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July 12, 2006
Project Number 1CD003.080

Deloitte & Touche Inc.
Suite 1900, 79 Wellington Street West
Toronto, ON, M5K 1B9
Canada

Attention: Doug Sedgwick

Dear Doug,

Re: Evaluation of the Vangorda Creek Diversion to Dixon Creek (2005-06, Task 20h)

1. Introduction

SRK Consulting (Canada) Inc. (SRK) has completed this evaluation of the proposed Dixon Creek route, which is one of the options currently under consideration for the long-term diversion of Vangorda Creek, at the Anvil Range Mining Complex, Faro, Yukon. This letter report was prepared for Deloitte & Touche on behalf of the Faro Mine Closure Planning Office and provides a summary of the results and conclusions of the evaluation.

2. Scope of work

The evaluation consisted of the following tasks:

Task 1: A geological and terrain stability evaluation overview
Task 2: Land survey along the proposed corridor
Task 3: Drilling program along the route
Task 4: Reporting

Details of the work carried out under Tasks 1, 2 and 3 are provided below.

3. Task 1: Geological and Terrain Stability Evaluation overview

EBA Engineering Consultants Ltd. (EBA) conducted a reconnaissance level assessment of the geology and terrain stability along the Dixon Creek route. The purpose of the evaluation was to assess the impact of the increased stream flow on the Dixon and Shrimp Creek catchments. The results and conclusions of the EBA evaluation are provided in Appendix A.

In summary, the stream flow of Dixon Creek is so small that there is no continuous surface channel of Dixon Creek for a considerable distance downstream from the proposed point of entry of the diversion channel. Significant potential environmental impacts to the upper watershed of Dixon Creek by the diversion of the full flow of Vangorda Creek would require extensive and major engineering works to mitigate.

4. Task 2: Land survey along the proposed corridor

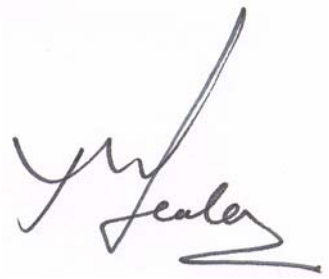
In order to fill some of the data gaps that were evident in the current topographic base map, SRK contracted Yukon Engineering Services (YES) to conduct a one-day survey of the corridor of the proposed Dixon Creek alignment. This survey extended from the junction with Vangorda Creek, along the route and down into Dixon Creek. The resulting alignments (horizontal and vertical) are shown in Figures 1 and 2.

5. Task 3: Drilling program along the route

To assess the soil conditions along the proposed route, SRK contracted Rocky Mountain Soil Sampling Inc. to drill a number of shallow holes along the proposed route of the diversion. Five holes (SRK05-DC-1 to SRK05-DC-5) were completed on August 12, 2005 using a small, manually portable drill. Each drill hole was logged by an inspector from SRK and representative samples were collected for potential laboratory analysis. Results of the field investigation and logs of the drillholes are provided in Appendix B. Based on the results of the overall evaluation, no laboratory testing was undertaken.

Yours truly,

SRK Consulting (Canada) Inc.



Peter Healey, P.Eng
Principal

Figures

Existing Earth Dam
at Head of Diversion

Vangorda Creek

TO DY

595,000 E

6,903,000 N

SRK05-DC-1

0+000

0+100

0+200

SRK05-DC-2

0+300

0+400

SRK05-DC-3

0+500

0+600

0+700

SRK05-DC-4

0+800

0+900

1+000

1+100

1+200

SRK05-DC-5

1+300

1+400

1+500

1+600

1+700

1+753.95

Dixon Creek

Vangorda Creek
Diversion

Original Channel of
Vangorda Creek

Existing
Diversion
Ditch

VANGORDA
PIT

W.L. 1089.88 (2003)

594,000 E

6,902,000 N

1210
1200
1190
1180
1170
1160
1150
1140
1130
1120
1110
1100

Date of Photography: 2003/07/25
Scale of Photography: 1:20000
Survey control derived from existing 1:20000 photography
Survey control based on: UTM Projection, NAD27
Compiled by The ORTHOSHOP, Calgary, September 2003
WO 8856

