CHILKOOT GEOLOGICAL ENGINEERS LTD.

Box 31146, Whitehorse, Yukon Y1A 5P7 chilkoot@northwestel.net (867) 667-6671 ph (867) 335-2085 c



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Geotechnical Evaluation

Faro Grum Sulphide Cell

Erosion Control Measures

Faro Mine Complex, Yukon - 2011

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Chilkoot Geological Engineers Ltd. was retained by *Boreal Engineering Ltd.* to conduct a preliminary geotechnical evaluation of the *Grum Sulphide Cell Cover* located in the *Faro Mine Complex* in Faro, Yukon. The purpose of the evaluation was to assess the condition of the recently constructed cell cover given the degree of soil erosion that had been observed by *Yukon Government* (YG) and *Denison Environmental Services* (DES) site personnel.

Following our (May 10 & 11, 2011) evaluation, it appeared that the design intent to control slope drainage was not being met and that immediate corrective action would be required to minimize the detrimental effects of the spring frechette and uncontrolled surface water run-off. As such, our firm recommended to Mr.E.Nyland, P.Eng., President of *Boreal Engineering Ltd.* that the functionality of the existing ditches be improved. We subsequently met with *Yukon Energy, Mines and Resources* personnel (Mrs.D.Pitt, ASLA, Senior Project Manager and Ms.K.Furlong, E.I.T., Project Manager) on May 13 to further evaluate the situation. After conference call discussions with the Engineer of Record, Mr.K.Scott, P.Eng. of *SRK Consulting Engineers and Scientists* (and the above noted personnel), it was determined that the best course of action to minimize soil erosion was to employ hand laborers to access the cell and clear the existing ditches of snow and ice such that their functionality would increase relative to their existing state. As such, our firm was employed at the request of *Yukon Government* and *Boreal Engineering Ltd*. to direct erosion control measures and provide recommendations for remedial action.

Our preliminary field assessment was conducted on May 10 & 11, 2011 by the undersigned along with the assistance of Mr.Nyland (*Boreal*) and Ms.Furlong (YG). Authorization to proceed with our services was granted by Mr.Erik Nyland, P.Eng., president of *Boreal Engineering Ltd.* on May 10, 2011.

Erosion control measures were conducted between May 16-21 & 24, 2011. Our firm was onsite to formulate and direct the erosion control plan between May 15-21, 2011.

A design team representative, Ms.O.Kosarewicz, P.Eng., was onsite May 17-19, 2011 to verify that the erosion control measures did not adversely affect the design. Mr.D.Rainey, Ph.D. (*YG*) Project Manager was also on site on May 17 & 18, 2011.

The following report summarizes the findings of our geotechnical evaluation and the erosion control measures that were undertaken. While preliminary recommendations have been formulated for discussion purposes, further evaluation is required to fully address all facets of the project that may require remedial work.

1.1 **Project Description** - Grum Sulphide Cell Cover Construction

The *Grum Sulphide Cell Cover Construction* project was conducted in the fall of 2010 and involved;

grading of waste tailings (sub-grade), placement of an impermeable plastic liner (approximately 400 m by 500 m), establishment of a 1 meter thick till cover above the liner, grading of surface water interception benches, construction of associated ditch-works, and hydroseeding of the cell cover (not yet conducted).

1.2 Weather

A general warming trend was noted between our two episodes onsite.

Generally fair weather was encountered on May 10 with temperatures in the order of 8 degrees Celsius, however, weather conditions deteriorated overnight and steady flurries were encountered on May 11, 2011.

The weather encountered during implementation of the erosion control measures was generally fair and ranged to 18 degrees Celsius with overnight temperatures generally above freezing. Overnight temperatures were noted to gradually increase during the course of the work.

The weather conditions for Faro, Yukon would have been recorded in more detail by Environment Canada if further information is required.

2.0 METHODOLOGY

Our geotechnical evaluation has been based upon our preliminary field assessment, brief review of the *SRK Consulting Engineers and Scientists* April 30, 2011 drawings (G-1 to G-14) and field conditions observed at the time erosion control measures were implemented.

Our observations and discussions with project personnel have been summarized in our Field Notes which have been attached as Appendix A.

A brief description of each facet of our evaluation and a summary of the work conducted during the erosion control measures has been provided below.

2.1 Preliminary Field Assessment (May 10 & 11, 2011)

A site reconnaissance was conducted the afternoon of May 10 and the morning of May 11, 2011. Mr.E.Nyland, P.Eng. of *Boreal Engineering Ltd.* and Ms.K.Furlong, E.I.T., (YG) Project Manager were both able to accompany us during our assessment.

In general, our reconnaissance was comprised of traversing portions of the site on foot, hand shoveling test pits along the toe berm bench and assessing the overall functionality of the cell cap relative to spring thaw conditions. During the work, photographs were taken to document the existing conditions. A digital copy of our photos has been attached.

At the time of our evaluation the majority of surficial cell cap materials were noted to be saturated or else covered in snow. Traversing the site on foot was difficult. Walking across saturated areas resulted in being inundated in approximately 0.2-0.3 meters of fine grained cap materials. The underlying cap materials were noted to be frozen below a depth of approximately 0.3 meters.

Several test-pits were hand excavated near the crest of the toe berm immediately above the as-built liner depth on May 10, 2011. While the liner was not encountered during the exploratory work, sand-bags (which were known to weigh down the liner during installation) were encountered along with buried snow and ice. The presence of deleterious (snow & ice) materials entrained within the backfilled materials was cause for concern. Thaw of the deleterious (snow/ice and frozen) material was likely the cause of zones of subsidence (which were observed as (approximately 1 by 4 meter) linear features oriented parallel to the crests of several benches) and general loss of local design continuity.

The bench ditches were essentially non-functional as they were noted to be filled with snow and ice. Water was noted to be flowing over the crests of the benches at a number of locations. The down-slope migration of surface water between the benches resulted in the formation of erosional rilling features in predominately areas were flow was identified in *Figure 1*. Similar erosion rills were noted on the eastern face of the toe berm near Station TB + 075.

Upon inspection of the 600 mm half culvert outflow flue, the noted conditions were considered poor. Specifically, undermining of the outflow flue was occurring. The undermining appeared to have been ongoing for some time and was in our opinion likely due to a combination of;

- a poor transition from the rip-rap lined outflow channel stemming from the final energy dissipation pond. Specifically, the half culvert outflow was oriented at an angle greater than 90° relative to the alignment of the discharge channel,
- its inability to handle the volume of flow discharging from the entire cell during the frechette,

the impact of local surface drainage, some of which appeared to have

originated from outside of the cell.

In addition to the undermining, an erosion gully was noted on the slope located directly east of the flue inlet area. The gully appeared to have formed as a result of overflow conditions at the location of the flue inlet.

Upon investigating the Grum Creek area (at the base of the erosion gully feature), granular outwash materials were noted to be lying on top of snow and the forest floor cover. It was apparent that this material originated from the erosion gully immediately upslope. Some incising through the forests organic floor mat and into the underlying sub-grade materials was noted. The presence of permafrost (which would be very susceptible to erosion) could not be ruled out in this region.

2.2 Erosion Control Measures (May 15-21 & 24, 2011)

2.2.1 Intent

Our firm employed a best practices approach to directing remedial measures with the intent to reduce the amount of surface water migrating down the face of the cell.

2.2.2 Establishment of Survey Control

For ease of reference, our firm established survey control on the benches and toe berm by marking survey lathe at approximately 25 meter intervals. Station 000 was established on the eastern edge of the benches with increasing chainage advancing to the west. In addition, the benches were simply numbered from 1 to 5 and TB (Toe Berm). Bench 1 equated to Bench elevation 1284 meters and subsequent benches were numbered sequentially relative to decreasing elevation (hence, Bench 5 is located immediately above the Toe Berm bench).

2.2.3 Initial Assessment (May 15, 2011)

Following our firms arrival onsite on May 15, 2011, it was apparent that the primary erosion features were comprised of incised rills on the cell cap culminating in prominent gullies located at the base of the toe-berm's east side. As such, a remedial

strategy was formulated to assess the effectiveness of hand clearing of the bench drainage ditches, reduce the flow of surface waters over the east side of the toe-berm and re-establish bench ditch drainage.

The erosion gully located near the flue inlet appeared to have increased in size since May 11 and as such our firm recommended that heavy equipment be utilized to place rip-rap in an attempt to reinforce/armor the slope and minimize erosion.

2.2.4 Methodology

T.Moon Construction was retained by YG and DES to provide personnel & equipment to assist in the erosion control measures. Their work effort was observed and summarized by our firm in Daily Construction Summaries which have been attached as Appendix B.

Initial efforts on May 16, 2011 to concentrate hand excavation at zones where surface waters were creating over the benches proved to be in-effective as snow and ice entrapped within the ditches prevented proper drainage of the bench ditches. As such, the methodology was modified to hand excavate from the confluence of the bench ditches and access road surface drains and progressively excavate to zones of concern. While this method resulted in improving the functionality of the bench ditches, there were areas (encountered on all benches) where sediment had completely filled the ditch (typically for a span of approximately 5-7 meters). This resulted in ditch blockage and bench overflow conditions. As the sediment within the ditch was saturated, hand-excavation of the material was not practical. As such, sandbag dikes were constructed to intercept down-slope surface waters and redirect the flow back into more functional areas of the bench ditches. Local native cap materials were utilized to fill the (~ 175) sand-bags as it was not logistically possible to hand carry filled sand-bags to the regions of concern. Once the majority of the surface water flow had been re-directed into the east side of Bench 5, T.Moon's crew was dispatched to clear bench ditches on the east side of the cell from a predominately

top-down approach. This allowed for water to be intercepted as soon as possible, to minimize cell degradation due to surface waters which were originating from outside the limits of the cell and the cell cap itself.

