Minto Explorations Ltd.

A SUBSIDIARY OF CAPSTONE MINING LTD.

APPENDIX Q
Dry Stack Tailings Storage Facility Update Report

September 7, 2010 EBA File: W14101068.001

Minto Explorations Ltd.
Suite 900, 999 West Hastings Street
Vancouver, BC V6C 2W2

Attention: Mr. Jaime Delgado
Acting General Manager

Dear Sir:

Subject: Dry Stack Tailings Storage Facility
Foundation Movement Monitoring Update, Minto Mine, YT

1.0 INTRODUCTION

Instrumentation installed within the footprint of the dry stack tailings storage facility (DSTSF) have indicated that movements are occurring deep within the foundation soils. This letter report can be considered a summary of current conditions from which recommendations for future action have been developed. The report summarizes data collected to date from instrumentation installed at the DSTSF, adjacent ore stockpile and airstrip laydown site. The significance of those movements to current operations, risk of failure and plans for continued monitoring are reviewed. The movements are not considered an imminent hazard to the system operation at present, however, they require an enhancement of the monitoring system and further evaluation of potential slope failure mechanisms in order to develop a clear understanding of their cause, potential effects and confirm appropriate mitigative measures.

Previous related reporting can be found in Technical Memo No. DSTSF-6 issued by EBA on April 23, 2010 and in the Dry Stack Tailings Storage Facility – 2009/2010 Annual Review report issued on July 20, 2010. Memo DSTSF-6 summarizes the soil stratigraphy and the instrumentation program undertaken as part of 2010 winter drilling program. The annual review report consists of a summary of the findings resulting from a site visit on April 27, 2010 completed to examine the physical condition of the DSTSF and the construction quality assurance and monitoring instrumentation data collected between May 1, 2009 and April 30, 2010. This report includes site observations from a reconnaissance trip by Messer's Berkers, Cutts, Hayley, and Dixon of EBA on August 18, 2010.

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2.0 MONITORING INSTRUMENTATION DATA

A summary of the instrumentation installed to date can be found in Table 1. The location of each instrument can be seen on Figures DSTSF-INC-4. Background information related to the rationale for the installations are presented in the annual report.

TABLE 1: DSTSF INSTRUMENTATION INSTALLED TO DATE

Instruments	Instrument Type	Date Installed	Still Functional?
96-G08	Ground Temperature Cable	1996	No
DST-1, -2, -5	Ground Temperature Cable	November 2007	Yes
DSP-1 -2	Vibrating Wire Piezometer	November 2007	Yes
DSSH-1, -2	Survey Hub	November 2007	Yes
DST-8, 9	Nested Single Bead Thermistors	March 2009	No
DST-3, -4, -6, -7	Ground Temperature Cable	February 2010	Yes
DSP-3, -4	Vibrating Wire Piezometer	February 2010	Yes
DSI-1, -2	Inclinometer	February 2010	No
DSI-3, -4	Inclinometer	April 2010	No
DSI-5	Inclinometer	April 2010	Yes
DSSH-5 to -12	Survey Hub	April 2010	Yes

2.1 GROUND TEMPERATURE CABLES

Ground temperature profiles from the seven functioning ground temperature cable locations within the DSTSF are presented in Figures T1 through T7. Readings from DST-8 and -9 and 96-G08 are presented in the annual report, no new data is available, as these instruments are no longer functioning.

The profiles for DST-1 through -4, which are located within the drainage blanket/starter bench, indicate that permafrost in the foundation soils beneath the DSTSF has been maintained to date. These instruments monitor the foundation soils to a depth of up to 10 m below the predevelopment surface. The results for DST-5, which acts as a background instrument outside the footprint of the DSTSF, indicate a seasonal active layer of approximately 2.5 m at that location. DST-6 and -7 have reached equilibrium and indicate permafrost conditions within the monitoring depths of approximately 5 m and 20 m below the predevelopment surface. The July 18, 2010 readings for DST-6 seem questionable; and as such, these values need to be confirmed with the September 2010 readings.

All the ground temperature cables currently installed terminate above the noted failure plane within the foundation soils. However, it should be noted that the permafrost was noted in all inclinometer boreholes with visual and non-visual ice observations throughout the overburden to bedrock.

2.2 VIBRATING WIRE PIEZOMETERS

Although two vibrating wire piezometers were installed in the fall of 2007 and an additional two in February of 2010, no piezometric data is currently available, as the foundation soils have remained frozen. Figures VP01 and VP02 show the ground temperature at each piezometer tip with time. The piezometers were installed to measure porewater pressure only if and when an active layer is established in the foundation soils.

2.3 SURVEY HUBS

Total lateral displacements, rates of displacement, and changes in elevations are presented in Table 2 for DSSH-1 to -12 (except for DSSH-3 and -4, which do not exist). This data presents the change in location from initial reading.

TABLE 2: SURVEY HUB SUMMARY TABLE – CHANGE IN LOCATION FROM INITIAL READING

Instrument	Total Lateral Displacement (mm)	Average Daily Lateral Displacement Rate (mm/d)	Total Change in Elevation (mm)
DSSH-1	1498	1.9	-471
DSSH-2	1725	2.0	0
DSSH-5	391	3.8	15
DSSH-6	402	3.9	-59
DSSH-7	377	3.6	63
DSSH-8	441	4.2	63
DSSH-9	247	2.4	92
DSSH-10	66	0.6	11
DSSH-11	17	0.2	5
DSSH-12	316	3.0	-140

Note: Reading period for DSSH-1 and -2 is November 23, 2009 to May 18, 2010. Reading period for DSSH-5 to -12 is April 6, 2010 to present.

It should be noted that DSSH-1 and -2 are surveyed differently than DSSH-5 though -12. DSSH-1 and -2 are surveyed with a GPS unit referenced to a steel bar at the location. Therefore, it is difficult to produce repeatable results although the relative movements are useful. To produce better repeatability, the installation of DSSH-5 through -12 allow for the GPS unit to be fixed to the survey hub. Therefore, these results are considered to provide greater accuracy.

The data indicates that all of the survey hubs, except for DSSH-11 are experiencing movement. The relative lateral movement rates for DSSH-1 and DSSH-2, presented in Figure SH01, indicate that the rates have increased since installation, and are currently in the order of 3 mm/day to 4 mm/day. These current rates are consistent with the readings from DSSH-5 through -12. The rate of lateral movement for the DSSH-5 through -12 locations, presented in Figure SH03, have been consistent over the last three months as determined by the linear nature of the data. Drawing DSTSF-INC-9 presents the movement rates and scaled vectors for the survey hubs. Survey hub coverage throughout the central and east portion of the waste rock shell is quite good; while additional coverage is required in the south and west portion of the facility.

The change in elevation for DSSH-1 and DSSH-2 are presented in SH04. DSSH-1 shows relatively consistent drop in elevation over time. DSSH-2 shows no change in elevation over time. The change in elevation for DSSH-5 through DSSH-12 are presented in SH05. DSSH-5, -10, and -11 show no change in elevation while DSSH-6 and -12 show a drop in elevation. The remaining hubs, DSSH-7, -8, and -9, show a rise in elevation. Drawing DSTSF-INC-10 presents the change in elevation for each survey hub since installation.

2.4 INCLINOMETERS

Inclinometers are slotted vertical pipes firmly grouted into deep boreholes into which an instrument is lowered from time-to-time to measure incremental changes in the slope of the pipe. These increments, when accumulated from the bottom-up accurately identify if and where lateral movements are taking place. Data from the five inclinometers (DSI-1 through -5) is summarized in the attached inclinometer plots (Shown in Figures Inclinometer DSI-1 to -5) and the associated movement rates presented in Figure I01 to I06. It should be noted that when a shear plane is identified within a slope indicator pipe the movement across that plane will eventually squeeze the pipe to the point where the instrument will no longer pass. This has recently happened to most of the instruments installed in April 2010 thus only DSI-5 remains functional at the time of report preparation.

Movement rates for DSI-1 were between 2.4 mm/day and 4.5 mm/day during the monitoring period. Accelerated movement was noted between February 24 and March 1, 2010, but become relatively consistent after that. Movement rates for DSI-2 were more consistent throughout the monitoring period and ranged between 2.0 mm/day and 2.8 mm/day.

With the exception of the second reading, which was taken a day after initialization, the movement rates for DSI-3 and DSI-4 have been consistent over the period of observation. The data scatter during the first few days can be attributed to curing of the grout backfill. Movement rates for DSI-3 and DSI-4 were between 3.6 mm/day and 4.2 mm/day and 2.7 mm/day and 3.4 mm/day, respectively.

Movement rates for DSI-5 (located furthest from the front face) indicate a decreasing trend with a rate of 0.6 mm/day on July 7, 2010 which is the lowest to date. The remaining readings range between 0.6 mm/day and 2.1 mm/day. Again, the second reading also shows the highest rate as in DSI-3 and DSI-4.

2.5 CORRELATION OF SURVEY HUBS AND INCLINOMETERS

Comparison of the survey hub and inclinometer data indicates that there is a good correlation with the lateral movements observed in the survey hubs and those observed in the inclinometers. Therefore, the survey hub data provides information on the extent of the deep-seated movement and a means of monitoring the movement as the inclinometers pinch off at the failure plane and are no longer readable. These results also confirm that all of the movement is occurring at substantial depth on a well defined shear zone within the foundation.

3.0 DSTSF CONSTRUCTION

Typical tailings placement within the DSTSF occurs at a rate of approximately 3000 tonnes per day and is placed within the grey shaded area on DSTSF-INC-4. The waste rock shell cover is placed in benches. The last two waste rock shell benches, Bench 6 and Bench 7, were constructed in July 2009 and January 2010, respectively. Bench 6 consisted of approximately 66,300 tonnes while bench 7 consisted of approximately 80,300 tonnes. Survey hubs DSSH-1 and -2 indicate increased movement rates during these time periods. This placement occurred before the installation of the inclinometers and survey hubs DSSH-5 through -12 so any effect on the movement rates cannot be commented on using these instruments.

4.0 ORE STOCKPILE AND AIRSTRIP LAYDOWN DATA

Two ore stockpile and laydown areas are located along the western extent of the DSTSF. These are labelled the North Green and Yellow Ore Stockpile and Airport Laydown and the South Green Ore Stockpile and are shown on Drawing DSTSF-Inc-4. These stockpiles add load to the DSTSF foundation soils therefore the data was examined to determine if there was a correlation between observed movements and pile size. The cumulative tonnage of these areas over time have been compared to the movement rates noted in the survey hubs and inclinometers and presented in Figure C01 through C03. The data suggests that that the size of the stockpile does have an effect on foundation movement rates.

5.0 STABILITY OF THE DSTSF

5.1 DSTSF FOUNDATION MOVEMENTS

The DSTSF occupies the south slope of Minto Creek valley, immediately east of the Area 1 Pit. A view of the facility from across the valley is shown in Photo 1. The geotechnical information available at the time the design was prepared in 2006 was collected in 1994 and 1996 for an earlier mine development proposal. This data did not include reference to the unexpected thickness of fine grained overburden soils that extend to depths greater than 50 m, which have been exposed in the mining at the Area 1 Pit and recorded in recent investigations. The soils are in a permafrost condition and contained substantial ground ice in the form of thick lenses and massive ice bodies with some thicknesses exceeding 1 metre. The presence of clay soils with substantial ground ice required a redesign of the final wall slopes and selective excavation of the frozen ground. Photo 2 is a photograph of an exposure of permafrost clay and till with massive ice exposures, taken during the site reconnaissance on August 18, 2010, as the excavation of the south wall 810 m bench was ongoing.

The deep deposits of ice-rich permafrost clay and clay till encountered in the south slope, of the Minto Creek valley and exposed in the Area 1 Pit were not expected nor were they predictable from the drilling that was conducted. These warm permafrost clay deposits extend eastward below the south valley wall and underlie the core of the DSTSF. Boreholes drilled in this area at various stages of mine planning in 1996 encountered mostly granular soils that are a weathering product of the upland country rock. Although these granular soils were in a permafrost condition, they were judged to not pose any particular hazard to the stability of the proposed tailings stack. Data obtained from the 2010 instrumentation program has shown that a substantial clay layer is present below the granular soils at depths below original ground in the order of 35 m (DSI4) and the clay seams extend to depths in the order of 60 m below original ground before bedrock is encountered. Drilling and sampling of the deep permafrost clay deposit identified layers of ice up to 1 metre thick (DSI4 at 53 m, April 2010). These deep clays with warm segregated ground ice have been clearly identified in all the slope indicator data as the zone where movements are now occurring.

The geomorphology of the Minto Creek valley is clearly much more complex than surface conditions within this glacial valley would imply. Deglaciation must have been marked by ice dams that formed deep ponds or lakes within the valley that were gradually infilled with lacustrine sediments. These clays and silts were subsequently overridden by glacial moraine and covered with residual and colluvial soils washed from the slopes above. When the glacier ice abated, the surfaces were exposed to a periglacial climate where permafrost formed from the surface down. Some of the massive ice found in the valley may be covered ancient glacier ice but much of it would have formed in place as the freezing front advanced, and ground water was fed from below.

The movement data, when correlated to the geometry of the valley slope and the location of the tailings stack clearly shows that the failure is confined to the deep clay seams and that a large block of the foundation is creeping slowly toward the centre of the valley taking the intact tailings stack with it. On a number of occasions, EBA personnel have carefully inspected the area around the perimeter of the tailings stack for visible indicators of movement. Visible indicators might include surface cracking on slope crests, a scarp that suggests failure, or heave within the valley bottom. The depth of the movement, as identified with existing instrumentation is along a plane inclined at about 5 degrees, located at approximately 20 m to 30 m below the deepest portion of the valley. There has been no surface evidence to suggest a slope failure is occurring nor is there any other surface indication that the tailings stack is moving. The movement mechanism is believed to be creep within the warm ice rich permafrost clay resulting from a slight increase in shear stress imposed by the tailings stack and ore storage pile. This mechanism is analogous to natural observations that ice near the melting point at the base of a glacier or within a rock glacier under shear stress creeps downslope at a relatively constant rate.

The dry stack has been designed to be self-supporting. There are no retaining structures to fail allowing rapid movement of materials into the valley bottom. The rock cover obvious on the outward face is a surface cover designed for closure and to prevent surface erosion. The stack by design would remain in place if the surface facing of rock fill were removed. In consideration of the importance of this statement and the need to confirm that an overall failure would not result in liquefaction or significant flow of tailings from the stack into the creek valley or the reservoir behind the dam, we are proposing to measure insitu properties and strength parameters within the deeper parts of the tailings stack.

5.2 THAW STABILITY OF THE DSTSF

Data from ground temperature cables and piezometer installations when examined together with the 2010 instrumentation drilling program confirm that the permafrost foundation soils present pre-development have remained frozen. The movements do not appear to be related to permafrost thaw.

6.0 CONCLUSIONS

At present, it is clear that the DSTSF is experiencing on-going creep-type deformation of warm frozen (permafrost) foundation soils along a very deep-seated zone within the underlying, native clay soils. Although different monitoring points and inclinometers are indicating slightly different rates of movement, it appears that the movement is essentially constant in a direction into the valley bottom. Creep of frozen soils can carry on until a threshold is reached and failure occurs or there is relaxation of shear stress. Any failure should be preceded by noticeable acceleration of the movements.

A failure in this case could include a loss of a portion of the rock slope cover and a bulging of the valley bottom (heave). Neither would be of severe consequence, as no material would be released to an uncontrolled environment (beyond the dam). Measures to counter the movement by valley infill have been suggested and discussed and are now in the development plan that is subject to the approval of the recently submitted permit application. It is EBA's understanding that there is no suitable waste rock available until early 2011 in order to implement that plan. A continuation with enhancements of the program of monitoring and observation with timely reporting is considered a reasonable work plan at present unless significant acceleration is noted.

7.0 RECOMMENDATIONS

EBA recommends the following components be enhanced or added to Minto's current action plan to allow analyses and improve our understanding of potential failure mechanisms. This will allow refinement of measures to reduce movement rates over the medium and long term.

On-going Monitoring and Testing

To determine the most practical and cost-effective mitigation options, it is important that the boundaries of the movement be more clearly defined and information continues to be collected that defines the magnitude, rate, and direction. The geotechnical characteristics of the clay layer, which appears to contain the slip surface, need to be further quantified for more detailed analyses and recommendations. The following recommendations will enable the collection of the needed data.

- Install ground temperature cables within accessible inclinometer pipe that have ceased to provide movement data to confirm the ground temperatures down to and within the vicinity of the failure plane. Additional holes may be required because many of the inclinometer pipes that have ceased to provide movement data are not open to a depth below the failure plane.
- Review the thermal design and the actual construction, instrumentation, and metrological data information.
- Monitor survey hubs DSSH-01 and 02 and DSSH-05 through -12 on a weekly basis. Note that DSSH-01 and 02 have not been read since May 18, 2010 and on-going monitoring is required.
- Upgrade DSSH-1 and -02 to allow the GPS unit to be attached to the survey hub, similar to DSSH-05 to -12.
- Survey and monitor the top of pipe location of DSI-1 through -5.
- Monitor DSI-5 on a weekly basis.
- Continue to observe on a systematic basis both the valley bottom and DSTSF for signs of distress.
- Install additional survey hubs and inclinometers to delineate the extent of movement. This could be a phased approach depending on the results observed.
- Complete and compile laboratory testing (currently underway) for the overburden samples from the 2010 winter drilling program. This should include some triaxial shear testing on samples of the permafrost clay layer that is known to be within the zone of movement. The current monitoring has allowed a determination of the depth of the failure surface, which will allow our laboratory test program to carefully select the appropriate samples.

- Compare compaction testing moisture content and the filter building moisture content data to assist with correlating moisture conditions within the DSTSF.
- Install protection for existing and future instrumentation.
- Develop a water management plan. Combine a review of the location of observed water and ice within the DSTSF, the effectiveness of the water diversion structures and other water inputs to provide recommendations to address surface and ground water issues that could affect slope movements.

Analysis

The complex movement of the slide cannot be modeled using standard 2-D limit equilibrium based software. We recommend that a finite-element software package, such as PLAXIS or FLAC, be used to model the time dependent nature of the movement. There appears to be a 3-D component to the movement, however, a standard finite-element model will allow us to more cost-effectively undertake a sensitivity type analysis.

Confirmation of Insitu Properties of Compacted Tailings

A program to evaluate the density and strength of the compacted tailings within the embankment is recommended. The purpose is to determine if the there is a risk of tailings liquefaction should an overall failure of the surface rock cover occur. The secondary objective will be to evaluate the potential effect that run-on water both surface and from the finger drains has on the behaviour of the tailings stack. This testing is for confirmation, as there is no indication that the integrity of the stack itself has been compromised by the deep-seated foundation movements.

The program would comprise a program of cone penetrometer tests (CPT) where the force to push a standard cone into the tailings stack is measured. Frozen layers within the stack will be identified and a series of properties related to strength and density will be measured. It is recommended that this testing program proceed as soon as practical in order to assure external reviewers that the consequences of a failure are not severe.

Current Mitigation Strategies

- Buttress
  - A "valley-fill" buttress below the DSTSF to arrest the slope movement given the nature and magnitude of the distress. At present, the geotechnical models are not complete enough to determine the geometry, volume, and layout of the probable buttress. This information will evolve from the tasks outlined above.
  - It is envisioned that the material for the buttress will be sourced from the development of the proposed new Area 2 Pit.
- Ore Stockpile and/or Airstrip Laydown Fill Placement

- An assessment of the effects of material loading should be completed prior to the development of future storage plans on and near the tailings stack.