CONTOUR INTERVAL = 2m

0 50 100 150 200 250 300 350 400 450 500m

Scale 1:5000

SRK05-DC-2 DRILL HOLE LOCATION

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Vancouver

SRK JOB NO.: 1CD003.080

Acad-Vangorda-Grum\Acad-2006\ site_plan-Diversions-2006.dwg

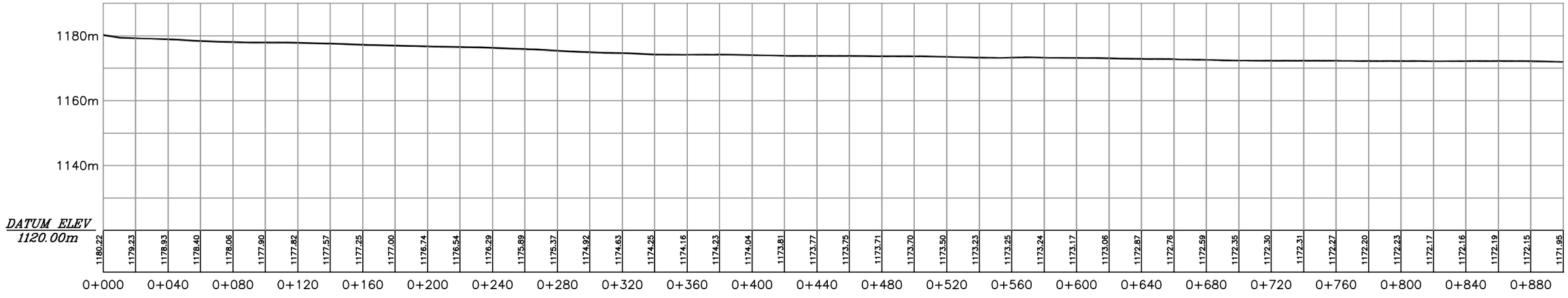
Deloitte & Touche

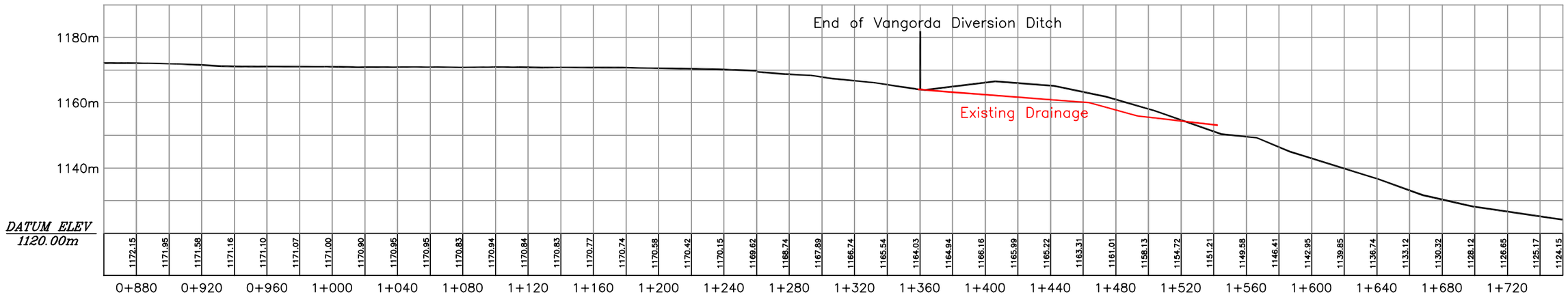
Vangorda Plateau Mine

Vangorda Creek Diversion - Dixon Creek Option

Proposed Realignment to Dixon Creek - Plan

DATE:	APPROVED:	FIGURE:
July 2006	PMH	1





Longitudinal Profile along ℄ of Existing Seepage Collection Ditch

0 25 50 75 100 125 150 175 200 225 250m

Scale 1:2500

VERTICAL EXAG. X2

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Engineers and Scientists
Vancouver

SRK JOB NO.: 1CD003.080

Acad-Vangorda-Grum\Acad-2006\ site_plan-Diversions-2006.dwg

Deloitte & Touche

Vangorda Plateau Mine

Vangorda Creek Diversion - Dixon Creek Option

Proposed Realignment to Dixon Creek - Profile

DATE:	APPROVED:	FIGURE:
July 2006	PMH	2

Photos

Photo 1: View of the existing seepage collection ditch above the Vangorda Pit. Site of one of the five drill holes completed by Rocky Mountain Soil Sampling Inc.

Photo 2: View of a soil sample from Drill Hole SRK05-DC-04

Photo 3: View along existing seepage collection ditch

Photo 4: Soil sample from Drill Hole SRK05-DC-03

Photo 5: Drilling along Dixon Creek

Photo 6: Sample extraction technique

Photo 7: Soil Sample from split spoon

Photo 8: Drillers with split spoon sampler

Photo 9: Opening the spilt spoon sampler

Appendix A
Geology and Terrain Stability Assessment (EBA)

March 9, 2006 EBA File No: 1200168

Steffen, Robertson and Kirsten (Can.) Inc.
Suite 800, 1066 West Hastings St.
Vancouver, BC V6E 3X2
TEL: [604] 681-4196 FAX: [604] 687-5532

Attention: Peter Healey, P.Eng.