A tracked Bobcat mini-excavator was subsequently employed (through *T.Moon Construction*) on May 18, 2011 to clear sediment from the bench ditches. This work initially concentrated on clearing sediment from the east side of the Bench 5 ditch immediately above the Toe Berm gullies. The intent was to increase its functionality and minimize down-slope migration of surface water that was impacting the prominent Toe Berm erosion features. Once capacity to the east side of Bench 5 ditch was restored, the mini-excavator concentrated its immediate efforts on the west side of the cell. The mini-excavator began at the west side of Bench 2 and progressively advanced down-slope. Clearing of the Bench 1 ditch was considered, however the ground conditions were too poor to allow for traverse with the excavator. As such, the laborers were employed on May 19, 2011 to promote drainage in this area (to allow for subsequent clearing with the mini-excavator). The mini-excavator switched to clearing east side ditches on May 20, 2011. Again, top-down methodologies were employed, beginning at Bench 2. The Bench 1 ditch was mechanically excavated on May 24, 2011.

Following hand clearing operations, the laborers were utilized to hand place rip-rap at the confluence of the bench ditches and access road surface ditches May 18-20, 2011.

While an attempt to place rip-rap in the region of the flue inlet area was made by *DES* on May 17, 2011, rip-rap placement operations could not be conducted as the poor condition of the quarry access road prohibited access. By May 18, 2011, the erosion gully had become essentially self-armored with large cobbles and boulders (as the fine-grained component of the slope materials had washed down-slope by this time). Following discussions with Mr.D.Rainey, Ph.D., (*YG*) Project Manager on May 18,

2011, it was determined that the best course of action would be to monitor the conditions and await direction from the *SRK* design team.

2.3 MISCELLANEOUS OBSERVATIONS & DISCUSSIONS

2.3.1 Evaluation of Constructed Bench Grades

It is our understanding that the benches were to be graded towards the cell cover and bench ditch at a 2% grade in an effort to retain downward migration of cell cap surface water. However, based upon our level survey, the grades noted actually promoted down-slope migration.

During our evaluation, the bench grades were measured utilizing a level transit as follows;

BENCH	Design Grade	As-built Grade	~ Measured Grade
Bench 1	2 %	2 %	Minus 2.8 %
Bench 2	2 %	2 %	Minus 2.3 %
Bench 3	2 %	2 %	Minus 3.6 %
Bench 4	2 %	2 %	Minus 4.5 %
Bench 5	2 %	2 %	Minus (TBD) %
Toe Berm	NA	NA	Minus 0.1 %

The large variation between the design/as-built and the bench grades measured during our survey is of concern.

2.3.2 Evaluation of Toe Berm Bench Area

A level survey of the toe berm was conducted relative to a local benchmark established by our firm. In brief, the survey indicated that there was an approximately 3.3 meter drop in elevation located at approximately Station TB + 075

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relative to the peak toe berm height in either direction. The presence of this low area essentially concentrated all the surface waters on the east side of the cell onto the face of the toe berm. This flow resulted in the formation of the erosion gully's. While onsite, the gully's were noted to incise approximately 2 meters in from the crest of the toe berm bench and create chasms which measured in the order of 2 meters deep.

From discussions with *SRK* personnel onsite, it was understood that the liner was temporarily established on the upslope side of the toe berm and then subsequently draped up and over the toe berm once it was established. Following this, additional bench material was utilized to create the bench. This would result in a region of liner where it's up-slope elevation is actually lower than the down-slope elevation. Further evaluation would be required to assess the impact of these potentially constructed conditions.

2.3.3 Failure of 600 mm half culvert outlet flue

Undermining of the outflow flue appeared to be ongoing since our initial investigation of May 10 & 11. The erosion gully (which formed on the eastern side of the adjacent slope descending to the Grum Creek area), was noted to have increased in size. While this increased the sediment load of the surface waters discharging from the cell cap, the oversized composition of the slope material resulted in (what appeared to be) essentially self-armoring slope conditions. The geotechnical durability and long-term environmental ramifications of the conditions should be assessed by qualified personnel.

The 600 mm half culvert was no longer operational after May 18, 2011. It's functionality prior to this time appeared to be limited.

2.3.4 Peripheral Drainage Regime

The surface drainage in peripheral regions outside the limits of the cell was undefined. At some locations, peripheral surface waters flowed onto the cell;

from areas of higher elevation that were outside of the cell limits, laterally into the surface ditches from the adjacent access roads, after exiting the confines of the lateral access road drainage ditches; and/or by flowing directly into the energy dissipation ponds.

These flow regimes have been illustrated in Figure 1.

2.3.5 Cell Cap Material Composition

Given the presence of what appeared to be oversized materials (up to 500 mm diameter), the design assumptions should be re-evaluated to ensure that the design intent has not been compromised. The presence of oversize materials will generally increase the permeability of the cap materials

2.3.6 Cell Cap Thickness

Our firm attempted to assess the thickness and condition of the cell cover by means of test pitting by hand excavation immediately up-slope of Bench 3 (east side). However, frozen soil conditions were encountered at a depth of approximately 0.4 meters and as such excavation of the test-pit was terminated.

Given the magnitude of rilling (which was at some locations observed to be in the order of 0.3 meters deep), it is possible that there are locations where the cap thickness is less than the 1 meter required by the design. This would be a concern as the design life may be compromised if adequate cap thickness was not attained as soil cover will ultimately erode through eolian, surface water and other erosion processes. The compromised areas are likely concentrated in regions were surface drainage was identified as noted in Figure 1.

The affect of potentially reduced cap thickness at the locations of the bench ditches would have to be assessed in greater detail as this area may promote percolation to the region of cover materials immediately above the liner.

Insufficient data was available to determine whether the cap was overbuilt to allow for subsequent consolidation/settlement.

2.3.7 Surface Drains

The surface drains were noted to be approximately 0.2 meters higher than the adjacent cell cap materials. This condition is likely due to a combination of settlement of the adjacent cell cap materials and composition/construction methodology associated with the surface drain materials. Specifically, construction activities and the materials composition may have resulted in a more durable and competent region of material. It was evident that the higher local elevations of the surface drains channeled surface water flow along the upslope periphery of the drains and actually promoted soil cap erosion particularly at the flow locations noted on Figure 1.

2.3.8 Moose Pond – Preliminary Evaluation of Slope Conditions

Poor back-slope conditions at Moose Pond were observed during the frechette by site personnel and as such, an evaluation of the slope conditions was conducted by our firm on May18 & 20, 2011.

At the time of our analysis, the inflow into Moose Pond had been redirected (by YG and DES) and the water levels in the pond were noted to be decreasing. The diversion was required as water levels within Moose Pond had exceeded the ponds maximum capacity marker due to the influx of surface water run-off from the Grum Cell Cap and surrounding areas.

In brief, our (May 20) work was comprised of establishing survey control at the top of the ponds southern slope by marking 25 meter intervals utilizing survey lathe (to MP + 150). Subsequently, the existing slope conditions were logged and summarized as noted on Page 45 of our Field Notes attached in Appendix C. In addition to recording the physical condition of the slope, a total of six (6) soil samples were obtained to

allow for subsequent geotechnical laboratory evaluation, if further assessment is required.

While some of the slope failure features appeared to be recent, others appeared to be historical in nature as re-vegetation of some failed areas was noted. As such, given the decreasing pond levels and observed conditions, our firmed deemed the risk of immediate slope failure to be relatively low. However, the slope conditions appeared to be progressively degrading and thus less stable conditions will likely prevail in the future unless remedial action is taken or a safe capacity for the pond is determined and strictly observed.

As it appears that pre-existing conditions of the Moose Pond back-slope have not been documented, it is difficult to assess the impact that the out-flow from the Grum Sulphide Cell has had upon it.

While there was concern that the natural attenuation capacity of the pond had likely been compromised (given the presence of a seepage zone that was identified near Station MP + 090), a detailed environmental study would be required to ascertain whether the ponds use in that capacity has been compromised.

2.3.9 Storm Water Management Pond

Our firm was approached by *Boreal Engineering Ltd.* and *YTG* on May 20, 2011 to assess the feasibility of constructing emergency storm water management ponds adjacent to the final energy dissipation pond. As such, our firm surveyed neat line layout of a potential pond area and discussed construction options during a conference call on May 21, 2011 with *YTG*, *SRK* and *Boreal Engineering* personnel. It was determined that *DES* would construct the ponds to allow for storage capacity in the event of heavy precipitation and so excavation began immediately.

and 21, 2011, it was ch, it should be noted

While our firm forwarded conceptual sketches on May 20 and 21, 2011, it was understood that *SRK* would facilitate their final design. As such, it should be noted that at the time of our departure from the site (on May 21, 2011), the side slopes of the storm water management pond excavated were not yet completed and so their final condition should be evaluated to ensure conformity with the design and side slope stability. A copy of our conceptual sketches has been attached in Appendix C.