8.0 CLOSURE

We trust that this letter satisfies your requirements for an update at this time. The work is ongoing and therefore we will continue to provide summary reports that document our continuing observations and discussions Additional information regarding the use of this report is presented in the attached General Conditions, which form a part of this report. If additional information or clarification of any recommendation presented in this report is required, please contact the undersigned.[1]

Respectfully submitted,
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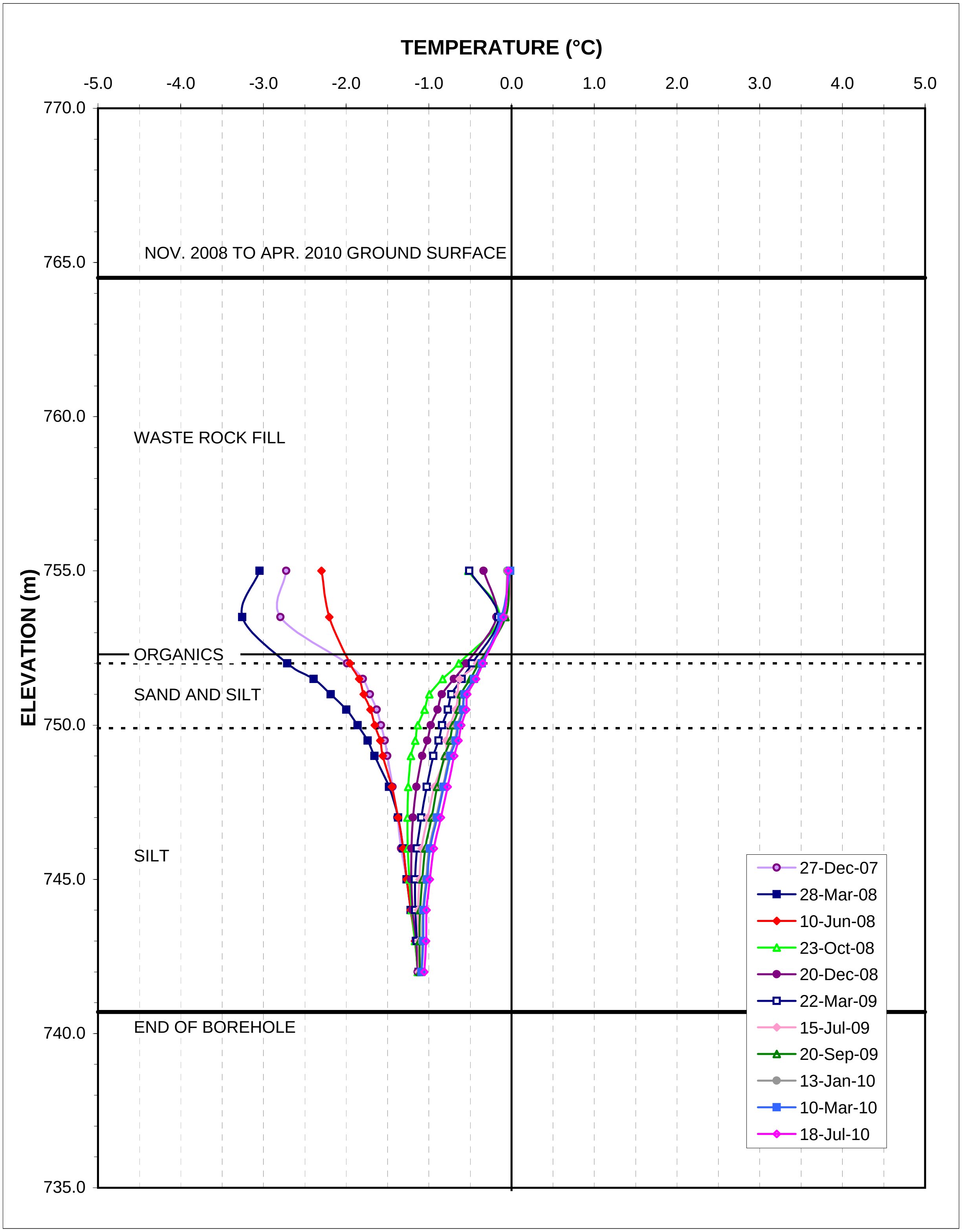
Attachments:

Figure T1 to 7	Ground Temperature Cable Profiles
Figure VP01 to 02	Ground Temperature Cable Profiles of Vibrating Wire Piezometers
Figure SH01 to 05	Survey Hub Movement Rates
Figure DSI-1 to -5	Slope Inclinometer Deflection Plots
Figure I01 to 06	Slope Inclinometer Movement Rates
Photographs	
Appendix A	Drawings
Appendix B	Geotechnical Report - General Conditions

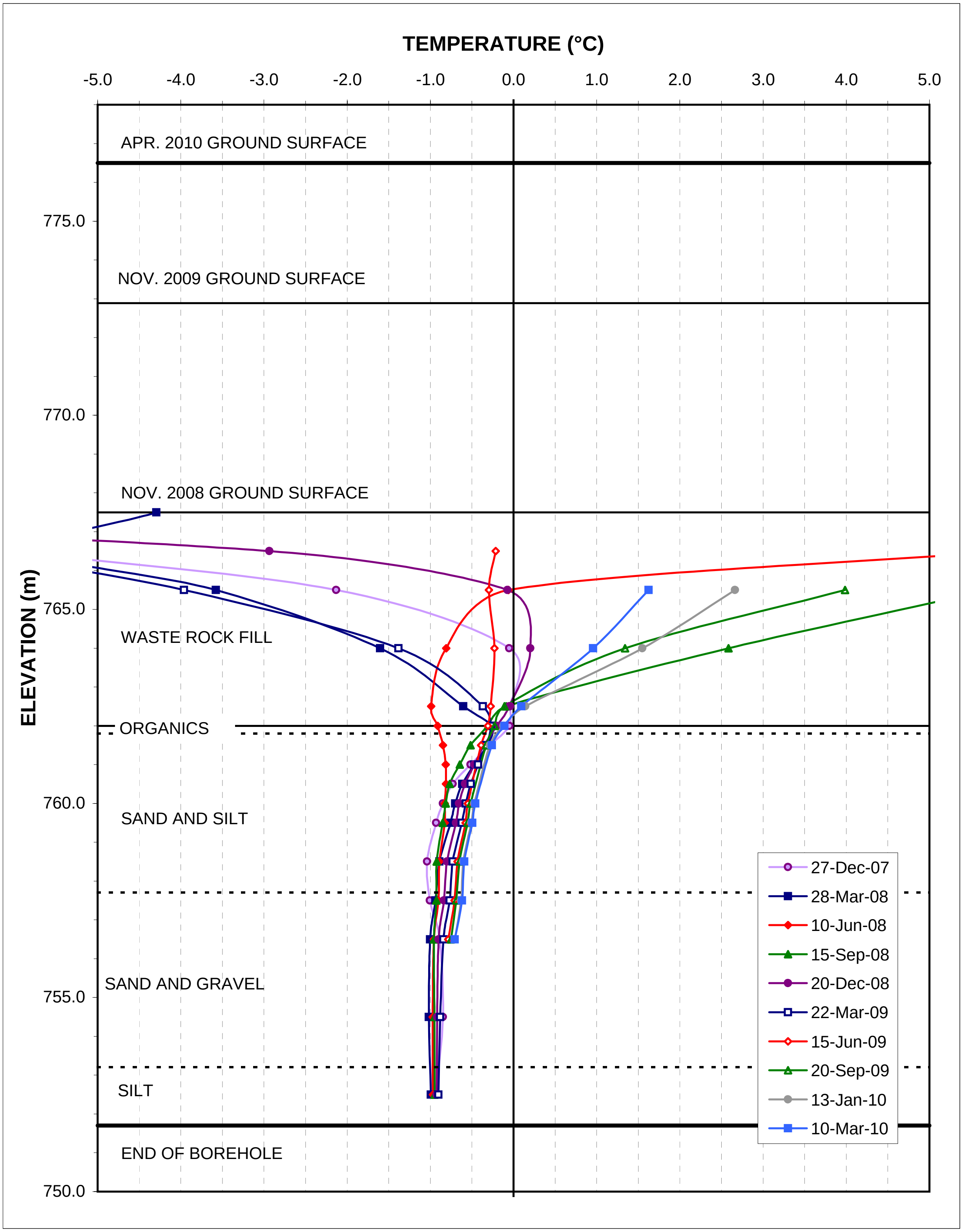
[1] Note: Original draft was prepared by Jason Berkers, P.Eng. dated August 20, 2010.

FIGURES



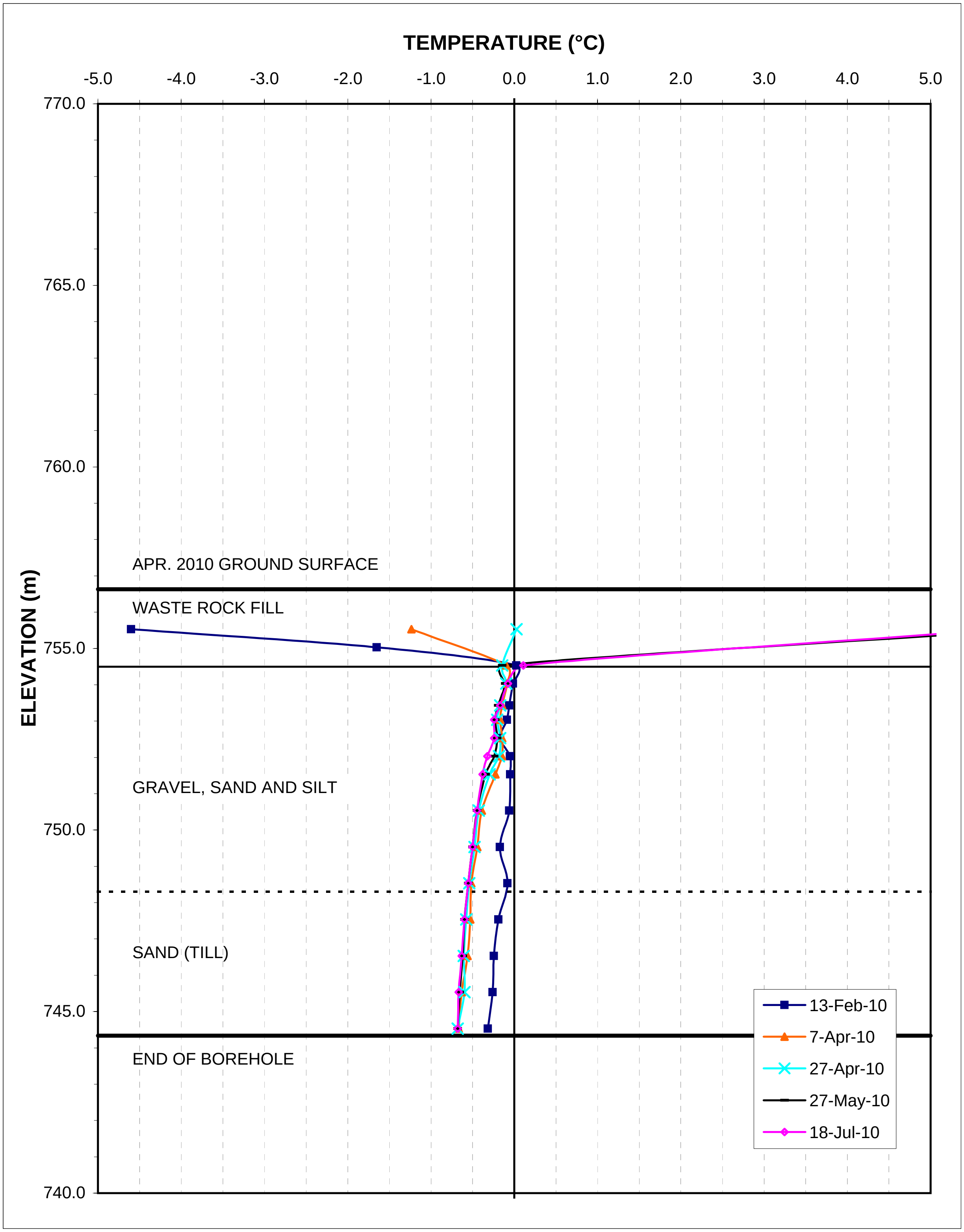
Installed: October 30, 2007

Dry Stack Tailings Storage Facility
Ground Temperature Profile - DST-1
Figure T1



Installed: November 1, 2007

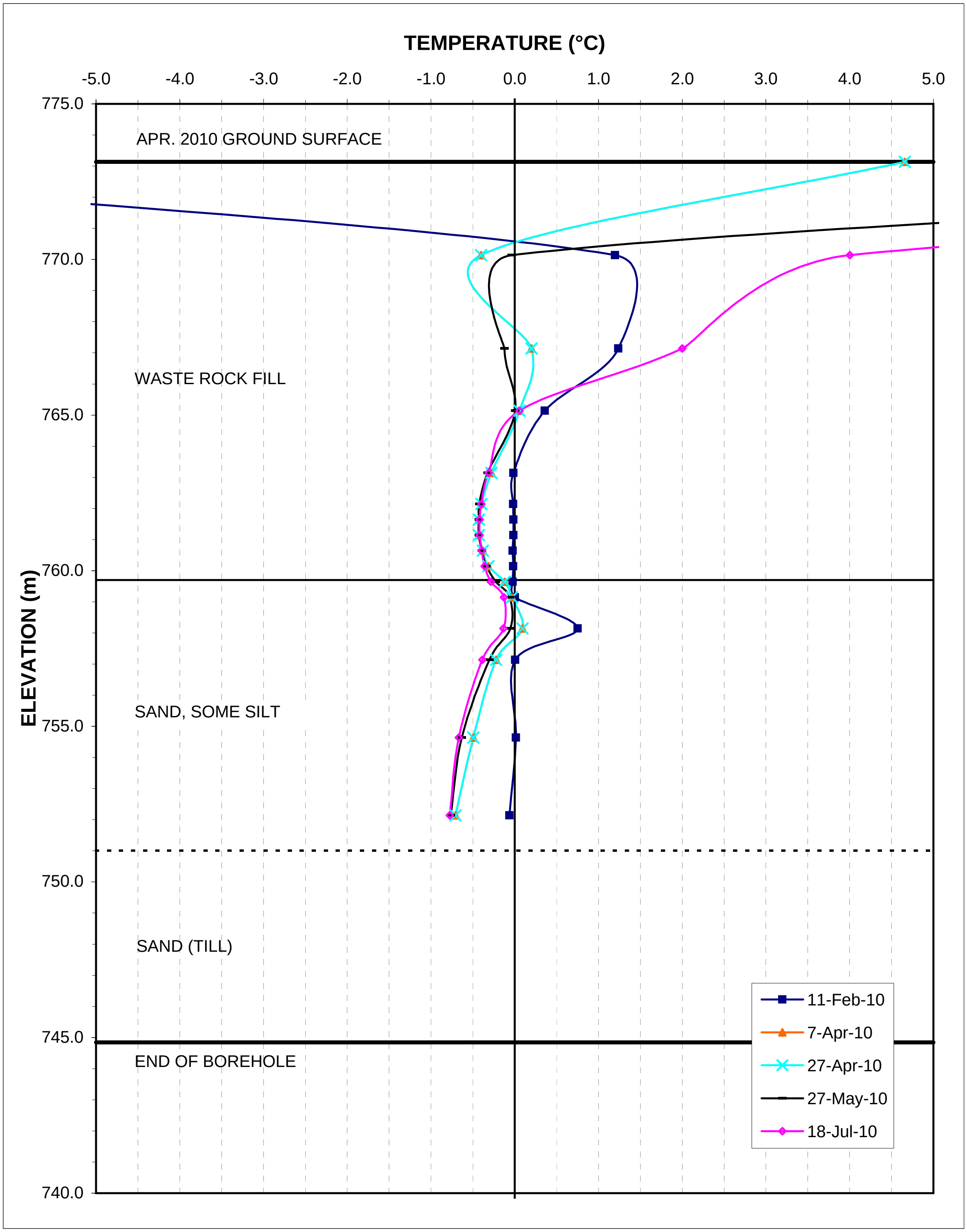
Dry Stack Tailings Storage Facility
Ground Temperature Profile - DST-2
Figure T2



Installed: January 22, 2010

Dry Stack Tailings Storage Facility
Ground Temperature Profile - DST-3
Figure T3

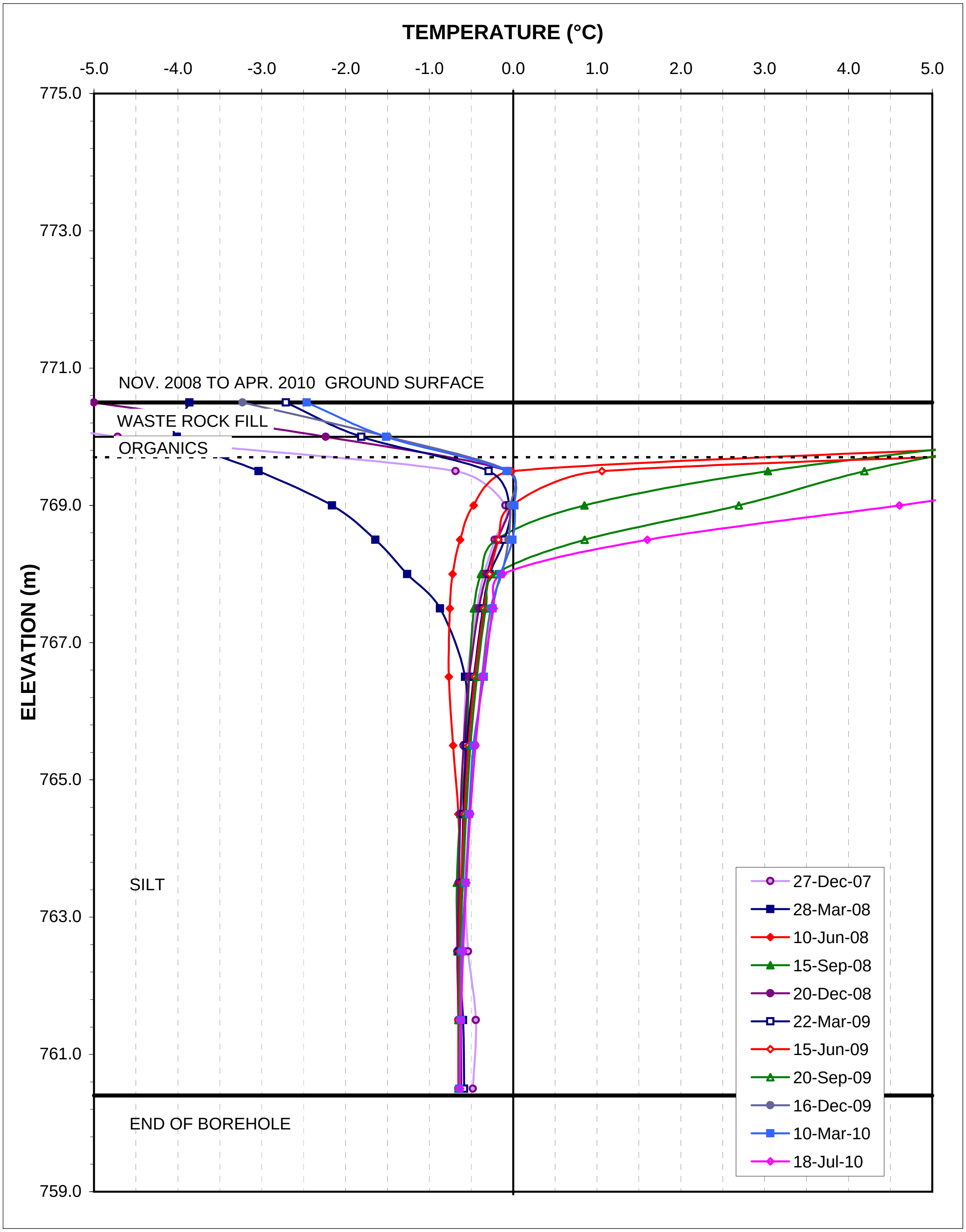
*Statigraphy subject to change following lab program



Installed: January 26, 2010

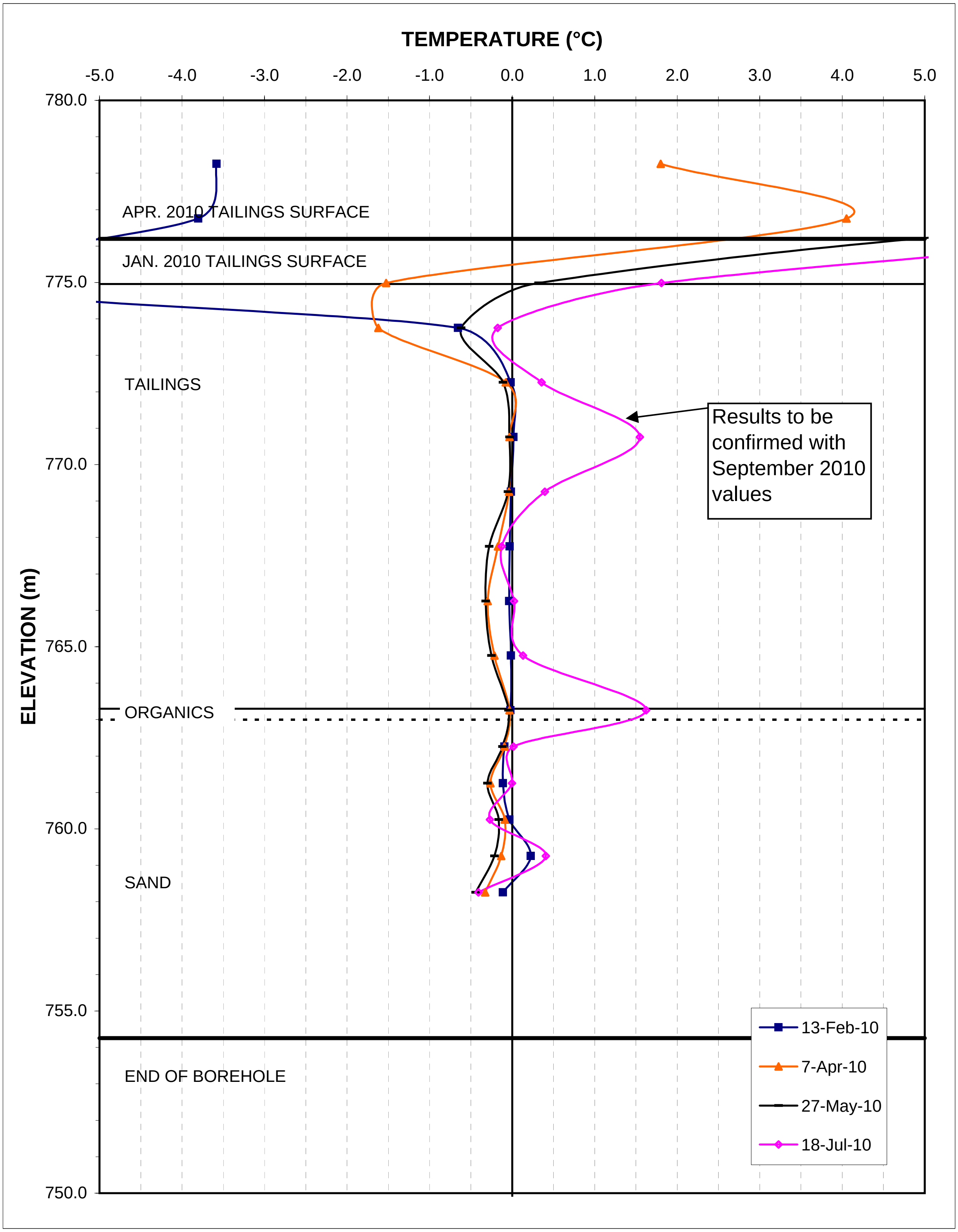
Dry Stack Tailings Storage Facility
Ground Temperature Profile - DST-4
Figure T4