Subject: Assessment of Geology and Terrain Stability
Upper Vangorda Creek Diversion Option 3, Anvil Mine Site, near Faro, YT

1.0 INTRODUCTION

This report presents the results of a reconnaissance level assessment of geology and terrain stability along a proposed route for a diversion channel of upper Vangorda Creek (Option 3) at the Anvil Mine Site near Faro, Yukon. Included herein is a description of the site, site conditions observed during a field assessment, and the resulting conclusions for route selection.

This report has been prepared in accordance with generally accepted geoscience practice and geoscience judgment has been used in the development of recommendations. For additional information regarding the use of this report, please refer to the attached General Conditions that form a part of this report.

Authorization to proceed with this project was provided by Mr. Peter Healey on July 29, 2005.

2.0 FIELDWORK

The site assessment was conducted on August 2, 2005 following an on-site meeting with P. Healey, P. Eng. The assessment included observations of exposed soil and bedrock along the existing interception ditch, a traverse of the route of the proposed Option 3 diversion to Dixon Creek and a traverse of the Dixon Creek channel from the point of entry of the proposed Vangorda diversion channel to a point about 1.5 km downstream.

3.0 OBSERVATIONS AND BACKGROUND INFORMATION

Vegetation Setting

The vegetation setting is spruce-aspen-willow-alpine birch-Labrador tea forest with some pine (Photograph 4). An area of spruce-sphagnum moss bog was crossed between the existing interception ditch and Dixon Creek (Figure 2). The floor of the Dixon Creek valley at the point of entry of the proposed Vangorda diversion channel is a wide, open bog/fen (Photograph 4).

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Geology

Bedrock exposed at the interception ditch (Station JD-1, Figure 2) is orange, rusty weathering, fissile phyllite (Photograph 1). This lithology was also exposed at an outcrop near Station 15+90 m (approximate location) and probably correlates with the Vangorda Creek Formation, which is mapped in the area. This unit was also exposed at an outcrop on steep valley side slopes of Dixon Creek near the road crossing, downstream of the proposed diversion plunge pool (Figure 2).

Existing Interception Ditch and Flow Direction

During development of the Anvil Mine, a surface water interception ditch was constructed upslope of the Vangorda open pit. The purpose of this structure was to collect and divert up-slope surface run-off, including small ephemeral watercourses, to reduce the volume of water entering Vangorda Pit. The proposed alignment of the Vangorda Creek diversion channel to Dixon Creek (Option 3) follows the existing ditch between Station 2+40 and Station 14+40 (Figure 2). The ditch is constructed in till from Station 2+40 m to about 11+00 m and Station 12+40 m to 14+40 m. From about Station 11+00 m to 12+40 m, the ditch is constructed in fissile, weathered phyllite bedrock, which is broken and soft enough to be machine-excavated. The interception ditch varies in depth from about 0.5 m (JD-5, Figure 2) to 4.0 m (JD-2). Note that Stations referred to are those shown on a conceptual plan dated April 2003 and do not correlate with a survey completed in August 2005.

A detailed survey to establish elevation and gradient of the interception ditch had not been completed prior to this assessment. Field observations indicated that a shallow, split gradient exists, with a drainage divide occurring at about Station 9+20 (Figure 2). A small, probably ephemeral stream intercepted by the ditch at Station 9+20 flows northwest towards Vangorda Creek. Southeast of Station 9+20, water collected by the interception ditch flows to the southeast.

A section prepared from a survey completed subsequent to the assessment indicates that the grade up to Station 3+40 is –1.8% to the southeast, a flat grade between actual station 3+40 and 9+10, a –5% grade to the southeast between 9+10 and 9+30 and a flat grade between 9+30 and 12+20. The proposed route of the Option 3 alignment southeast of approximate Station 14+20 (Figure 2) was not surveyed.

Most of the channel northwest of Station 9+20 m was dry and there was no evidence of recent scouring. Intercepted flows in this section are intermittent or ephemeral at best and probably most of the run-off collected in this section infiltrates the permeable loose till and fissile weathered bedrock. The effectiveness of the ditch to collect and divert surface run-off is probably limited.

Assessment of Terrain Between Present Interception Ditch and Dixon Creek

Station 14+00 m (Figure 2) is the approximate southeastern extent where the present interception ditch is aligned with the Option 3 diversion route. From this point the proposed alignment traverses gentle gradient forested terrain. At about Station 16+40 m (Figure 2), a 3 m high phyllite outcrop forms the northern edge of a 200 m wide flat, open, poorly-drained blueberry/willow/sphagnum moss bog with standing water in shallow depressions. A gentle to flat gradient, moderately to imperfectly drained spruce/aspen/willow forest forms a 60 m wide "island" within the bog.

