

VICTORIA GOLD CORPORATION

EAGLE GOLD PROJECT DUBLIN GULCH, YUKON

2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

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Dear Mr. Padula,

Re: **EAGLE GOLD PROJECT: 2011 GEOTECHNICAL INVESTIGATION FOR MINE
SITE INFRASTRUCTURE FACTUAL DATA REPORT**

Please find attached the final version of the aforementioned report. Should you have any questions or comments, please do not hesitate to contact the undersigned.

Yours sincerely,

BGC ENGINEERING INC.
per:

Pete Quinn, Ph.D., P.Eng.
Senior Geotechnical Engineer

EXECUTIVE SUMMARY

This report summarizes the findings of geotechnical site investigation work conducted in the summer of 2011 at the Eagle Gold Project, at Dublin Gulch, located near Mayo, Yukon. Several areas on site were previously explored as part of a pre-feasibility study for development of an open pit heap leach gold mine. BGC is providing engineering recommendations for foundations, waste rock storage areas, open pit slopes and pit depressurization under separate cover. The present report addresses investigations conducted in 2011 in support of a Feasibility Study for proposed mine site infrastructure other than the open pit, including: the heap leach pad; waste rock storage areas; crushers and conveyors; water diversion structures; plant site buildings; solution and water management ponds; and other miscellaneous buildings and facilities.

The 2011 geotechnical investigation program involved the excavation of ninety-six test pits, advancement of forty-six drill holes (29 Diamond holes and 17 Auger holes), and mapping of fifty-nine outcrops (natural exposures, existing road cuts and drill pads cuts) to characterize subsurface conditions relevant for foundation and earthworks design. Samples were taken from select test pits and boreholes for index testing of soil and rock. Bulk samples of rock and placer tailings were also analyzed for a range of parameters related to the potential for re-use as select fill or aggregate. This report presents factual data only, and does not include any engineering interpretation of the data nor engineering recommendations in relation to the proposed mine facilities.

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LIMITATIONS

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1.0 INTRODUCTION

1.1. General

Victoria Gold Corporation (VIT), with assistance from Wardrop Engineering Inc. (WEI) is completing a feasibility study (FS) for development of the proposed Eagle Gold mine at Dublin Gulch, Yukon. BGC Engineering Inc. (BGC) was contracted by VIT to complete geotechnical investigations for mine site infrastructure. This report presents factual data resulting from geotechnical investigations performed between June and August 2011. This work was conducted to investigate subsurface conditions at the proposed locations for the mine site infrastructure, other than the open pit, including: the heap leach pad; waste rock storage areas; crushers and conveyors; water diversion structures; plant site buildings; solution and water management ponds; and other miscellaneous buildings and facilities.

1.2. Project description

The Eagle Gold property is located approximately 40 km north of Mayo, and 15 km northwest of Elsa, as illustrated in Drawing 01. The mine will comprise an open pit and heap leach pad; haul roads; waste rock storage areas; process plant; crushers and conveyors; truck shop; camp; water diversion structure; process water ponds; drainage ditches; sediment control structures and various ancillary facilities.

The current layout for the proposed mine facilities, as received from Wardrop on November 23, 2011, is illustrated in Drawing 02. The field investigation plan was made on the basis of an earlier general arrangement received from Wardrop on June 14, 2011, and was updated during execution of the field work as the layout was adjusted on August 10, 2011. Geotechnical engineering recommendations will be provided at a later date under separate cover in a foundation design report.

1.3. Previous Investigations

Site conditions at the Eagle Gold site have been partially described in several reports as follows, and Drawing 03 shows the location of all the historic holes within the area of interest:

- Report on 1995 Geotechnical Investigations for Four Potential Heap Leach Facility Site Alternatives, First Dynasty Mines, Dublin Gulch Property. (Knight Piésold, 1996a).
- Report on Feasibility Design of the Mine Waste Rock Storage Area, First Dynasty Mines, Dublin Gulch Property. (Knight Piésold, 1996b).
- Field Investigation Data Report, Dublin Gulch Project, New Millennium Mining. (Sitka Corp, 1996).
- Hydrogeological Characterization and Assessment, Dublin Gulch Project, New Millennium Mining. (GeoEnviro Engineering, 1996).
- BGC Engineering Inc. 2009. Site Facilities Geotechnical Investigation Factual Data Report. Eagle Gold Project, Victoria Gold Corporation.

- Stantec. 2010. Project Proposal for Executive Committee Review. Pursuant to the Yukon Environmental and Socio-Economic Assessment Act. Eagle Gold Project, Victoria Gold Corporation.
- BGC Engineering Inc. 2011a. 2010 Geotechnical Investigation for Mine Site Infrastructure, Factual Data Report. Eagle Gold Project, Victoria Gold Corporation.

Knight Piésold completed a feasibility level geotechnical study to evaluate the surficial materials and bedrock conditions at four potential heap leach pad locations, two potential waste rock storage areas, and the open pit. Groundwater wells and two thermistors were installed in selected drillholes at that time. Test pitting and diamond drilling were completed from June to September 1995 at upper Bawn Bay Gulch, lower Dublin Gulch, the north side of Lynx Creek, and at the confluence of Haggart and Lynx Creeks.

In 1996, Sitka Corp completed test pits and diamond drillholes in Bawn Bay Gulch, Eagle Pup, Stewart Gulch, and Platinum Gulch for preliminary design of the heap leach and waste rock facilities. Auger holes were drilled in Gill Gulch to evaluate it as a potential borrow source of silt material for use as a liner for the heap leach facility. Monitoring wells were installed in Bawn Bay Gulch and Eagle Pup. Eight thermistor strings were installed.

In 2009, BGC was engaged to gather factual data describing subsurface conditions at the proposed heap leach and waste rock storage facility locations. The work involved the excavation of sixty-nine test pits and advancement of seven boreholes. Thermistor strings were installed in three boreholes to gather temperature data. Dynamic cone penetration profiles were obtained at two borehole locations to obtain information about material density. Dynamic cone soundings were attempted in two other holes and met with refusal. Groundwater monitoring wells were installed by Stantec in two of the seven BGC boreholes. In addition, Stantec supervised the logging and installation of several other monitoring wells around the site.

In 2010, Stantec presented a Project Proposal which included general site conditions such as regional geology, physiography, drainage, climate and seismicity. Air-photo based terrain mapping and an evaluation of geological hazards affecting the project area were both also described in this report.

In 2010, BGC was engaged to develop a geotechnical site investigation program in support of the FS for proposed mine site infrastructure. A total of forty-nine test pits and twenty-five drill holes were completed to characterize the overburden material and bedrock conditions. Additionally, three cut slopes were logged for exposed soil and rock conditions, and core from one client-drilled condemnation hole was logged for geotechnical purposes. Laboratory testing was completed on selected samples for moisture content, and representative samples were also tested for Atterberg Limits and grain size analysis. Various other lab tests were also completed on bulk samples of placer tailings being considered for potential use as select fill or aggregate.

Several engineering reports were also issued in draft by BGC in early 2011, with preliminary foundation and earthworks recommendations for a number of key facilities.

1.4. Scope of Work

BGC was engaged to provide further geotechnical investigation work to address gaps in the data required in support of FS design for mine site infrastructure. The 2011 site investigation program was conducted between June and August 2011 and the field activities involved the excavation of ninety-six test pits, advancement of forty-six drill holes (29 Diamond holes and 17 Auger holes), and mapping of fifty-nine outcrops (natural exposures, existing road cuts and drill pads cuts) to characterize subsurface conditions relevant for foundation and earthworks design. Samples were taken from select test pits and boreholes for index testing of soil and rock. Bulk samples of rock and placer tailings were also analyzed for a range of parameters related to the potential for re-use as select fill or aggregate.

2.0 SITE CONDITIONS

2.1. Surficial Geology

The surficial geology of the Dublin Gulch area has been mapped by Bond (1998) and is illustrated in Drawing 04. Pleistocene and Holocene colluvial deposits are abundant in the project area and generally consist of diamicton, gravel, shattered bedrock, and lenses of sand and silt derived from bedrock and surficial materials by a variety of chemical and physical weathering processes. Transport of surface material occurs as creep, sheetwash, and mass wasting and is common on all slopes in the area.

Glacial till is infrequently observed because the valley is aligned transverse to the regional Cordilleran ice flow. Where till does occur, it is generally either a silty or sandy clay matrix with some proportion of larger clasts up to cobble size. The valley bottom is dominated by alluvium and placer mining tailings. The north facing uplands are covered by an apron or blanket of colluvium over bedrock, as compared with the southern facing uplands, where bedrock is nearer to surface and covered by a thinner veneer of colluvium. The Haggart Creek Valley to the west of the project site is filled with a mix of alluvial deposits and placer tailings. A till blanket has been mapped along the east side of Haggart Creek, south of its confluence with Dublin Gulch.

The project site is located in a region of discontinuous permafrost. Permafrost distribution within the project area is controlled by factors such as soil texture, soil moisture, aspect, vegetation and snow depth. Permafrost occurs typically on north- and east-facing slopes at higher elevations, and within poorly drained valley bottoms.

Frozen ground, when observed, is generally encountered immediately below the organic cover, although frozen organics are also encountered on north-facing slopes and under a dense spruce forest canopy. The distribution of frozen ground is highly variable across the site. Permafrost was confirmed with temperature measurements from thermistors installed on site by Knight Piesold, Sitka Corp and BGC. However, in all cases of confirmed permafrost, the measured ground temperatures showed the permafrost to be relatively warm, between 0°C and -1°C.

2.2. Bedrock Geology

The Eagle Gold mineral deposit is located in a granodiorite intrusion at the western limit of the Dublin Gulch property at its narrowest extent. It is hosted within Upper Proterozoic to Lower Cambrian aged metasedimentary rocks of the Hyland group. The metasedimentary clastic rocks consist of intercalated and deformed quartzites, and phyllites, and to a lesser degree, schists and carbonates. The dominant feature in the Dublin Gulch deposit area is the Potato Hills stock which outcrops above Haggart Creek and consists of a medium-grained phaneritic granodiorite intrusive dated at approximately 93 million years (Smit et al., 1995). The stock is elongated 70°E and is roughly 2 km wide by 5.5 km long. The long axis of the stock coincides with the axis of the Dublin Gulch anticline. Sheet-like sills of

granodiorite extend from the stock and cut the meta-sedimentary strata at low angles. The intrusive-meta-sedimentary contact dips shallowly to steeply to the north on the northern side of the intrusive, and steeply to the north or south along its southern margin.

The Dublin Gulch Property is located in the northern portion of the Selwyn Basin. The Selwyn Basin is a fault controlled epicratonic basin that is underlain by four main metasedimentary units. Dominantly clastic, these units are, from oldest to youngest, the Proterozoic Hyland Group, the Paleozoic Upper Schist, and the Mesozoic Lower Schist and Keno Hill Quartzites.

These units have been juxtaposed by Cretaceous-aged laterally extensive, northward directed thrust sheets. There are three main thrust sheets, from east to west: the Dawson, Tombstone and Robert Service.

Four generations of deformation are documented; however, two initial events are responsible for the dominant structural orientation. The first resulted in widespread development of foliation that was subsequently deformed by gentle, regional-scale folding. The second deformational event resulted in east-trending, south-plunging anticlines within the Dublin Gulch area which are the dominant structures at the Eagle Gold occurrence.

At least four periods of faulting have occurred in the Dublin Gulch area, including two low angle thrust and bedding plane faults, NW trending faults, NE trending faults, E trending faults, and N-S trending faults. It is the latter that may have the most significant impact on mineralization, which appears to displace the Dublin Gulch stock.

The Dublin Gulch area is characterized by four intrusive episodes. The most significant is the occurrence of Cretaceous aged granodiorite which has intruded the Hyland Group metasediments near their contact with the underlying Upper Schist. Quartz diorite, quartz monzonite, leucogranite and aplite comprise the younger intrusive phases that occur predominantly as dikes and sills, and cut both the granodiorite and surrounding country rocks.

The meta-sedimentary rocks are the product of greenschist-grade regional metamorphism, and proximal to the Dublin Creek area have undergone metasomatism and contact metamorphism. A hornfels thermal aureole surrounds the deposit and within which the coarse clastic components of the Hyland Group have been altered to quartz-biotite, the argillaceous components to sericite-biotite-chlorite schist, and the carbonates to marble, wollastonite-quartz skarn and pyroxenite skarn. The aureole extends from 800 to 2,000 m outward from the intrusive.

3.0 GEOTECHNICAL INVESTIGATION

3.1. Overview

The field work for this project was conducted between June and August, 2011. The site investigation involved a number of different components which are listed below and are described in detail in the following sub-sections and corresponding appendices:

- Mapping of natural cut slopes, existing cut roads and drill pad cuts;
- Excavation of test pits;
- Geotechnical borehole drilling using solid stem auger, hollow stem auger, CRREL barrel (Cold Regions Research and Engineering), and triple tube diamond drill coring;
- Field Testing:
 - Penetration testing;
 - Downhole geophysics;
 - Plate load testing;
 - Thermistors and ground ice classification;
 - Point load testing;
- Groundwater investigation:
 - Standpipe piezometers;
 - Vibrating wire piezometers (VWP);
 - Slug Testing;
 - Seep observations;
- Laboratory Testing:
 - Soil index testing;
 - Rock strength testing;
 - Modified Proctor and permeability testing;
 - Aggregate testing;

The geographic coordinates and elevations of most of the boreholes were surveyed with differential GPS by Underhill in September 2011. Boreholes BH-BGC11-39, -40A, -41, -61, -65 and -67 were not surveyed by Underhill and thus their location and ground elevation were estimated using a handheld GPS unit, as was also done with all test pits and outcrop mapping locations. This test hole location information should be considered approximate.

Visual classification and description of overburden materials was done according to the Unified Soil Classification System (USCS, ASTM D24887), frozen soils were classified according to ASTM D4083, and intact rock and rock mass characterization was done following ISRM (1981) and other industry recognized classification systems such as Rock Mass Rating (RMR, Bienawski, 1976) and Geological Strength Index (GSI, Hoek and

Marinos, 2000; Cai et al., 2004). Appendix A provides a basic legend describing most of the parameters observed during soil and rock description.

3.2. Outcrop Mapping

A total of fifty-nine natural outcrops, road cuts and platform cuts were mapped in detail and are presented in Appendix B. The location of these cut slopes is shown on Drawing 05 and Table B-1 summarizes the rock mass characteristics of the cut slopes.

Geological mapping on existing road cuts, drill pad cuts and natural slope cuts was undertaken to describe the exposed rock mass and characterize its expected engineering behavior according to the type and orientation of the structures and intact rock strength. The presence of mapped outcrops paired with drilled boreholes provides the opportunity to compare the rock mass as seen in the borehole with that of the nearby outcrop and give insights of its strength when recovery was poor near the surface. The information gathered during the outcrop mapping included an overburden characterization (soil description and classification) and detailed lithological and structural description of the bedrock, together with a rock mass characterization.

3.3. Test Pitting

In 2011 a total of ninety-six test pits were excavated throughout the project area using a 325D Cat excavator. All test pit locations are illustrated on Drawing 05 and test pit logs are included in Appendix C and are summarized in Table C-1. Selected test pit photographs are included in Appendix D.