*Statigraphy subject to change following lab program



Installed: October 30, 2007

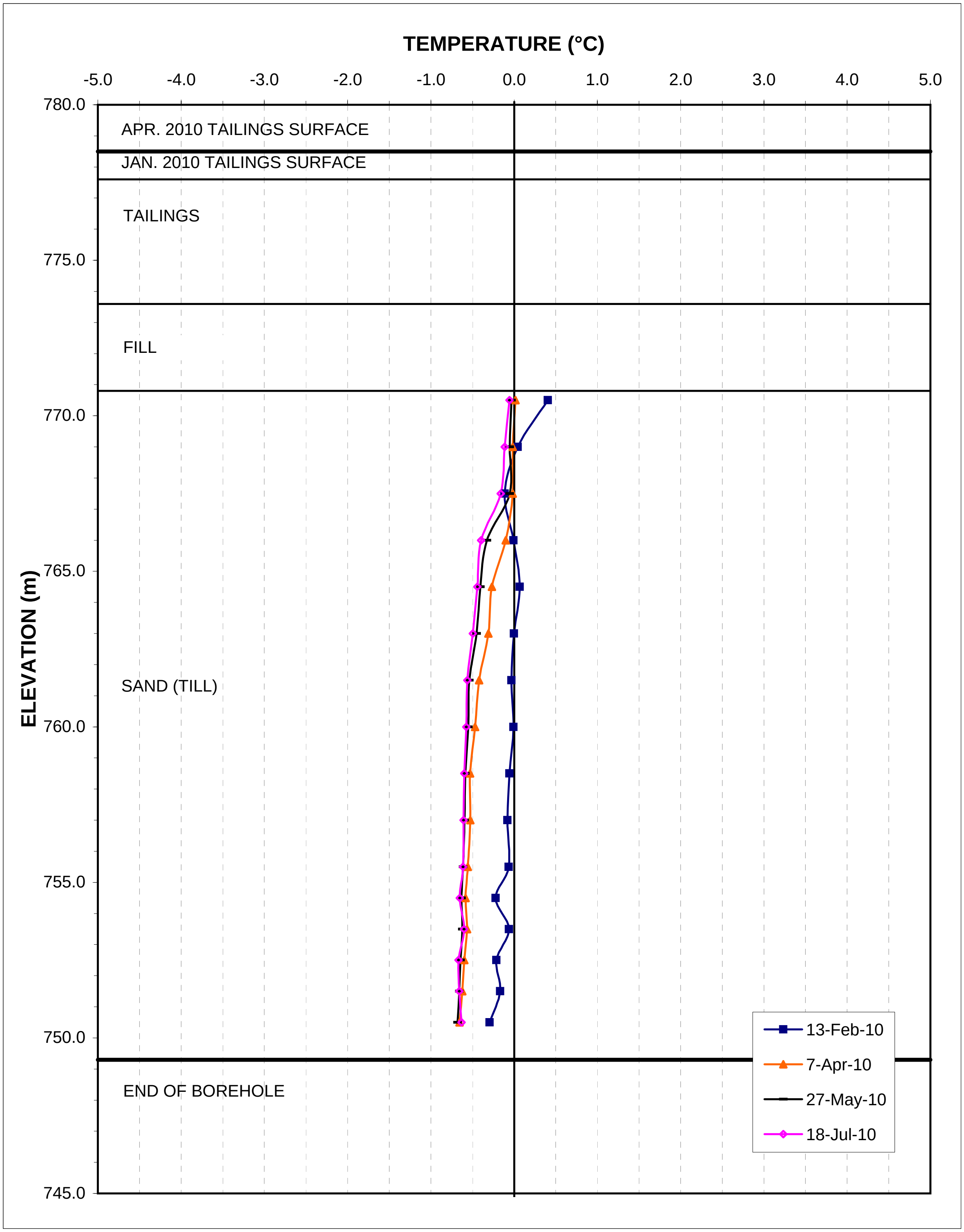
Dry Stack Tailings Storage Facility
Ground Temperature Profile - DST-5
Figure T5



Installed: January 27, 2010

Dry Stack Tailings Storage Facility
Ground Temperature Profile - DST-6
Figure T6

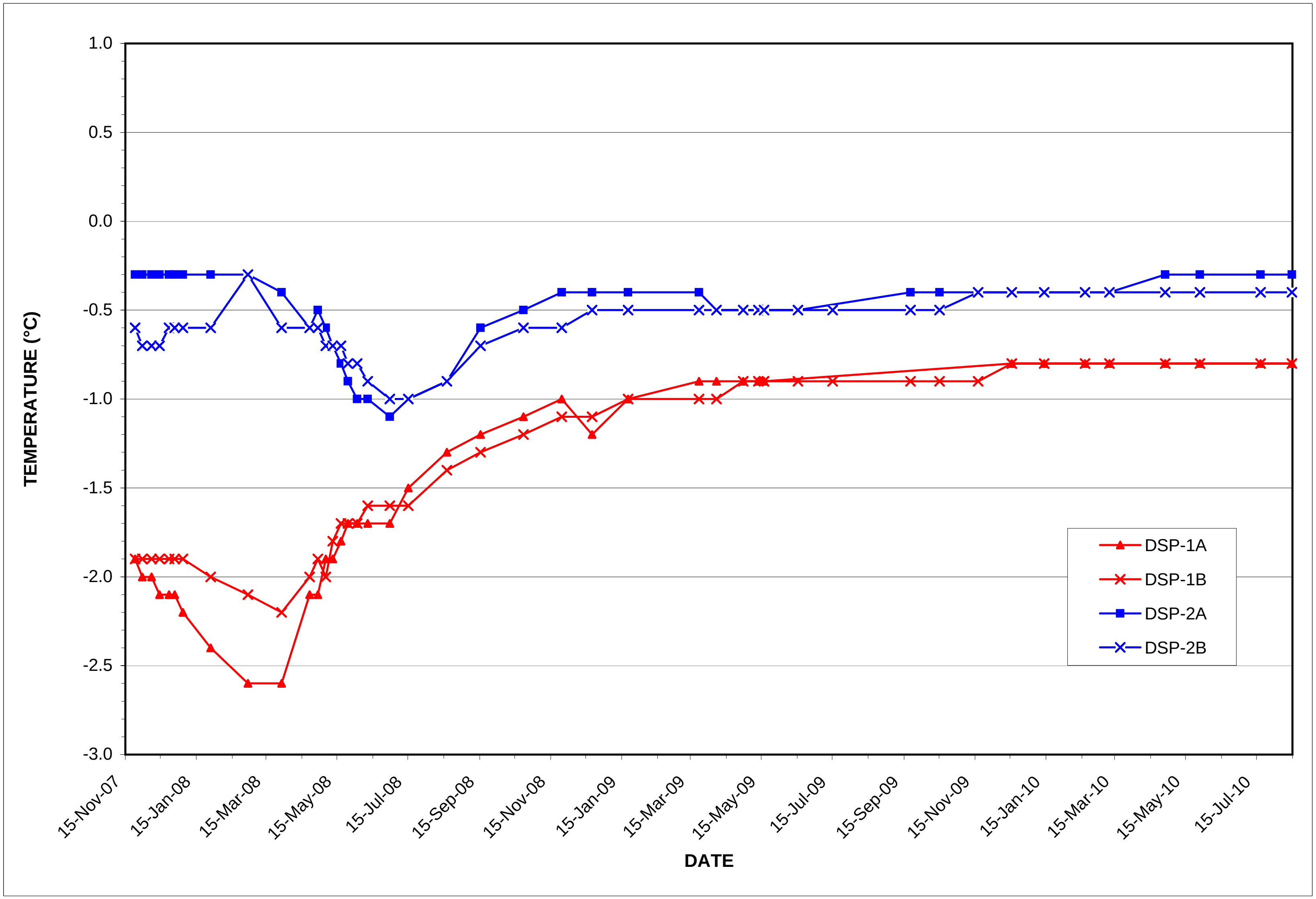
*Statigraphy subject to change following lab program



Installed: January 28, 2010

*Statigraphy subject to change following lab program

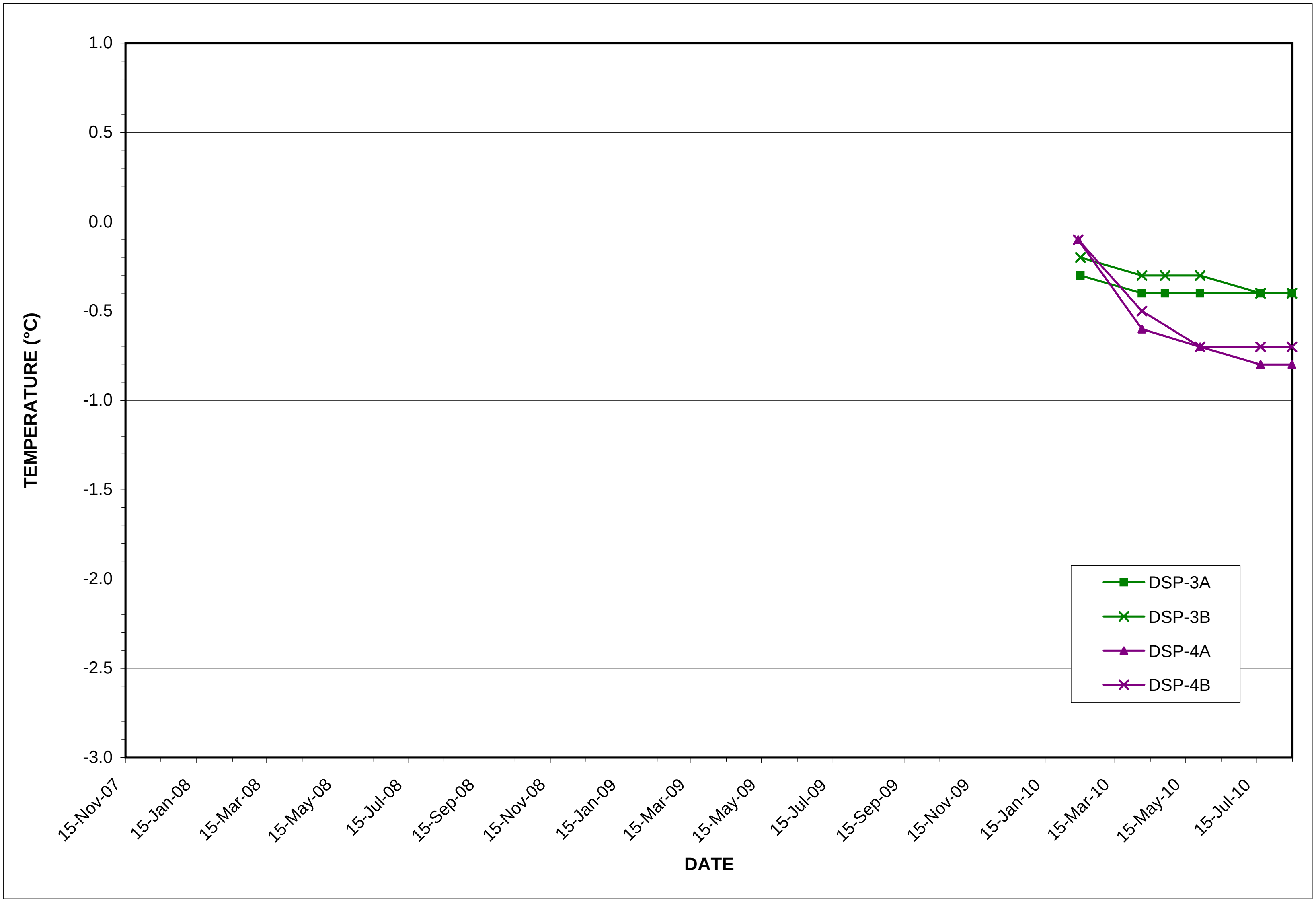
Dry Stack Tailings Storage Facility
Ground Temperature Profile - DST-7
Figure T7



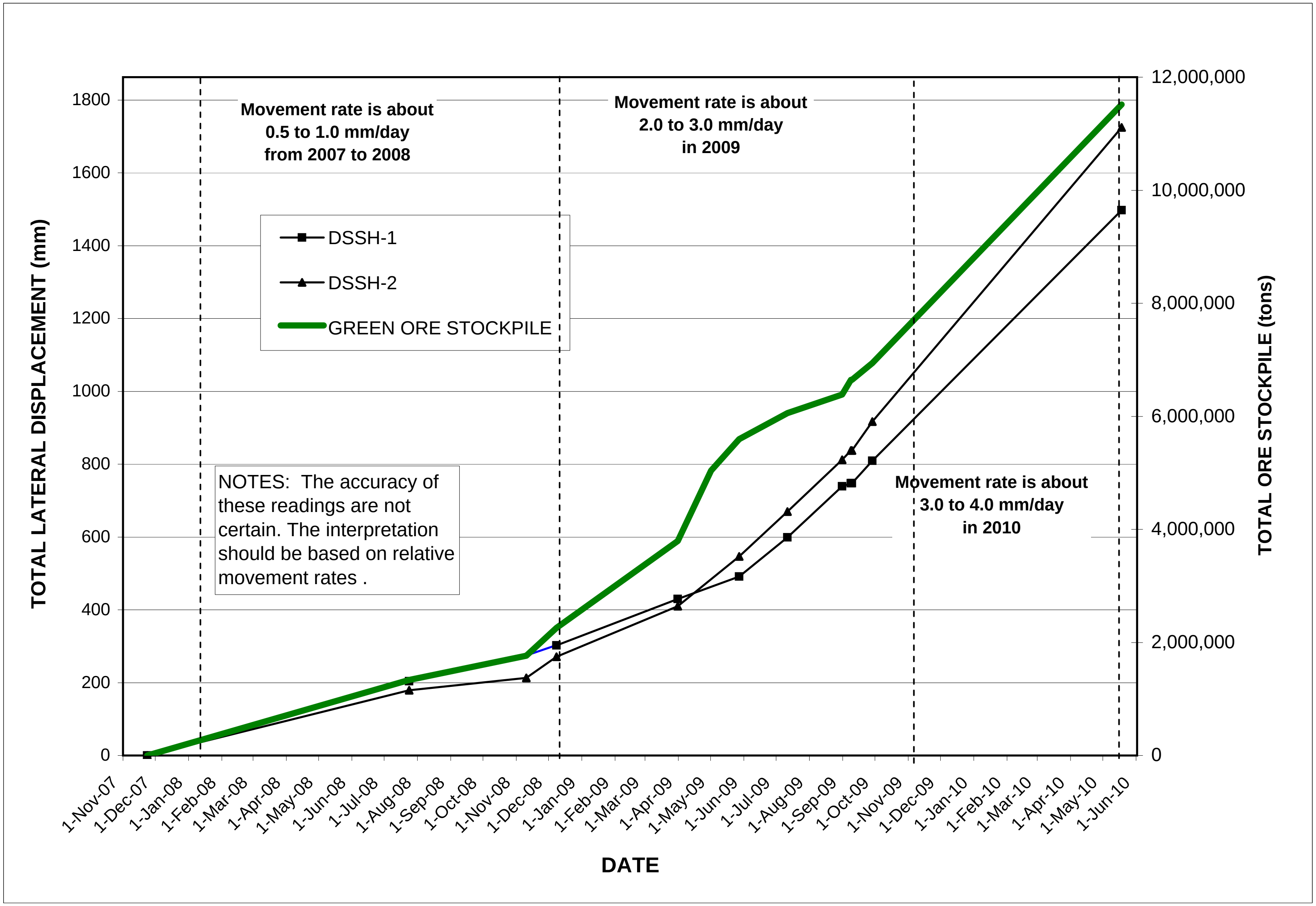
Dry Stack Tailings Storage Facility

Ground Temperature Profile of VW Piezometer - DSP-1A, DSP-1B, DSP-2A, DSP-2B

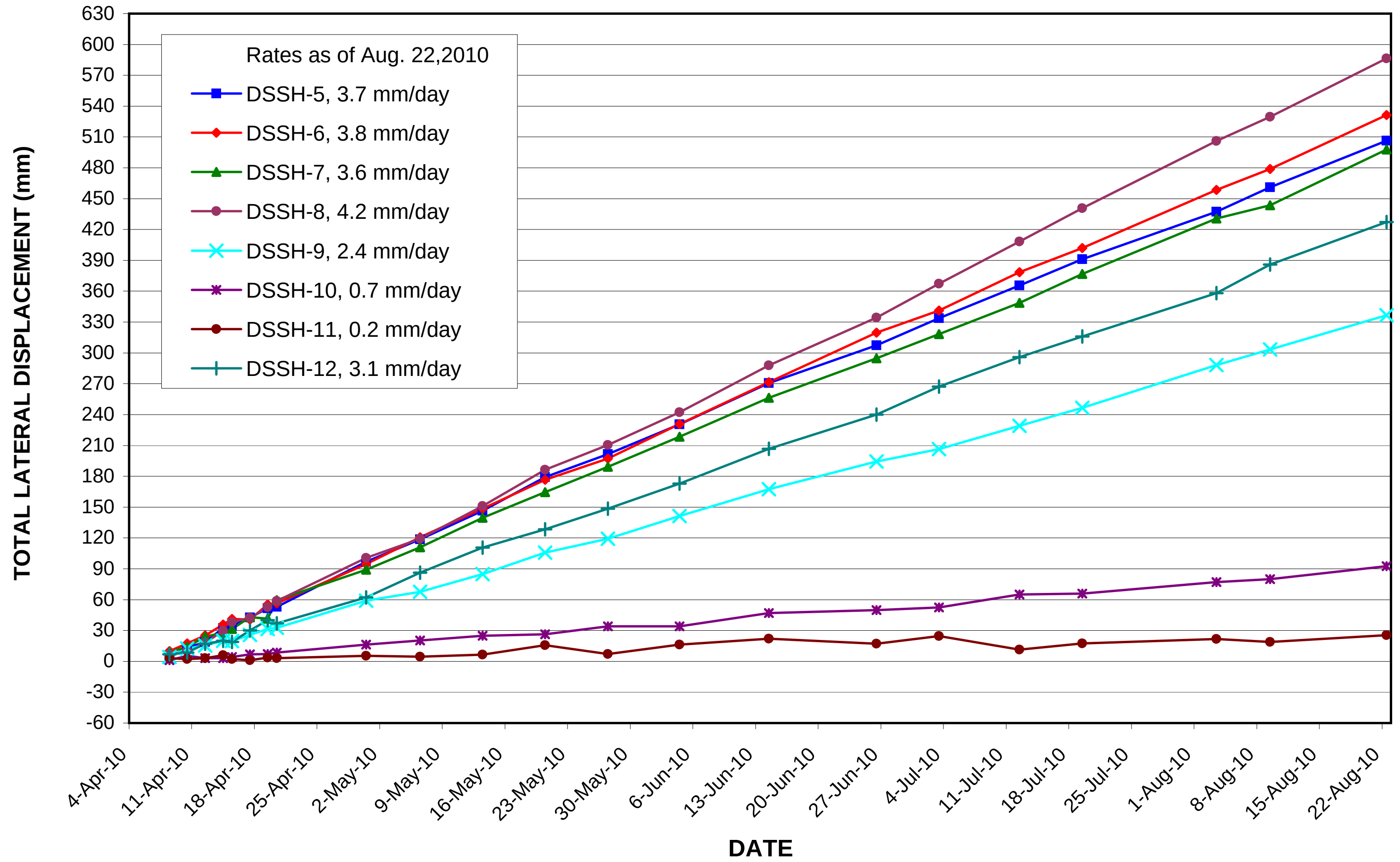
Figure VP01



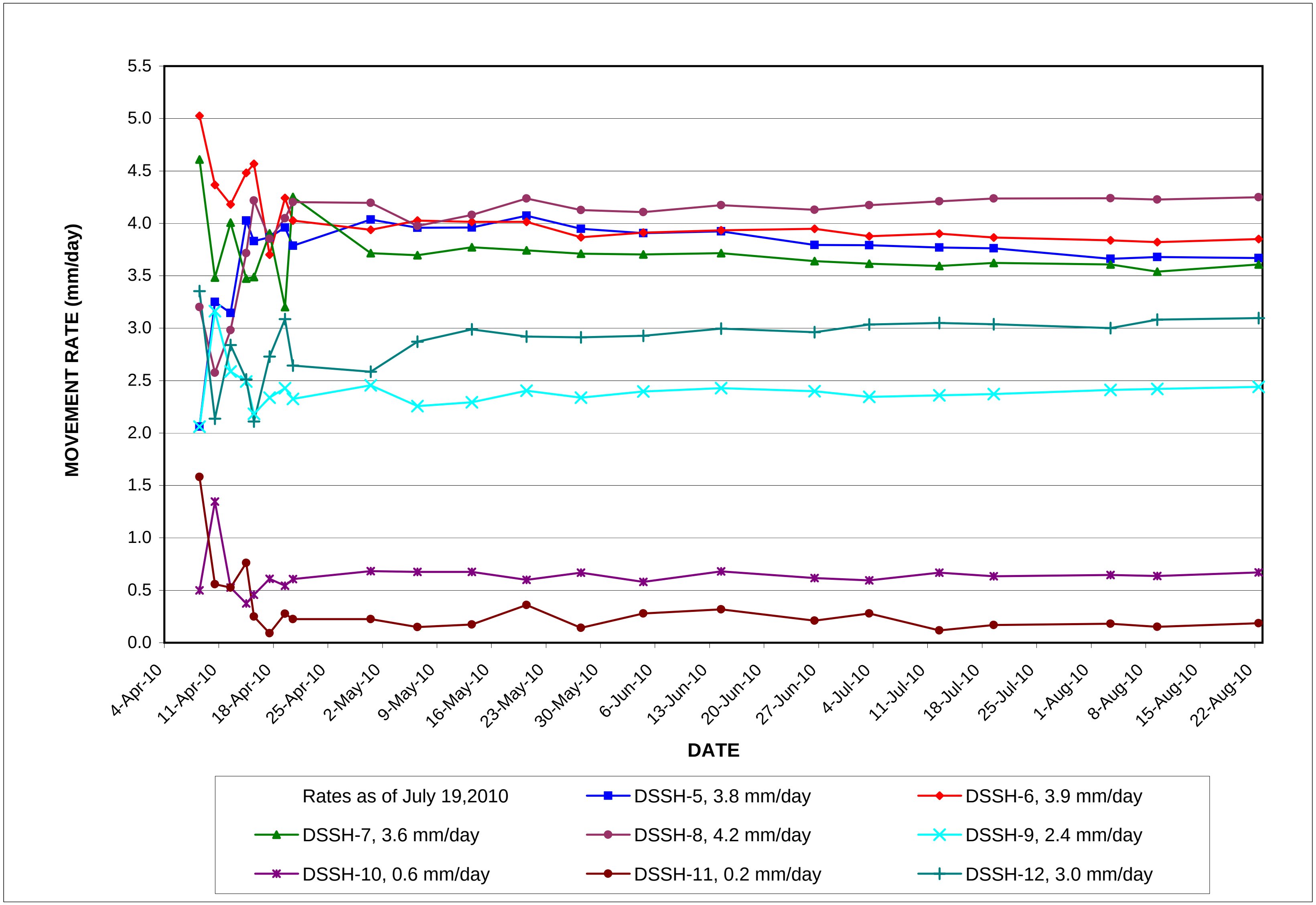
Dry Stack Tailings Storage Facility
Ground Temperature Profile of VW Piezometer - DSP-3A, DSP-3B, DSP-4A, DSP-4B
Figure VP02