2.3.10 Sample Retention

Our firm retained several grab samples to assist in characterizing the existing conditions at the time the erosion control measures were implemented. Specifically this included obtaining samples from areas of the surface drains, cell cap and Moose Pond slope failure.

2.4 As-built Drawing Review

While a detailed review of the As-Built Drawings was beyond our scope-of-work, based upon our observations and use of the drawings during remedial action, it appears that the intended as-built drawings do not accurately reflect the existing conditions. Specifically, the liner appears to be absent at the location of the toe berm bench gully's, however, this is difficult to evaluate as a detail illustrating the liners end-point at the location of the toe berm appears to be absent in this version of the drawings.

3.0 RECOMMENDATIONS & CONCLUSIONS

3.1 General

Our recommendations have been provided for discussion purposes only. They are not meant to be utilized directly as recommendations for remedial action.

For ease of reference, we have compiled *Figure 1 – May, 2011 Drainage Regime and Proposed Recommendations,* which denotes the approximate locations of potential geotechnical liabilities and provides point form recommendations for future evaluation/rehabilitation. In addition, the figure shows the approximate location and direction of the selection of photos that have been attached in Appendix E. We have elaborated upon a selection of these recommendations to provide insight as to the complexity of formulating remedial options. Additional measures may be required in order to effectively address the site and degrading conditions. Other geotechnical concerns will become more apparent over the course of time as erosion continues.

Erosion of the cell will be ongoing until remedial action is taken and a protective vegetative cover is fully established. As such, it is likely that soil erosion during precipitation events and next spring's thaw will be heavy (as a vegetative cover will not likely be fully established by the fall) and this should be taken into account from a project management perspective.

The remedial action envisioned will likely require the use of heavy equipment in order to import and/or re-grade and compact the cell and peripheral areas.

3.2 Formulate an Engineered Design

A detailed assessment of the behavior of the cap materials should be conducted to verify the overall stability of the soil mass, especially given the presence of the Geotechnical Evaluation Report Grum Sulphide Cell Cover Erosion Control Measures Faro Mine Complex, Faro, Yukon - 2011 impermeable liner. This assessment should evaluate the local and regional thermal, groundwater and surface drainage regimes to evaluate the soils behavior during times of thaw. The assessment should also evaluate the avalanche hazard at times when the cell is covered in snow.

If imported material is required to re-establish the cap thickness, 'bottom-up' construction methodologies should be employed. Subsequent layers should be well bonded and keyed into underlying/adjacent materials. The construction of a reinforced (geo-grid/rip-rap) slope should be considered for steeper areas of the toe berm's face. The gully areas should be excavated utilizing a tracked excavator equipped with a clean-up bucket to minimize the potential for unconsolidated fill from infilling the existing fissures/gullies.

The impact of terminating the liner near the top of the toe berm should be reevaluated considering the long-term effect of sub-surface groundwater (above the liner) discharging at this location.

3.3 Re-grade & Compact the Cell Cover

Once the thickness of the cell cap has been confirmed and the prepared surface has been re-graded, the cell should be compacted utilizing a sheeps-foot drum roller. The use of this equipment should greatly assist in promoting laminar flow as well as reducing down-slope flow velocities. In addition, the prepared surface will be ideal to capture and harbor hydro-seed grasses, which will promote uniform establishment of a soil cap. The feasibility of utilizing compaction equipment on the cell slope will be heavily dependent upon the moisture condition of the cell cap materials and weather conditions.

3.4 Hydro-seed the Cell Area

Additional applications of hydro-seeding may be required depending upon how well it is established following the spring thaw.

3.5 Re-configure/Re-establish Surface Drains

The surface drains should be re-established such that they lie just below the cell cover surface in such a manner as to intercept and break-up fall line slope drainage.

3.6 Re-grade the Benches

The benches (including the toe berm bench) should be re-graded with a slope considerably greater than 2% drainage towards the bench ditches.

3.7 Re-habilitate the Ditches & Energy Dissipation Ponds

The bench and access road ditches should be lined with rip-rap armor to minimize erosion. The use of a near surface liner should be considered.

A toe berm ditch should be constructed on the toe berm bench. The use of baffles in this area should be considered given the existing gradients.

Sediment should be excavated from the energy dissipation ponds to restore their functionality.

3.8 Re-grade Peripheral Areas

The peripheral areas outside the limits of the cell should be regarded to promote drainage away from the cell limits. Interception ditches should be considered.

3.9 Survey the Cell Cap, Ancillary and Moose Pond Areas

A 0.5 meter contour interval survey plan (forwarded by *Boreal Engineering Ltd.* on May 20, 2011) verifies that the intended as-built conditions as noted on *SRK's* drawings (G-1 through G-14) were not likely fulfilled at the time the survey was conducted. As such, given the degree of erosion that was noted, we recommend that a detailed topographical survey be conducted in such a way that the generated surface can be compared to the above noted 0.5 m contour intervals. From this, an isopach thickness map can be generated to assess the change in (cell cover thickness) conditions relative to the initial fall, 2010 survey. Areas of concern should be explored in the field based upon areas identified during evaluation of the isopach map.

In addition to survey of the cell limits, the ancillary areas should also be surveyed such that remedial options can be better assessed. As a minimum, this should include surveying all areas up-slope of the cell, peripheral areas adjacent to the cell (up to and including the haul road), areas down-slope of the cell (including the base of the Grum Tailings directly east of the energy dissipation pond) the Grum Creek drainage area, and the Moose Pond and slope failure areas.

The 0.5 meter contour interval plan has been attached in Appendix D.

3.10 Complete Storm Water Management Pond(s)

The discharge from the energy dissipation pond outflow into the storm water management pond flow along a very low gradient and/or else heavily armored slope to prevent soil erosion from occurring. Additional consideration will be required to evaluate the ponds side-slope stability given the additional surcharge loads that may be carried by truck traffic/equipment utilizing the nearby quarry access road. We recommend that the access road be shifted such that it lies immediately adjacent to the access road surface ditch to allow for a greater factor of safety in this regard.

3.11 Assess Grum Creek Area

The presence of local and regional permafrost should be identified prior to formulation of remedial options in this area.

3.12 Assess Moose Pond & Slope

We recommend that a slope stability analysis be conducted to assess the structural integrity of the pond side slopes, particularly considering the number of personnel that work near this area and the apparently degrading slope conditions. The slope should be actively monitored by specifically designated and qualified geotechnical personnel to increase worker safety in this region.

3.13 Review Cell Cap Performance

While a review of the cell caps performance was beyond our scope-of-work, we suggest that the impact of up-slope water (which may percolate downwards and promote recharge of the groundwater regime of the waste tailings materials below the existing liner) be assessed. If required, materials up-slope of the cell should be regraded to promote positive drainage away from the cell liner area.

3.14 As-built Drawing Revisions

We recommend that the As-Built drawings be revised to better reflect the actual field conditions such that remedial options can be better assessed (particularly in the region of the toe berm).

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3.15 Construction Monitoring & Inspection

We recommend that any remedial work be monitored and inspected by qualified geotechnical personnel to verify that the remedial work is conducted in accordance with the design intent.

3.16 Time is of Essence

Time is of essence.

While further evaluation would be required to assess all facets of cell and peripheral area rehabilitation, it is critical that remedial construction work be conducted this summer to minimize the amount of soil erosion particularly during precipitation events and next year's frechette. This may require performing remedial work prior to receiving remedial recommendations from the design team. Construction based upon an interim design should be considered, considering that it was not possible to eliminate the soil erosion potential at the time of our operations due to the scale of work required.

4.0 **REPORT LIMITATIONS**

This report is intended for the sole use of Boreal Engineering Ltd.

No portion of this report may be used as a separate entity; it is intended to be read in its entirety. Any use of this report by a third party, or any reliance or decisions based upon it, are the responsibilities of such third parties.

The comments contained within this report reflect our best judgment in light of the information available to *Chilkoot Geological Engineers Ltd.* at the time of our field services. They are based upon a compilation of field observations, field conditions and generally accepted engineering practices.

The observations made in our Daily Construction Summary's reflect the general trend of work but are by no means an absolute measure of the work conducted.

Our evaluation is limited in that most of our observations were obtained while providing erosion control measure oversight and this should be taken into account. Our observations and recommendations should be assessed from a regional standpoint considering that conditions will change over time.

It should be reiterated that while the Erosion Control Measures were successful in minimizing down-slope migration of surface waters, deleterious conditions still exist and as such degradation of the cell cap is ongoing until corrective action is completed and final cell equilibrium has been reached.

As with all remedial work projects, adjustment to the remedial (assessment, design and construction) methodologies may be required to accommodate actual/potential field and soil conditions.

Should any newly found geotechnical concerns become apparent, our firm should be notified immediately in order to confirm the suitability of our recommendations and conclusions, which may be altered or modified in writing by the undersigned.

Thank you for allowing our firm to provide you with the above noted services. We trust the information we have provided will suit your purposes at this time, however, if you should have any questions regarding the information provided herein, please feel free to contact the undersigned at your convenience.