Southeast of the bog area, the gentle gradient terrain is moderately well to well-drained aspen/spruce/alpine birch/willow forest. A 10 m long, 28% grade slope separates the gentle gradient terrain from the relatively flat terrain of the Dixon Creek valley floor (Photograph 4). The Dixon Creek valley floor is estimated to be 125 wide at this point.

The discontinuous, mostly sub-surface channel of Dixon Creek was located in the approximate centre of the valley floor, which is likely a remnant glaciofluvial outwash channel.

Dixon Creek Channel

The channel of Dixon Creek is discontinuous and flow is mostly sub-surface to a point about 700 m downstream of the proposed diversion channel (JD-6, Figure 2). At this point it joins a small tributary from the south to form a continuous channel, intermittently braided, up to 1 m wide and 0.25 m deep. The channel gradient here is about 5% where it is confined on the north bank by steep (70% gradient) valley side slopes of silty sand till with some gravel to gravelly, overlying shale bedrock (Road River Group?). The stream channel gradient appears to increase slightly downstream. The channel was not traversed downstream of this point.

4.0 CONCLUSIONS

Existing Interception Ditch

Field observations of soil texture suggest that the till and weathered bedrock substrate are permeable and unconsolidated. Near-surface bedrock is fissile, deeply weathered and incompetent. In sections where the proposed Vangorda Creek Diversion channel gradients exceed very shallow to flat grades, the probability of erosion is high and liners or other structures would likely be required to prevent scouring and infiltration.

Terrain Between Present Interception Ditch and Dixon Creek

Most of the proposed alignment crosses gentle, relatively consistent gradient terrain and there were no topographic features that would preclude construction of a diversion channel. However, in some sections of the proposed alignment the grade is greater than 5% and the flow would likely have to be confined within a structure to control potential erosion. The stream would also have to be confined within a structure where it crosses the area of bog that was identified during the field assessment (Figure 2).

Dixon Creek Channel

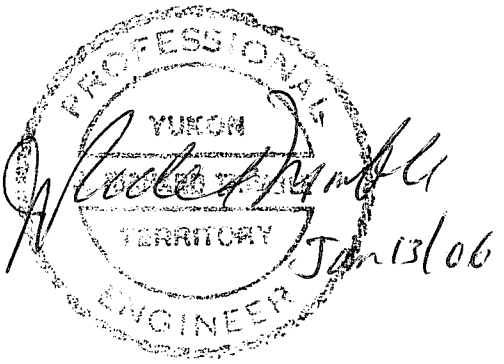
Likely the most significant limiting factor with the proposed Option 3 diversion of Vangorda Creek is the incompatibility of Vangorda Creek flows with the existing Dixon Creek valley and stream channel.

The area of the Vangorda Creek watershed upstream of the diversion is 17.2 km^2, which is over 6 times greater than the Dixon Creek watershed above the point of entry of the proposed diversion channel (2.8 km^2). The stream flow of Dixon Creek is so small that there is no continuous surface channel of Dixon Creek for a considerable distance downstream from the proposed point of entry of the diversion channel. Significant potential environmental impacts to the upper watershed of Dixon Creek by the

diversion of the full flow of Vangorda Creek would require extensive and major engineering works to mitigate. To further pursue this option, additional studies are recommended to assess environmental implications and the feasibility of construction of many kilometres of artificial channel or other containment structures.

We trust that this assessment report satisfies your requirements at this time. If clarification of any aspect of the report is required, or if you need additional information or assistance, please contact the undersigned.

Your truly,
EBA Engineering Consultants Ltd.