The test pit program was designed to develop an understanding of the engineering properties of the overburden materials across the site and to allow collection of disturbed soil samples for laboratory testing. Paired test pits and boreholes were conducted in areas where poor recovery from the diamond holes was expected. Test pits were also tailored to provide additional data and samples within the proposed silt borrow areas. In addition, plate load tests were conducted at the bottom of two test pits in the proposed plant site.

Observations of material compactness were inferred from excavator performance and from penetration resistance of the side walls of shallow pits. All frozen ground encountered was classified for ice content in the field. All test pits were terminated due to refusal on bedrock or frozen ground, sloughing/collapse of side walls, or due to the limits of excavator reach.

3.4. Geotechnical Borehole Drilling

3.4.1. General

The borehole drilling program was planned to characterize overburden and bedrock conditions at selected key facility locations. The drilling was focused on determining depth to, and competence of, bedrock, as well as characterizing the nature of overburden soils with respect to bearing capacity and potential settlements, or for design of cut slopes. Two

different drilling methods were used; auger drilling tailored to the soil unit characterization, and diamond drilling tailored to the rock characterization.

3.4.2. Diamond Drilling

In 2011 a total of twenty-nine diamond drill holes were drilled within the proposed Heap Leach pad, the Dublin Gulch valley bottom, the proposed plant site, the proposed Eagle Pup Waste Rock Storage Area, the proposed Platinum Gulch Waste Rock Storage Area, and the Crushers. All diamond drill hole locations are illustrated on Drawing 05 and diamond borehole logs are included in Appendix E and summarized in Table E-1. Selected diamond drill core photographs are included in Appendix F.

Diamond drilling was conducted by Lyncorp of Smithers, BC using a skid mounted diamond drill rig equipped with HQ3 drill rods. During the program two different rigs were used, the CS10 (June-July) and the CS1000 (August). The drill used a triple-tube core barrel, in which the inner tube was isolated from the drill fluid and had a split (longitudinal) sleeve. The split sleeve allowed the core to be recovered and viewed in a relatively undisturbed state. The diamond holes were typically drilled with a mix of water and polymer as drilling fluid.

Previous investigations met significant challenges getting good core recovery in the overburden materials and highly weathered bedrock. The 2011 site investigation was planned to make best efforts to improve core recoveries. Where poor recoveries were anticipated, the diamond drill holes were paired with test pits to provide some information about the shallow overburden characteristics. The same idea was applied whenever an outcrop was mapped in the vicinity of a borehole, as detailed in section 3.2. Diamond drill holes ranged in completed depth from 20.1 m to 50.5 m.

Installations in completed diamond drill holes included slotted PVC pipe standpipe piezometers (seventeen boreholes), vibrating wire piezometer (one borehole) and solid PVC casing for down-hole geophysics (ten boreholes). Standpipe piezometers typically included 3 m slotted pipe screened intervals near the bottom of the hole, using coarse sand/fine gravel quartz as the sand pack material, plugged with bentonite, grouted to the surface using a cement-bentonite grout and capped with bentonite. PVC pipe for downhole geophysics was grouted in place using a cement-bentonite grout, with the grout mixture proportioned to achieve a finished stiffness in the grout similar to that of the surrounding formation. The typical grout mix was 0.3:1:2.5 (bentonite:cement:water) by weight. All piezometers and PVC pipe for downhole geophysics were 50 mm diameter PVC.

3.4.3. Auger Drilling

In 2011 a total of seventeen auger holes were drilled, located in the Dublin Gulch valley bottom, proposed silt borrow area, proposed truck shop, proposed Eagle Pup Waste Rock Storage Area, and the proposed Plant Site. All auger test hole locations are illustrated on Drawing 05 and auger logs are included in Appendix E and summarized in Table E-1. Selected auger hole photographs are included in Appendix F.

Auger drilling was conducted by Midnight Sun Drilling of Whitehorse, Yukon, using a MARL 4CT track-mounted rig. Three techniques, solid stem augering, hollow stem augering and coring with the CRREL barrel were used depending on the purpose of the drilling and nature of the formation. Solid stem was used for the majority of the drilling, with grab samples obtained from the auger flights. The hollow stem was used as casing to conduct standard penetration testing (SPT) testing or to install standpipe piezometers. The CRREL barrel was used for core sampling of frozen ground, and also to obtain core of completely weathered rock at one hole. Auger holes were advanced to the target depth, limits of drilling capability (i.e. the length of auger flights available) or to refusal on boulders or bedrock.

Holes ranged in completed depth from 9.2 m to 28.2 m, with three short holes of 2.7 m, 4.3 m and 6.9 m which were drilled either in weathered bedrock or placer tailings, and due to the coarse nature of the formation they met refusal at shallow depths. SPT was completed at selected intervals in the auger holes to obtain some measure of in-situ density of the soils. The tests were conducted in all the areas where auger drilling was conducted, with the exception of the proposed Eagle Pup waste rock storage area where frozen ground was encountered.

Soil identification was based on observations of material retained on the auger flights, in split spoon samples from the SPT, and extruded from the CRREL barrel. SPT samples were typically taken at 1.5 m to 3 m intervals. Auger flights, SPT samples, and CRREL barrel core samples were photographed and logged at the rig and selected samples were collected for laboratory testing. Auger drilling was typically completed in 1.5 m runs and CRREL barrel drilling was typically completed in 0.6 m to 0.75 m runs.

Installations in completed auger holes included slotted PVC pipe standpipe piezometers (four boreholes), vibrating wire piezometer (one borehole), solid PVC casing for down-hole geophysics (one borehole), and solid PVC casing for installation of thermistor strings (seven boreholes). Installations in the auger holes for downhole seismic and VWP followed the same methodology as described previously for the diamond holes. Installations for standpipe piezometers and thermistor strings were completed with bentonite chips use to backfill between the casing and the formation to the surface.

3.5. Field Testing

3.5.1. Penetration Testing

Three types of penetration testing were completed as part of the drilling and test pitting program. Standard Penetration Testing (SPT) was performed in twelve of the sixteen auger holes completed, Dynamic Cone Penetration Testing (DCPT) was completed in one location adjacent to BH-BGC11-67/69 at the proposed plant site, and Wildcat Penetrometer testing was completed at 23 locations, most of them within the Dublin Gulch valley bottom. Some SPT and DCPT tests were completed using an instrumented rod and SPT Energy Analyzer (Pile Dynamics Inc., Cleveland, Ohio) to measure the energy transferred (ETR) from the hammer to the rods, allowing corrections of field data to a standard energy value (N_{60}).

Appendix G contains a detailed description of the different tests, Table G-1 summarizes all penetration testing completed, and Drawing 06 illustrates the location of the different tests.

3.5.1.1. SPT N-blow count

SPT testing was completed at selected intervals in the auger holes to obtain some measure of in-situ density of the formation materials. SPT testing was completed from the auger rig using standard SPT equipment: 34.9 mm (1 3/8") diameter split spoon, 63.5 kg (140 lb) automatic trip hammer, and 762 mm (30") drop height.

Previous observations of the placer tailings, colluvium, and weathered bedrock indicated the presence of a large fraction of cobbles and boulders reducing the applicability of SPT testing. During the 2011 field investigation, in an attempt to identify interference by coarse materials, SPT blowcounts were typically recorded for each inch (25 mm) during the SPT tests. Coarse particles sometimes interfered with the tests, causing the tests to reach refusal before the end of the 450 mm test interval. In this situation, a 'corrected' N and consequently 'corrected' N_{60} value was estimated for the sandier portion of the test. For detailed information on the methodology followed to calculate corrected N-values, refer to Appendix G and Tables G-2 and G-3.

SPT tests were carried out in all boreholes where soil conditions allowed, with a total of 77 tests (Drawing 06). From the entire suite of tests completed, thirty were considered invalid either because they were performed in frozen ground (seven), recoveries were less than 150 mm (eighteen), or were influenced by coarse clasts (five).

3.5.1.2. Dynamic Cone Penetrometer (DCPT)

Dynamic cone penetration testing (DCPT) involves a cone driven into the formation at the base of a borehole. The number of blows needed to achieve this penetration is taken as an indicator of the density or consistency of the formation material. DCPT testing was completed with the auger rig and the same automatic trip hammer as was used for SPT testing. The DCPT tests were also monitored using the SPT energy analyzer. Blows were counted in 150 mm increments.

A total of four DCPT tests were completed adjacent to BH-BGC11-67/69 in the proposed plant site (Drawing 06). Refusal was met at 2.5 m, 3.2 m, 0.5 m and 0.6 m in the four tests. Refer to Appendix G and Table G-4 for detailed information on the DCPT testing.

3.5.1.3. Wildcat Penetrometer

Wildcat penetrometer testing is conducted with a hand operated penetration testing tool. The device consists of a safety hammer, rods, a sacrificial cone tip and a lubricating system. The falling portion of the hammer weighs 15.9 kg (35 lbs) and the drop height is 0.377 m (15 inches). The rods are 1 m long and 2.8 cm (1.1 inches) in diameter. The rods are grooved every 10 cm to allow blow counts to be recorded in 10 cm increments. Testing is completed when refusal is met (considered to be 50 blows in one 10 cm increment).

A total of 25 Wildcat penetrometer tests were completed mostly within Dublin Gulch and a few in the crusher area, 100 day Storage area and along the road to Platinum Gulch (Drawing 06). Of the 25 tests, three were terminated at the maximum length of rods available. The remaining tests were terminated at refusal. For detailed information on testing results and methodology followed refer to Appendix G and Tables G-5 and G-6.

3.5.2. Geophysics

Frontier Geosciences Inc. of Vancouver, B.C. was engaged to complete seismic refraction and down-hole seismic testing between August 24 and September 8, 2011. The surface investigation was supplemented with downhole surveys, including eleven compression wave and shear wave soundings in completed boreholes. A detailed summary of the geophysical investigation program and results (seismic refraction, shear and compression wave velocities) is presented in Appendix H.

3.5.2.1. Seismic Refraction

The work included approximately 5.4 km of seismic refraction surveying along and across the Dublin Gulch valley bottom, and to the east of Anne Gulch, up in the northern side of Dublin Gulch valley within the proposed Heap Leach footprint (Drawing 05).

Seismic refraction involves the use of small explosive charges to create a shock wave at one end of a series of geophones, which are placed on the ground surface at even intervals. The time taken for the shock wave to arrive at each geophone can be interpreted to make inferences of compression wave velocity with depth between instruments. This information can be used to infer the contact between different geological units, providing additional spatial information between (or in the absence of) test holes. The main purpose for this work was to identify the transition from overlying overburden to undisturbed bedrock within the Dublin Gulch valley and obtain an indication of material stiffness over an extended area.

3.5.2.2. Downhole Seismic Profiling

This work included shear wave (S-wave) and compressional wave (P-wave) profiling in eleven boreholes, all of them selected within key proposed facilities as follows: plant site (one), Dublin Gulch valley bottom (four), Heap Leach (three), and Crushers (three). Drawing 07 illustrates the location of all the boreholes where downhole seismic profiling was conducted.

The surveying was carried out in drillholes lined with PVC casing. Data was obtained at 0.5 m intervals for approximately the full depth of each borehole (ranging between 21.3 m to 50.3 m) using surface energy sources (typically a sledgehammer blow to the end of a heavy timber in firm contact with the ground surface) and a down-hole triaxial geophone package held to the wall of the casing by a spring steel carrier.

The main purpose of this work was to obtain S-wave velocity profiles, which provide an indirect indication of the shear strength of the formation. The P-wave velocity data can be used for other related purposes, for example estimating excavatability of rock.

3.5.3. Plate Load Testing

Plate load tests were carried out by BGC personnel in August 2011 to assess the stiffness properties of different exposed subgrade materials. Plate load tests were conducted in two excavated test pits located in the proposed plant site (TP-BGC11-103, TP-BGC11-105) and at one excavation in the proposed secondary crusher area (Drawing 08). Four different plate sizes were used: 1.2 m, 0.76 m, 0.53 m, and 0.3 m, and at each test location at least two different plate sizes were tested. The purpose of this testing program was to:

- Measure the modulus of subgrade reaction of different rock types; and,
- Test various diameters of plate to estimate the variation of material stiffness with bearing pressure and investigate the influence of plate size for extrapolation of results to full size footings.

A detailed description of the test methodology is presented in Appendix I and a summary of test locations, characteristics and results is presented on Tables I-1 to I-4.

3.5.4. Thermistors

Seven thermistor strings were installed during the site investigation program. The thermistors were located in areas where visible ice was observed during drilling and frozen ground conditions were expected to be found at depth (Drawing 07). The targeted areas were:

- Proposed silt borrow, two 10 m long thermistor strings (one broken) and one 25 m-long thermistor string;
- proposed truck shop, two 10 m long thermistor strings; and,
- ice-rich colluvium at Eagle Pup, two 25 m long thermistor strings.

For installation details and temperature readings refer to Appendix J and Tables J-1 and J-2.

3.6. Groundwater Investigation

3.6.1. Groundwater Levels

Groundwater observations were made throughout the site and consisted of: measuring water levels in standpipe and vibrating wire piezometers installed in 2009 and 2011 by BGC and installed by others in previous years; estimating the hydraulic conductivity of different soil units and weathered bedrock by means of slug testing in selected 2011 BGC standpipes; and, recording details of surface seepage locations throughout the site, when observed. Details of the groundwater investigation can be found in Appendix K and summary tables K-1 to K-5. Ground and surface water observations are illustrated in Drawing 09.

3.6.1.1. Standpipe Piezometers

Standpipe piezometers were installed to enable observations of groundwater elevations and also to support estimates of the hydraulic conductivity of formation materials through slug tests. During the 2011 investigation, twenty-two slotted PVC standpipe piezometers were installed in both auger and diamond drill holes (Drawings 07 and 09). Water levels in select standpipes installed by others were measured on July 3rd and July 4th, 2011. Water levels in standpipes installed by BGC in 2011 along with select standpipes installed by others were read between August 25th and August 29th, 2011.

3.6.1.2. Vibrating Wire Piezometers (VWP)

A total of two VWPs were installed during the 2011 site investigation, one in an auger hole in the proposed Eagle Pup Waste Rock Storage Area and the other in an inclined diamond hole in the proposed plant site (Drawings 07 and 09). Water levels in VWPs installed by BGC in 2011 were read between August 25th and August 29th, 2011.

3.6.2. Slug Testing

Five slug tests were conducted in selected boreholes in the Dublin Gulch valley bottom (BH-BGC11-32, -33, -34, -39 and -52) (Drawings 07 and 09). The hydraulic conductivity of weathered bedrock and placer tailings was assessed by conducting rising head tests in these monitoring wells. The tests were completed after water levels had stabilized following drilling and piezometer installation. Three iterations of a 1L rising head slug test were performed for each well; a standard 1L bailer was used to remove each slug. Water level measurements were recorded with a down-hole data logger, with manual observations as a check of instrument records. Data logger measurements were corrected with a barometric pressure logger.

3.6.3. Seep Observations

Within the area of interest, twenty-eight locations of surface seepage were observed. The spatial coordinates of each seep were noted and the approximate seepage rate was estimated if possible. A complete list of the observations can be found in Table K-5 in Appendix K, and the locations are shown in Drawing 09.

3.7. Sampling and Laboratory Testing

Laboratory testing conducted on select samples collected in 2011 included soil index testing, compaction testing and permeability testing, rock strength testing, and aggregate testing. Two labs were engaged: Golder Associates from Burnaby, BC, which conducted some of the soil index testing and all of the rock strength testing, and GeoNorth, from Prince George, BC, which conducted soil index testing, compaction testing and permeability testing, and aggregate testing. The original lab testing reports are included in Appendix L, and for summary plots and tables of tabulated test results refer to Appendix M.