Respectfully Submitted,

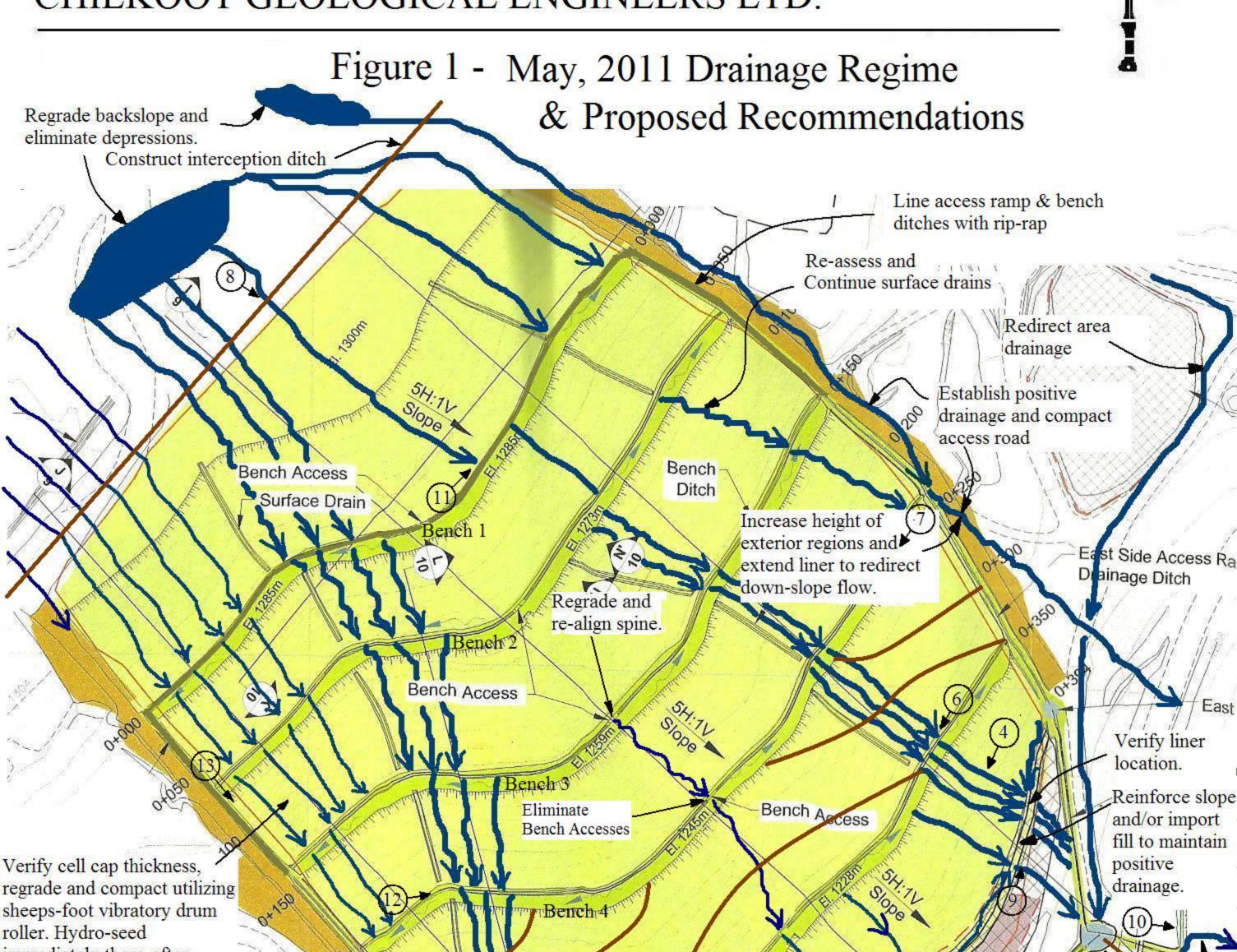
CHILKOOT GEOLOGICAL ENGINEERS LTD.



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immediately there-after.

Regrade and compact benches (minimum 10% grade). Import fill if required.

Re-assess drainage regime & re-align surface drains if required.

Construct and armor drainage ditch Regrade and place baffles and rip-rap Assess potential for slope failure. Construct storm water management pond(s)

Q^

Regrade areas outside of cell cap. Assess snow loading/deposition and avalanche hazard both in/outside of cell limits.

Assess and install silt barriers/fences down-gradient.

Assess down-slope erosion features. Assess Moose Pond slope failure. Align quarry access road adjacent to collection ditch Compact access road. Reconfigure and out emband

Toe

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Reconfigure and cut embankment for out-flow. ' Follow fall line of prevailing regional slope. Maintain constant grade to elevation of Grum Creek. Install liner, geofabric and heavy rip-rap. Re-evaluate long-term stability of outflow channel particularly with respect to under-cutting.

LEGEND

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Drawn May 31, 2011 by T.Dhara - N.T.S.

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APPENDIX A

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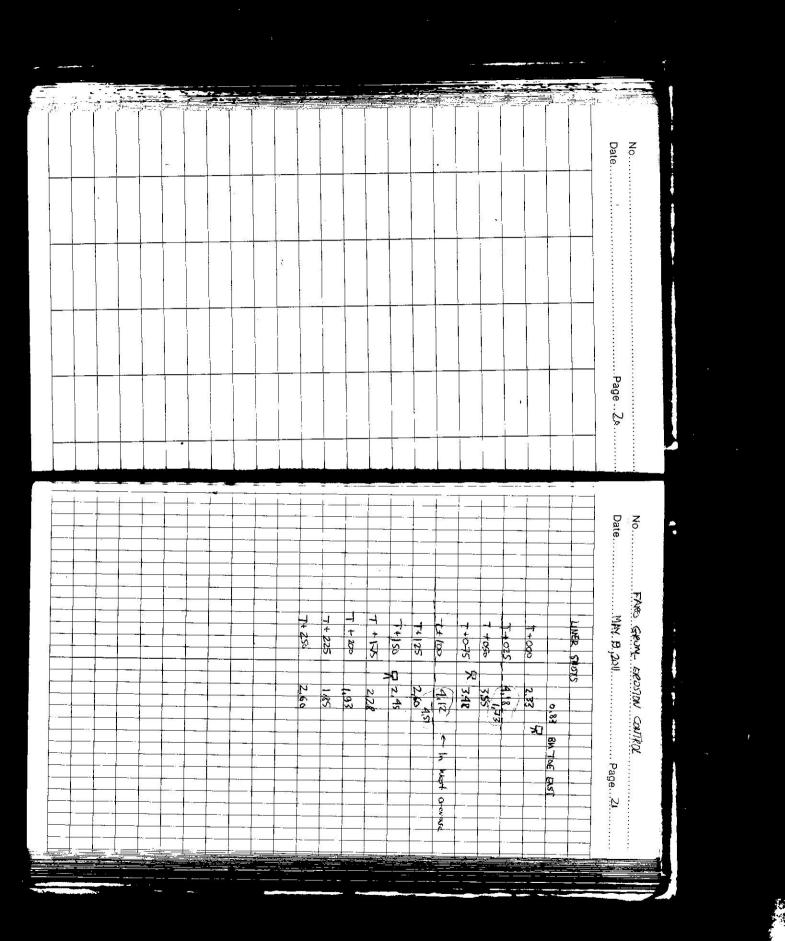
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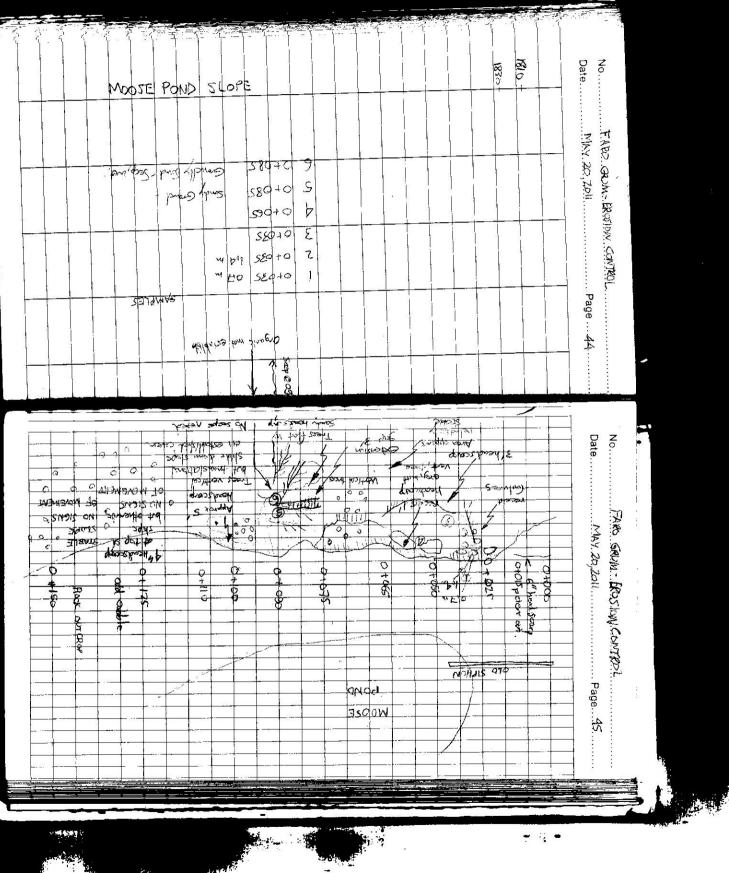
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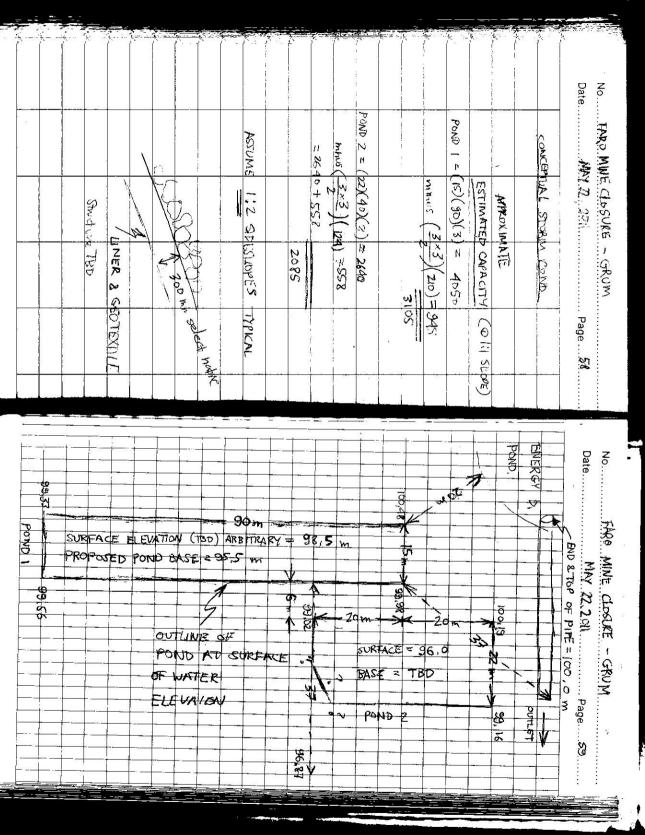
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APPENDIX B