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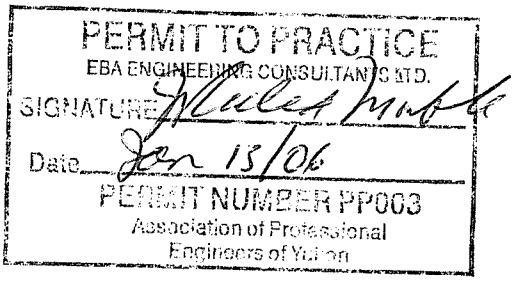
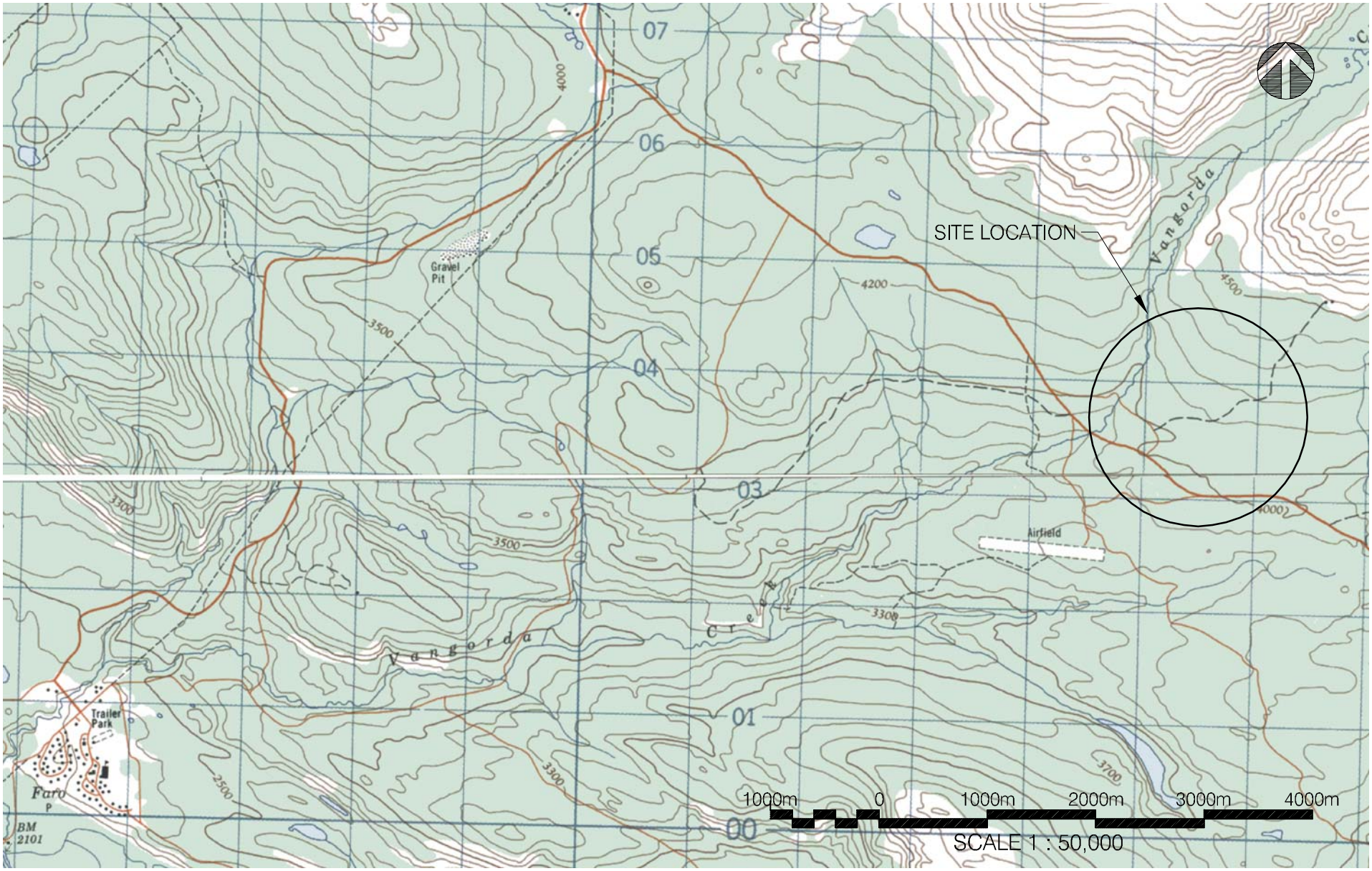


FIGURES



EBA Engineering Consultants Ltd.

PROJECT: UPPER VANGORDA CREEK DIVERSION OPTION 3 - GEOLOGY & TERRAIN STABILITY ASSESSMENT, ANVIL MINE SITE - FARO, YT.

CLIENT: STEFFEN, ROBERTSON & KIRSTEN (CAN.) INC.

TITLE: KEY PLAN

DATE	DWN.	CHKD.	FILE NO.	DRWG.
JAN. 2006	JSB	JTD	1200168	FIGURE 1

\\eba.local\corp\Whitehorse\Data\0201Drawings\Faro\1200168 Anvil Mine -Vangorda Crk Dvsn\1200168 Figure 1 Key Plan.dwg, 1/13/2006 4:01:11 PM, Adobe PDF, jbuyck

Contours interpolated from NTS 1:50,000 Map Sheet

Proposed Headworks Embankment

Vangorda Creek

SHALLOW GULLY IN TILL

6,903,000 N

595,000 E

SPHAGNUM MOSS BOG

GENTLE GRADIENT TILL VENEER OVER PHYLLITE BEDROCK

FLAT GRADIENT ORGANIC BLANKET/ VENEER

JD-1

JD-2

JD-3

JD-4

JD-5

JD-6

JD-7

Extent of Earthworks

Plunge Pool

Existing Diversion Ditch

℄ of Proposed Realignment to Dixon Creek

594,000 E

TILL SLOPE GENTLE GRADIENT HUMMOCKY

TILL VENEER OVER BEDROCK MODERATELY STEEP

TILL STOCKPILE

Dixon Creek

LEGEND:

- TERRAIN BOUNDARY
- FIELD OBSERVATIONS
- SUB-SURFACE WATERCOURSE
- 4WD TRAIL

NOTES:

1. DATUM - NAD 27
2. COORDINATES = UTM
3. CONTOUR INTERVAL 2 m.

0 50 100 150 200 250

Scale 1:5000

EBA Engineering Consultants Ltd.