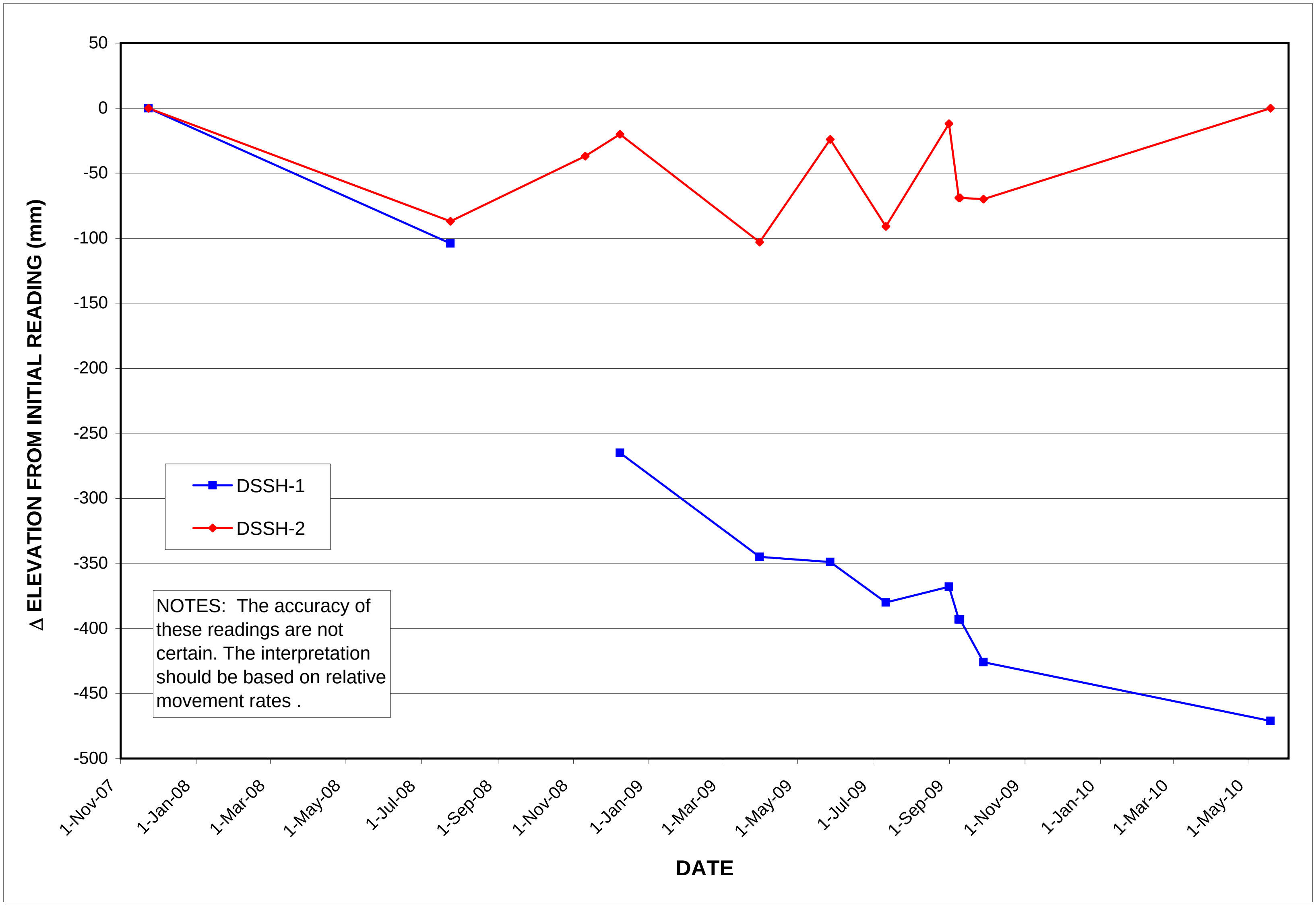
Dry Stack Tailings Storage Facility
Survey Hub-DSSH-1 & DSSH-2
Figure SH01



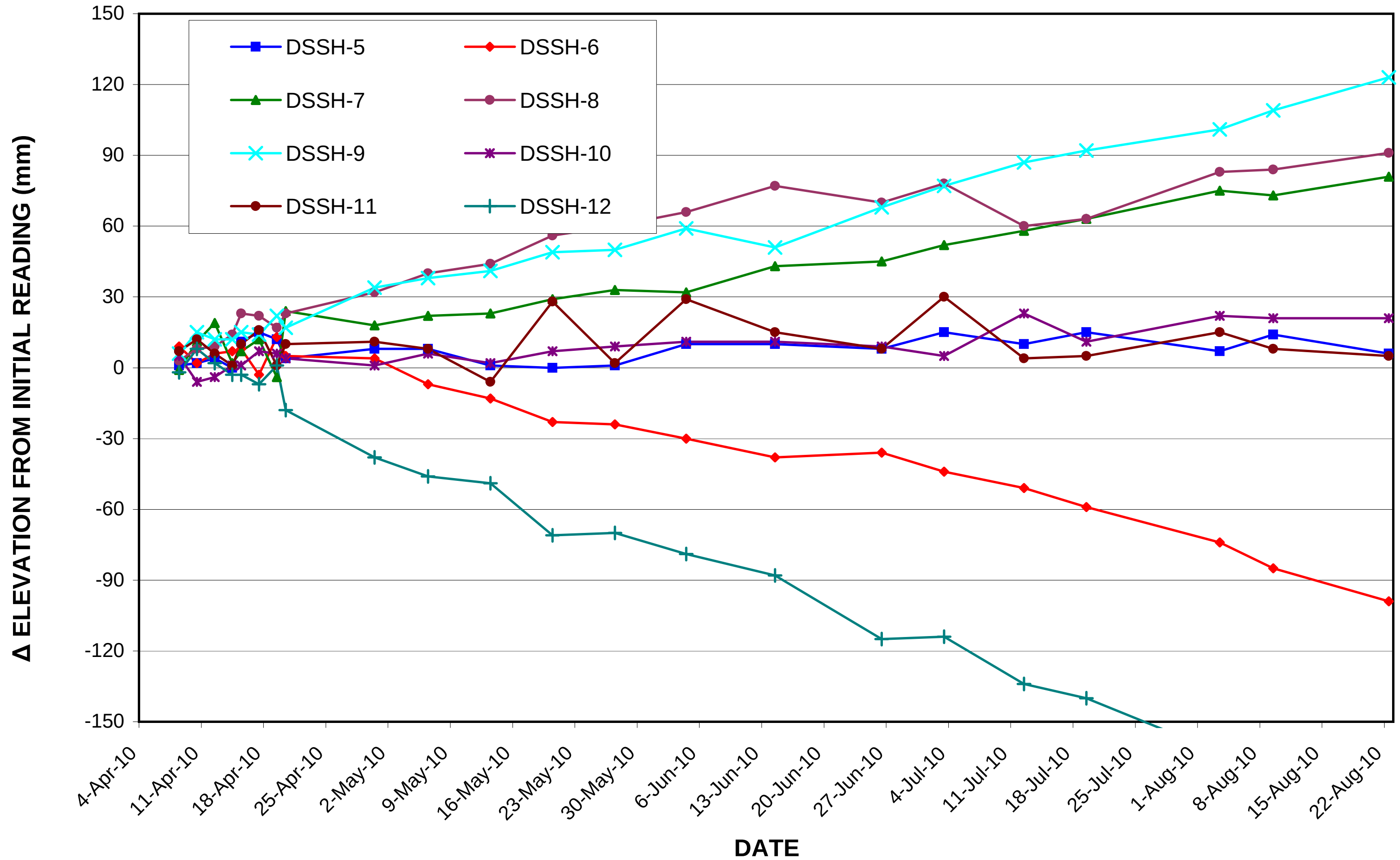
Dry Stack Tailings Storage Facility
Survey Hub-DSSH-5 to DSSH-12
Figure SH02



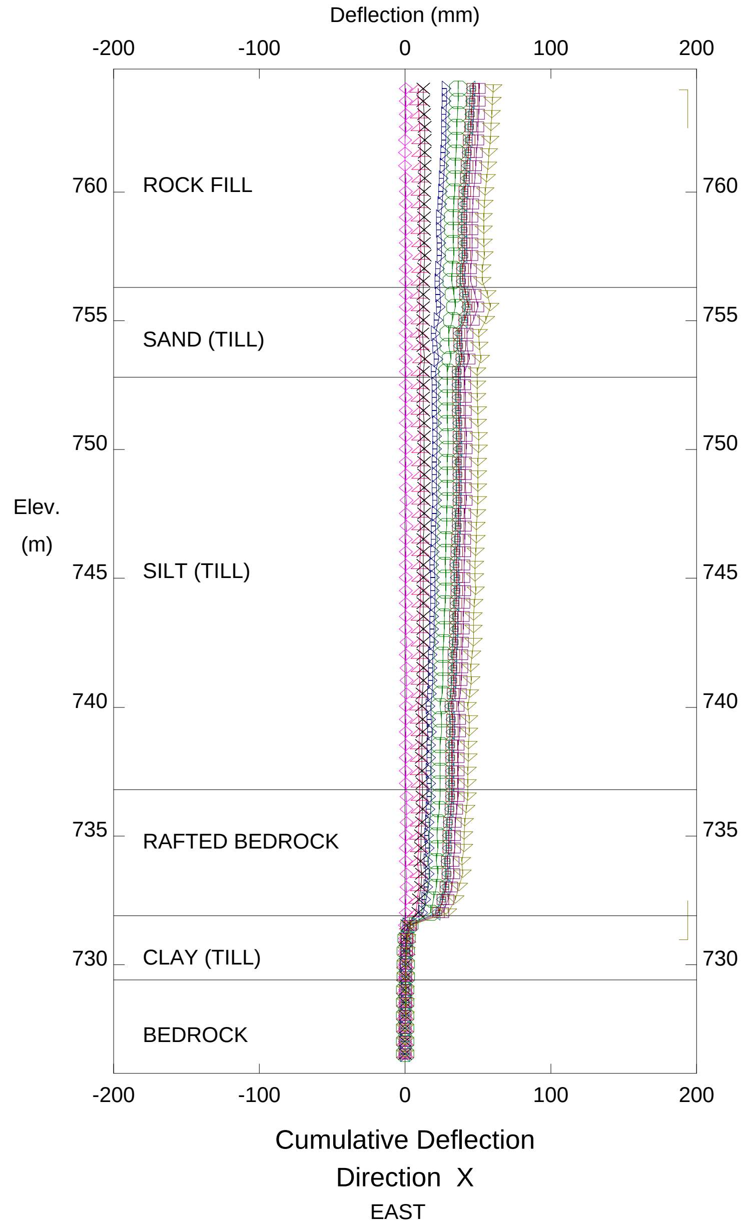
Dry Stack Tailings Storage Facility
Survey Hub-DSSH-5 to DSSH-12
Figure SH03



Dry Stack Tailings Storage Facility
Survey Hub-DSSH-1 & DSSH-2
Figure SH04



Dry Stack Tailings Storage Facility
Survey Hub-DSSH-5 to DSSH-12
Figure SH05



LEGEND

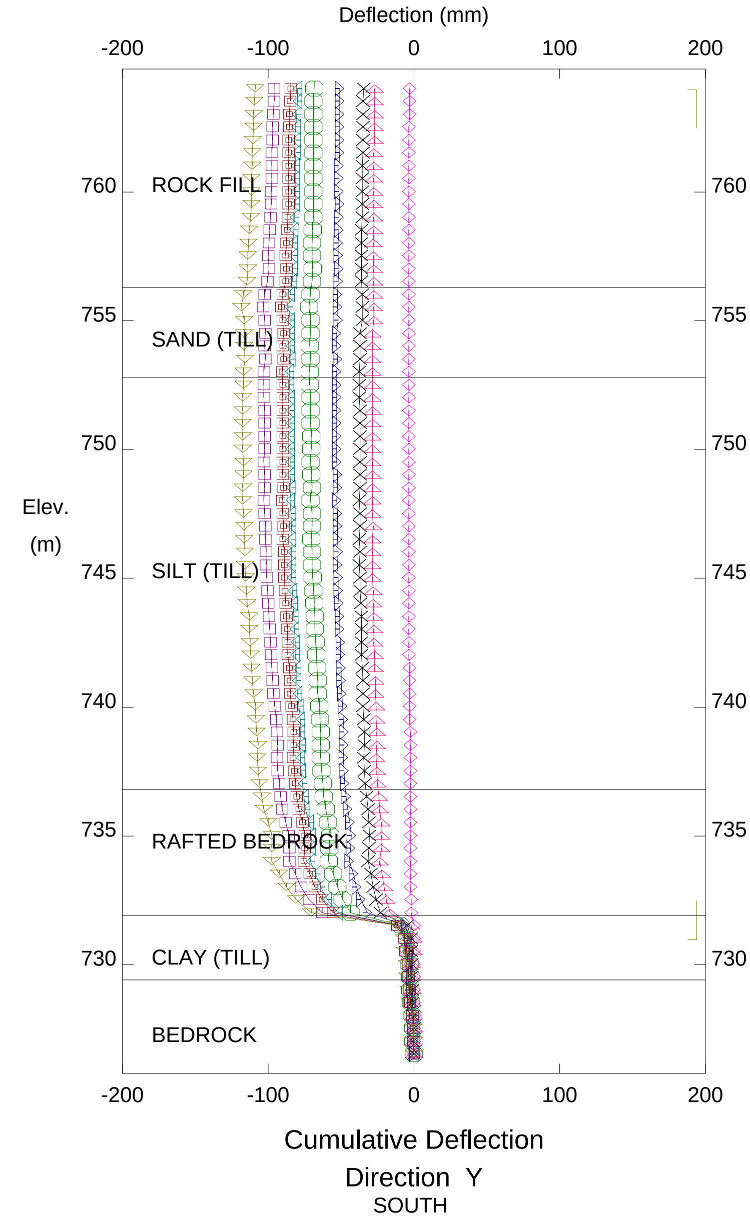
Initial	11 Feb 2010
	12 Feb 2010
	21 Feb 2010
	24 Feb 2010
	1 Mar 2010
	6 Mar 2010
	11 Mar 2010
	13 Mar 2010
	17 Mar 2010
	22 Mar 2010

Pinched off at failure plane April 7, 2010

N 6945021.46
E 385563.53

Ref. Elevation 764.78 m

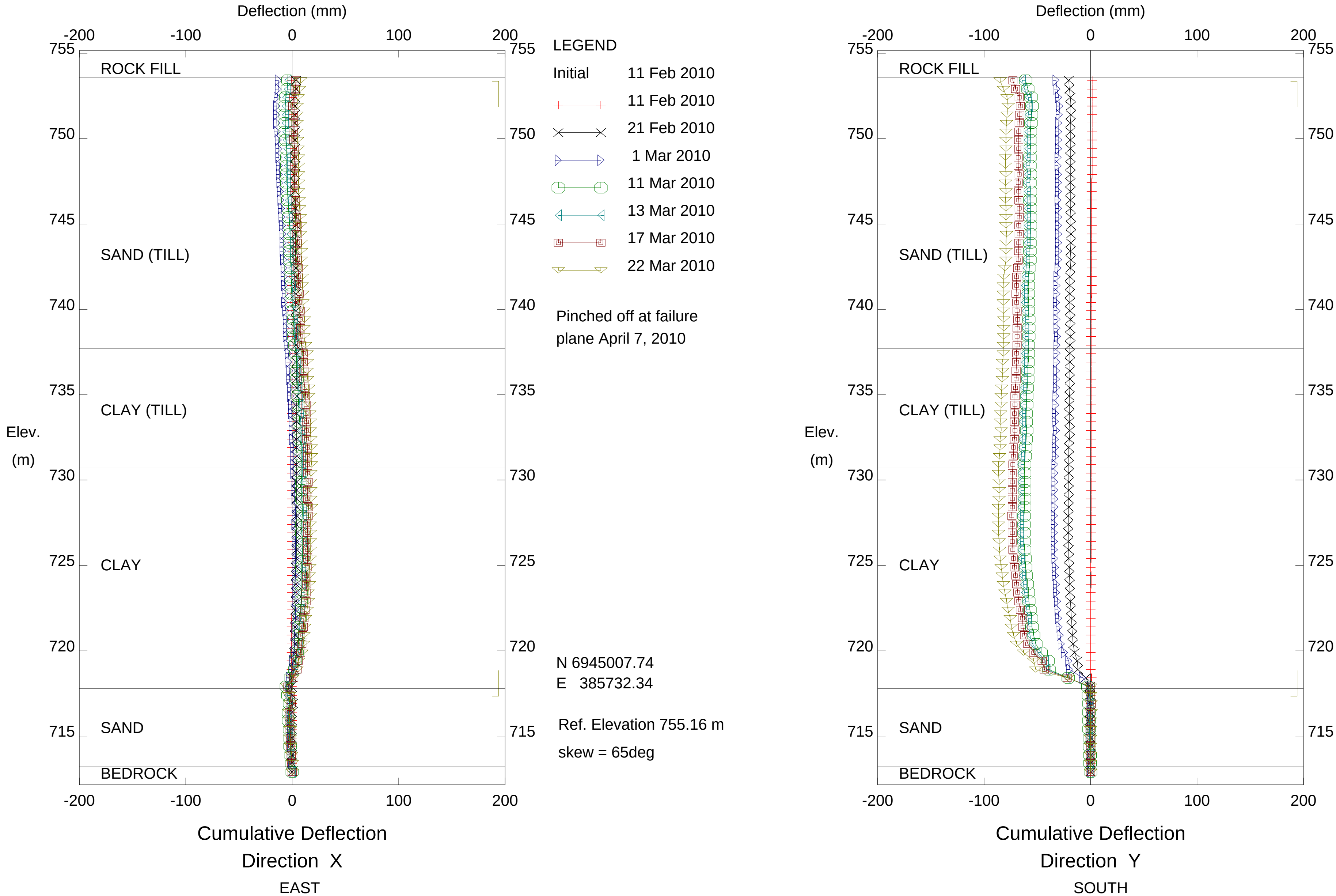
skew = 97deg



, Inclinometer DSI-1

Dry Stack Dump

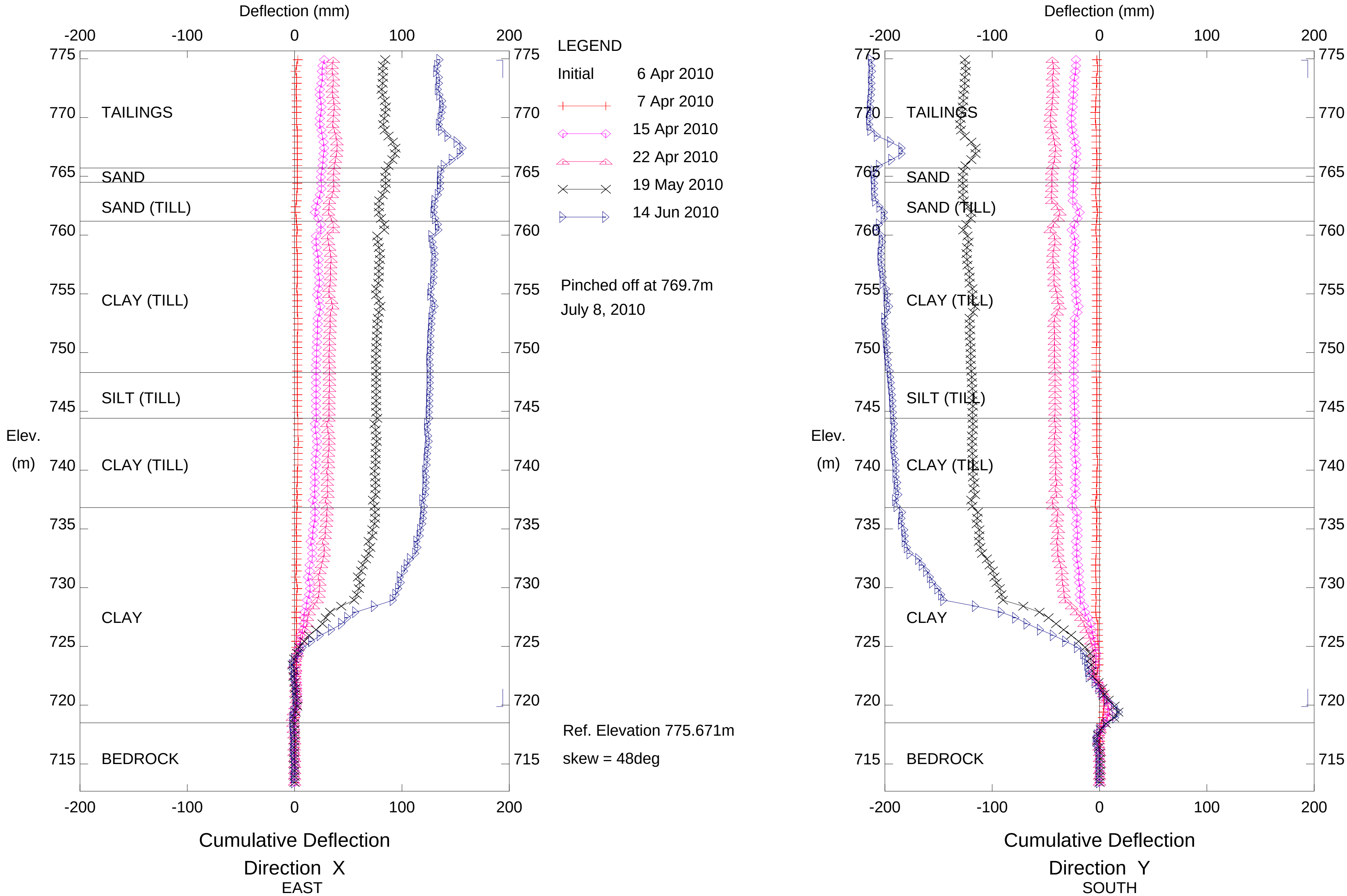
Q:\Whitehorse\Data\0201 Inclinometer Data\Minto SI Readings\Gtilt files\DSI1.gtl