Daily Construction Summary

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Construction				t 15	15	15	15	15	15	f5	15	lunch	h4	h4	h4	h4	h4	h4	h4	h3	h3	t					25.5	0.0	3.0	0.0	3
Laborers 3.4 & 5 K f5 f5 </td <td>L</td>														L																	
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Bobcat Tracked Excavator Image: Second														1.																	
Bobcat Tracked Exeavator Image: Second																															
Christian Christian MP MO 0.0 <td>_</td>														_																	
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DES Jay I I I I I I I I I I I I I I I I I I I															0.																
тю	D.Rainey, Ph.D.						τ	τ	τ	τ	τ	τ	τ	τ	mu	τ	m	m	m	m	m	τ					3.0	0.0	5.0	0.0	8
We	eather																														
																			,												
 assess bench 	hes & slope	k													ad Co							Deres		F 1	1	ork Ar			0		
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: - cut/fill roadwa	ау	n - safety		eting				\A/-													2 3		n ∠ - ch 3 -	-	-		r H			Road	;
l - - excavate ditc	haa	p - parke o - redire		~~~				- Wo	Drk										J				2n 3 - 2h 4 -				н Р			Road Rap Pit	
- excavate ditc - filling sandbag		r - rip-rap						Mic	scellan	00110	ĺ			Misc	cellan	00116			1		+ 5		211 4 - 2h 5 -		-		F		Rip-R	аргі	
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, , ,	9 snow from ditches	t - travel		om sito				- Tra	امريد			0		26				-15			P	F	Energy	-	1						
-		w -	0/11	on site				. 116													P		Noose								
	COMMENTS: Conti		once	ntrate e	fforts t	n san	had h	ench	5 surf	aca fl	ow an	d redi	rect fl	ow int	to her	ch 5 (litch	Effort	s to e	tom th	e flow	anne	ared	offecti	ve an	d as e	such				
	hand clearing operat																														
	While DES was able																											allow	or use	of a	
	rock truck being sup																														

									CH	HILI	KO	OT	GE	OLO	DGI	[CA	LΕ	NG	INE	EER	S L	TD	•								Ţ	
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												DA	ILY (CONS	STRL	JCTIC	DN SI	JMM	ARY													
			Clier Proje Task	ect :	Farc	Mine	Com	ing Lto plex Cl ion Co	losure		ures										Date Locat Work	tion :	s :	Grun	18, 20 n Sulp se Se	hide	Cell C	over				
Equ	lipment	0430	0200	0230	0800	0830	0060	0830	1000	1030	1100	1130	1200	1230	1300	1330	1400	1430	1500	1530	1600	1630	1700	1730	1800	1830	1900	Work Hours	Miscellaneous	Travel	Downtime	Total Hours
CHILKOOT	Tares Dhara, P.Eng.	1	t	m	0 mMI	^o mMF	mF	mB	sTB	sTB	sTB	sTB	a4	m	m	m0	m	s3	s3	s4	s4	m	m1-3	m1-3	m0	t		10.5	0.5	1.0	0.0	12
BOREAL	G.Nyland		t	m	0 m0			f5	f5	f5	f5	f5	h4	h4	h4	h4	h4	s3	s3	s4	s4	m	m1-3	m1-3	m0	t		10.5	0.5	1.0	0.0	12
	T.Moon		t	ml		offsi																						0.5	0.0	1.0	0.0	1
-	Robert (Foreman)		t	h		h1		h1		h1	h1		lunch			h1	h1	h2	h2	h2		h3		r1-3				10.0	0.0	1.0	0.0	1'
T.Moon	Laborer 1 & 2		t	h		h1	h1	h1		h1	h1	h1	lunch			h1	h1	h2	h2	h2	h3	h3		r1-3				20.0	0.0	2.0	0.0	22
Construction	Laborers 3,4 & 5		t	h		h1	h1	h1	h1	h1	h1	h1	lunch		h1	h1	h1	h2	h2	h2	h3	h3						30.0	0.0	3.0	0.0	33
Tracked Excavator (Neil) t t t t t t t n0 n0 n0 t e5 e5<																																
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Argo (6 wheel drive) t mc																																
Argo (6 wheel drive) t mc														10																		
Tracked Excavator (Neil) t </td <td></td>																																
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- assess benche	es & slope	k -						_					_	_		ad Co							_				ork Ar					
) - 			miscel						- Do	wntim	e		N	lateri	al	Da	aily	Т	otal			-			Elev			0		Securi		
- cut/fill roadway	Ý		safety i		ting																	2		-	Elev	-		F			w Flue	•
-			barked						- Wo	ork												3			Elev			H			Road	
- excavate ditch			edirec						1											1		4		-	Elev	-		P			ap Pit	
- filling sandbags			p-rap		ement				- Mi	scella	neous		<u> </u>			cellar	eous		40			5	Ben		Elev			<u>c</u>			Сар	
- grade existing			urvey						ı –				Ci	ew Si	ze			11	-13			B			Berm			В	l	Backslo	pe Are	ea
- hand shovel si -	now from ditches		avel to	o/froi	m site				- Tra	avel												:P 1P			y Pon e Pon							
-		w -											L					I		I	IV			141005		u	1					
(COMMENTS: Shifte	ed her	nd cles	arino		to all		nch 1	Δmi	ni-evo	avato	r arriv	ed on	sita tr	د مام د	r sadii	ment f	rom t	he he	nch dit	chee	Soft I	Rench	1.00	nditio	ns res	tricter	4				
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												DA	ILY (CONS	STRU	CTIC	ON SU	JMM	ARY													
			Clier				jineeri														Date				19, 20		0-110					
			Proje Task				Comp			e Measu	iros										Locat Work				n Sulpi se See		Cell C	over				
			Task	•	Imple	ment	EIUSI		millori	weasu	lies										WORK	Alea	5.	Fleas	se see	e Dei	OW					
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Ec	uipment	30	8	0730	8	0830	0060	0630	1000	1030	1100	1130	1200	1230	1300	1330	1400	1430	1500	1530	1600	1630	1700	1730	1800	1830	1900	Work Hours	Miscellaneous	Travel	Downtime	
	luipinent	0430	0200	20	0800	08	60	60	10	10	1	-	12	12	13	13	4	4	15	15	16	16	17	17	18	18	19	ž	Sells	Tra	0 V	
																												Ň	Mise		Õ	
CHILKOOT	Tares Dhara, P.Eng.	1	t	m0	sTB	sTB	sTB	m	m	m	m	m	m	m	m	m	s5	s5	s5	s5	a1	a1	a5	a5	m0	t		10.5	0.5	1.0	0.0	
BOREAL	G.Nyland	1	t	-			sTB			m	m	m	m	m	m	m	s5	s5	s5			m	m	m	m0	t		10.5	0.5	1.0	0.0	
	T.Moon		t			mC	t	offsite																				1.5	0.0	1.0	0.0	T
	Robert (Foreman)		t	mC						r1-5											m	m	m	t				9.5	0.0	1.0	0.0	
T.Moon	Laborer 1 & 2		t	mC						r1-5											m	m	m	t				19.0	0.0	2.0	0.0	
Construction	Laborers 3,4 & 5		t	mC						r1-5			lunch							m	m	m	m	t				28.5	0.0	3.0	0.0	
	Argo (6 wheel drive)		t	mC	mC					mC			li un ala	mC			mC	mC	mC			mC	mC	t			_	9.5	0.0	1.0	0.0	
	Tracked Excavator (Neil) Volvo Rock Truck	_	t Ha	Dm Ha	e3 pH	e3 pH	e3 pH	e3 pH	e3 pH	e3 pH	e3 pH	t pH	lunch pH		e4 pH	e4 pH	e4 pH	e4 pH	t pH		-	10.0	0.0	1.0	0.0	_						
SRK	Olga Kosarewicz, P.Eng.		рп	pn t	m0					mMP				lunch		m	pn t	m m	m			рп m	m5	m	mMP	?		7.5	0.0	2.5	0.0	+
Orar	Christian				mo	mo	mo				mo			union						111-7			1110					7.5	0.0	2.0	0.0	
	Chris																															t
DES	Jay														mF	mF	mF	mF	mC	mC	mC	mC	m5	mC	mMP	?		5.5	0.0	0.0	0.0	T
	Cat 345 Excavator																															
	D9 Cat Bulldozer																															
YTG																																
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W	eather																															
- assess benc	haa 8 alana	k-											1		1.02	d Co	unte			1						W	ork Aı	1026				_
- assess benc	nes a siope		miscel	laneoi	15				- Do	wntim	þ		Ν	lateri			aily	Тс	otal		1		Ben	ch 1 -	Elev '			0	l –	Securi	tv Gat	e
- cut/fill roadw	av		afety						00		•						,				2				Elev '			F		Outflo		
-	.,		arked		.9				- Wo	ork											3				Elev '			H			Road	
- excavate dite	ches		edirec	t flow																	4				Elev '			Р			ap Pit	
- filling sandba		r - rij	p-rap	blacer	nent				- Mis	scellar	neous				Misc	ellan	eous				5		Ben	ch 5 -	Elev 2	1228	i.	С		Cell	Ċap	_
- grade existin	g	s - s	urvey						•				Cı	rew Si	ze			11	-13		Т			Toe	Berm			В	E	Backslo	pe Ar	ea
- hand shovel	snow from ditches	t - tra	avel to	/from	site				- Tra	avel											E				y Pono							_
-		w -																			M	Р		Moos	e Pono	d						
	COMMENTS: The																					d the	west	side o	of Ben	ch 3	and th	nen the	west s	ide of E	Bench	4
	T.Moon's crew conti	nued t	to plac	e rip-i	an at	the co	onflue	nce of	f the c	ditch b	enche	es to t	he lat	eral si	irface	drain	s (pre	domir	natelv	east s	side).											