3.7.1. Soil Testing

Samples for soil testing were recovered as SPT samples, auger grab samples, CRREL barrel core samples, and grab samples from test pit walls. Table 3-1 below provides a summary of soil tests performed in the work.

Table 3-1. Summary of Soil Samples Tested

Laboratory Test	Testing Standard	Number of Tests
Grain size	ASTM C136	147
Moisture Content	ASTM 2216	193
Hydrometer	ASTM 422	81
Atterberg Limits	ASTM 4318	93
Specific Gravity - Soil	ASTM 854	4
Soluble Sulphate in Soil	CSA A23.2-3B	3
Modified Proctor	ASTM 698	4
Permeability Testing	ASTM D5856	4

3.7.2. Rock Strength Testing

3.7.2.1. UCS and Brazilian Tensile Strength

Nine rock core samples from the proposed Heap Leach (BH-BGC11-29, -34), crushers (BH-BGC11-40A, -40B, -62), Plant Site (BH-BGC11-69), and Platinum Gulch Waste Rock Storage Area (BH-BGC11-43, -45, -46) were selected for laboratory testing to perform Unconfined Compressive Strength (UCS) testing with strain measurements, Brazilian tensile strength, and specific gravity. Table 3-2 provides a summary of tests performed on rock samples.

Table 3-2. Summary of Rock Samples Tested

Laboratory Test	Testing Standard	Number of Tests
UCS with Strain Measurement	ASTM D7012	8
Brazilian Tensile Strength	ASTM D3967	17
Specific Gravity - Rock	C127	9

3.7.2.2. Point Load Testing

During the 2011 Site Investigation Program a total of 320 core samples of rock were tested using a point load tester to estimate the intact rock strength of the metasedimentary and intrusive rock. These samples were taken from rock core obtained from twenty-seven boreholes at the site. From the 320 core samples, 117 were invalid, as outlined by ISRM (1985), due to incomplete development of the fracture surface. Therefore, 203 valid tests were performed. For more details on the point load testing procedure and a complete record of the samples tested refer to Appendix N and tables N-1 to N-28.

3.7.3. Borrow Source Testing

Several bulk samples were obtained from different locations identified as potential borrow areas suitable for construction material. The lab testing was tailored to aid in the characterization of the materials according to the requirements of each potential borrow source.

3.7.3.1. Silt Borrow

Fine grained soil materials may be required to construct liners for the heap leach facility or ponds. Previous investigations (see BGC 2011b) identified potential sources of silt borrow, and this component of the work was executed to determine if sufficient quantities of low permeability material are available on site. Five test pits (TP-BGC11-82, -119, -120, -121, and -122) and four boreholes (BH-BGC11-44, -47, -49, and -51) were sampled, with selected materials identified for testing for grain size, moisture content, Atterberg limits, Modified Proctor moisture-density relationship, and permeability testing (at 95 % Modified Proctor). Table 3-3 below summarizes the testing completed for potential silt borrow materials.

Table 3-3. Summary of tests conducted on potential silt borrow materials

Hole ID	Northing (m)	Easting (m)	Number of Soil Samples Tested					
			Grain Size	Moisture Content	Hydrometer	Atterberg Limit	Modified Proctor	Permeability Testing
			ASTM C136	ASTM 2216	ASTM 422	ASTM 4318	ASTM 698	ASTM D5856
Test Pits								
TP-BGC11-82	7100691	458604	5	5	2	5	--	--
TP-BGC11-119	7100548	458587	--	1	--	1	1	1
TP-BGC11-120	7100484	458511	--	1	--	1	1	1
TP-BGC11-121	7100588	458502	--	1	--	1	1	1
TP-BGC11-122	7100707	458684	--	1	--	1	1	1
Boreholes								
BH-BGC11-44	7100547	458690	5	10	5	8	--	--
BH-BGC11-47	7100699	458812	5	6	4	3	--	--
BH-BGC11-49	7100636	458747	5	6	5	5	--	--
BH-BGC11-51	7100744	458644	3	11	2	5	--	--

Notes:

1. Location for BGC test pits from handheld GPS measurements, and location of BGC boreholes from Underhill survey dated September, 2011.
2. Coordinates in UTM NAD 83, Zone 8N.

3.7.3.2. Aggregate Testing

Samples of silicified metasediments (5 x 20L pails) and granodiorite (5 x 20L pails) within the footprint of the open pit, grab samples of cobbles from the placer tailings (10 x 20L pails), and 1 x 20L pail with rock samples from the Steiner outcrop were obtained for the purpose of testing the material for potential use as concrete aggregate or other high quality aggregate. The samples from the open pit were combined and crushed to form one composite sample that was later tested; a similar procedure was followed with the samples from the placer tailings. Table 3-4 provides a summary of the aggregate testing.

Table 3-4. Summary of Aggregate testing

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Comments	Sample Testing
CONC-BGC11-01	7101270	459788	888	Single composite sample prepared by combining placer tailings oversize materials obtained from indicated locations	<ul style="list-style-type: none"> • Clay lumps (CSA A23.2-3A) • Low Density Granular Materials (CSA A23.2-4A) • Material Finer than 80 µm, fine aggregate (CSA A23.2-2A&5A) • MgSO₄ soundness of coarse aggregate (CSA A23.2-9A) • MgSO₄ soundness of fine aggregate (CSA A23.2-9A) • Flat and elongated particles (CSA A23.2-13A) • Micro-Deval, fine aggregate (CSA A23.2-23A) • Micro-Deval, coarse aggregate (CSA A23.2-29A) • Unconfined Freeze thaw (CSA A23.2-24A) • LA Abrasion, small size coarse aggregate (CSA A23.2-16A) • Relative Density SDD & Absorption, fine aggregate (CSA A23.2-6A) • Sulphate in soil (CSA A23.2-3B) • Detection of Alkali-Silica Reactive Aggregate by Accelerated Expansion of Mortar Bars (CSA A23.2-25A) • Petrographic No., coarse aggregate (CSA A23.2-15A)
CONC-BGC11-02	7101200	459716	886		
CONC-BGC11-03	7101160	459626	881		
CONC-BGC11-04	7101220	459484	859		
CONC-BGC11-16	7100940	458852	826		
CONC-BGC11-GRD	7099352	460190	1285	Single composite sample prepared by combining samples of granodiorite and quartzite clasts obtained from two indicated locations	<ul style="list-style-type: none"> • Clay lumps (CSA A23.2-3A) • Low Density Granular Materials (CSA A23.2-4A) • Material Finer than 80 µm (%), fine aggregate (CSA A23.2-2A&5A) • MgSO₄ soundness of coarse aggregate (CSA A23.2-9A) • MgSO₄ soundness of fine aggregate (CSA A23.2-9A) • Flat and elongated particles (CSA A23.2-13A) • Micro-Deval, fine aggregate (CSA A23.2-23A) • Micro-Deval, coarse aggregate (CSA A23.2-29A) • Unconfined Freeze thaw (CSA A23.2-24A) • Small coarse aggregate LA abrasion (CSA A23.2-16A) • Relative Density SDD & Absorption, fine aggregate (CSA A23.2-6A) • Sulphate in soil (CSA A23.2-3B) • Detection of Alkali-Silica Reactive Aggregate by Accelerated Expansion of Mortar Bars (CSA A23.2-25A) • Petrographic No., coarse aggregate (CSA A23.2-15A)
CONC-BGC11-SSED	7099338	460188	1284		
BGC-Steiner	7100278	459034	907	Single sample using material from the Steiner outcrop only	<ul style="list-style-type: none"> • Micro-Deval, coarse aggregate (CSA A23.2-29A) • Relative Density SDD & Absorption, fine aggregate (CSA A23.2-6A)

Notes:

1. Location for BGC samples from hand held GPS measurements.
2. Coordinates in UTM NAD 83, Zone 8N.

3.8. Placer Tailings Inventory

A visual reconnaissance of the reworked valley bottom placer tailings of Dublin Gulch and Haggart Creek was completed to develop opinions as to their potential for use as construction materials in development of the proposed mine. The placer tailings materials excavated in the course of foundation preparation may comprise a useful construction material, as a potential source for general fill, structural fill, concrete aggregate or heap overliner material, if suitably selected and processed through screening, crushing, and/or washing. Appendix O contains a description of the placer tailings encountered in Dublin Gulch and Haggart Creek, and Drawing 10 illustrates the locations of material observations per area. Visual observations of surficial materials in the Dublin Gulch valley bottom are summarized in Table O-1, and visual observations of surficial materials in the Haggart Creek valley bottom are summarized in Table O-2.

4.0 OBSERVED SUBSURFACE CONDITIONS

4.1. Summarized Data

Summary tables with key data results are tabulated below.

4.1.1. Penetration Data

Detailed observations of penetration resistance are presented in Appendix G, and raw blowcount data are displayed on the borehole logs in Appendix D. These results are summarized for the various geological units in Table 4-1, below.

Table 4-1. Summary of Penetration Data

Borehole	# of Valid Tests	Average N ₆₀	Max N ₆₀	Min N ₆₀	Standard Deviation N ₆₀
Placer Tailings					
BH-BGC11-39	11	39	67	1	20
BH-BGC11-65	5	27	36	17	7
Alluvium					
BH-BGC11-44	5	50	99	9	33
BH-BGC11-47	2	10	15	5	-
BH-BGC11-49	1	10	-	-	-
BH-BGC11-51	5	18	26	8	6
Till					
BH-BGC11-53	4	44	55	23	13
Colluvium					
BH-BGC11-55	6	20	35	14	7
BH-BGC11-56	2	44	62	26	-
BH-BGC11-57	3	83	100	71	-
Weathered Metasedimentary Rock					
BH-BGC11-55	1	86	-	-	-
BH-BGC11-57	1	98	-	-	-

Notes:

BH-BGC11-58 and BH-BGC11-60 had no valid SPT tests.

BH-BGC11-57 has one SPT test where the calculated N₆₀ value is 128, this is reported as 100 as N₆₀ values are truncated at 100.

4.1.2. Plate Load Testing

Detailed results of the plate load testing are presented in Appendix I. Key results from the testing are summarized in Table 4-2, below.

Table 4-2. Summary of Plate Load Test Results

Location	Test #	Depth Below Grade (m)	Plate Diameter (m)	Vertical Subgrade Reaction Modulus	
				(kPa/mm)	
				Virgin Curve, k_v	Rebound Curve, $k_{v(u-r)}$
Plant Site (TP-BGC11-103)	PT-BGC11-01	5.5	0.76	130	220
Plant Site (TP-BGC11-103)	PT-BGC11-02	5	0.76	140	310
Plant Site (TP-BGC11-103)	PT-BGC11-03	5	0.53	130	290
Plant Site (TP-BGC11-103)	PT-BGC11-04	5	0.3	540	1170
Plant Site (TP-BGC11-103)	PT-BGC11-05	5.5	0.53	130	250
Plant Site (TP-BGC11-105)	PT-BGC11-06	8.5	1.2	30	60
Plant Site (TP-BGC11-105)	PT-BGC11-07	8.5	0.76	40	80
Plant Site (TP-BGC11-105)	PT-BGC11-08	8.5	0.76	50	120
Plant Site (TP-BGC11-105)	PT-BGC11-09	8.5	0.53	40	120
Secondary Crusher	PT-BGC11-10	3.2	0.53	190	600
Secondary Crusher	PT-BGC11-11	3.2	0.3	640	1600
Secondary Crusher	PT-BGC11-12	3.2	0.53	200	500
Secondary Crusher	PT-BGC11-13	3.8	0.3	220	1000
Secondary Crusher	PT-BGC11-14	4.2	0.53	400	1040

4.1.3. Temperatures

Detailed observations of ground temperature measurements are presented in Appendix J. Key observations are summarized in Table 4-3, below.

Table 4-3. Summary of 2011 Thermistor Readings

Borehole	Location	Readings Dates	Depth of Frozen Ground (m)	Temperature of Frozen Ground (°C)
BH-BGC09-STU-3	Lower Stuttle Gulch	11 July 25-29 August	4 m to >10 m	0 to -0.3
BH-BGC09-STU-4	Lower Stuttle Gulch	11 July 25-29 August	4 m to >10 m	0 to -0.2
BH-BGC09-AG-3	Lower Ann Gulch/ Valley Bottom	7 July	10m	0.6
BH-BGC10-07 ¹	Between Stuttle Gulch and Eagle Pup	11 July 29-30 August	10 m	0.0
BH-BGC11-42	Proposed Eagle Pup Waste Rock Storage Area	29-30 August	5 m to >25 m	-0.1 to -0.4
BH-BGC11-63	Proposed Eagle Pup Waste Rock Storage Area	29-30 August	1.2 m to >25	0 to -0.3
BH-BGC11-44	Proposed Silt Borrow Area	11 July 29-30 August	7 m to >10 m	0 to -0.1
BH-BGC11-47	Proposed Silt Borrow Area	11 July 29 August	7 m to >10 m	0 to -0.1
BH-BGC11-49	Proposed Silt Borrow Area	11 July	4 m to >10 m	0
BH-BGC11-51	Proposed Silt Borrow Area	26-29-30 August	15 m to >25 m	-0.1
BH-BGC11-57	Proposed Truck Shop	29-30 August	1.5 m to >10 m	-0.1 to -0.2
BH-BGC11-58	Proposed Truck Shop	30 August	1 m to >10 m	-0.1 to -0.7

¹: Thermistor consisted of a single thermocouple at 10m depth

4.1.4. Groundwater Observations

Groundwater observations have been divided into groundwater level readings and permeability testing results.

4.1.4.1. Water Level

Detailed observations of groundwater have been presented in Appendix K. Groundwater elevations observed at selected monitoring well locations in summer 2011 are summarized in Table 4-4, below.