PROJECT: UPPER VANGORDA CREEK DIVERSION OPTION 3 - GEOLOGY & TERRAIN STABILITY ASSESSMENT, ANVIL MINE SITE - FARO, YT.

CLIENT: STEFFEN, ROBERTSON & KIRSTEN (CAN.) INC.

TITLE: SITE PLAN WITH PROPOSED OPTION 3 ALIGNMENT

DATE	DWN.	CHKD.	FILE NO.	DRWG.
JAN. 2005	JSB	JTD	1200168	FIGURE 2

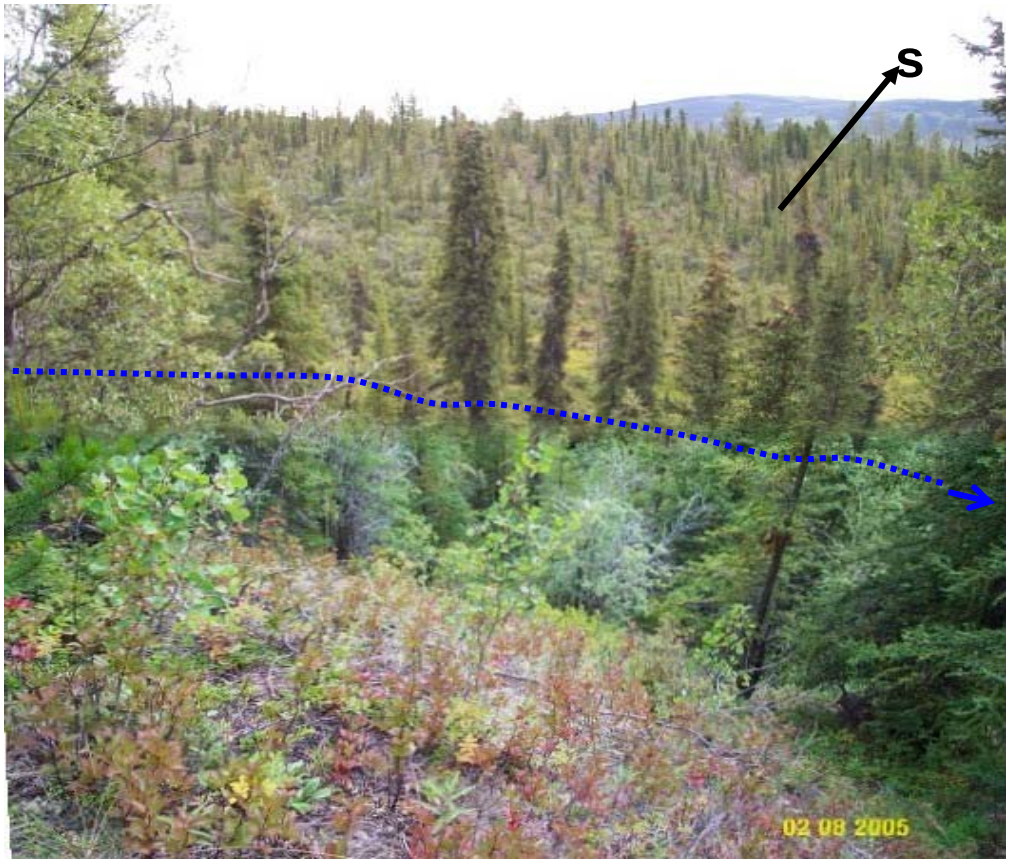
\\eba.local\corp\Whitehorse\Data\0201Drawings\Faro\1200168 Anvil Mine -Vangorda Crk Dvsn\1200168 Figure 2 Site Plan.dwg, 1/13/2006 4:05:18 PM, Adobe PDF, jbuyck

PHOTOGRAPHS

Photograph 1: Station JD1. Interception Ditch is approximately 4 m deep into rusty weathering, steeply dipping (~45° SW) fissile phyllite.

Photograph 2: Station JD7. Interception Ditch is approximately 3 m deep into till blanket overlying weathered phyllite.

Photograph 3: Station JD4. Berm constructed on downslope side of shallow Interception Ditch is about 1 m deep into till blanket.



Photograph 4: View southeast from vicinity of Station 20+00 to Dixon Creek Valley. Blue broken line shows estimated location of sub-surface watercourse. There is no continuous open stream channel at this location.