									(86						-lighwa ⊉north					673 f	ax										Ĩ															
												DA		CONS	STRU	СТІС	N SL	JMMA	٩RY																											
			Clien Proje Task	ect :	Faro	Mine	Comp	ing Lto blex Cl on Co	losure		ures										Date Locat Work	tion :		Grun	20, 20 n Sulp se See	hide	Cell Co	over																		
Eq	juipment	0430	0200	0730	0800	0830	0060	0930	1000	1030	1100	1130	1200	1230	1300	1330	1400	1430	1500	1530	1600	1630	1700	1730	1800	1830	1900	Work Hours	Miscellaneous	Travel	Downtime															
CHILKOOT	Tares Dhara, P.Eng.		t	m0	m	m	m	m	mG	mG	mMF	mMP	mMP	mMF	m	m	m	m	m	m	mEP	sEP	sEP	sEP	sEP	t		11.0	0.0	1.0	0.0	T														
BOREAL	G.Nyland	Ì	t	m0	m	m	m	m	m	m	m	m	m	m	m	m	m	m	m		mEP							11.0	0.0	1.0	0.0	T														
	T.Moon		t	mC	mC	t	offsite	e																				1.5	0.0	1.0	0.0	T														
	Robert (Foreman)		t	mC											r1-5		t											6.0	0.0	1.0	0.0															
T.Moon	Laborer 1 & 2		t	mC	r1-5	r1-5	r1-5	r1-5	r1-5	r1-5	r1-5	r1-5	lunch	r1-5	r1-5	r1-5	t											12.0	0.0	2.0	0.0															
Construction	Laborers 3,4 & 5		t	mC	r1-5	r1-5						r1-5	lunch	r1-5	r1-5	r1-5	t											18.0	0.0	3.0	0.0															
	Argo (6 wheel drive)		t	mC	mC	mC	mC	mC	mC	mC	mC	mC		mC	mC	mC	pН	pН	рН	рН	рН	pН	pН	pН	pН			6.0	0.0	0.5	0.0															
	Tracked Excavator (Neil)		t	m0	m0	e5	e5	e5	e5	e5	t	e2	lunch	e2	e2	e2	e2	t	e3	e3	e3	e3	e3	e3	t			10.0	0.0	1.0	0.0															
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$																																														
															Chris																															
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	Cat 345 Excavator																																													
	D9 Cat Bulldozer																																													
YTG	Matt (Yukon Environment)										mMF																					Ι														
																																_														
W	eather																																													
- assess benc	has & clopa	k-													1.03	d Co	inte		1	1						W/-	ork Ar	0.26																		
- assess benc	nes a siope		niscell	laneoi	IS				- Do	wntim			۸	lateri			aily	То	tal			1	Ben	ch 1 -	Elev			0		Securit	v Gat	P														
- cut/fill roadwa	av		afety r										<u> </u>				,	<u> </u>			2	-			Elev		1	F		Outflo																
-	~ <u>,</u>		arked	noculi	9				- Wo	ork											4				Elev			H		Haul																
 excavate dite 	thes		edirect	flow					- ••0	μN			L								4				Elev			P		Rip-R																
 filling sandbag 			o-rap p		hent				- Mie	scellar					Mier	ellan	20115				5	-	-		Elev			C		Cell																
- grade existin	•		urvey	nacen	GIII				- 19113	Joenal	10003		Cr	ew S		Shall	5543	9-	10		T		Don		Berm	1220		B	F	Backslo		62														
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		u - u a w -		, nom	5110				- 110	1100			I									IP			e Pon			SP		Water																
		**											L													-	· · · ·		5.0.11																	
	COMMENTS: The n	nini-e	xcavat	tor clea	ared s	edim	ent fro	om Be	nch 5	west	side	then r	oroare	ssed	to Ber	nch 2	east s	ide ar	nd Ber	nch 3	east s	ide.																								
	T.Moon crew comple																																													
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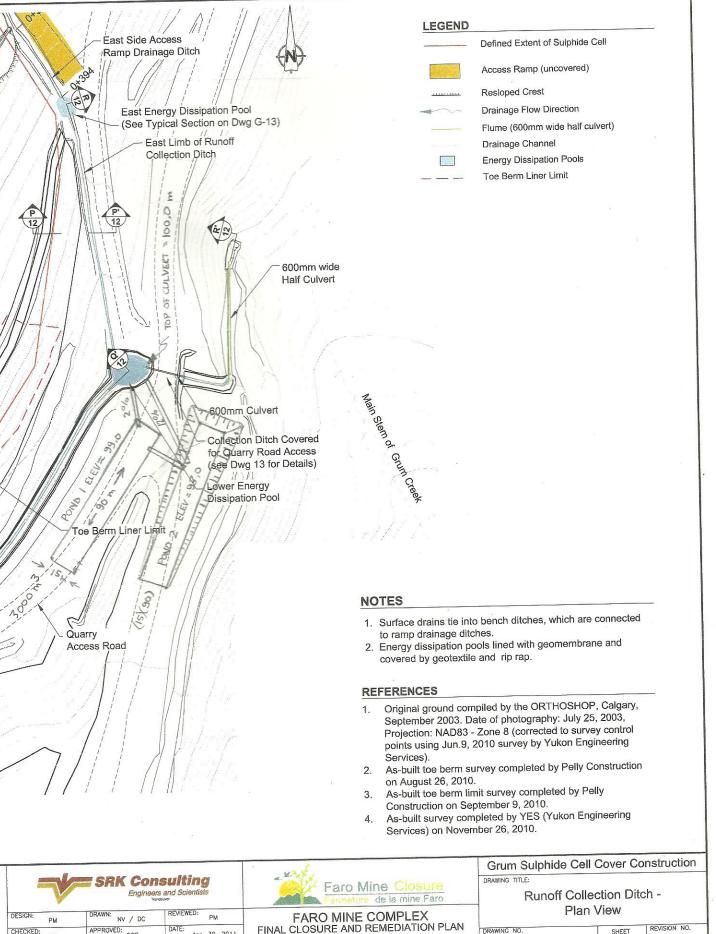
									CI	HIL	KO	OT	GE	OL	OGI	[CA	LE	NG	INI	EER	S L	TD	•							1		
									(8			'1 ph.	chil	koot@		weste	el.net	(867) 667.	∙ 6673 ⁻	fax										Ĭ	
												DA	AILY (CON	STRL	ICTIC	ON S	UMM	ARY													
			Clien Proje Task	ect :	Faro	al Eng Mine ement	Com	plex C	losur	e Meas	ures										Date Loca Worl		IS :	Grun			Cell C ow	over				
Eq	uipment	0430	0200	0730	0800	0830	0060	0660	1000	1030	1100	1130	1200	1230	1300	1330	1400	1430	1500	1530	1600	1630	1700	1730	1800	1830	1900	Work Hours	Miscellaneous	Travel	Downtime	Total Hours
CHILKOOT	Tares Dhara, P.Eng.			t	m0	mEP	mFF	2 m0	m0	mFP	sFP	sEP	mFF	mFF	SFP	sEP	mFF	mFF	mG	mG	mFP	mFP	mC	t				9.5	0.0	1.0	0.0	12
BOREAL	G.Nyland	1		t		mEP														mG							1	9.5	0.0	1.0	0.0	12
	T.Moon	l –																										1				<u> </u>
T.Moon Construction	Robert (Foreman) Laborer 1 & 2 Laborers 3.4 & 5																															
Construction			nL	nЦ	54	nЦ	ъЦ	ъЦ	~L	54	24	pН	nLl	۳Ц	۳Ц	nЦ	nL	~L	nH	24	nЦ	ъЦ	ъЦ	24	۶Ц							
	Argo (6 wheel drive)		pH	pH	pH			pH			pH				pH								pH									
	Tracked Excavator (Neil) Volvo Rock Truck		pH	pH	pH			pH			pH		pH		pH								pH									
SRK	Volvo Rock Truck Olga Kosarewicz, P.Eng.		pН	pН	pН	рн	pН	pН	pН	pН	pН	pН	pН	рн	pН	pН	pН	pН	pН	pН	pН	pН	pН	pН	pН	+	+					
SILIN	Christian					+						1	+		+		+	+		+	+				1		+	-				
	Chris		1					1	+	1		+	+	+	+		+	+		+					+	+	+	<u> </u>				
DES	Jay					1						1	1		1		1	1		1	1				1		1	1				
220	Cat 345 Excavator		t	mEP	mEP	mEP	mEF	mEF	mEF	mEF	mEP	mEP	lunch	mEF	mEF	mEF	mEF	mEF	mEF	mEP	mEP	mEP	t	pН	1			9.0	0.0	1.0	0.0	10
	D9 Cat Bulldozer		t	mEP	mEP	mEP	mEF	mEF	mEF	mEP	mEP	mEP	lunch	mEF	mEF	mEF	mEF	mEF	mEF	mEP	mEP	mEP	t	pH	1			9.0	0.0	1.0	0.0	10
YTG	Matt (Yukon Environment)	i –						1		1		1						1							1	1		1				
	. ,	-		•	•	•				•		•	•					•		•	•	•	•	÷	•	•	•		-	-		
We	eather																															
- assess bench	ies & slope	k -						_								ad Co		1 -		-			-				ork Ar				_	
-			niscell						- Do	owntim	e			Vater	ial	D	aily	T	otal	4		1		ch 1 -				0		Securi		
- cut/fill roadwa	У		afety r	neetin	ng				.							I		I		-		2	-	ch 2 -		-		F		Outflo		
-			arked						- W	ork						I		I		1		3		ch 3 -				H		Haul		
- excavate ditc			edirect					_	.	11					M.: -	aller				7		4		ch 4 -				P		Rip-R		
- filling sandbag			p-rap p	blacem	nent				- Mi	scella	neous	;		rou 0	-	cenar	neous		4	-		5 "B	Ben	ch 5 -				C B	-		Cap	
- grade existing			urvey	16					Т т.,	avel				rew S	ize			I	4	4		В Р		Loe Energ	Berm			в G		Backslo		
- nand shovel s	snow from ditches	t - tra w -	avel to	/Irom	site				- ID	avei										_	_	1P		Moose				SP		Water		
	COMMENTS: DES in		ed exca	avatio	n of a	storn	n wate	er ma	nagen	nent p	ond d	own-g	radie	nt of t	he fina	al ene	ergy di	ssipat	tion p	ond.			•									
	Hours and work effor	t refle	ect time	e onsi	ite ba	sed u	on o	ur obs	servat	ions a	nd are	e appr	oxima	ate.																		