Table 4-4. Summary of Water Level Readings for data collected in 2011 site investigation

Hole ID	Elevation (masl)	Date Collected	Depth (mbgs)	Piezometer Type
BH-BGC11-26	1139.6	27-Aug-11	16.4	Standpipe
BH-BGC11-29	1045.3	27-Aug-11	7.8	Standpipe
BH-BGC11-30	952.0	27-Aug-11	16.3	Standpipe
BH-BGC11-32	818.7	24-Aug-11	10.8	Standpipe
BH-BGC11-33	833.0	26-Aug-11	4.2	Standpipe
BH-BGC11-34	848.3	24-Aug-11	8.3	Standpipe
BH-BGC11-35	986.3	27-Aug-11	24.6	Standpipe
BH-BGC11-36	1002.5	27-Aug-11	19.9	Standpipe
BH-BGC11-38	1013.0	25-Aug-11	26.3	Standpipe
BH-BGC11-39	804	24-Aug-11	9.8	Standpipe
BH-BGC11-40B	1049.8	25-Aug-11	8.5	Standpipe
BH-BGC11-45	1354.4	27-Aug-11	8.4	Standpipe
BH-BGC11-46	1246.1	27-Aug-11	15.3	Standpipe
BH-BGC11-48	1140.0	27-Aug-11	31.3	Standpipe
BH-BGC11-52	909.0	25-Aug-11	4.1	Standpipe
BH-BGC11-54	883.8	29-Aug-11	11.8	Vibrating Wire
BH-BGC11-55	881.0	27-Aug-11	dry	Standpipe
BH-BGC11-57	859.2	27-Aug-11	dry	Standpipe
BH-BGC11-59	883.7	27-Aug-11	29.4	Standpipe
BH-BGC11-60	859.2	27-Aug-11	dry	Standpipe
BH-BGC11-62	1017.9	27-Aug-11	20.2	Standpipe
BH-BGC11-63	1100.7	29-Aug-11	18.1	Vibrating Wire
GT96-26	n/a	3-Jul-11	30.1	Standpipe
GT96-35	n/a	27-Aug-11	3.4	Standpipe
MW09-AG1	1017	25-Aug-11	15.7	Standpipe
MW09-AG2	1009	27-Aug-11	12.7	Standpipe
MW09-DG1	840	4-Jul-11	1.8	Standpipe
MW09-DG1	840	27-Aug-11	2.3	Standpipe
MW09-DG2	824	4-Jul-11	0.9	Standpipe
MW09-DG4	787	4-Jul-11	5.6	Standpipe
MW09-DG5	810	27-Aug-11	10.7	Standpipe
MW09-OG2	1332	3-Jul-11	2.2	Standpipe

Hole ID	Elevation (masl)	Date Collected	Depth (mbgs)	Piezometer Type
MW09-OG3	1065	3-Jul-11	0.4	Standpipe
MW09-STU1	967	4-Jul-11	14.3	Standpipe
MW09-STU1	967	27-Aug-11	14.4	Standpipe
MW09-STU2	857	4-Jul-11	2.1	Standpipe
MW10-DG06	859	3-Jul-11	2.2	Standpipe
MW10-OBS1	796	4-Jul-11	7.2	Standpipe
MW10-OBS2	793	4-Jul-11	5.3	Standpipe
MW96-01	1398	3-Jul-11	23.1	Standpipe
MW96-02	1394	3-Jul-11	8.1	Standpipe
MW96-03a	1384	3-Jul-11	1.9	Standpipe
MW96-04	1373	3-Jul-11	4.6	Standpipe
MW96-05	1363	3-Jul-11	2.7	Standpipe
MW96-06a	1405	3-Jul-11	2.6	Standpipe
MW96-07b	n/a	3-Jul-11	0.0	Standpipe
MW96-08	1339	3-Jul-11	4.9	Standpipe
MW96-09a	1361	3-Jul-11	4.5	Standpipe
MW96-09b	1361	3-Jul-11	6.7	Standpipe
MW96-10a	1367	3-Jul-11	2.2	Standpipe
MW96-10b	1368	3-Jul-11	2.0	Standpipe
MW96-13a	984	3-Jul-11	5.3	Standpipe
MW96-13b	984	3-Jul-11	14.9	Standpipe
MW96-14a	976	3-Jul-11	dry	Standpipe
MW96-14a	976	27-Aug-11	dry	Standpipe
MW96-14b	976	27-Aug-11	3.6	Standpipe
MW96-14b	976	3-Jul-11	3.6	Standpipe
MW96-15a	943	4-Jul-11	0.8	Standpipe
MW96-15a	943	27-Aug-11	1.5	Standpipe
MW96-26	1316	3-Jul-11	6.5	Standpipe
MW96-27	1308	3-Jul-11	9.3	Standpipe

4.1.4.2. Permeability

A detailed discussion of the in-situ permeability testing is provided in Appendix K. Estimated hydraulic conductivities are summarized in Table 4-5, below.

Table 4-5. Summary of Hydraulic Conductivity Results by Tested Formation

Hole ID	Elevation (masl)	Formation/Unit	Top and Bottom of sand pack (mbgs)	Average conductivity, k (m/sec)
BH-BGC11-32	819	Placer Tailings and Weathered Bedrock ¹	19.5 – 23.6	2.5×10^{-6}
BH-BGC11-33	833	Weathered Bedrock	36.3 – 41.4	1.1×10^{-7}
BH-BGC11-34	848	Weathered Bedrock	30.1 – 34.4	2.6×10^{-6}
BH-BGC11-39	804	Placer Tailings	14.6 – 18.6	8.1×10^{-5}
BH-BGC11-52	909	Weathered Bedrock	18.3 – 22.6	9.7×10^{-6}

Note:

Screened section of pipe was installed across two units, therefore the conductivity (k) is not representative of a specific lithological unit.

4.1.5. Point Load Test

Detailed observations from point load tests are provided in Appendix N. A summary of completed and valid tests is provided in Table 4-6, below.

Table 4-6. Summary of Point Load Test Results by Rock Type

Hole ID	Metasedimentary Rock				Intrusive Rock			
	Number of Tests		Is ₅₀ Values (MPa)		Number of Tests		Is ₅₀ Values (MPa)	
	V	IN	Range	Mean	V	IN	Range	Mean
BH-BGC11-24	9	4	0.4 - 2.9	1	0	0	N/A	N/A
BH-BGC11-25	6	5	0.5 - 2.9	1.8	0	0	N/A	N/A
BH-BGC11-26	5	3	1.1 - 4.3	1.9	0	0	N/A	N/A
BH-BGC11-27	4	3	0.8 - 3.0	2.1	0	0	N/A	N/A
BH-BGC11-28	10	5	0.3 - 3.1	1.5	0	0	N/A	N/A
BH-BGC11-29	13	4	0.3 - 11.9	2.2	0	0	N/A	N/A
BH-BGC11-30	5	3	0.4 - 1.3	0.8	0	0	N/A	N/A
BH-BGC11-31	2	0	0.3	0.3	0	0	N/A	N/A
BH-BGC11-33	2	3	0.3 - 0.5	0.4	0	0	N/A	N/A
BH-BGC11-34	5	5	0.4 - 2.1	1	0	0	N/A	N/A
BH-BGC11-35	5	4	0.4 - 7	3	0	0	N/A	N/A
BH-BGC11-36	12	6	0.3 - 8.6	1.9	0	0	N/A	N/A
BH-BGC11-37	9	5	0.3 - 5.7	2.3	0	0	N/A	N/A
BH-BGC11-38	8	5	0.2 - 1.9	1.9	0	0	N/A	N/A
BH-BGC11-40A	10	6	0.5 - 9.6	2.5	0	0	N/A	N/A
BH-BGC11-40B	12	6	0.4 - 6.5	2.2	0	0	N/A	N/A
BH-BGC11-43	2	2	5.6 - 8.7	7.2	7	1	7.5 - 10.8	9.4
BH-BGC11-45	12	7	0.2 - 18.5	3.7	0	0	N/A	N/A
BH-BGC11-46	2	0	3.0 - 6.4	4.7	7	2	0.7 - 10.3	4.9
BH-BGC11-48	2	1	0.2 - 0.3	0.2	2	0	0.4 - 0.9	0.7
BH-BGC11-50	7	7	1.3 - 6.1	3.9	1	0	7.5	7.5
BH-BGC11-52	5	2	0.3 - 4.7	2	0	0	N/A	N/A
BH-BGC11-54	9	3	0.3 - 0.9	0.4	0	0	N/A	N/A
BH-BGC11-59	3	5	0.3 - 1.6	0.7	0	0	N/A	N/A
BH-BGC11-62	11	7	0.3 - 1.6	0.9	0	0	N/A	N/A
BH-BGC11-64	0	0	N/A	N/A	1	2	1.3	1.3
BH-BGC11-66	0	0	N/A	N/A	9	6	0.6 - 8.1	5.3
BH-BGC11-69	6	5	0.3 - 2.2	1.1	0	0	N/A	N/A
Total	176	106	0.2 - 18.5	2	27	11	0.4 - 10.8	4.8

Notes:

- (1) Range and mean values only consider the valid tests.
- (2) V: valid test; IN: invalid test; N/A: not applicable.

4.1.6. Concrete Aggregate Testing results

Results of aggregate testing are summarized in Table 4-7. Both the pit aggregate and the placer tailings aggregate samples failed two test parameters typically used for evaluation of materials proposed for concrete aggregate. The pit aggregate sample failed Flat and Elongated Particles (CSA A23.2-13A) and Material Finer than 80 μm for Fine Aggregate (CSA A23.2-5A), and the placer tailings aggregate sample failed Micro-Deval for Coarse Aggregate (CSA A23.2-29A) and Material finer than 80 μm for Fine Aggregate (CSA A23.2-5A).

The petrographic examination for both pit and placer tailings composite samples revealed that the aggregate material contained rock types with mineralogy that may have a potential for alkali silica reactivity (ASR). Alkali-silica reactivity tests through mortar bar expansivity testing (CSA A23.2-25A) showed that both samples may be classified as “non-reactive.” Both samples were observed to have a petrographic number within the recommended limits for use as normal concrete aggregate.

Aggregate testing performed in the BGC Steiner sample included only Relative Density and Absorption for fine aggregate (CSA A23.2-6A) and Micro Deval for coarse aggregate (CSA A23.2-29A) due to the small size of the sample. The Steiner aggregate sample passed the abovementioned tests.

Additional study will be necessary to confirm suitability of these aggregate sources for use as concrete aggregate.

Table 4-7. Summary of Aggregate Testing Results

Test Type	Test Method	Pit Aggregate	Placer Tailings Aggregate	BGC Steiner	Maximum Allowable Value
Clay Lumps (%)	CSA A23.2-3A	0.0	0.0	N/A	1.0
Low Density Granular Materials (%)	CSA A23.2-4A	0.0	0.0	N/A	0.5
Material Finer than 80 µm (%), fine aggregate	CSA A23.2-5A	4.8	3.6	N/A	3.0
MgSO ₄ Soundness Loss, fine aggregate (%)	CSA A23.2-9A	3.16	3.86	N/A	16
MgSO ₄ Soundness Loss, coarse aggregate (%)	CSA A23.2-9A	0.97	0.27	N/A	12
Flat and Elongated Particles (%)	CSA A23.2-13A	35	19	N/A	20
Micro-Deval, Fine Aggregate (%)	CSA A23.2-23A	13.3	11.6	N/A	20
Micro-Deval, Coarse Aggregate (%)	CSA A23.2-29A	12.9	26.9	11.0	17
Unconfined Freeze-Thaw Test	CSA A23.2-24A	0.9	1.2	N/A	6
LA Abrasion (%), small size coarse aggregate	CSA A23.2-16A	13.7	29.9	N/A	50
Relative Density, fine aggregate	CSA A23.2-6A	2.657	2.704	2.718	-
Absorption (%), fine aggregate	CSA A23.2-6A	0.999	1.399	1.122	-
Sulphate in Soil (%)	CSA A23.2-3B	0.0008	0.0004	N/A	0.1
Detection of Alkali-Silica Reactive Aggregate by Accelerated Expansion of Mortar Bars	CSA A23.2-25A	0.14	0.11	N/A	0.15
Petrographic No., Coarse Aggregate	CSA A23.2-15A	121	117	N/A	140

Notes:

1. Cells are highlighted when results doesn't meet the specification.
2. N/A: not applicable, test was not performed

4.1.7. Material Testing Commissioned by Tetra Tech for the Heap Leach Facility

Some material samples were obtained for laboratory tests relative to the heap leach facility design; testing of these was commissioned by Tetra Tech Inc. Samples tested included durable rock, silt, colluvium, placer tailings, and weathered rock. Golder Associates performed a variety of tests, which are summarized in Table 4-8. Test results are included in Appendix L.

Table 4-8. Summary of Testing Commissioned by Tetra Tech for the Heap Leach Facility

SAMPLE DESCRIPTION	LAB CRUSH	SIEVE ANALYSIS	MIN./MAX. DENSITY	RELATIVE DENSITY & ABSORPTION					UNCONFINED FREEZE & THAW (2)	LAB ABRASION/MICRO-DEVAL	SLAKE DURABILITY	POINT LOAD	PROCTOR COMPACT	SPECIFIC GRAVITY	HYDROMETER	ATTERBERG LIMITS
				50X37.5MM	37.5X25MM	25X12.5MM	12.5X4.75MM	MINS 4.75MM								
SECTION 1 – DURABLE ROCK (1)																
ODF, as received		X														
ODF, minus 50mm crushed	X	X	X	X	X	X	X	X			O					
ODF, minus 37.5mm crushed	X	X	X					X		X						
ODF, minus 25mm crushed	X	X	X			X	X	O								
SECTION 2 – SILT																
												X	X	X	X	
SECTION 3 – COLLUVIUM																
		X							O	X		X	X	X	X	
SECTION 4 – PLACER TAILS																
		X								X	O	X	X	X	X	
SECTION 5 - WEATHERED ROCK																
									O	O	X	X				
	X = completed															
	O = still to do															
Notes:	1. Completed using ASTM standard test method 2. Freeze-thaw test uses CSA 24A.															

4.2. Material Descriptions

4.2.1. General

Subsurface conditions within the project area can be generalized as comprising a thin cover of organic soil and vegetation underlain by colluvium and variably thick, variably altered/weathered bedrock. Often, alluvial sediments, glacial till or placer tailings (fill) were encountered in or near the valley bottom close to Dublin Gulch. Ground conditions around the project site were found to be highly variable and influenced by the degree of bedrock weathering, slope angles, ground elevation, and presence/absence of frozen ground.

Frozen ground was observed at many test hole locations, and its presence was observed to vary considerably with respect to the depth to frozen ground, thickness, and ground ice content. Excess ice was observed in some locations. The spatial distribution of observed frozen ground is illustrated in Drawing 11.

Groundwater observations in the valley bottom near Dublin Gulch varied between 0.8 m and 10.8 m, however, at higher elevations groundwater was encountered between 0.4 m and 31.3 m, with the shallower readings typically observed near secondary drainages. Two dry wells were drilled in the truck shop area (BH-BGC11-57, -60), and one dry well was drilled in the south abutment of the proposed Heap Leach embankment (BH-BGC11-55).

4.2.2. Overburden

Several types of overburden materials were encountered within the project site, as discussed below.

4.2.2.1. Organics

A thin organic cover is widespread across the project site overlying the other overburden units and primarily consists of vegetative rootmat, moss, silt and sand, and other organic matter in varying proportions. The typical observed thickness was in the order of 0.2 to 0.3 m, and was not usually encountered in previously disturbed areas containing fill materials, such as the placer tailings in the valley bottom. Buried layers or lenses of peat and/or organic silt were observed in the site within other overburden types, such as colluvium or alluvium. The most common areas where buried organics were noted are the proposed camp site (depths between 1.0 m to 6.5 m in TP-BGC11-123, -124, -128, and -129), silt borrow area (depths between 1.0 m to 7.5 m in BH-BGC11-44, -47; TP-BGC11-82, -119, and -143), proposed truck shop (depths between 0.3 m and 0.5 m in TP-BGC11-135, -136), and proposed diversion channel embankment in Dublin Gulch (depths between 0.1 m and 3.5 m in TP-BGC11-135, -136).

4.2.2.2. Colluvium

Colluvial materials were generally encountered on sloping ground throughout the site at or near the ground surface, below the organic cover. Colluvium was not typically observed near the valley bottom or in the previously disturbed areas in Dublin Gulch.