, Inclinometer DSI-2

Dry Stack Dump

Q:\Whitehorse\Data\0201 Inclinometer Data\Minto SI Readings\Gtilt files\DSI2.gtl



, Inclinometer DSI-3

Dry Stack Dump

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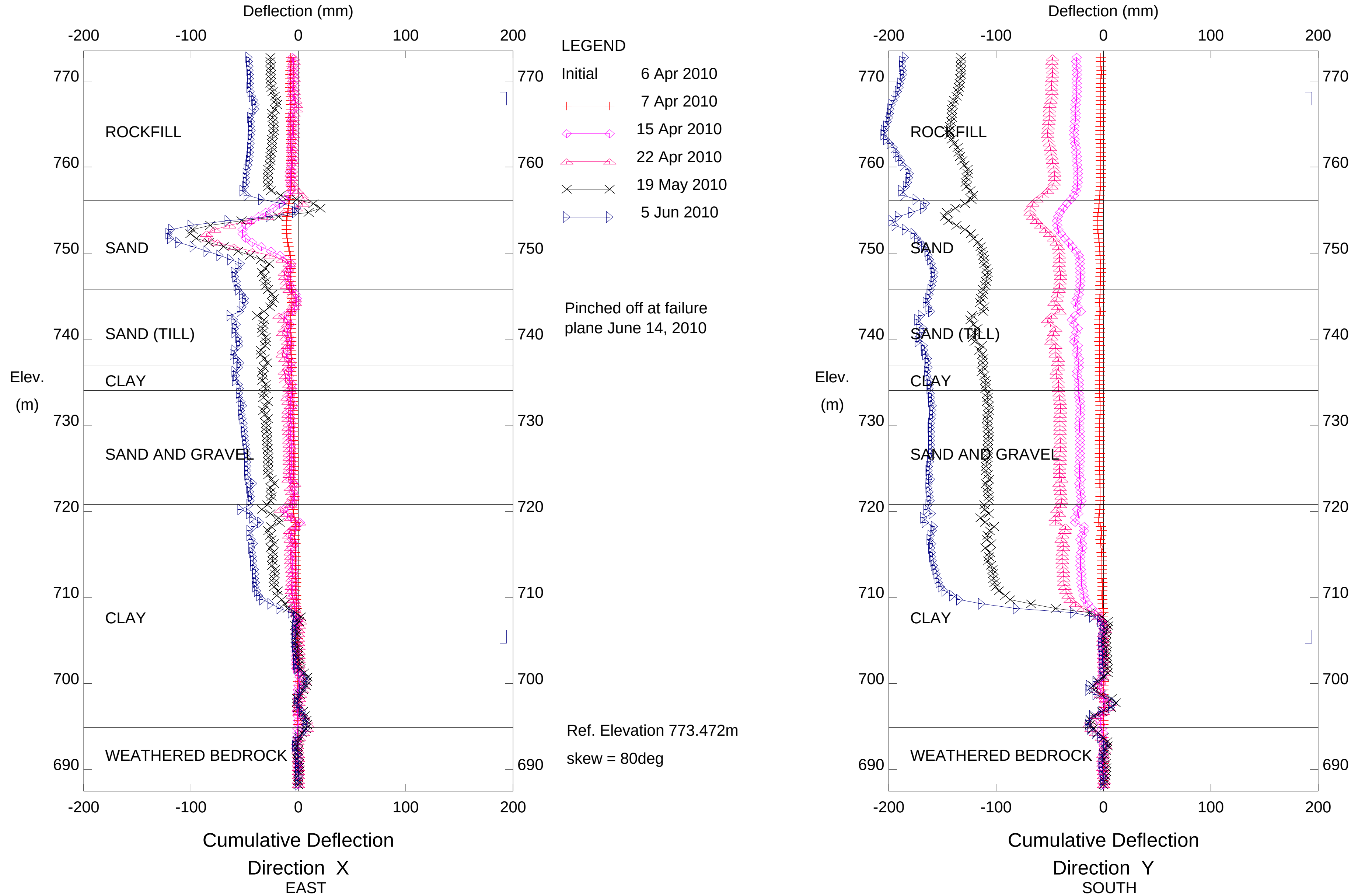

, Inclinometer DSI-4

Dry Stack Dump

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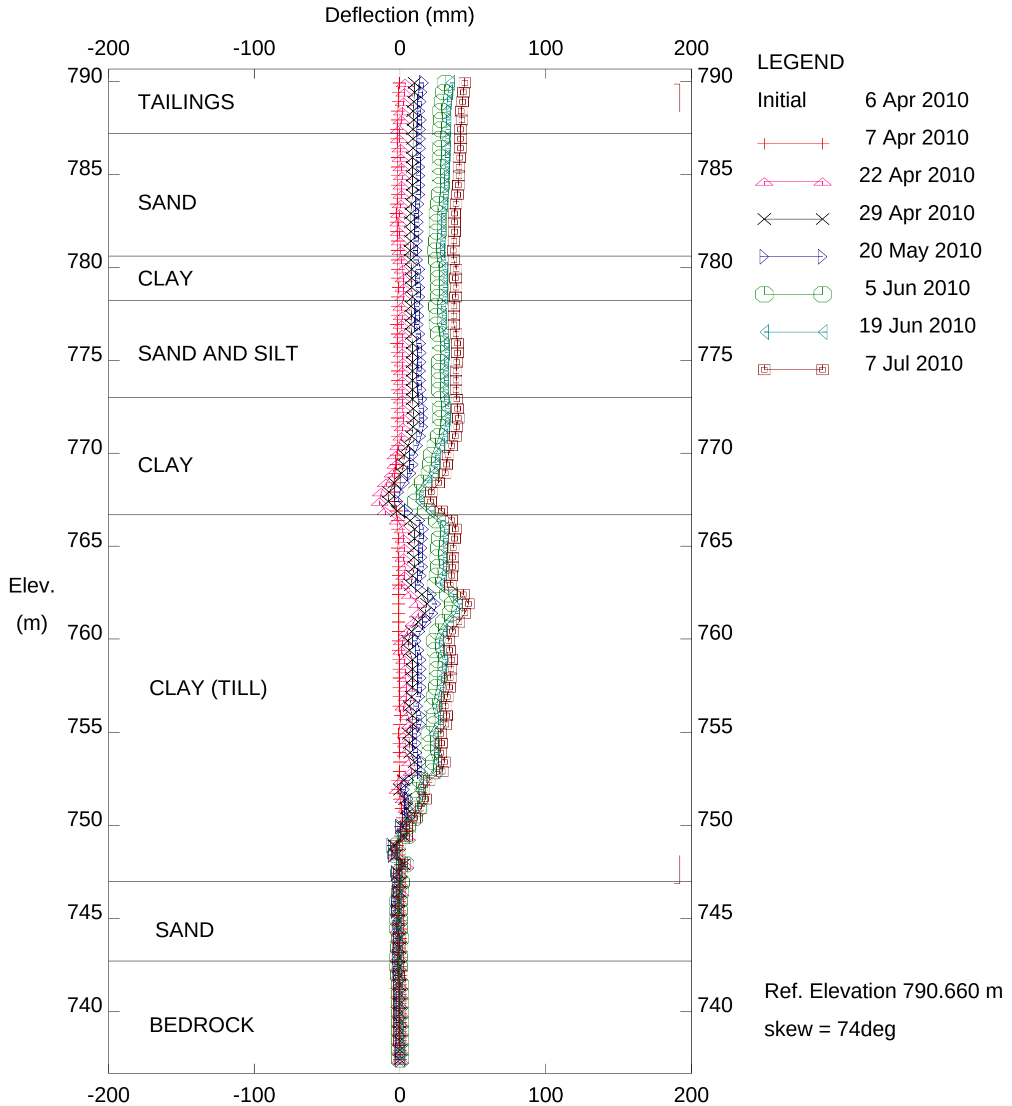
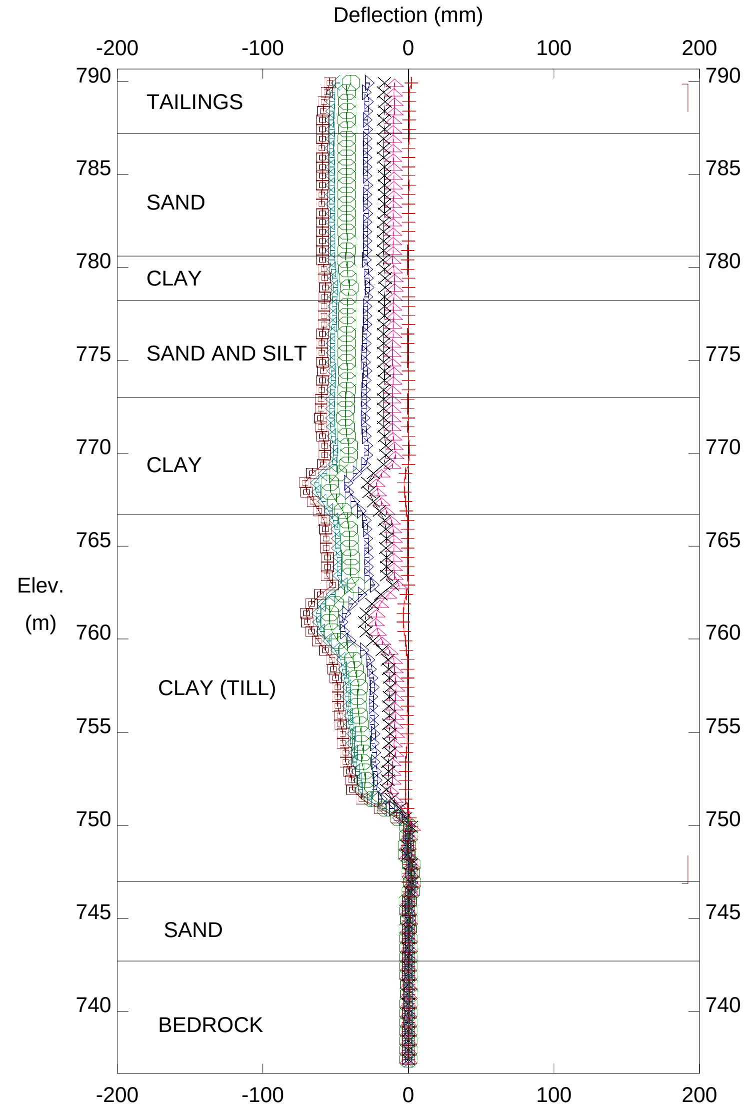