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											DA	AILY (CON	STRL	JCTIC	ON S	UMN	1ARY													
			Client : Project : Task :	Farc	o Mine	Com	ring Lt plex C ion Co	losure		ures											: ation : k Area		Grun			Cell C ow	Cover				
Eq	uipment	0430	0700 0730	0800	0830	0060	0830	1000	1030	1100	1130	1200	1230	1300	1330	1400	1430	1500	1530	1600	1630	1700	1730	1800	1830	1900	Work Hours	Miscellaneous	Travel	Downtime	Total Hours
CHILKOOT	Tares Dhara, P.Eng.											1																_			
BOREAL	G.Nyland	1						1	1	1	1	1		1		1				1	1	1	1				1	İ –			1
	T.Moon																														
	Robert (Foreman)																														
T.Moon	Laborer 1 & 2																														
Construction	Laborers 3,4 & 5																														
	Argo (6 wheel drive)																														
	Tracked Excavator (Neil)		t m	C e4	e4	e4	e4	e4	t	e1	e1	lunch	n e1	e1	e1	e1	e1	e1	e1	e1	e1	e1	e1	t			10.0	0.0	1.0	0.0	11.0
0.51/	Volvo Rock Truck																														
SRK	Olga Kosarewicz, P.Eng.																														
	Christian																									_					
DES	Chris Jay				_											_										_					
DES	Cat 345 Excavator			_																											
	D9 Cat Bulldozer				_																					-					
YTG	Do Out Duildozei																														
110				1									1		1																
W	eather															1															
4																															
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b -		m - r	miscellane	ous				- Do	wntim	ie		I	Mater	ial	D	aily	Т	otal			1		ch 1 -				0		Securi		
c - cut/fill roadwa	ау		afety mee	ting				_													2		ch 2 -				F		Outflo		
d -			arked					- W	ork												3		ch 3 -				Н			Road	
e - excavate dito			edirect flow																-		4		ch 4 -				Р			ap Pit	
f - filling sandbag			p-rap plac	ement				- Mi	scella	neous	6				cellar	neous			_		5	Ben	ch 5 -				С			Cap	
g - grade existin	•		urvey				_					С	rew S	ize			-	1	-		ГВ			Berm		_	В		Backslo	pe Are	ea
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APPENDIX C

Conceptual Sketches

Storm Water Management Ponds



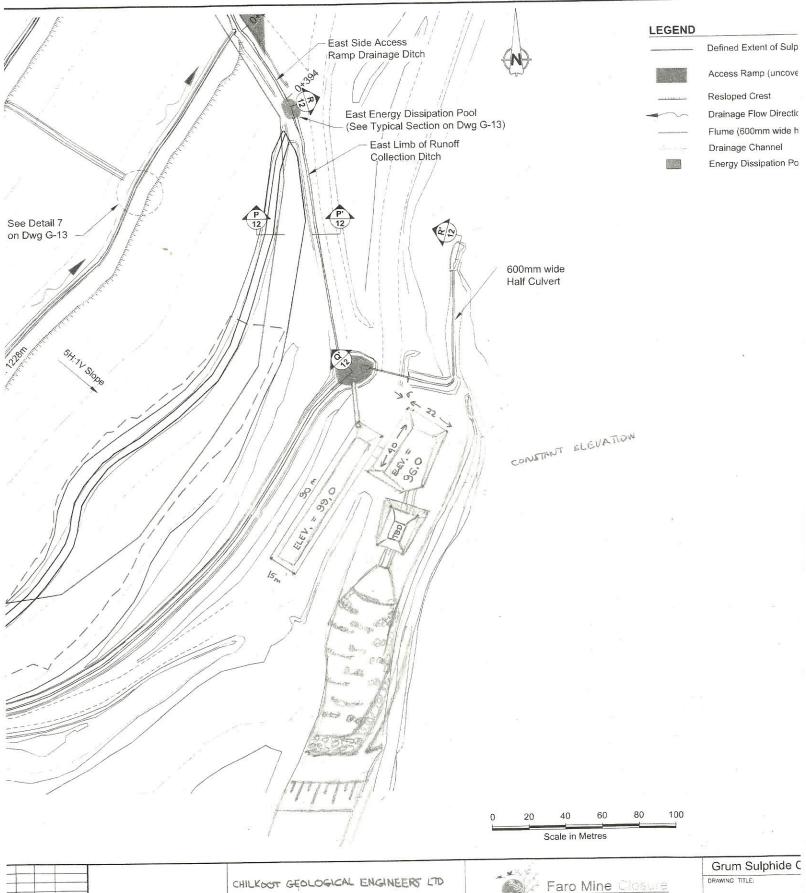
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 DATE:
 Apr. 30, 2011
 FINAL CLOSURE AND REMEDIATION PLAN
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 FILE NAME:
 1CY001_029-G-6_NAD83.dwg
 SRK JOB NO.:
 1CY001.029
 DRAWING NO