The relative density of the colluvium was variable and generally ranged from loose to compact. The gradation and thickness of the colluvium was observed to be highly variable, predominantly ranging from boulders and cobbles with some silt and sand to silty sand with gravel and some cobbles. The colluvium is typically derived from transported weathered metasedimentary and/or intrusive bedrock. Gravel, cobbles, and boulders were generally observed to be angular to subangular. Finer grained colluvium characterized by silt with trace sand and gravel is generally only encountered on the lower flanks of the north and west-facing slopes such as at the vicinity of the proposed truck shop, proposed Platinum Gulch pond, south abutment of the proposed heap leach embankment and conveyors.

According to the 2011 test hole information, a typical average colluvium thickness for the site is 3.0 m; however in the vicinity of the proposed crushers it could be as thick as 7.0 m or more. It must be noted that this average thickness is likely understated since some test holes didn't penetrate through the full thickness of the colluvium layer. The colluvium may also include variable amounts of organics as noted in section 4.2.2.1, which are often observed in distinct layers or lenses within the colluvium.

A distinct colluvial unit was observed within a lobate landform in the Eagle Pup drainage area, at BH-BGC11-42 and BH-BGC11-63. This material contains completely weathered rock fragments mixed with excess ice, including frequent inclusions of massive ice. The lobate landform containing this ice-rich colluvium is approximately 1 ha in plan area, and is tentatively interpreted to be a relict rock glacier, or similar peri-glacial creep feature.

4.2.2.3. Fill

The surficial materials in the valley bottom near Dublin Gulch and Haggart Creek have been reworked by several decades of placer mining activities. Old sedimentation ponds and stockpiles of variable washed sand and gravels, with varying proportions of cobbles and boulders, are present throughout this area.

The placer tailings consist of highly variable materials, with a wide range of grain size and density. The materials range from isolated deposits of cobbles and boulders, to gravelly sand or sandy gravel, to loose silty sand or soft sandy silt. The latter materials appear to be confined to specific areas, which are inferred from aerial photographs to have been sediment control ponds. The observed thickness of placer tailings at eight test holes had a mean value of about 11 m, with a range between 1.7 m and 19.8 m. Drawing 10 shows a rough classification and distribution of materials surveyed from surface observations. Subsurface observations of these materials were conducted by auger drilling and test pitting, and grab samples were obtained from the ground surface.

4.2.2.4. Alluvium

Alluvial deposits were encountered within the proposed laydown area and potential silt borrow area. These materials comprise frozen and occasionally ice-rich stratified low plastic silts to silty sands with trace gravel and some interlayers of silty gravels. None of the test holes that encountered alluvial deposits in the laydown area fully penetrated the deposit. The deepest test hole in the area is auger hole BH-BGC11-51 which drilled through 24.9 m of frozen alluvium and reached the targeted depth without exploring the full thickness of the unit.

There is some uncertainty in the geological interpretation of the materials encountered within the proposed laydown area. It is possible that glacial till may have been encountered at depth below alluvium; however the contact between the two units is not clearly defined, due to sample disturbance associated with the auger drilling method.

4.2.2.5. Debris Flow Deposits

Materials tentatively classified as debris flow deposits were observed around the area where Stewart Gulch meets Dublin Gulch (BH-BGC11-31, -52 and TP-BGC11-131, -134, -136) and within the area of the south abutment of the proposed Heap Leach embankment (BH-BGC11-55 and OC-BGC11-49). The deposits were generally observed below shallow colluvium or fill, and typically consisted of subrounded to subangular cobbles and boulders embedded in a silty sand matrix.

4.2.2.6. Glacial Till

Glacial till had been observed in previous investigations on the lower flanks of north and south facing slopes above Dublin Gulch, and can be described as a compact well graded sand and gravel with some silt and trace to some cobbles. Interpretations of glacial till materials were generally made in the field on the basis of the absence of angularity of gravel and cobble size clasts, and presence of mixed lithology, subrounded to rounded clasts (i.e. both granodiorite and metasediments). Subrounded to rounded materials are indicative of glacially transported soils in contrast to more locally transported colluvium, which tends to be more angular, as described previously. Glacial till was observed in TP-BGC11-103 and -104. In TP-BGC11-103 the glacial till was observed beneath approximately 3.5 m of colluvium and above weathered bedrock, while in TP-BGC11-104 it was observed beneath approximately 0.7 m, also overlying completely weathered bedrock.

4.2.2.7. Highly to Completely Weathered Bedrock

Colluvium was typically observed to be underlain by a horizon of variably-weathered rock. The weathering profile varies substantially across the site, depending on parent rock type and other local factors.

The metasedimentary rock (e.g. quartzite, schist) nearest the ground surface was often observed to be completely weathered to silt with some to trace gravel or sand and gravel

with cobbles and trace to some silt and clay. The gravel and cobble clasts tended to be friable, platy and exhibit a 'soapy' film due to the weathering/alteration. The transition from highly or completely weathered rock to a more competent, unweathered rockmass is highly variable; unweathered rock was generally not observed in test pits, and usually not observed at shallow depths in drill holes.

The near-surface granodiorite intrusive rock was often observed to be either completely weathered to a silty sand, or sandy silt, or highly weathered to a poorly graded sand. The thickness of the weathered horizon was highly variable.

Weathered rock is considered to be part of the overburden where it is completely weathered (i.e. W5) or residual soil (i.e. W6). Less weathered rock, including highly weathered (i.e. W4) rock, is considered for the purposes of this report to be part of the bedrock.

4.2.3. Bedrock

Two major rock types were encountered at the site below the overburden soils: metasedimentary and intrusive. The metasedimentary bedrock type was the most predominant bedrock type encountered during the 2011 field program, since the intrusive stock extends across the uplands, which were not a primary focus of investigation. However, some intrusive rock was encountered in boreholes and at outcrops in the Platinum Gulch and Eagle Pup valleys.

Completely weathered rock is included as part of the overburden soils, as discussed previously. Less weathered rock is considered part of the bedrock, which has been subdivided into three broad categories, or types, for the purpose of engineering classification.

"Type 3" rock is the first "rock-like" material underlying the overburden materials, and is defined as being rock that is highly or less weathered (i.e. W4 or better), and has intact strength greater than R0 (i.e. minimum UCS strength 1 MPa). It is expected that Type 3 rock can be excavated with normal excavating equipment.

"Type 2" rock is defined as rock with Geological Strength Index (GSI, Hoek and Marinos, 2000) or Rock Mass Rating (RMR, Bieniawski, 1976) of 30 or greater, and core recovery during drilling of 50 % or greater. Alternatively, where GSI and RMR data are unavailable, average Rock Quality Designation (RQD) of 10 or greater serves as an equivalent criterion. It is expected that Type 2 rock will require ripping.

"Type 1" rock is defined as having GSI, RMR or average RQD exceeding 40. It is expected that Type 1 rock may require blasting.

The outcrop logs in Appendix B and borehole logs in Appendix E provide descriptive data that may be used to infer bedrock "type" as just described.

4.2.4. Frozen Ground

Frozen ground was encountered at 48 test locations (13 boreholes and 35 testpits) in the 2011 field program. Frozen soil was generally encountered immediately within the upper 2.0 m beneath the surficial organic cover; however, frozen ground was encountered to depths of up to 23 m below grade at the ice-rich colluvium lobate feature (BH-BGC11-42 and BH-BGC11-63) and the proposed silt borrow area (BH-BGC11-51). Observations of frozen ground are illustrated on Drawing 11, which also shows frozen ground observations from previous exploration programs. It should be noted that the presence of shallow frozen ground at the time of investigation does not automatically imply the presence of permafrost, since the maximum thaw may occur later in the year than the time of exploration; however, the presence of frozen ground does indicate that frost will likely persist at the observed locations until sometime late in the construction season.

5.0 CLOSURE

We trust the above satisfies your requirements at this time. Should you have any questions or comments, please do not hesitate to contact us.

Yours sincerely,

BGC ENGINEERING INC.

per:

Daniela Welkner, M.Sc.,
Engineering Geologist

Pete Quinn, Ph.D., P.Eng.,
Senior Geotechnical Engineer

Reviewed by:

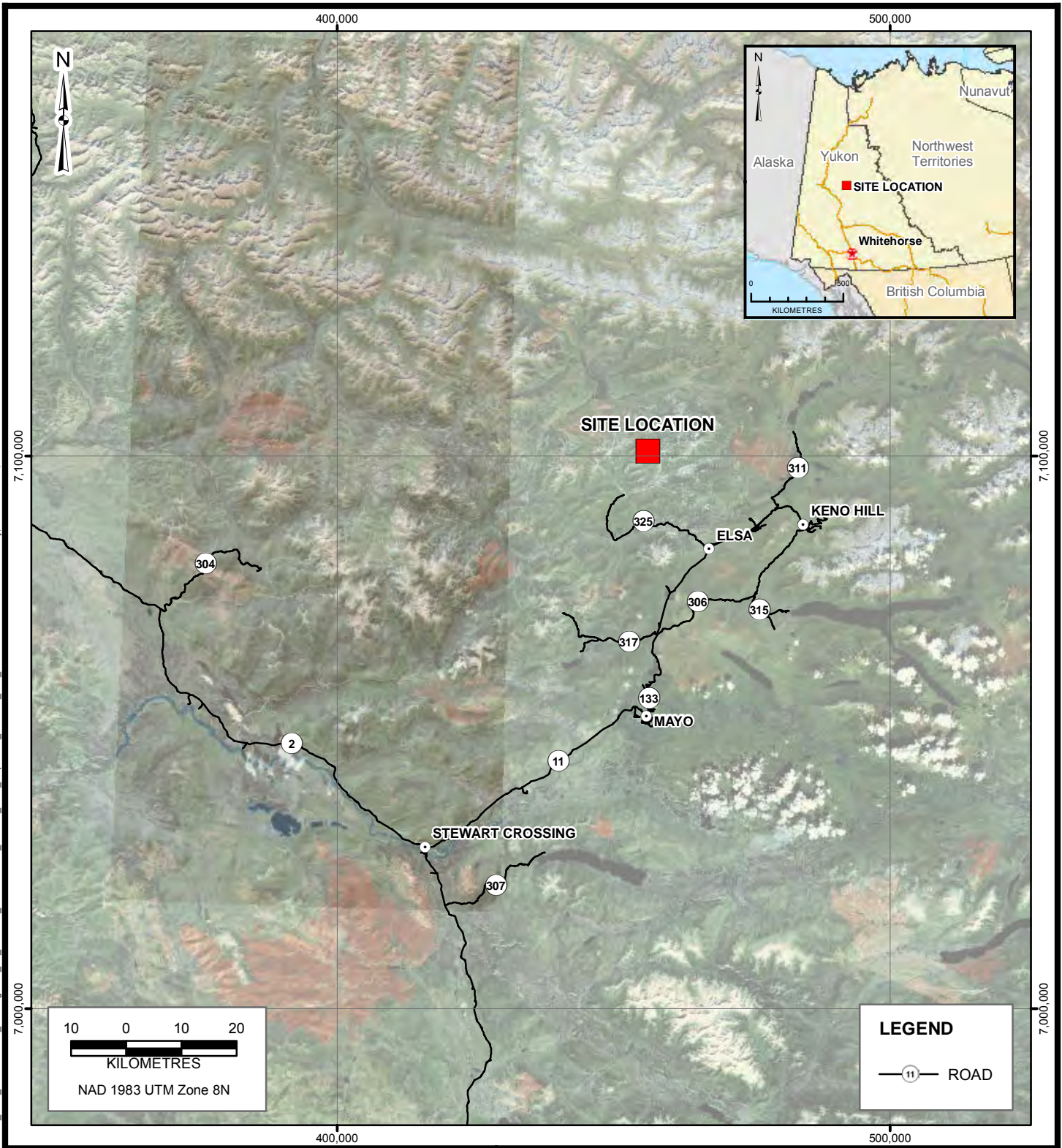
Thomas G. Harper, P.E
Senior Civil Engineer

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DRAWINGS

X:\Projects\0792_Victoria Gold\06\Workspace\20111128_REPORT_2011_Geotechnical_Investigation_for_Minesite_Infrastructure_Factual_Data_Report\01_General_Site_Location.mxd Date: Wednesday, December 14, 2011 Time: 11:02 AM



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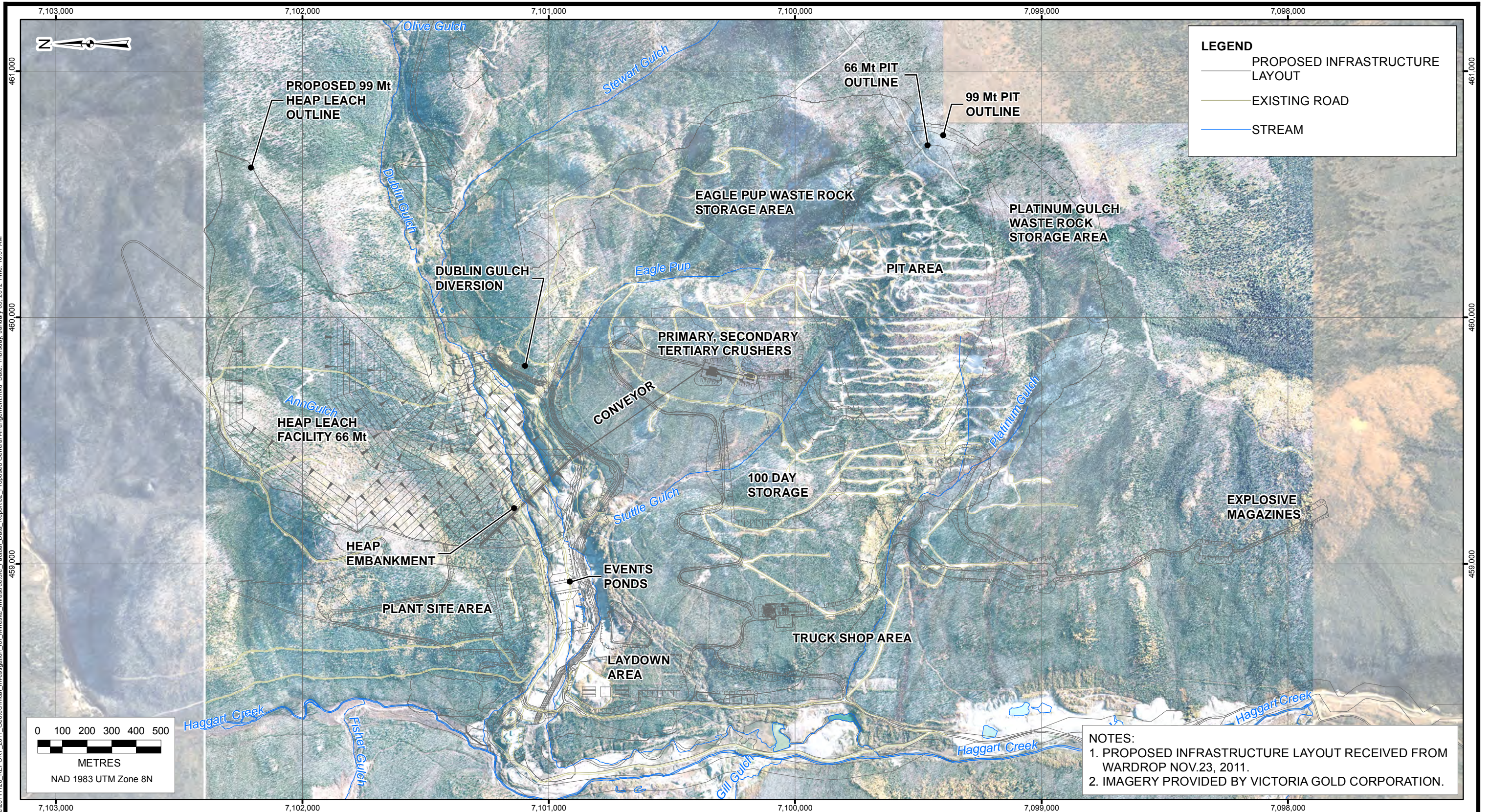
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FACTUAL DATA REPORT**