Cumulative Deflection

Direction X

EAST



Cumulative Deflection

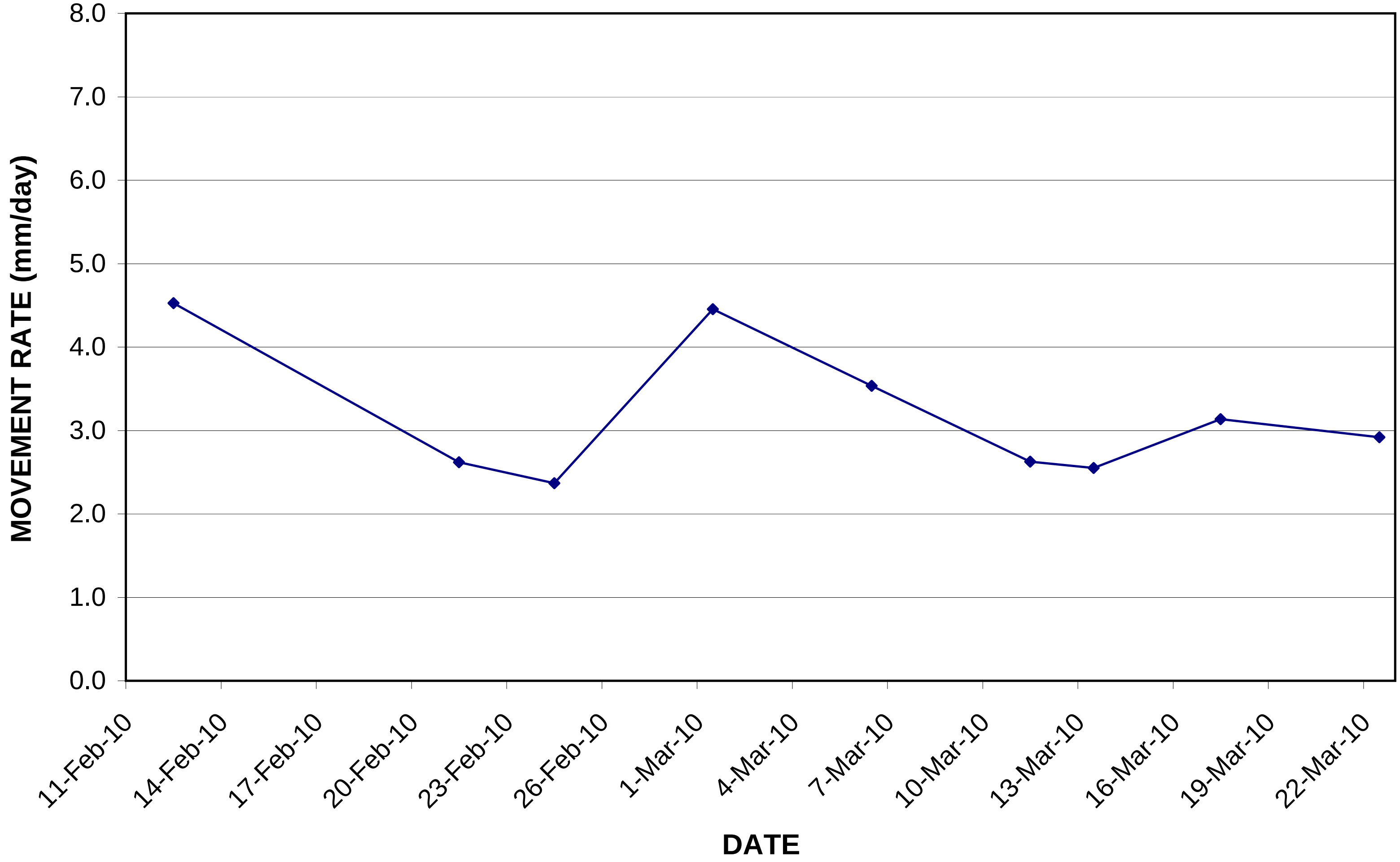
Direction Y

SOUTH

, Inclinometer DSI-5

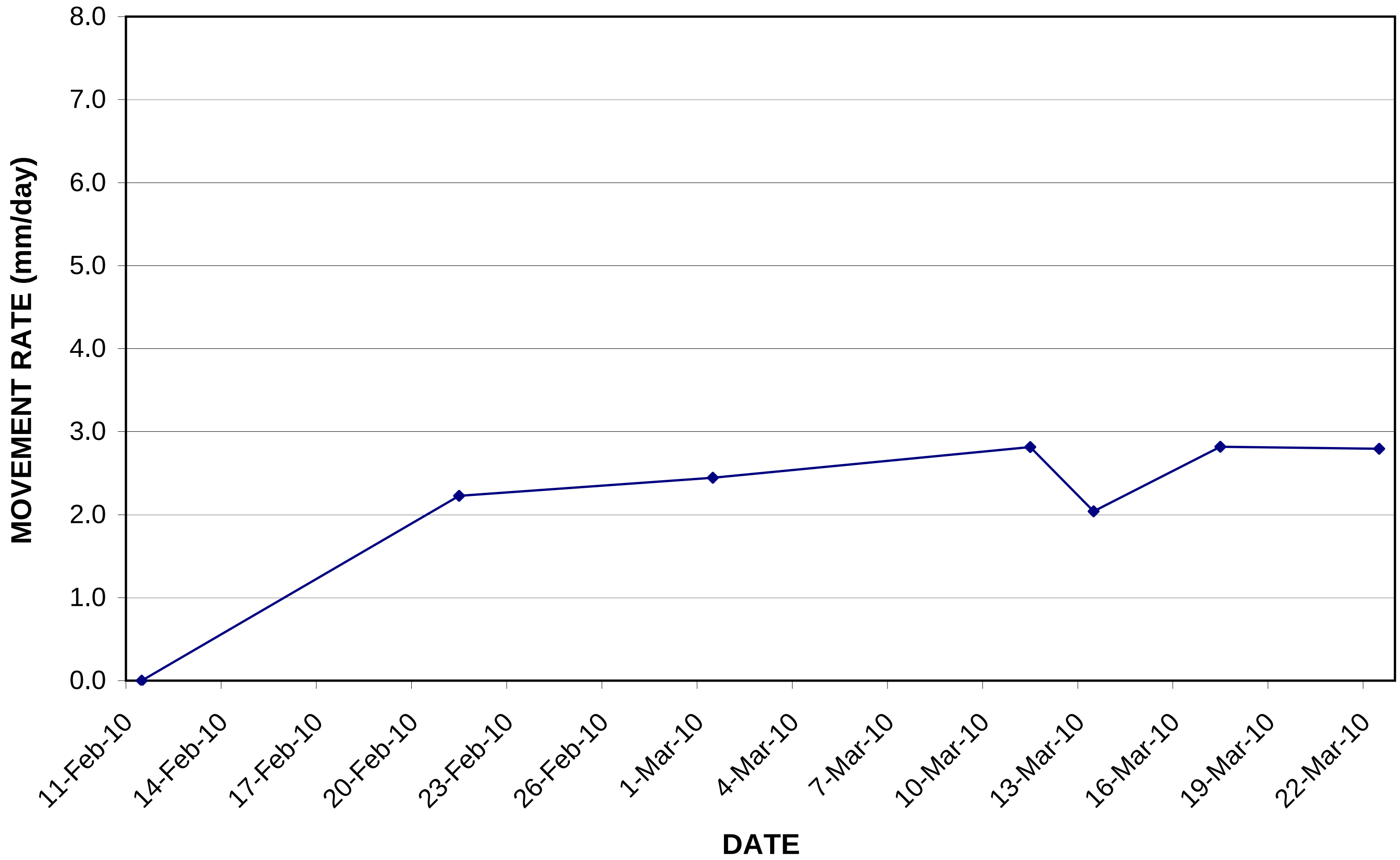
Dry Stack Dump

Q:\Whitehorse\Data\0201 Inclinometer Data\Minto SI Readings\Gtilt files\DSI5.gtl



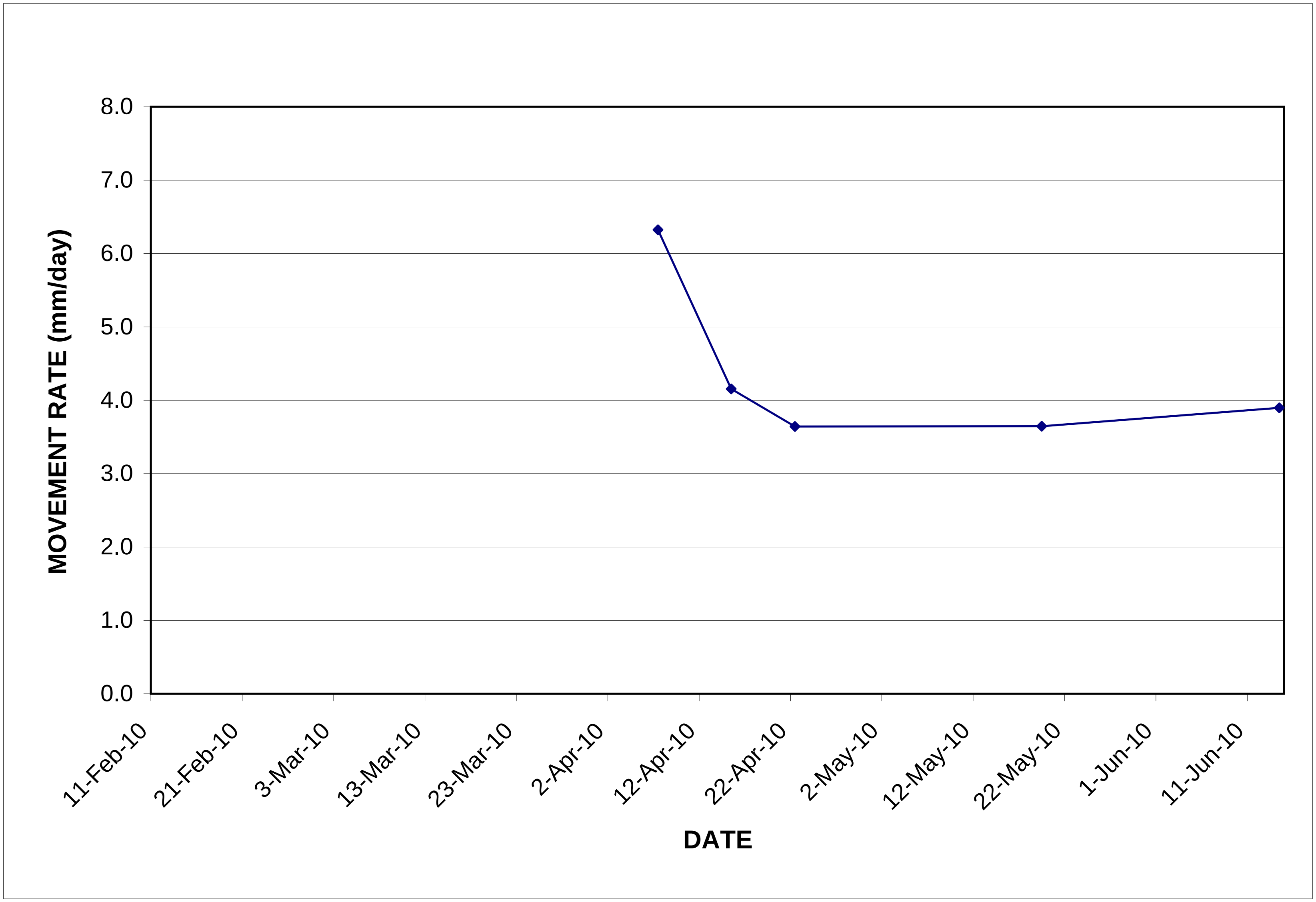
Installed: January 25, 2010

Dry Stack Tailings Storage Facility
DSI-1 Inclinometer Movement Rate
Figure I01



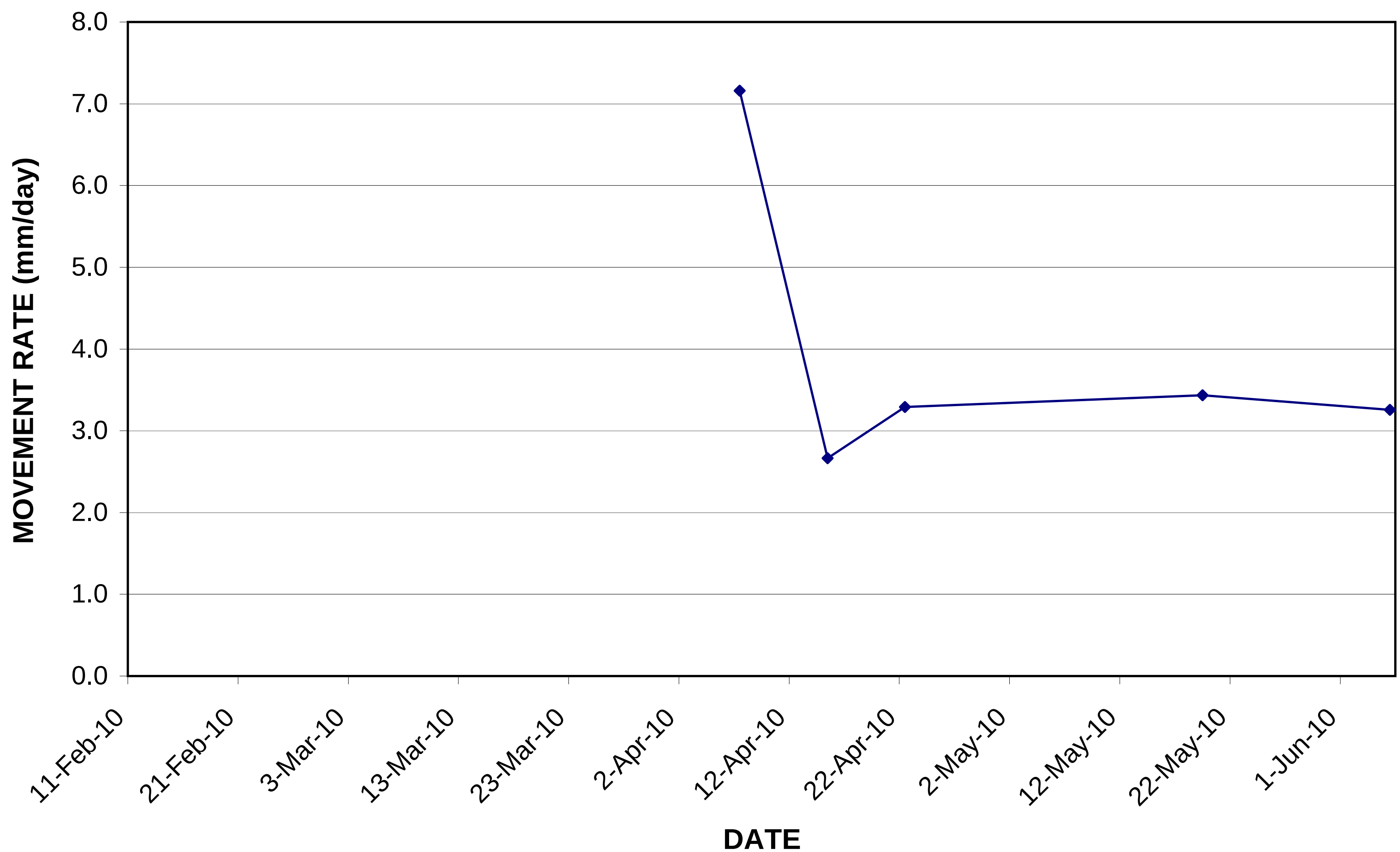
Installed: January 24, 2010

Dry Stack Tailings Storage Facility
DSI-2 Inclinometer Movement Rate
Figure I02



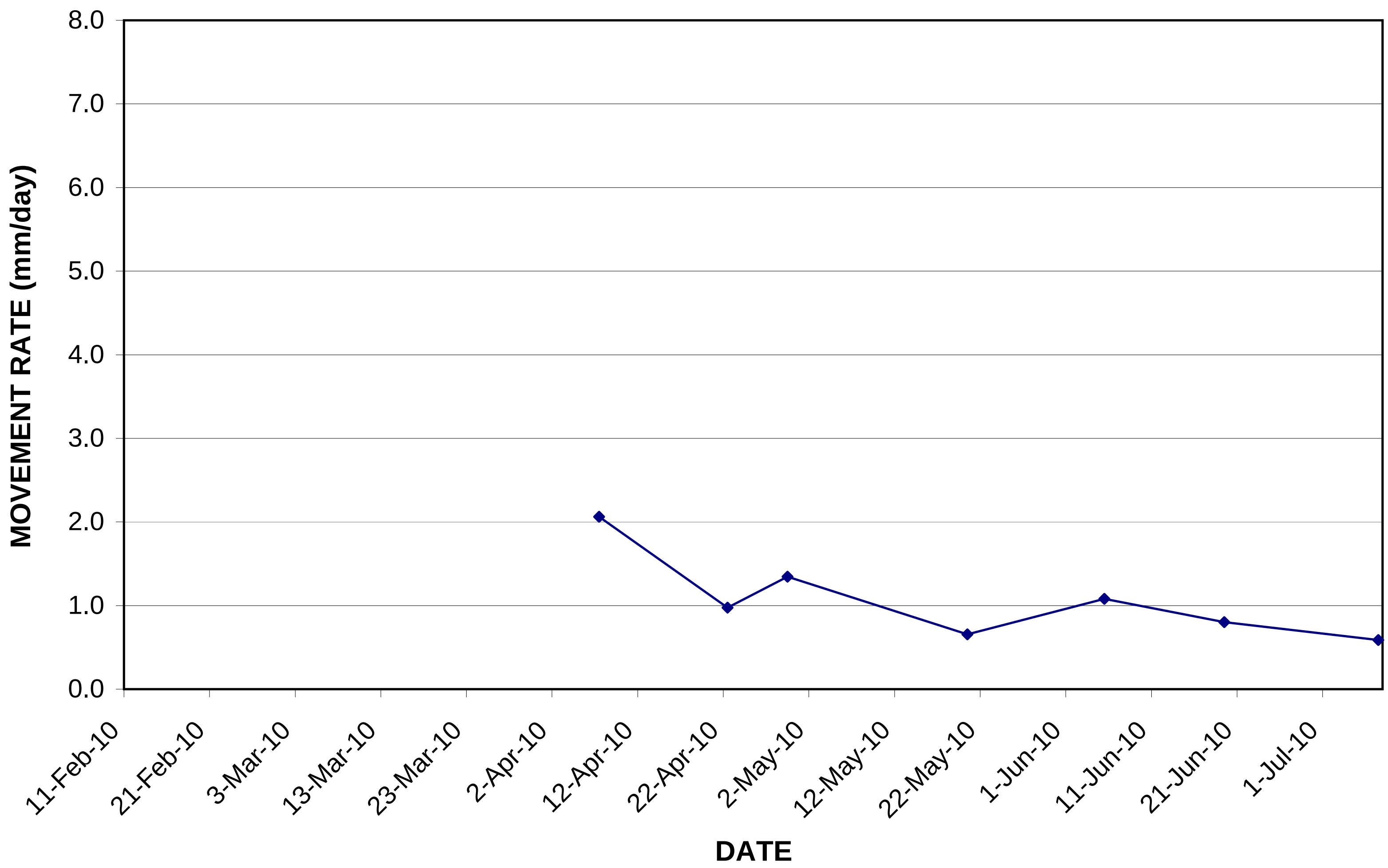
Installed: April 5, 2010

Dry Stack Tailings Storage Facility
DSI-3 Inclinometer Movement Rate
Figure I03



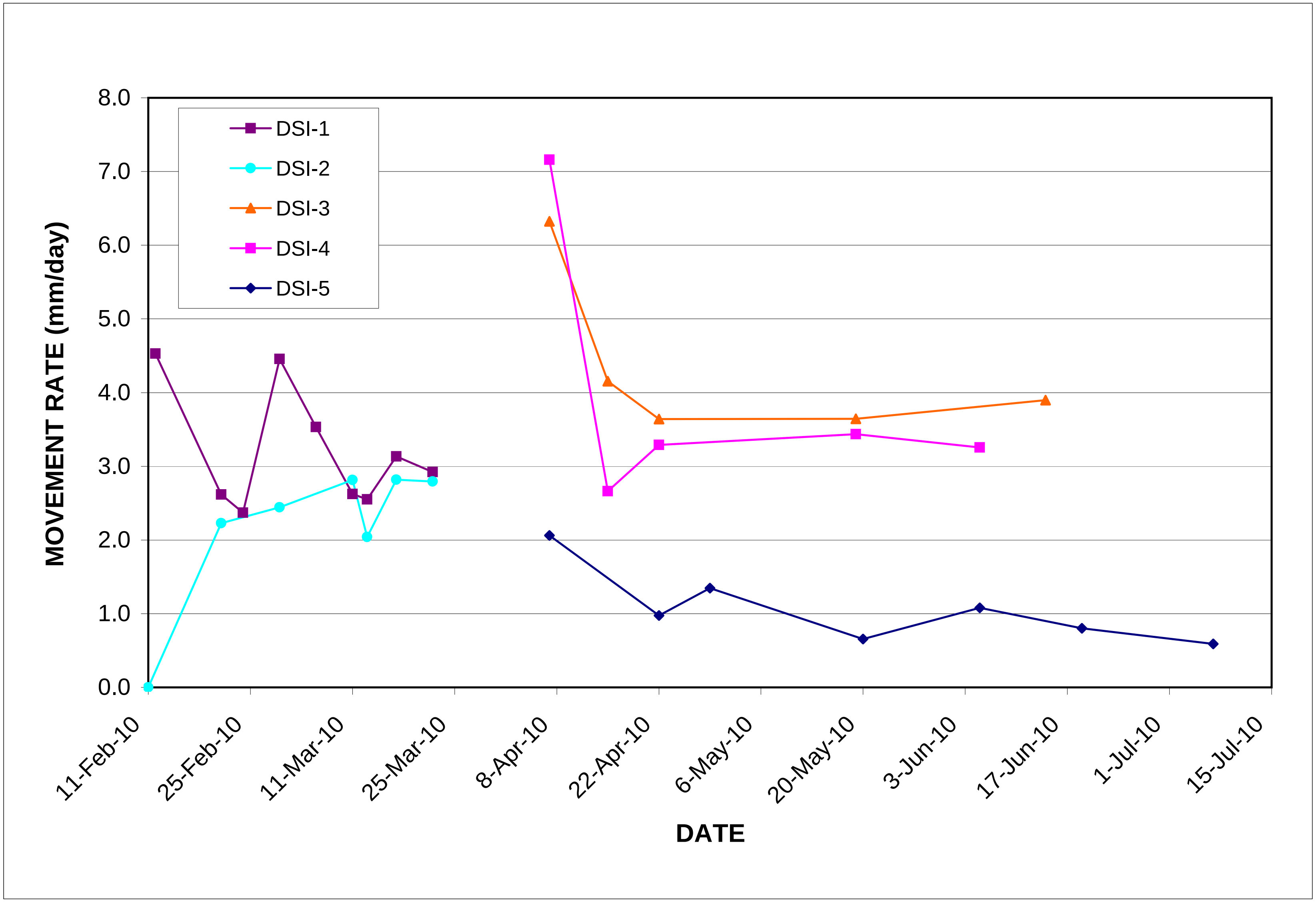
Installed: April 4, 2010

Dry Stack Tailings Storage Facility
DSI-4 Inclinometer Movement Rate
Figure I04



Installed: April 5, 2010

Dry Stack Tailings Storage Facility
DSI-5 Inclinometer Movement Rate
Figure I05



Dry Stack Tailings Storage Facility
Inclinometer Movement Rates
Figure I06

PHOTOGRAPHS

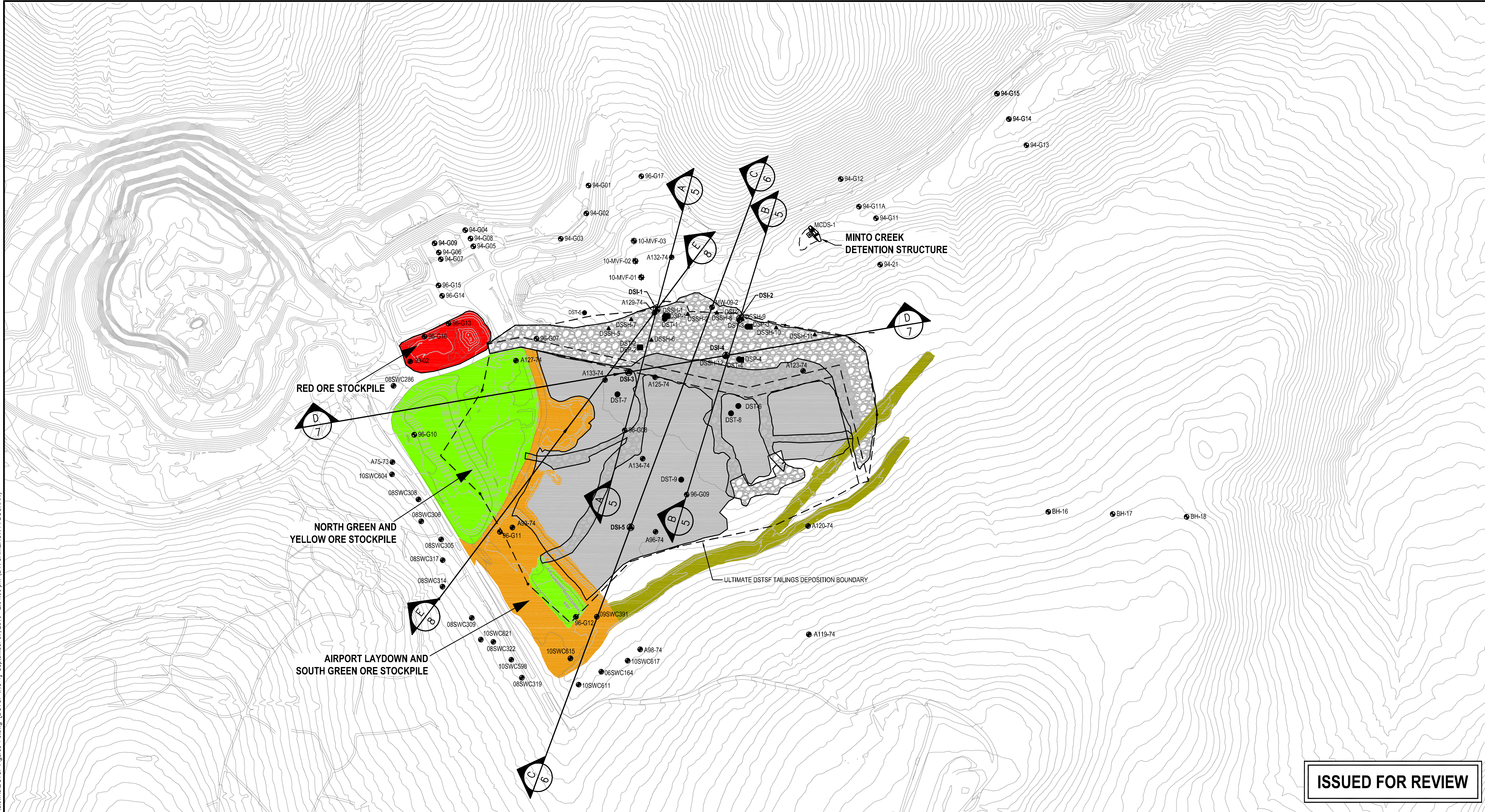
Photo 1
Overview of DSTSF Aspect: Southeast (Photo taken August 18, 2010)

Photo 2
Ice exposed in cut for the 810 m bench in Area 1Pit (Photo taken August 18, 2010)

APPENDIX

APPENDIX A DRAWINGS

C:\Documents and Settings\cbuchan\Desktop\Minto for Kelowna\DSTSF Figures 4-8.dwg [DSTSF-INC-4] September 07, 2010 - 2:04:06 pm (BY: CAMERON F. BUCHAN)



LEGEND

- BOREHOLES LIMITED TO DSTSF AREA
- INCLINOMETERS LIMITED TO DSTSF AREA
- SURVEY HUBS LIMITED TO DSTSF AREA
- GROUND TEMPERATURE CABLES LIMITED TO DSTSF AREA
- PIEZOMETERS LIMITED TO DSTSF AREA
- TESTPITS LIMITED TO DSTSF AREA
- EXPLORATION BOREHOLES LIMITED TO DSTSF AREA

- WASTE ROCK
- STOCKPILE
- STOCKPILE
- LAYDOWN
- PLACED TAILINGS
- SURFACE DIVERSION BERMS

NOTE :
3m INTERMEDIATE AND 15m INDEX CONTOURS BASED ON APRIL SURVEY DATA PROVIDED BY MINTO AND PRE-DEVELOPMENT 2 m CONTOUR DATA.

PLAN

SCALE 1:7500

50 0 50 150 300m

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EBA Engineering Consultants Ltd.