APPENDIX

APPENDIX A *

Notes from field reconnaissance, August 2, 2005

STATION	NOTES
JD-1	gentle gradient terrain; well drained; spruce aspen willow forest; ditch is approximately 5 m deep trench into rusty weathering, deeply weathered, fissile phyllite; bedrock is steeply dipping (approx. 45^{o}) SW; bedrock is overlain by a veneer of till; Mv-Mx/phRj; Photo 4 2:27 pm;
JD-2	Ditch is approximately 4 m deep into till; no bedrock exposed; terrain is same as at JD1
JD-3	Flow in ditch downstream (southeast) of this point emanates from a very small (ephemeral?) stream that breaches the east bank; ditch is dry upstream (northwest) of this point; ditch depth is 1 m; till is silty sand with some gravel; some weathered, rusty weathering phyllite is present indicating shallow bedrock; Mb/phR;
JD-4	Thick willow on down slope side of ditch berm suggests wet (moderately drained) conditions suitable for this vegetation; this suggests that the ditch was less than successful in the interruption of near surface groundwater, i.e., substrate, possibly both till and weathered bedrock, is permeable; till is sandy silt to silty sand with some gravel, well-drained; gentle gradient terrain; UTM 594292E; 6902794N;
JD-5	Ditch depth is 0.5 m in till
JD-6	Dixon Creek stream channel at approximately 625 m downstream of road crossing; at 500 m downstream of road crossing, the stream gradient is flat (<3%); no continuous developed stream channel at this point (intermittent); at 625 m, stream channel is developed and intermittently braided and 1 m wide, 0.25 m deep with a gradient of $\leq$ 5%; stream is confined by steep (70% gradient) NE valley side slopes; Mb/shR; SW valley side slopes are moderate gradient and vegetated (till?); UTM: 594475E; 6902248N; GPS elevation 1110m;
JD-7	Photo 3: till blanket; 2:21 pm.

This report incorporates and is subject to these "General Conditions".

A.1 USE OF REPORT AND OWNERSHIP

This geotechnical report pertains to a specific site, a specific development and a specific scope of work. It is not applicable to any other sites nor should it be relied upon for types of development other than that to which it refers. Any variation from the site or development would necessitate a supplementary geotechnical assessment.

This report and the recommendations contained in it are intended for the sole use of EBA's client. EBA does not accept any responsibility for the accuracy of any of the data, the analyses or the recommendations contained or referenced in the report when the report is used or relied upon by any party other than EBA's client unless otherwise authorized in writing by EBA. Any unauthorized use of the report is at the sole risk of the user.

This report is subject to copyright and shall not be reproduced either wholly or in part without the prior, written permission of EBA. Additional copies of the report, if required, may be obtained upon request.

A.2 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. EBA does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

A.3 LOGS OF TEST HOLES

The test hole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance that requires precise definition of soil or rock zone transition elevations may require further investigation and review.

A.4 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. EBA does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.

A.5 SURFACE WATER AND GROUNDWATER CONDITIONS

Surface and groundwater conditions mentioned in this report are those observed at the times recorded in the report. These conditions vary with geological detail between observation sites; annual, seasonal and special meteorologic conditions; and with development activity. Interpretation of water conditions from observations and records is judgmental and constitutes an evaluation of circumstances as influenced by geology, meteorology and development activity. Deviations from these observations may occur during the course of development activities.

A.6 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance that can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

A.7 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

A.8 INFLUENCE OF CONSTRUCTION ACTIVITY

There is a direct correlation between construction activity and structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer, when the final design and construction techniques are known.

A.9 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, as well as the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

A.10 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems that will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

A.11 BEARING CAPACITY

Design bearing capacities, loads and allowable stresses quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition assumed. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions assumed in this report in fact exist at the site.

A.12 SAMPLES

EBA will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the client's expense upon written request, otherwise samples will be discarded.

A.13 STANDARD OF CARE

Services performed by EBA for this report have been conducted in a manner consistent with the level of skill ordinarily exercised by members of the profession currently practising under similar conditions in the jurisdiction in which the services are provided. Engineering judgement has been applied in developing the conclusions and/or recommendations provided in this report. No warranty or guarantee, express or implied, is made concerning the test results, comments, recommendations, or any other portion of this report.

A.14 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, EBA has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

A.15 ALTERNATE REPORT FORMAT

Where EBA submits both electronic file and hard copy versions of reports, drawings and other project-related documents and deliverables (collectively termed EBA's instruments of professional service), the Client agrees that only the signed and sealed hard copy versions shall be considered final and legally binding. The hard copy versions submitted by EBA shall be the original documents for record and working purposes, and, in the event of a dispute or discrepancies, the hard copy versions shall govern over the electronic versions. Furthermore, the Client agrees and waives all future right of dispute that the original hard copy signed version archived by EBA shall be deemed to be the overall original for the Project.