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VICTORIA GOLD CORPORATION

PROJECT No.:	DWG No.:	REV.:
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A	OCT 2011	ISSUED FOR COMMENT	LL	DW	PQ

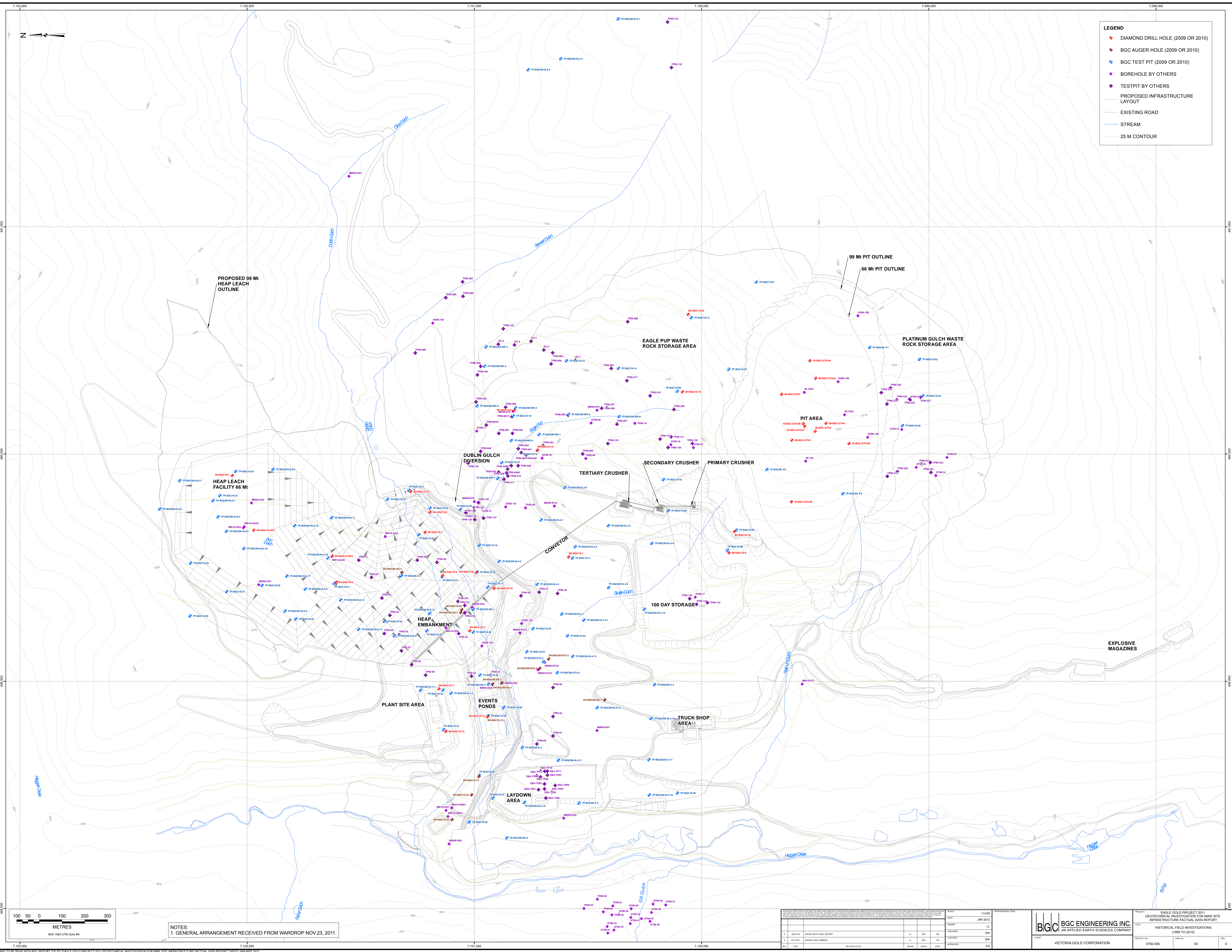
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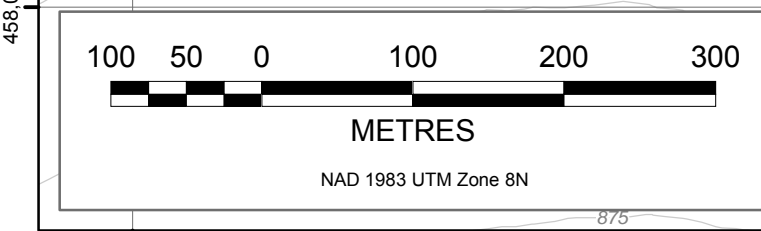
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TITLE: PROPOSED GENERAL ARRANGEMENT		
PROJECT No.: 0792-006	DWG No.: 02	REV.: 0



LEGEND

- ◆ DIAMOND DRILL HOLE (2009 OR 2010)
- BGC AUGER HOLE (2009 OR 2010)
- BGC TEST PIT (2009 OR 2010)
- ◆ BOREHOLE BY OTHERS
- ◆ TESTPIT BY OTHERS
- PROPOSED INFRASTRUCTURE LAYOUT
- EXISTING ROAD
- STREAM
- 25 M CONTOUR



NOTES:
1. GENERAL ARRANGEMENT RECEIVED FROM WARDROP NOV.23, 2011.

REV	DATE	DESCRIPTION	BY	CHKD	APPD
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2	OCT 2011	REVISED FOR COMMENT	LL	DW	PG

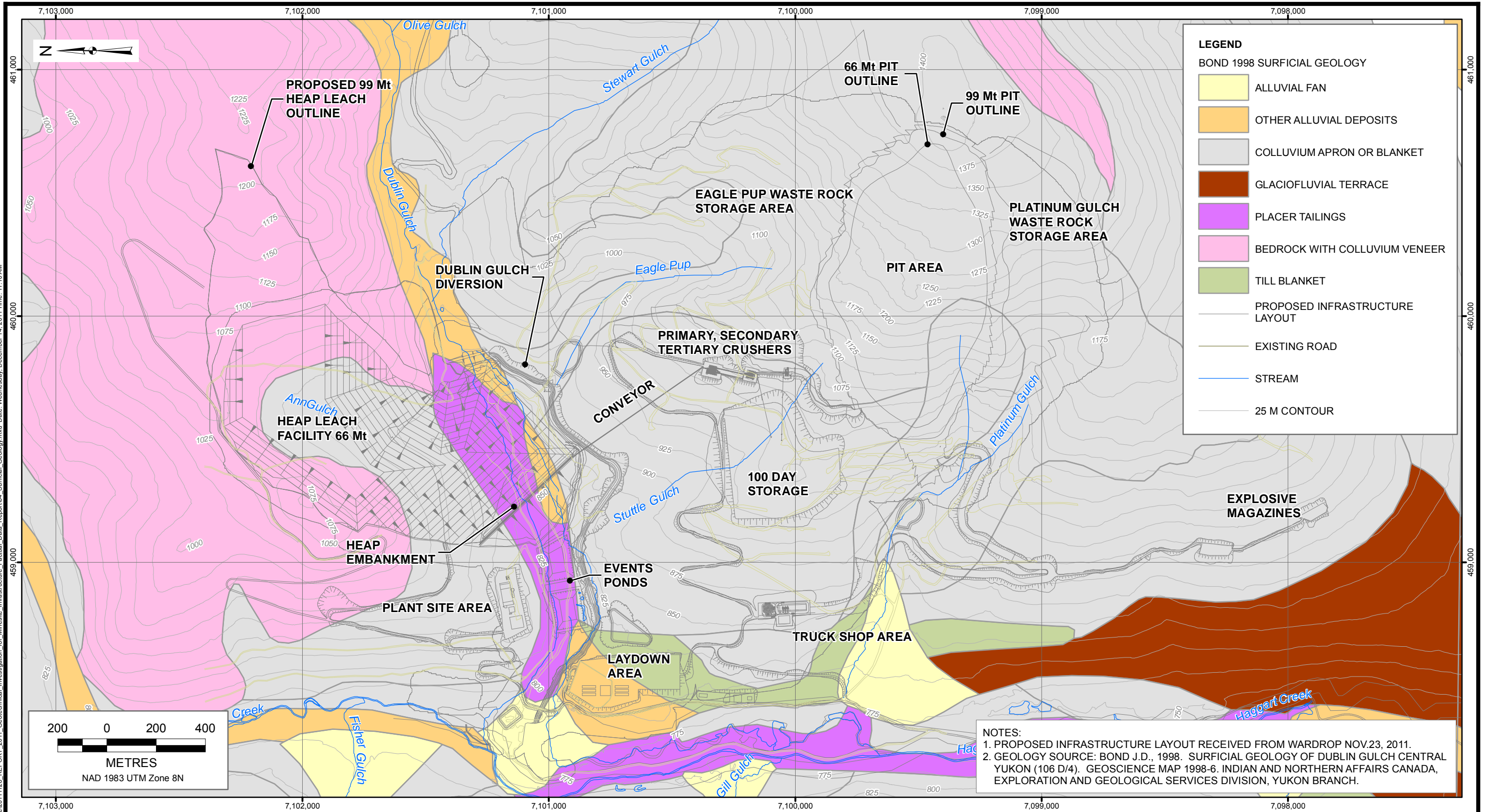
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EAGLE GOLD PROJECT 2011
GEOLOGICAL INVESTIGATION FOR MINE SITE
INFRASTRUCTURE FACTUAL DATA REPORT
HISTORICAL FIELD INVESTIGATIONS
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THIS TO BE READ WITH BGC REPORT FILED "EAGLE GOLD PROJECT 2011 GEOLOGICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT" DATED JANUARY 2012.



LEGEND

BOND 1998 SURFICIAL GEOLOGY

- ALLUVIAL FAN
- OTHER ALLUVIAL DEPOSITS
- COLLUVIUM APRON OR BLANKET
- GLACIOFLUVIAL TERRACE
- PLACER TAILINGS
- BEDROCK WITH COLLUVIUM VENEER
- TILL BLANKET
- PROPOSED INFRASTRUCTURE LAYOUT
- EXISTING ROAD
- STREAM
- 25 M CONTOUR

NOTES:

1. PROPOSED INFRASTRUCTURE LAYOUT RECEIVED FROM WARDROP NOV.23, 2011.
2. GEOLOGY SOURCE: BOND J.D., 1998. SURFICIAL GEOLOGY OF DUBLIN GULCH CENTRAL YUKON (106 D/4). GEOSCIENCE MAP 1998-6. INDIAN AND NORTHERN AFFAIRS CANADA, EXPLORATION AND GEOLOGICAL SERVICES DIVISION, YUKON BRANCH.

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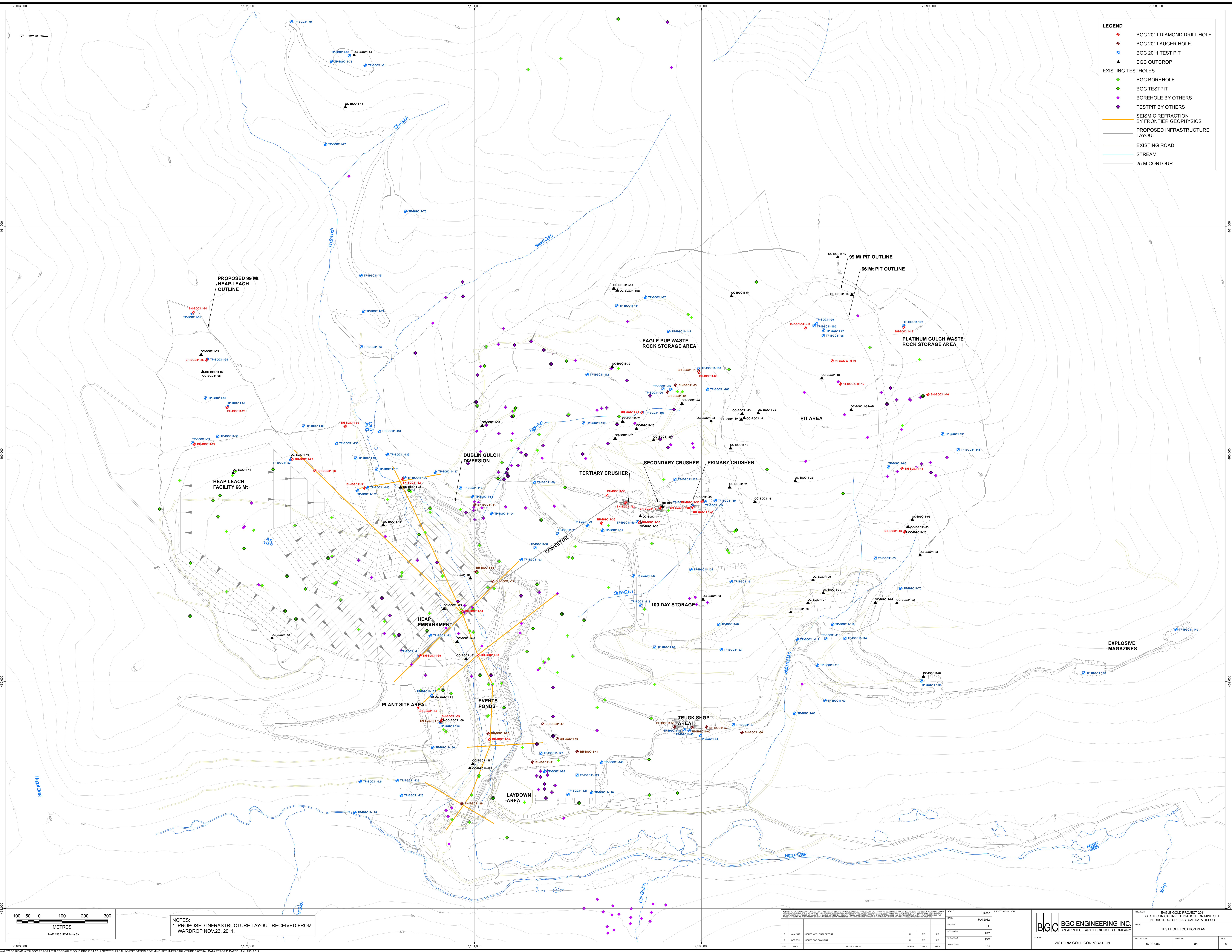
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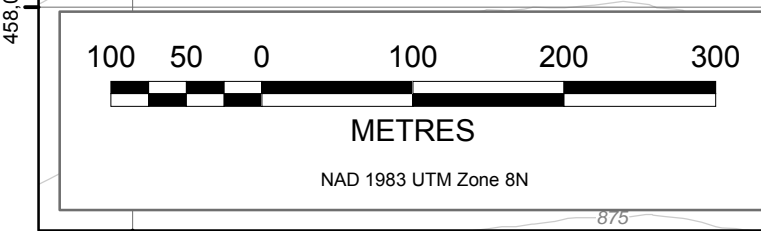
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PROJECT: EAGLE GOLD PROJECT		
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TITLE: SURFICIAL GEOLOGY		
PROJECT No.:	DWG No.:	REV.:
0792-006	04	0