DRY STACK TAILINGS STORAGE FACILITY MINTO MINE, YUKON

SITE PLAN WITH SECTIONS

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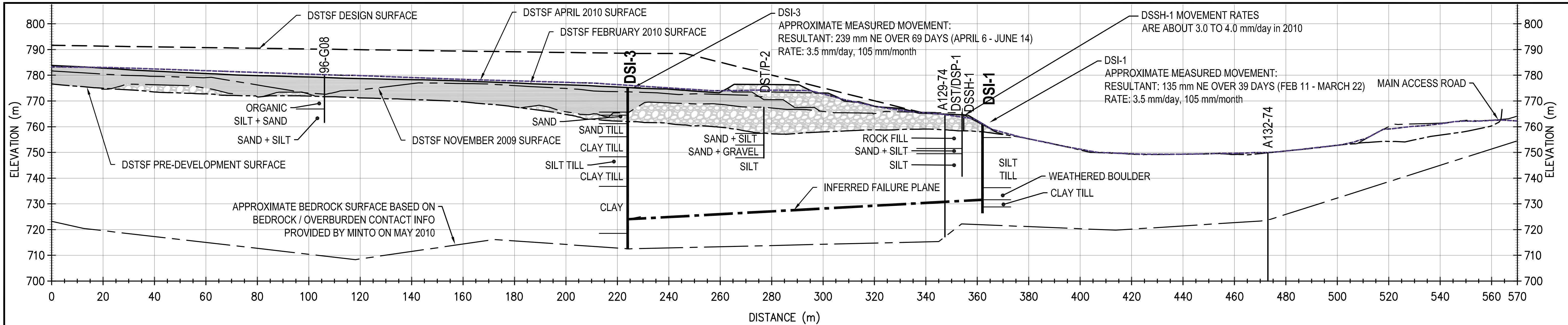
WHSE	September 7, 2010		

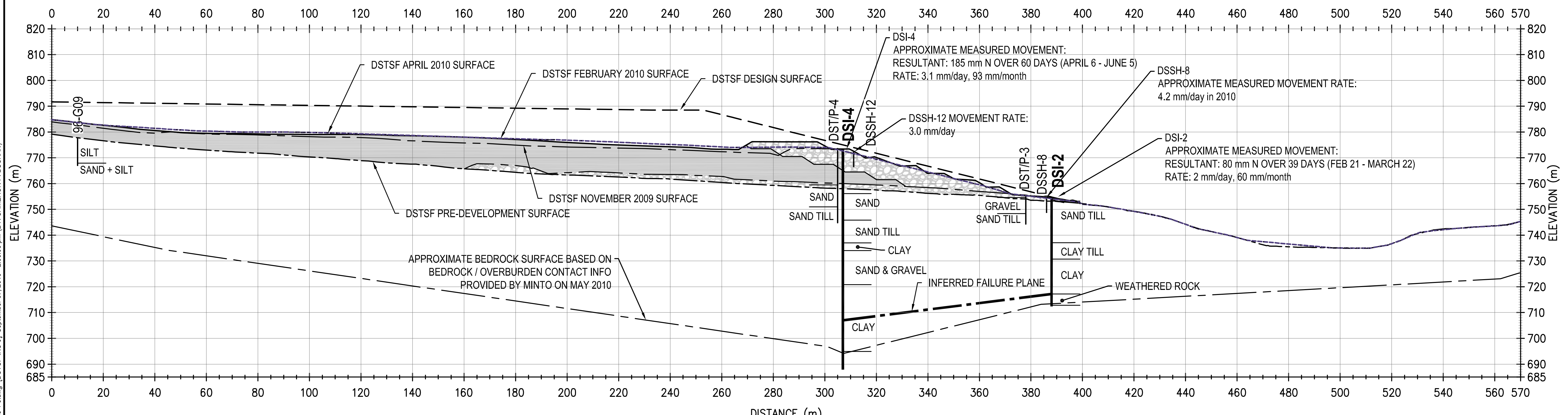
DSTSF-INC-4

C:\Documents and Settings\cbuchan\Desktop\Minto for Kelowna\DSTSF Figures 4-8.dwg [DSTSF-INC-5] September 07, 2010 - 2:05:08 pm (BY: CAMERON F. BUCHAN)



A 1 SECTION A
SCALE 1:1500

Scale: 1: 1 500 (metres)



B 1 SECTION B
SCALE 1:1500

Scale: 1: 1 500 (metres)

ISSUED FOR REVIEW

LEGEND

- TAILING
- WASTE ROCK

NOTES :

- STRATIGRAPHY TO BE CONFIRMED ON COMPLETION OF INDEX LAB TESTING
- PRE-DEVELOPMENT SURFACE DERIVED FROM 2m CONTOUR DATA FROM AIR PHOTO INTERPRETATION

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SECTION A AND SECTION B

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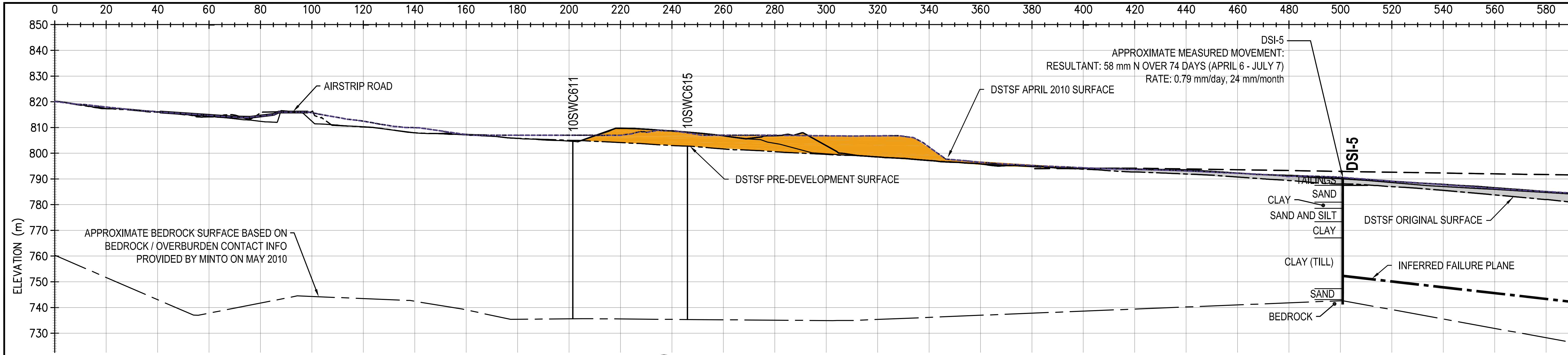
WHSE	September 7, 2010		

DSTSF-INC-5



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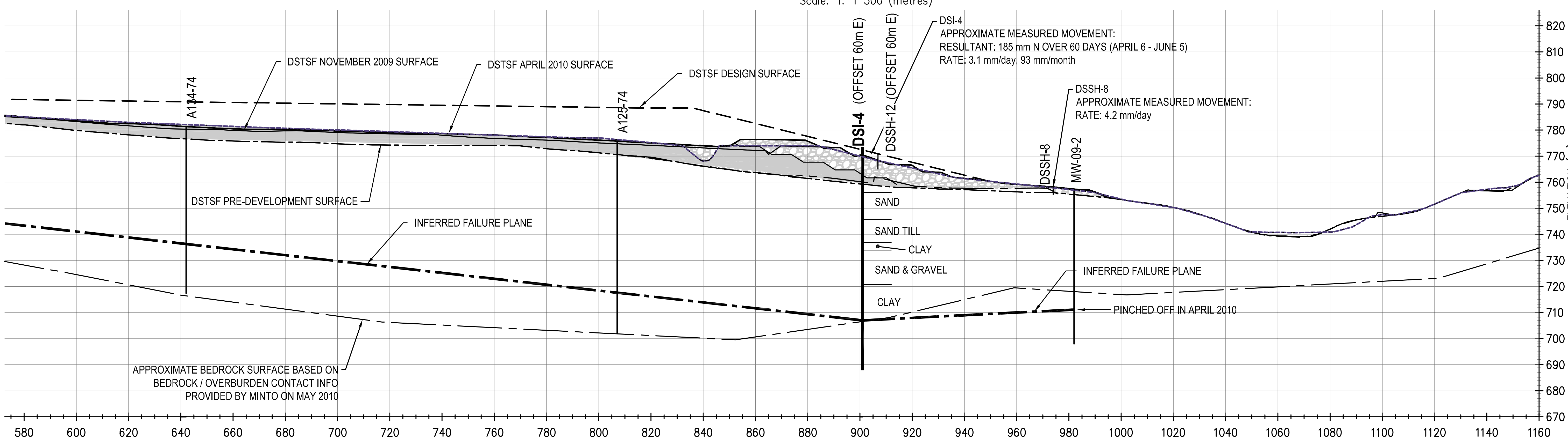
SCALE 1:1500

Scale: 1: 1 500 (metres)



SECTION C (C/1)
SCALE 1:1500

Scale: 1: 1 500 (metres)

LEGEND
- TAILING
- LAYDOWN
- WASTE ROCK

NOTES :

- STRATIGRAPHY TO BE CONFIRMED ON COMPLETION OF INDEX LAB TESTING
- PRE-DEVELOPMENT SURFACE DERIVED FROM 2m CONTOUR DATA FROM AIR PHOTO INTERPRETATION

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SECTION C

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WHSE	September 7, 2010		

DSTSF-INC-6

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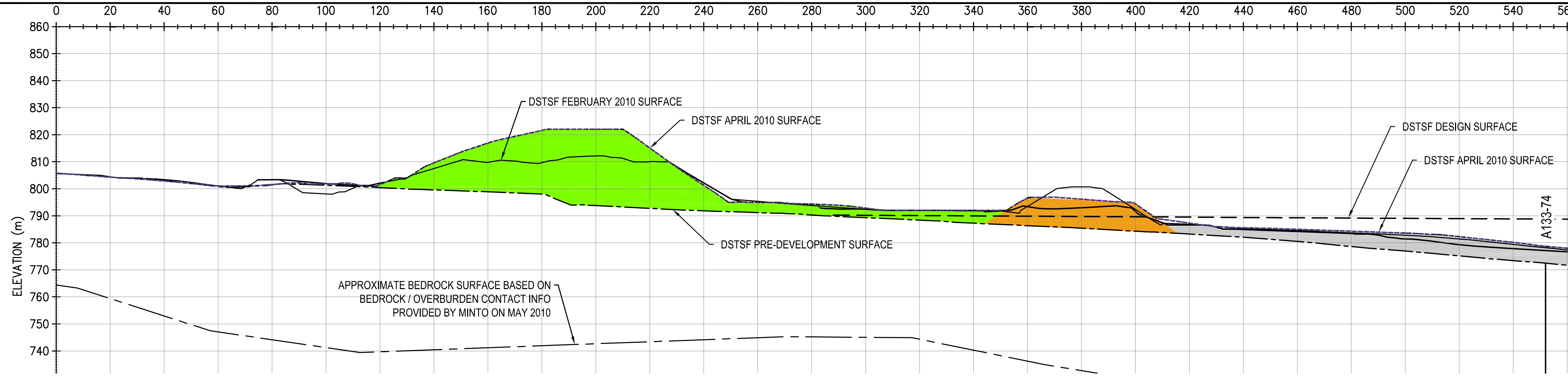
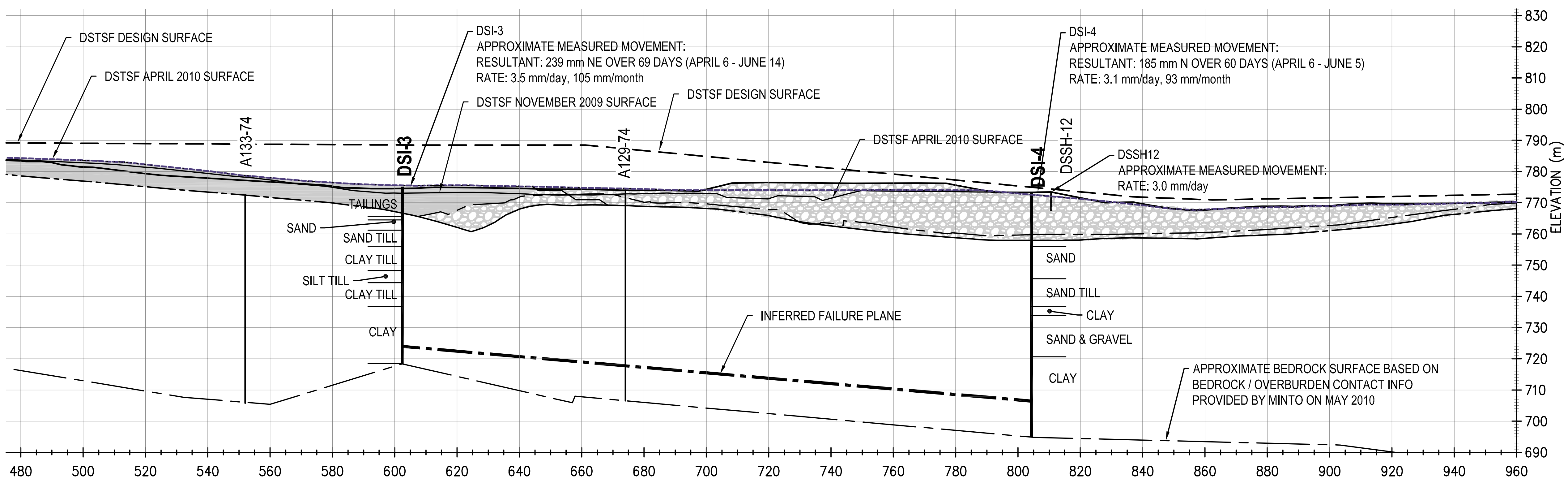


SECTION D (D/1)
SCALE 1:1500

Scale: 1: 1 500 (metres)



SECTION D (D/1)
SCALE 1:1500

Scale: 1: 1 500 (metres)

ISSUED FOR REVIEW

LEGEND

- TAILING
- LAYDOWN
- STOCKPILE
- WASTE ROCK

NOTES :

- STRATIGRAPHY TO BE CONFIRMED ON COMPLETION OF INDEX LAB TESTING
- PRE-DEVELOPMENT SURFACE DERIVED FROM 2m CONTOUR DATA FROM AIR PHOTO INTERPRETATION

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SECTION D

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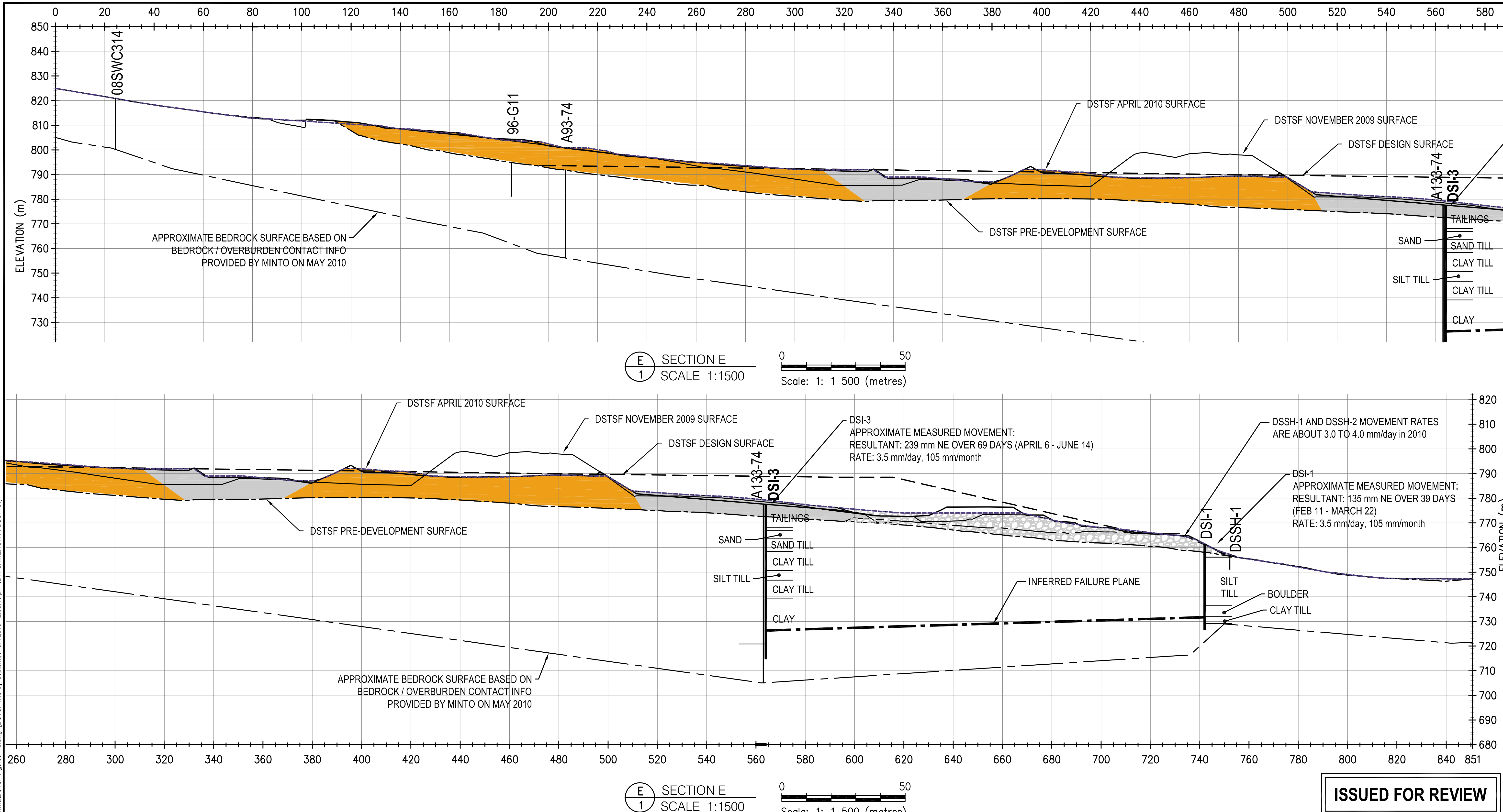
OFFICE	DATE		
WHSE	September 7, 2010		

DSTSF-INC-7



ISSUED FOR REVIEW

LEGEND

- TAILING
- LAYDOWN
- WASTE ROCK

NOTES :

- STRATIGRAPHY TO BE CONFIRMED ON COMPLETION OF INDEX LAB TESTING
- PRE-DEVELOPMENT SURFACE DERIVED FROM 2m CONTOUR DATA FROM AIR PHOTO INTERPRETATION

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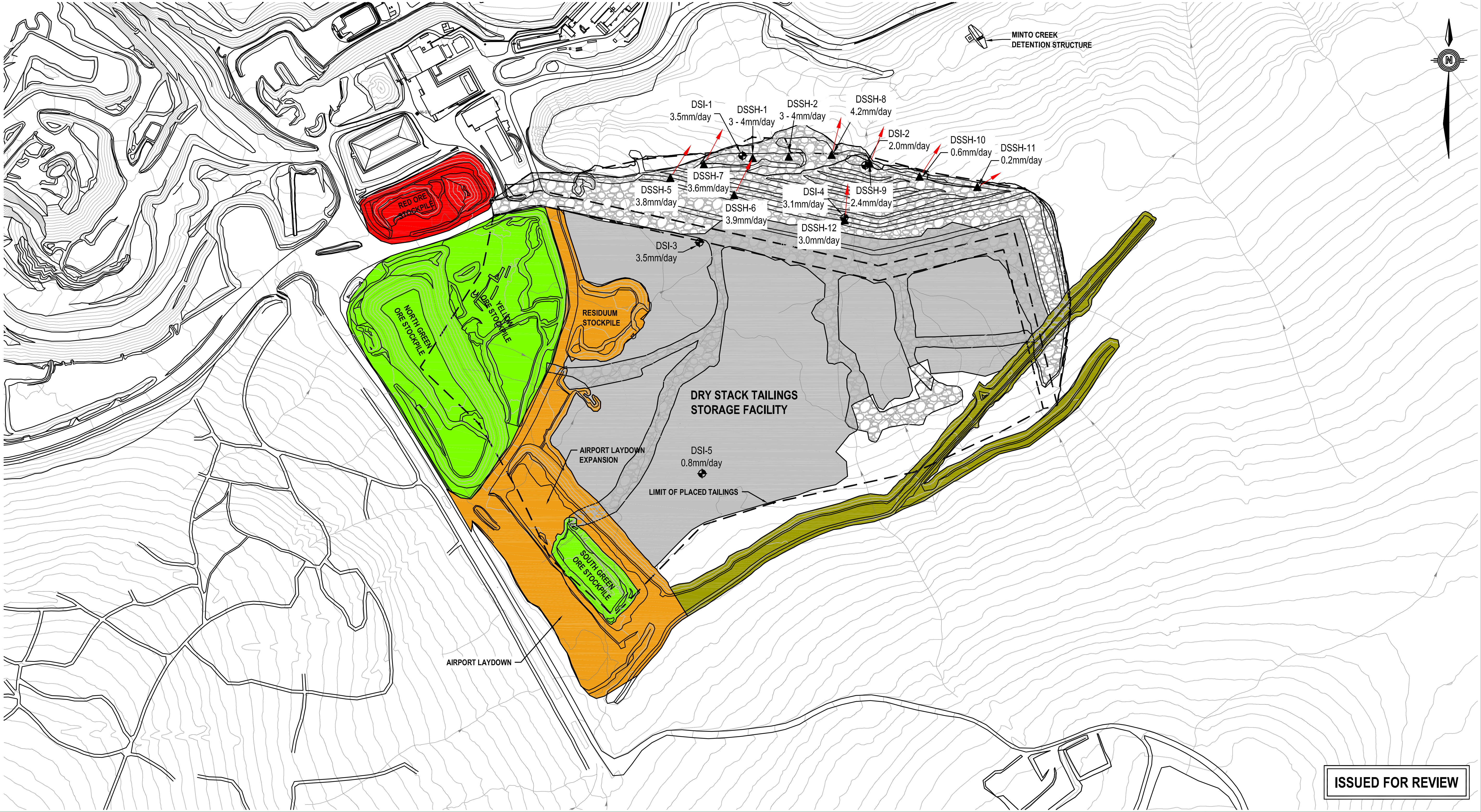
MINTO MINE, YUKON

SECTION E

PROJECT NO.	DWN	CKD	REV
W14101068.001	CC	BJC	0
OFFICE	DATE		
WHSE	September 7, 2010		

DSTSF-INC-8



ISSUED FOR REVIEW

LEGEND:
- INCLINOMETERS LIMITED TO DSTSF AREA
- SURVEY HUBS LIMITED TO DSTSF AREA
- STOCKPILE (red)
- WASTE ROCK
- STOCKPILE (green)
- LAYDOWN
- PLACED TAILINGS
- SURFACE WATER DIVERSION BERMS

NOTE:
3m INTERMEDIATE AND 15m INDEX CONTOURS BASED ON APRIL SURVEY DATA PROVIDED BY MINTO AND PRE-DEVELOPMENT 2 m CONTOUR DATA.