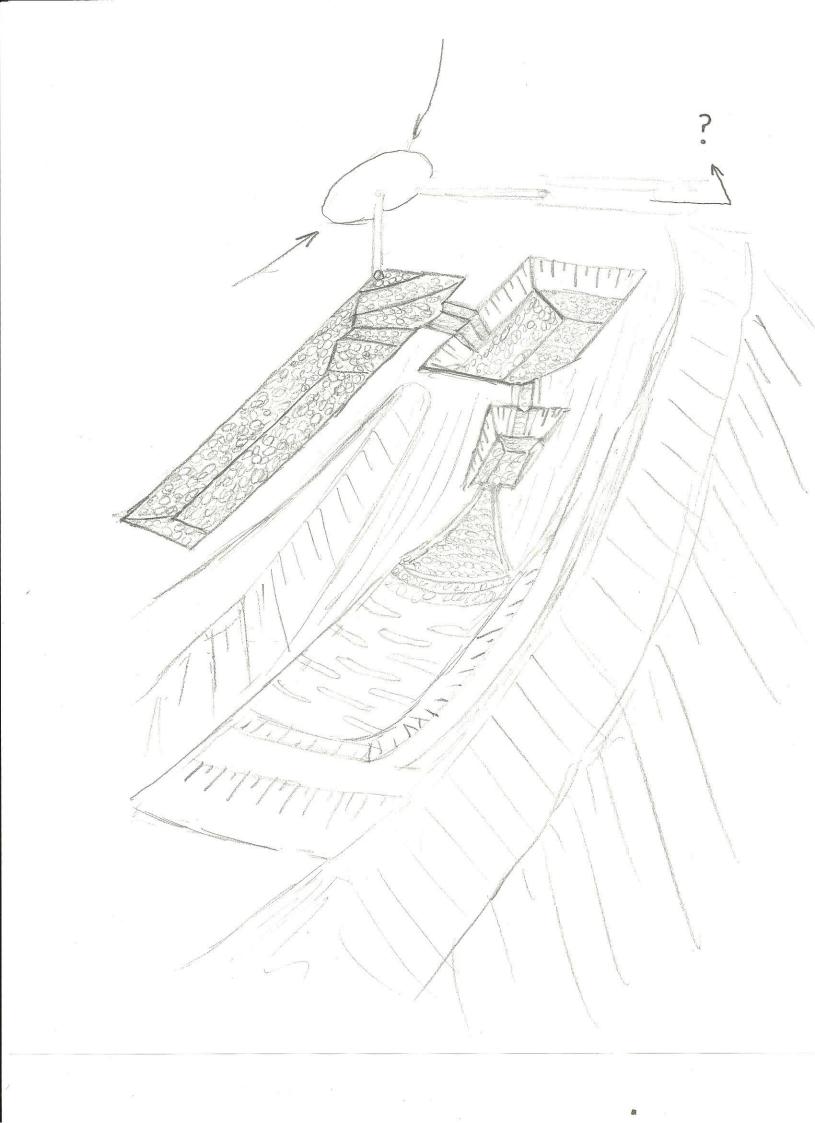
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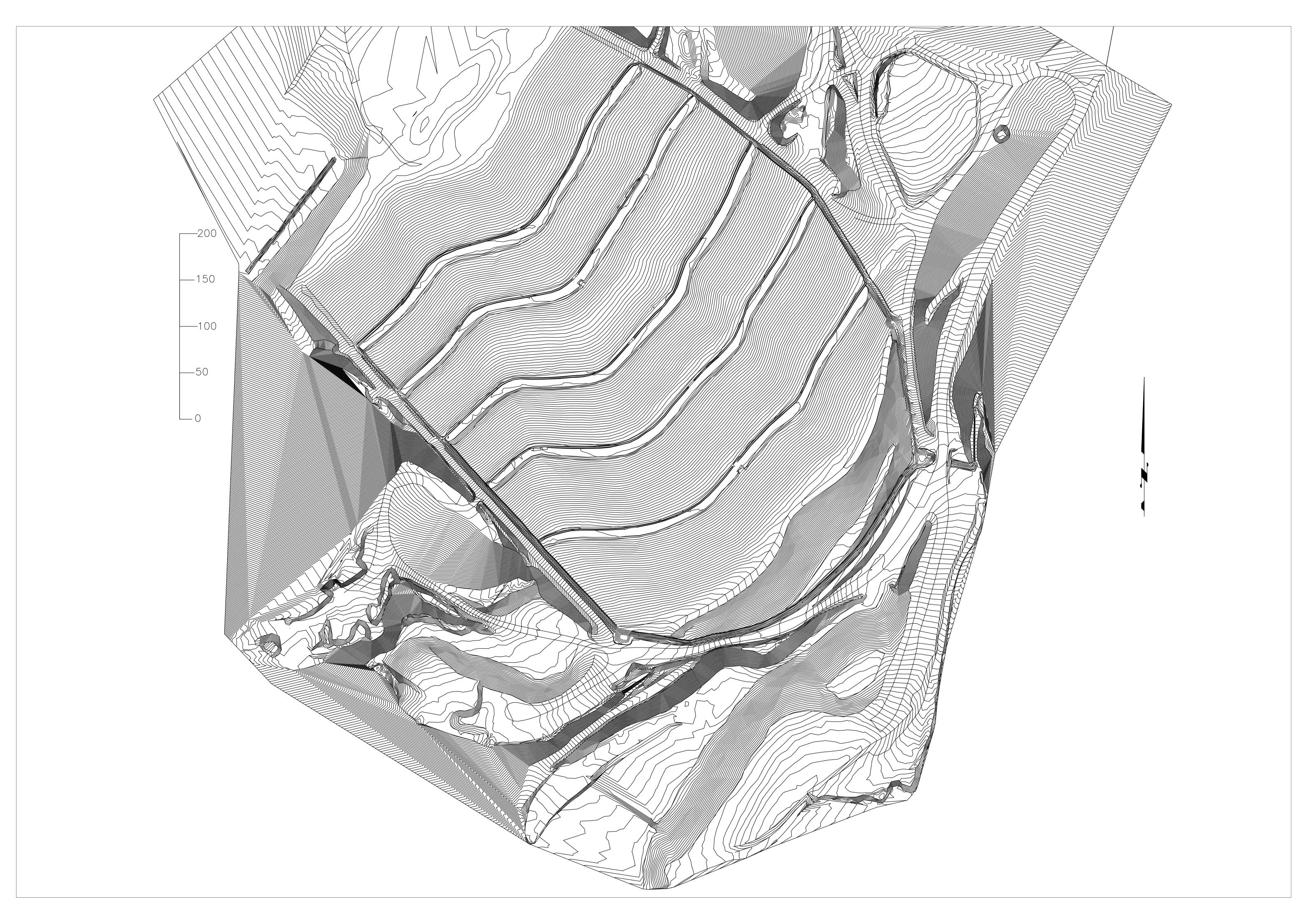


			(20)	CHILKOG	T GEOLOGICAL ENG	INEERS LTD	Faro Mine Closure Fersieure de la mine Faro	DRAWING TITLE:
PM		22Feb11		DESIGN:	DRAWN: TD	REVIEWED:	CONTRACTOR AND A CEMENT PONDS	DRAWING NO.
PM AT	CCS	28July10 18Jun10		CHECKED:	APPROVED:	DATE:	STORM WATER MANAGEMENT PONDS	
CHK'D APP'D DATE			PROFESSIONAL ENGINEER'S STAM	FILE NAME: 1CY001_029-G-6_NAD83.dwg			SRK JOB NO .: 1CY001 029	



APPENDIX D

0.5 m contour intervals of Fall 2010 As-builts



APPENDIX E

Selection of Photos





#1 - May 10, 2011 - Site Conditions as viewed from the Van Gorda Tailings piles



2 - May 20, 2011 – Cell conditions.

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#3 - May10, 2011 – Toe Berm conditions near liner elevation. Note buried snow.



#4 - May 16, 2011 – Erosion rills on cell cover below east side of bench 5.



5 - May 16, 2011 – Toe Berm conditions. Note sediment and erosion gullyies.



#6 - May 16, 2011 – Bench 5 ~ Station 0 + 080 Sand-bagging to redirect overflow.



#7 May 17, 2011 – Hand excavation of east side of Bench 4 ditch.



#8 - May 17, 2011 – Up-slope (beyond elev. 1300) conditions.



#9 - May 17, 2011 – Erosion gully at Toe Berm Crest ~ Station 0 + 080.



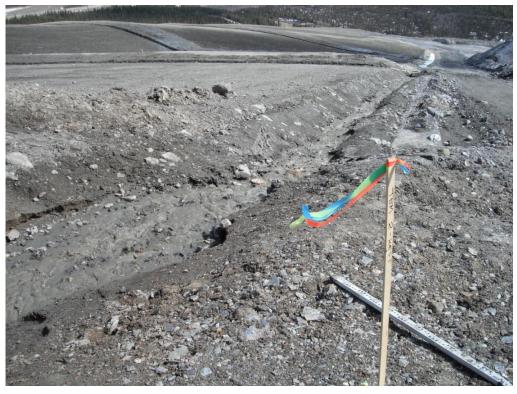
10 - May 18, 2011 – Outflow flue conditions.



11 - May 19, 2011 - Sedimentation of bench ditch. Bench 1 + 180 facing east.



12 - May 19, 2011 – Excavating ditch sediment from Bench 4 ditch.



#13 - May 21, 2011 – Bench 2 west side facing down slope. Note incising of ditch.



14 - May 21, 2011 – Excavation of emergency storm water management pond. Geotechnical Evaluation Faro Grum Sulphide Cell – Erosion Control Measures

Faro Mine Complex, Yukon 2011