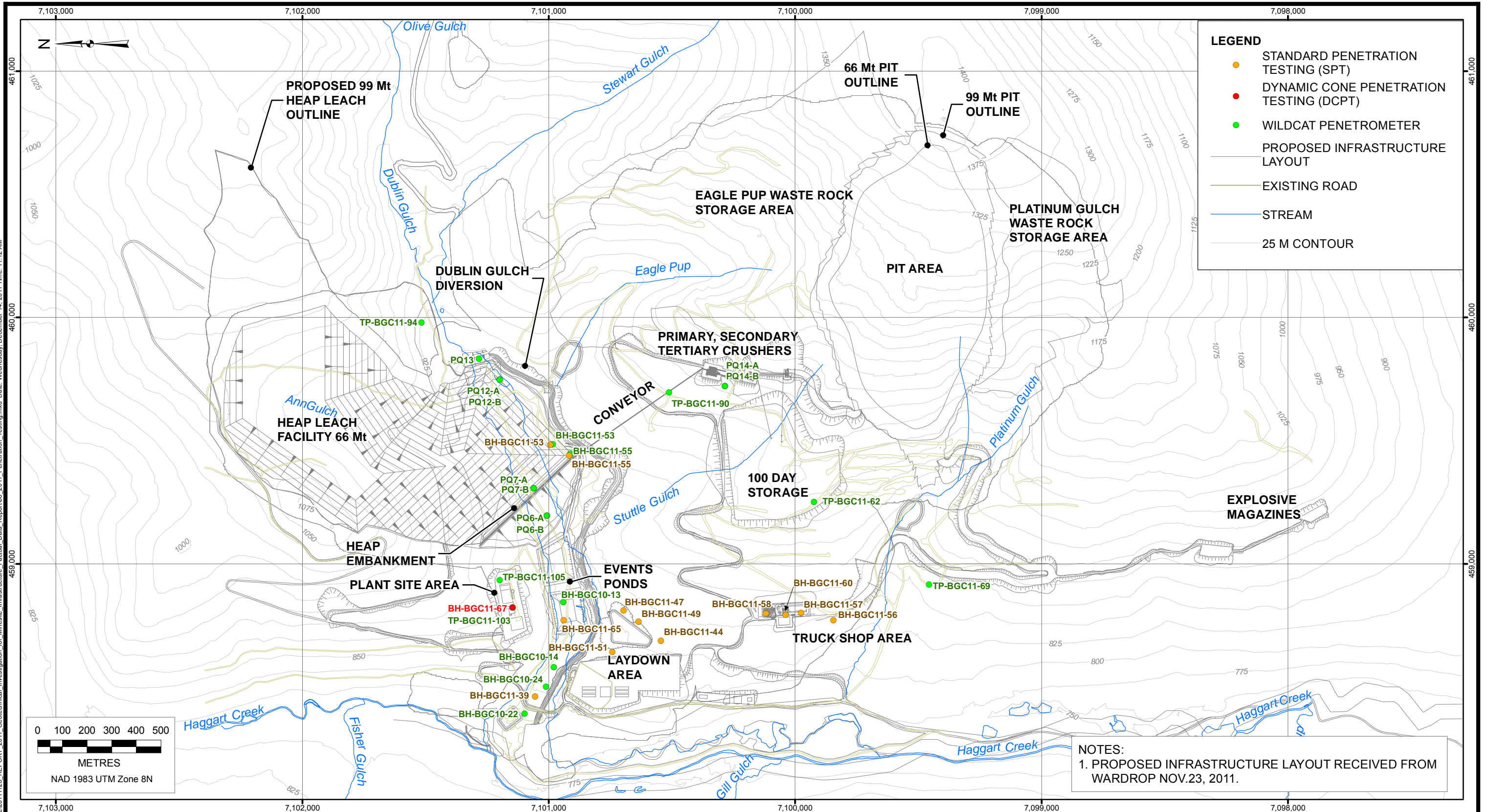


- LEGEND**
- ◆ BGC 2011 DIAMOND DRILL HOLE
 - ◇ BGC 2011 AUGER HOLE
 - BGC 2011 TEST PIT
 - ▲ BGC OUTCROP
 - EXISTING TESTHOLES
 - ◆ BGC BOREHOLE
 - ◇ BGC TESTPIT
 - ◇ BOREHOLE BY OTHERS
 - ◇ TESTPIT BY OTHERS
 - SEISMIC REFRACTION BY FRONTIER GEOPHYSICS
 - PROPOSED INFRASTRUCTURE LAYOUT
 - EXISTING ROAD
 - STREAM
 - 25 M CONTOUR



NOTES:
1. PROPOSED INFRASTRUCTURE LAYOUT RECEIVED FROM WARDROP NOV.23, 2011.

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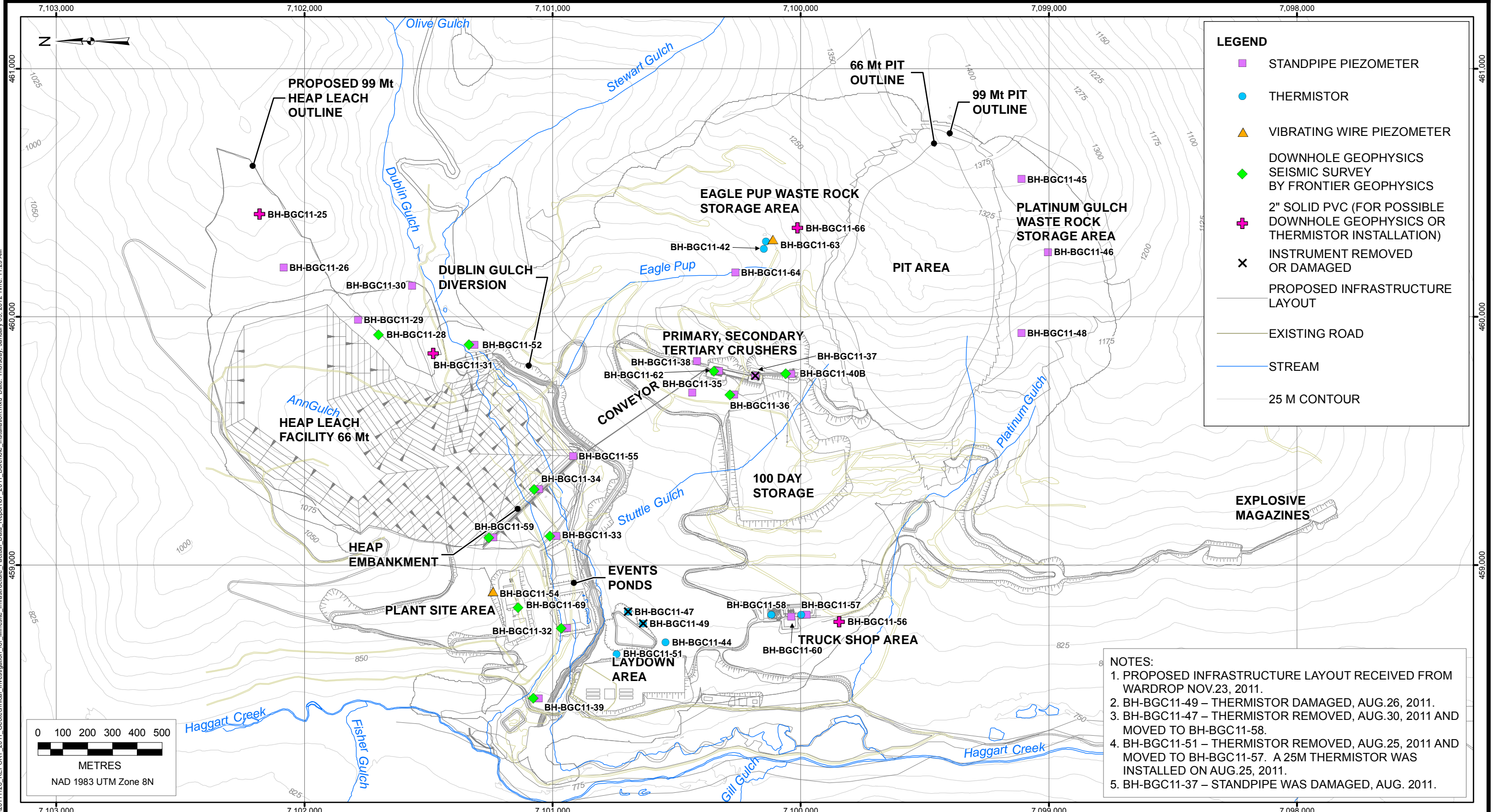
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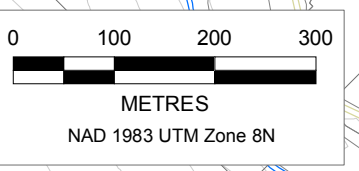
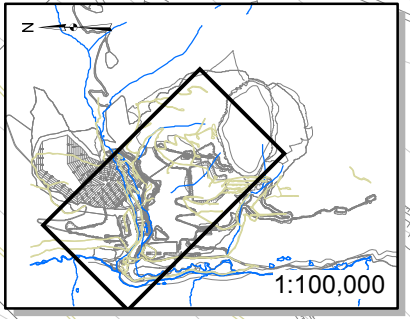
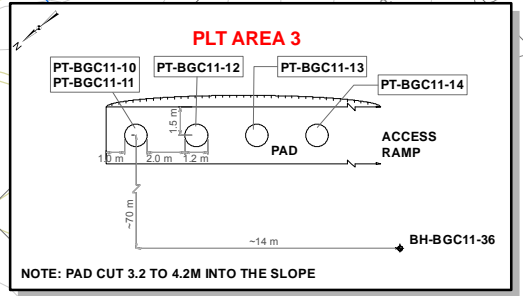
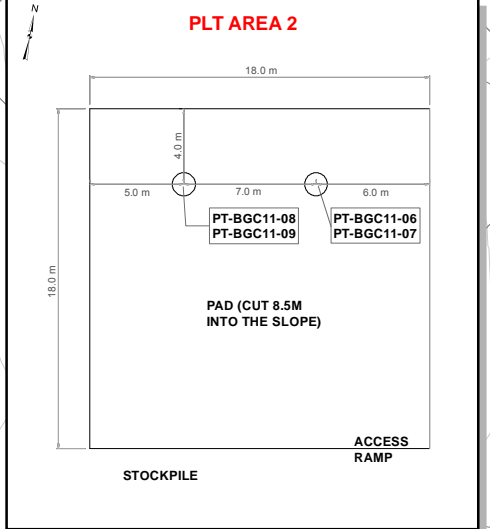
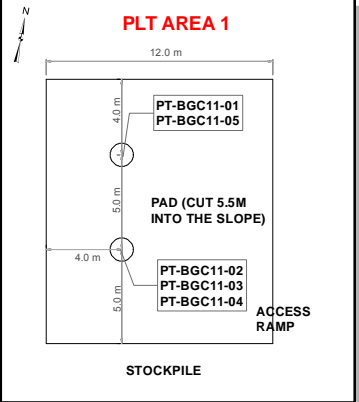
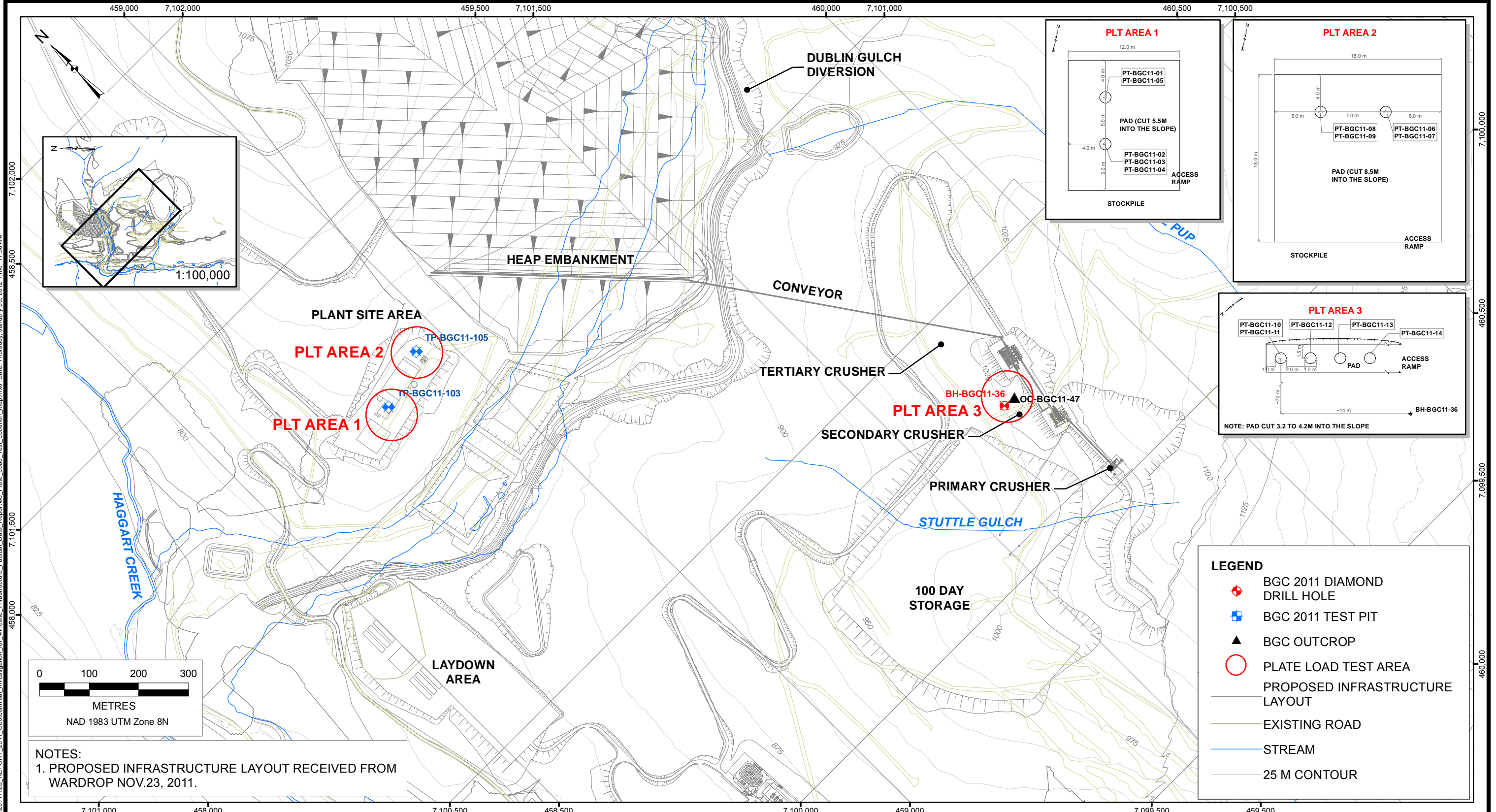
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TITLE:	2011 BOREHOLE INSTALLATIONS		
PROJECT No.:	0792-006	DWG No.:	07
REV.:			0



NOTES:
 1. PROPOSED INFRASTRUCTURE LAYOUT RECEIVED FROM WARDROP NOV.23, 2011.