0 — 250

Scale: 1: 5 000 (metres)

NUM	DATE	APR	DESCRIPTION

DRAWING STATUS

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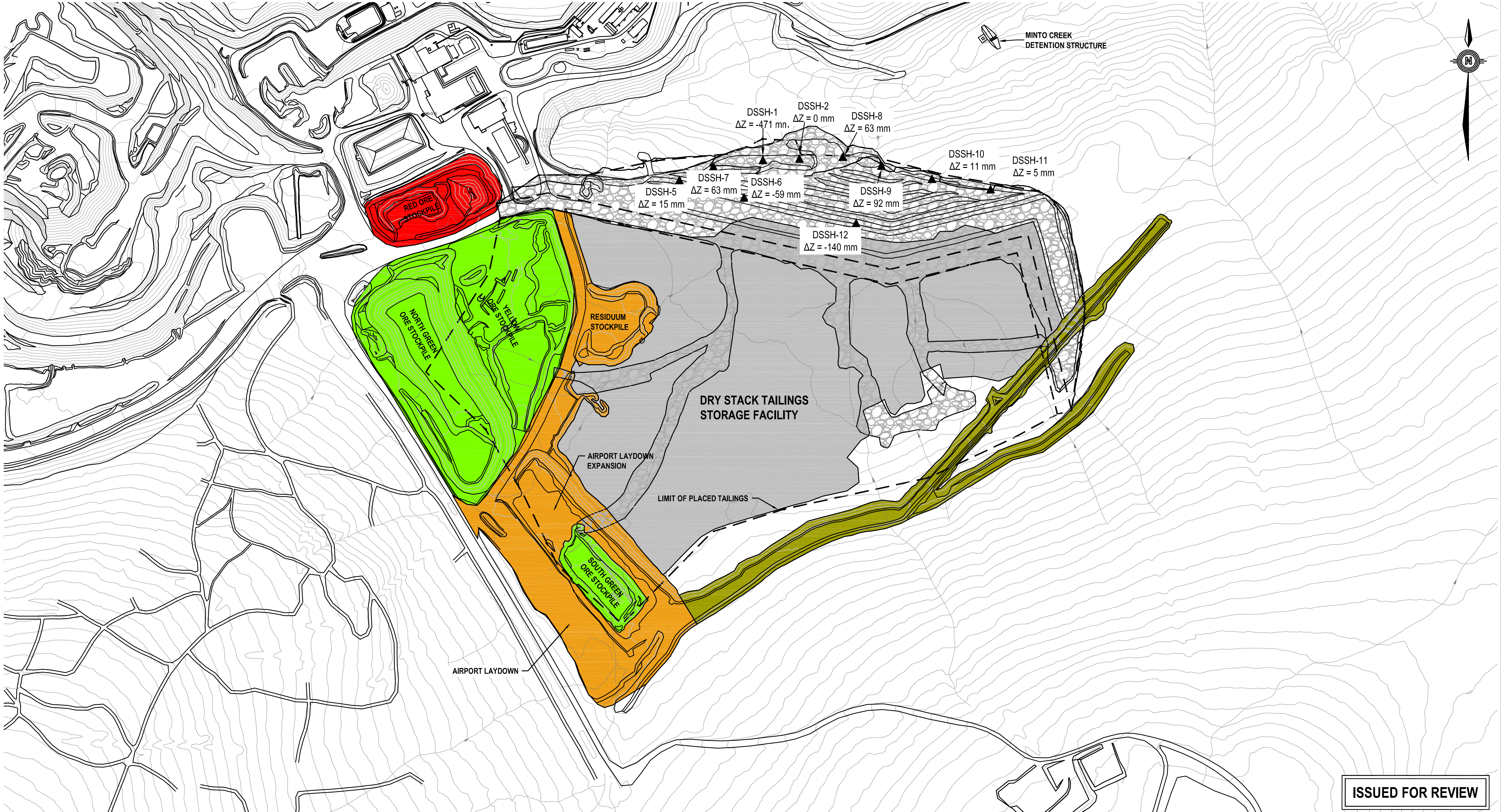
DRY STACK TAILINGS STORAGE FACILITY
MINTO MINE, YUKON

SURVEY HUBS AND INCLINOMETERS
LATERAL MOVEMENTS AND RATES

PROJECT NO.	DWN	CKD	REV
W14101068.001	CB	BJC	0
OFFICE	DATE		
EBA-WHSE	September 7, 2010		

DSTSF-INC-9



ISSUED FOR REVIEW

LEGEND:
- INCLINOMETERS LIMITED TO DSTSF AREA
- SURVEY HUBS LIMITED TO DSTSF AREA
- STOCKPILE (red)
- STOCKPILE (green)
- LAYDOWN
- PLACED TAILINGS
- SURFACE WATER DIVERSION BERMS
- WASTE ROCK

NOTE:
3m INTERMEDIATE AND 15m INDEX CONTOURS BASED ON APRIL SURVEY DATA PROVIDED BY MINTO AND PRE-DEVELOPMENT 2 m CONTOUR DATA.

0 — 250

Scale: 1: 5 000 (metres)

NUM	DATE	APR	DESCRIPTION

DRAWING STATUS

CLIENT

MINTO EXPLORATIONS LTD.

EBA Engineering Consultants Ltd.

DRY STACK TAILINGS STORAGE FACILITY
MINTO MINE, YUKON

SURVEY HUBS
CHANGE IN ELEVATION

PROJECT NO.	DWN	CKD	REV
W14101068.001	CB	BJC	0

OFFICE: EBA-WHSE
DATE: September 7, 2010

DSTSF-INC-10

APPENDIX

APPENDIX B GEOTECHNICAL REPORT - GENERAL CONDITIONS

GEOTECHNICAL REPORT – GENERAL CONDITIONS

This report incorporates and is subject to these "General Conditions".

1.0 USE OF REPORT AND OWNERSHIP

This geotechnical report pertains to a specific site, a specific development and a specific scope of work. It is not applicable to any other sites nor should it be relied upon for types of development other than that to which it refers. Any variation from the site or development would necessitate a supplementary geotechnical assessment.

This report and the recommendations contained in it are intended for the sole use of EBA's Client. EBA does not accept any responsibility for the accuracy of any of the data, the analyses or the recommendations contained or referenced in the report when the report is used or relied upon by any party other than EBA's Client unless otherwise authorized in writing by EBA. Any unauthorized use of the report is at the sole risk of the user.

This report is subject to copyright and shall not be reproduced either wholly or in part without the prior, written permission of EBA. Additional copies of the report, if required, may be obtained upon request.

2.0 ALTERNATE REPORT FORMAT

Where EBA submits both electronic file and hard copy versions of reports, drawings and other project-related documents and deliverables (collectively termed EBA's instruments of professional service), only the signed and/or sealed versions shall be considered final and legally binding. The original signed and/or sealed version archived by EBA shall be deemed to be the original for the Project.

Both electronic file and hard copy versions of EBA's instruments of professional service shall not, under any circumstances, no matter who owns or uses them, be altered by any party except EBA. EBA's instruments of professional service will be used only and exactly as submitted by EBA.

Electronic files submitted by EBA have been prepared and submitted using specific software and hardware systems. EBA makes no representation about the compatibility of these files with the Client's current or future software and hardware systems.

3.0 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, EBA has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

4.0 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. EBA does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

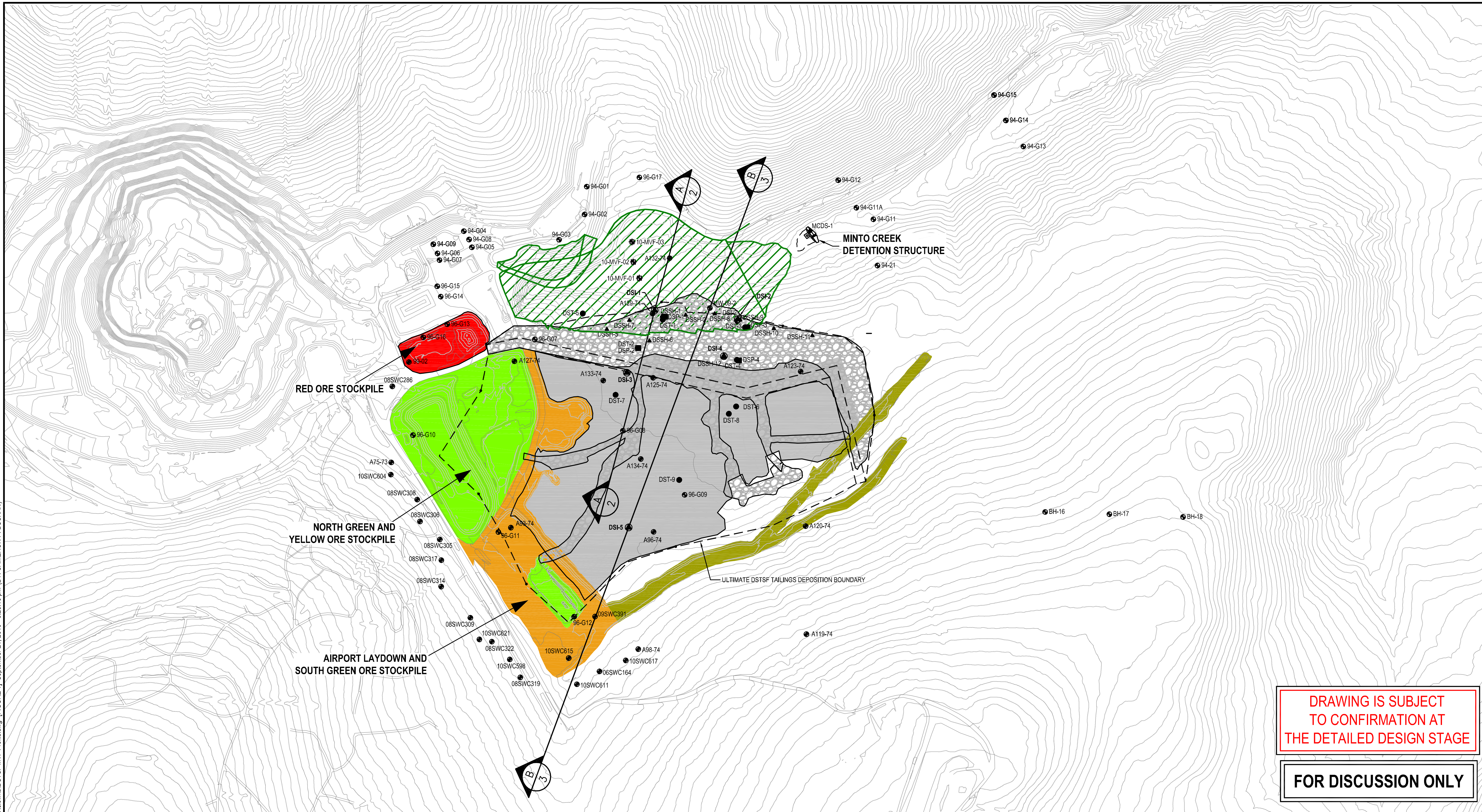
Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

5.0 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

6.0 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. EBA does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.



DRAWING IS SUBJECT TO CONFIRMATION AT THE DETAILED DESIGN STAGE

FOR DISCUSSION ONLY

LEGEND

- BOREHOLES LIMITED TO DSTSF AREA
- INCLINOMETERS LIMITED TO DSTSF AREA
- SURVEY HUBS LIMITED TO DSTSF AREA
- GROUND TEMPERATURE CABLES LIMITED TO DSTSF AREA
- PIEZOMETERS LIMITED TO DSTSF AREA
- TESTPITS LIMITED TO DSTSF AREA
- EXPLORATION BOREHOLES LIMITED TO DSTSF AREA

- WASTE ROCK
- STOCKPILE
- STOCKPILE
- LAYDOWN
- PLACED TAILINGS
- SURFACE DIVERSION BERMS
- MILL VALLEY FILL

NOTE :
3m INTERMEDIATE AND 15m INDEX CONTOURS BASED ON APRIL SURVEY DATA PROVIDED BY MINTO AND PRE-DEVELOPMENT 2 m CONTOUR DATA.

PLAN

SCALE 1:7500

50 0 50 150 300m

CLIENT

MINTO EXPLORATIONS LTD.

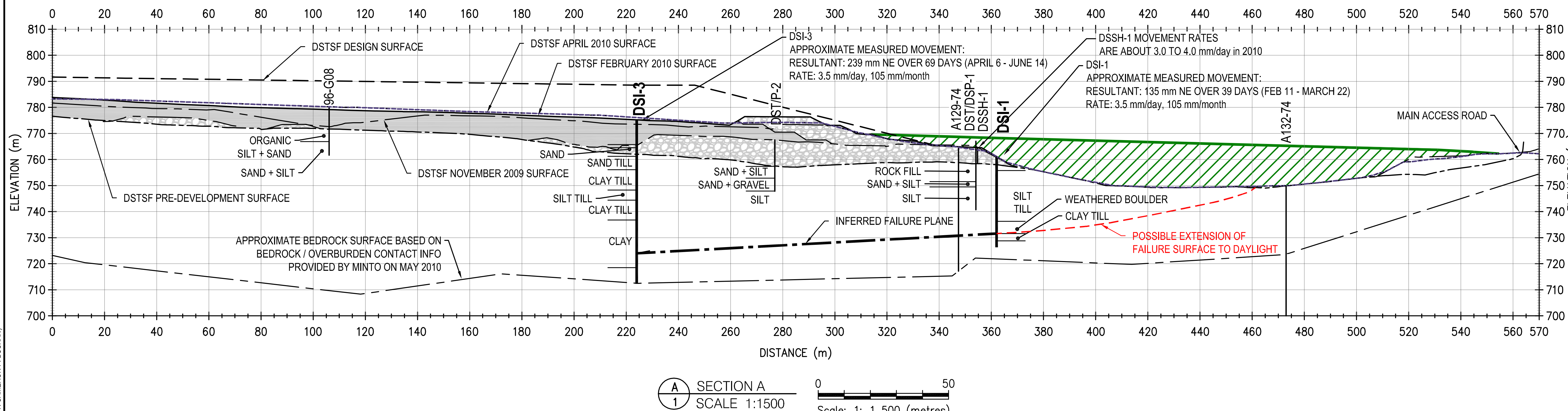
EBA Engineering Consultants Ltd.

MILL VALLEY FILL PLANNING
MINTO MINE, YUKON

SITE PLAN WITH SECTIONS

PROJECT NO.	DWN	CKD	REV
W14101068.001	CB	BJC	0
OFFICE	DATE		
EBA-WHSE	September 20, 2010		

Figure 1



DRAWING IS SUBJECT TO CONFIRMATION AT THE DETAILED DESIGN STAGE

FOR DISCUSSION ONLY

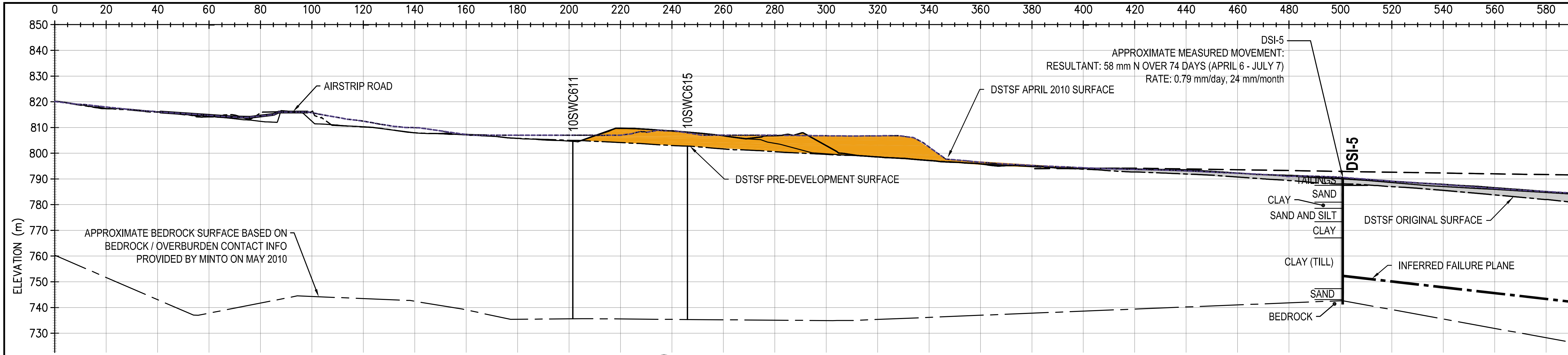
LEGEND

- TAILING
- WASTE ROCK
- MILL VALLEY FILL

NOTES :

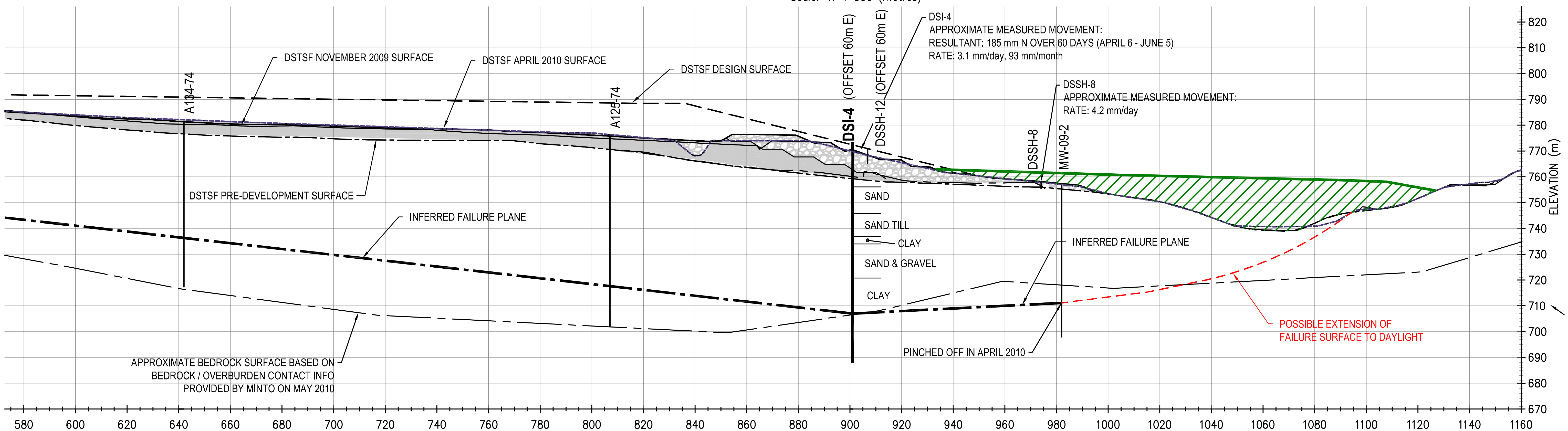
- STRATIGRAPHY TO BE CONFIRMED ON COMPLETION OF INDEX LAB TESTING
- PRE-DEVELOPMENT SURFACE DERIVED FROM 2m CONTOUR DATA FROM AIR PHOTO INTERPRETATION
- INFORMATION WAS COPIED FROM DSTSF UPDATE REPORT (DATED AUGUST 12, 2010) TO SHOW MILL VALLEY FILL SURFACE

CLIENT	MILL VALLEY FILL PLANNING MINTO MINE, YUKON				
MINTO EXPLORATIONS LTD.	SECTION A				
EBA Engineering Consultants Ltd.	PROJECT NO. W14101068.001	DWN CB	CKD BJC	REV 0	Figure 2
	OFFICE EBA-WHSE	DATE September 20, 2010			



C 1 SECTION C
SCALE 1:1500

Scale: 1: 1 500 (metres)



C 1 SECTION C
SCALE 1:1500

Scale: 1: 1 500 (metres)

LEGEND

- TAILING
- LAYDOWN
- WASTE ROCK
- MILL VALLEY FILL

NOTES :

- STRATIGRAPHY TO BE CONFIRMED ON COMPLETION OF INDEX LAB TESTING
- PRE-DEVELOPMENT SURFACE DERIVED FROM 2m CONTOUR DATA FROM AIR PHOTO INTERPRETATION
- INFORMATION WAS COPIED FROM DSTSF UPDATE REPORT (DATED AUGUST 12, 2010) TO SHOW MILL VALLEY FILL SURFACE

DRAWING IS SUBJECT TO CONFIRMATION AT THE DETAILED DESIGN STAGE

FOR DISCUSSION ONLY

CLIENT

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EBA Engineering Consultants Ltd.

MILL VALLEY FILL PLANNING
MINTO MINE, YUKON

SECTION B

PROJECT NO.	DWN	CKD	REV
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OFFICE	DATE		
EBA-WHSE	September 20, 2010		

Figure 3