The Client agrees that both electronic file and hard copy versions of EBA's instruments of professional service shall not, under any circumstances, no matter who owns or uses them, be altered by any party except EBA. The Client warrants that EBA's instruments of professional service will be used only and exactly as submitted by EBA.

The Client recognizes and agrees that electronic files submitted by EBA have been prepared and submitted using specific software and hardware systems. EBA makes no representation about the compatibility of these files with the Client's current or future software and hardware systems.

Appendix B
Drill Hole Logs

SRK05-DC-1

GPS (NAD27) readings		Drill hole completed on August 12, 2005 by Rocky Mountain Soil Sampling Inc. Logged by SRK (M. Prado)		
E 594,593	N 6,903,426			
Interval (m)				
From	To	Soil type	Soil description	Notes
0.0	0.4	Sand	Fine to coarse sand, trace silt, minor fine gravel, brown, loose, damp.	
0.4	3.3	Sandy silt	Sandy silt (fine to coarse sand), minor clay, with fine to coarse, sub-angular to sub-rounded gravel, brown-gray, friable, damp.	Till. Sample collected.
3.3	4.0	Silty sand	Silty fine to coarse sand, trace clay, with fine to coarse, sub-angular to sub-rounded gravel, brown, loose, damp. Or silty sand.	
4.0	5.3	Sandy silt	As above, with increased amount of fines (silt, minor clay). From ~ 4.5m, increased amount of clay (sandy clay-silt).	

SRK05-DC-2

GPS (NAD27) readings		Drill hole completed on August 12, 2005 by Rocky Mountain Soil Sampling Inc. Logged by SRK (M. Prado)		
E 594,480	N 6,903,314			
A				
Interval (m)				
From	To	Soil type	Soil description	Notes
0.0	0.2	Sandy-clayey silt	Sandy-clayey silt (fine to coarse sand), with fine to coarse, sub-angular to sub-rounded gravel, brownish-gray, with organic matter (roots).	
0.2	2.8	Sandy-clayey silt	Sandy-clayey silt (fine to coarse sand), with fine to coarse, sub-angular to sub-rounded gravel, gray with scattered brown nodes (diam~5mm), damp, with low plasticity.	Till. Increased amount of clay.
Refusal at 2.87m, no indication of bedrock, borehole re-located about 5m along the ditch (see description below).				

B				
Interval (m)				
From	To	Soil type	Soil description	Notes
0.0	0.6	Silty sand	Silty fine to medium sand, trace clay, with minor fine gravel, brown, with organic matter, loose, damp.	
0.6	5.3	Sandy-clayey silt	Sandy-clayey silt (fine to coarse sand), with fine to coarse, sub-angular to sub-rounded gravel, gray with scattered brown nodes (diam~5mm), damp, with low plasticity.	Clayey till. Harder from ~ 4.0m. Sample collected (3.0-5.3m).

SRK05-DC-3

GPS (NAD27) readings		Drill hole completed on August 12, 2005 by Rocky Mountain Soil Sampling Inc. Logged by SRK (M. Prado)		
E 594,523	N 6,903,122			
Interval (m)				
From	To	Soil type	Soil description	Notes
0.0	5.3	Sandy-clayey silt	Sandy-clayey silt (fine to coarse sand), with fine to coarse, sub-angular to sub-rounded gravel, gray, stiff, damp, with low plasticity (thread diam~3mm).	Till gets harder and slightly more plastic with depth. Sample collected.

SRK05-DC-4

GPS (NAD27) readings		Drill hole completed on August 12, 2005 by Rocky Mountain Soil Sampling Inc. Logged by SRK (M. Prado)		
E	N			
594,705	6,902,977			
Interval (m)		Location description: Inside ditch, about 3m below road level		
From	To	Soil type	Soil description	Notes
0.0	0.5	Sandy-clayey silt	Sandy-clayey silt (fine to coarse sand), with fine to coarse, sub-angular to sub-rounded gravel, gray, loose, damp.	Till. Sampled.
0.5	1.0	Bedrock	Weathered bedrock (schist).	Refusal at 1.0 mbgl.

SRK05-DC-5

GPS (NAD27) readings		Drill hole completed on August 12, 2005 by Rocky Mountain Soil Sampling Inc. Logged by SRK (M. Prado)		
E	N			
594,986	6,902,503			
Interval (m)		Location description: in the bush, about 50m from the access road		
From	To	Soil type	Soil description	Notes
0.0	0.1	Top cover	Top organic soil layer (moss covered). Peat, dark brown to black.	
0.1	0.3	Clayey silt	Clayey silt, with minor fine-coarse sand, brown, loose, damp, with roots/organic matter.	Sample.
0.3	0.9	Bedrock	Weathered bedrock (schist). Well preserved foliation, gray, friable, dry.	Sample. Refusal at 0.9m.