LEGEND	
	BGC 2011 DIAMOND DRILL HOLE
	BGC 2011 TEST PIT
	BGC OUTCROP
	PLATE LOAD TEST AREA
	PROPOSED INFRASTRUCTURE LAYOUT
	EXISTING ROAD
	STREAM
	25 M CONTOUR

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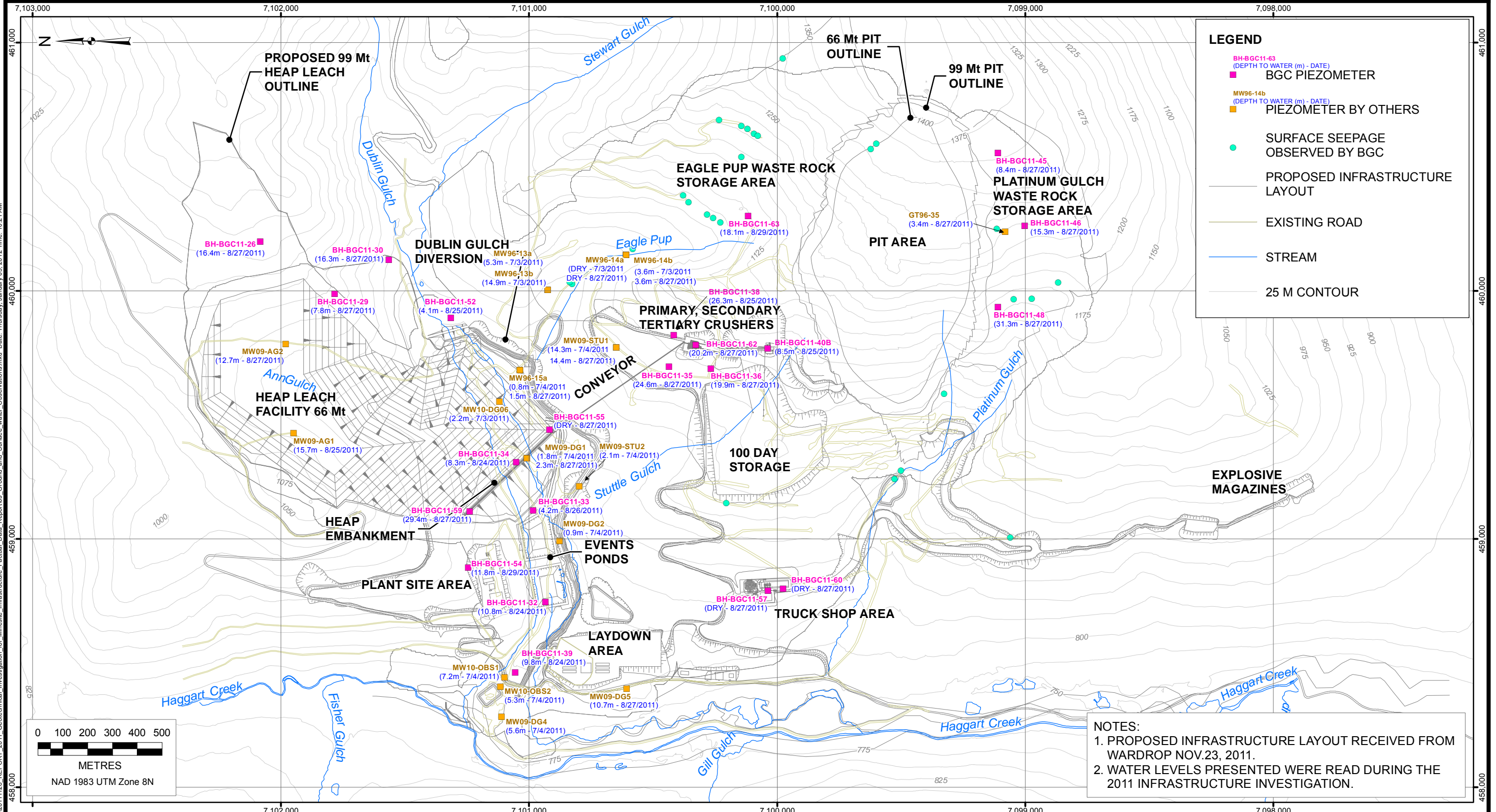
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TITLE: PLATE LOAD TEST LOCATION MAP		
PROJECT No.:	DWG No.:	REV.:
0792-006	08	0



LEGEND

- BH-BGC11-63 (DEPTH TO WATER (m) - DATE) BGC PIEZOMETER
- MW96-14b (DEPTH TO WATER (m) - DATE) PIEZOMETER BY OTHERS
- SURFACE SEEPAGE OBSERVED BY BGC
- PROPOSED INFRASTRUCTURE LAYOUT
- EXISTING ROAD
- STREAM
- 25 M CONTOUR

NOTES:

1. PROPOSED INFRASTRUCTURE LAYOUT RECEIVED FROM WARDROP NOV.23, 2011.
2. WATER LEVELS PRESENTED WERE READ DURING THE 2011 INFRASTRUCTURE INVESTIGATION.

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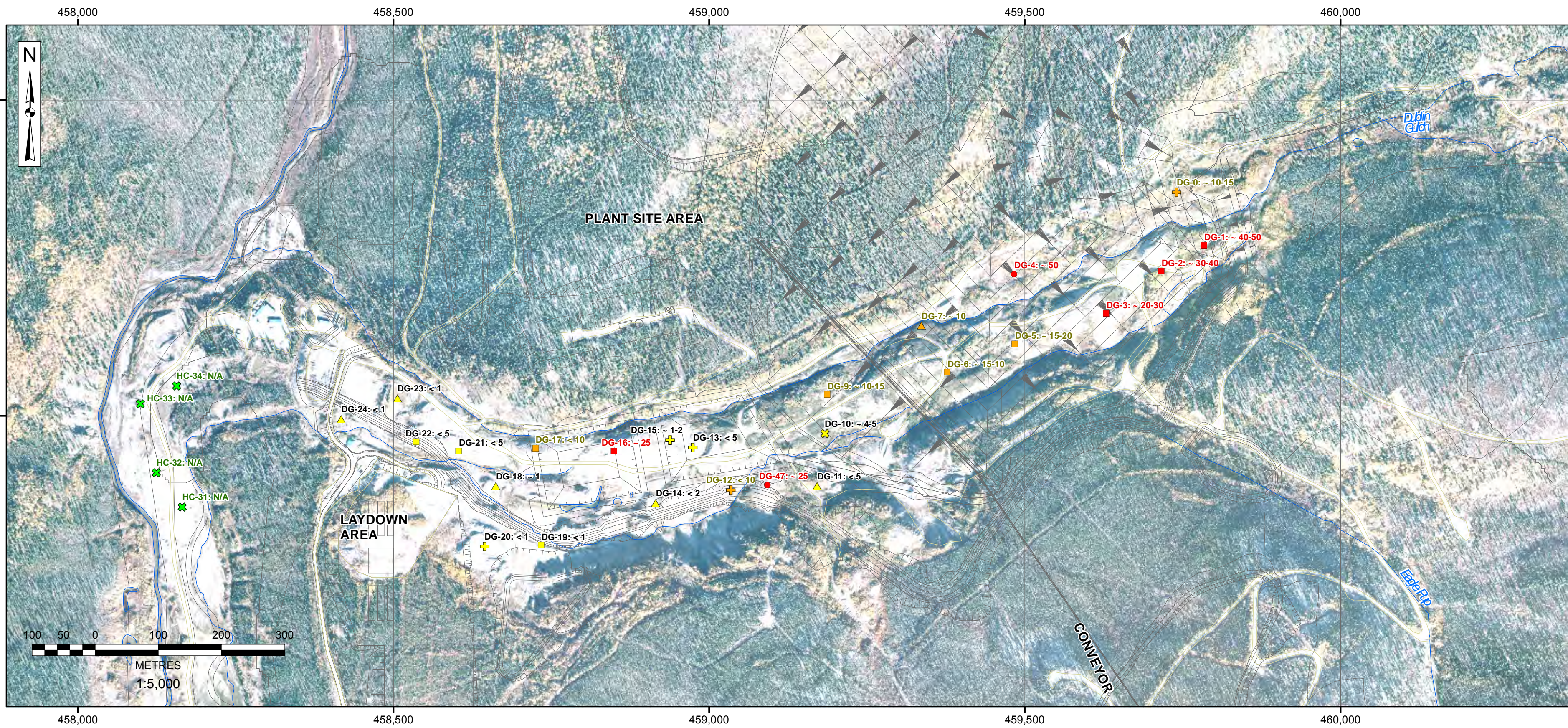
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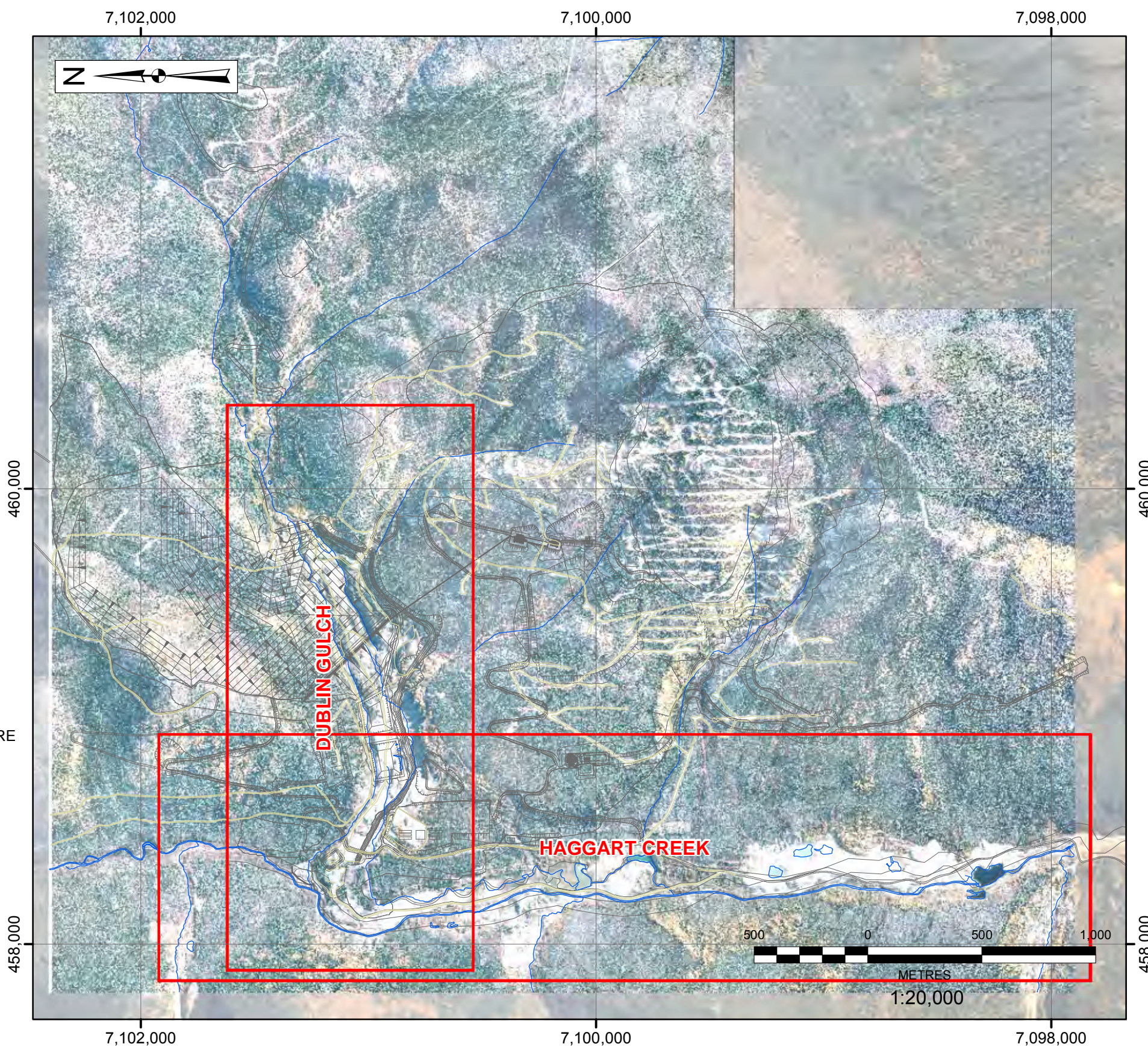
PROJECT:	EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT		
TITLE:	GROUND AND SURFACE WATER OBSERVATIONS		
PROJECT No.:	0792-006	DWG No.:	09
REV.:			0

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DUBLIN GULCH

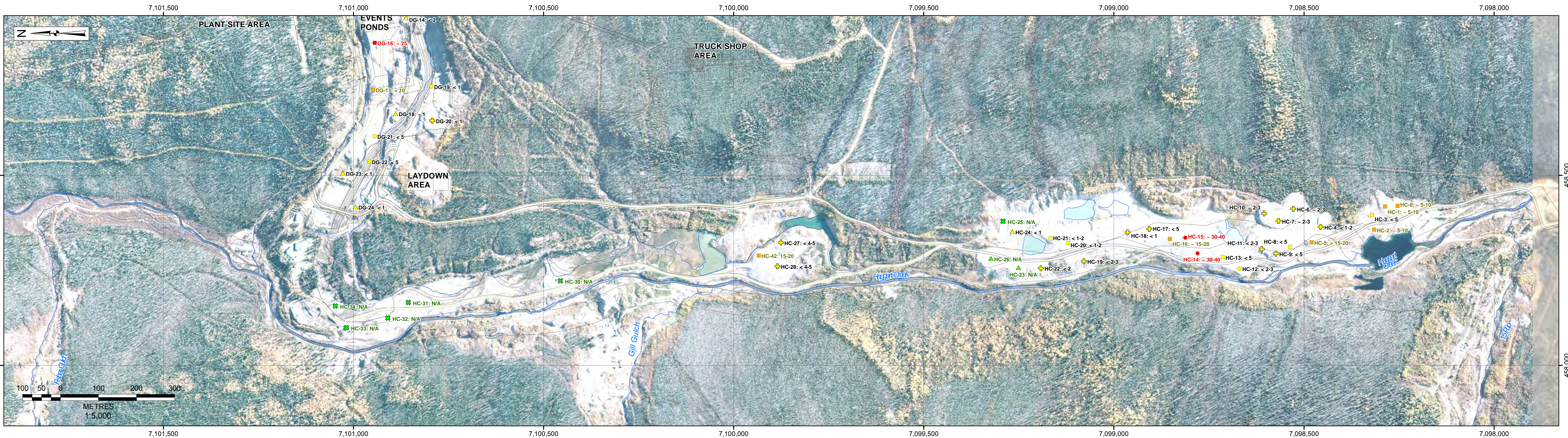


OVERVIEW MAP



- LEGEND**
- ESTIMATED % OF OVERSIZE PARTICLES (>75 MM)
- N/A
 - 0-5 %
 - 5-20 %
 - >20 %
- SOIL CLASSIFICATION BY UNIFIED SOIL CLASSIFICATION SYSTEM (USCS)**
- GP - POORLY GRADED GRAVEL
 - GW - WELL GRADED GRAVEL
 - ▲ SM - SILTY SAND
- EXAMPLE: \oplus HC-22: <2
- HAGGART CREEK SAMPLE N#22: <2% OF AGGREGATE IS GREATER THAN 75MM.
- PROPOSED INFRASTRUCTURE LAYOUT
- EXISTING ROAD
- STREAM

HAGGART CREEK



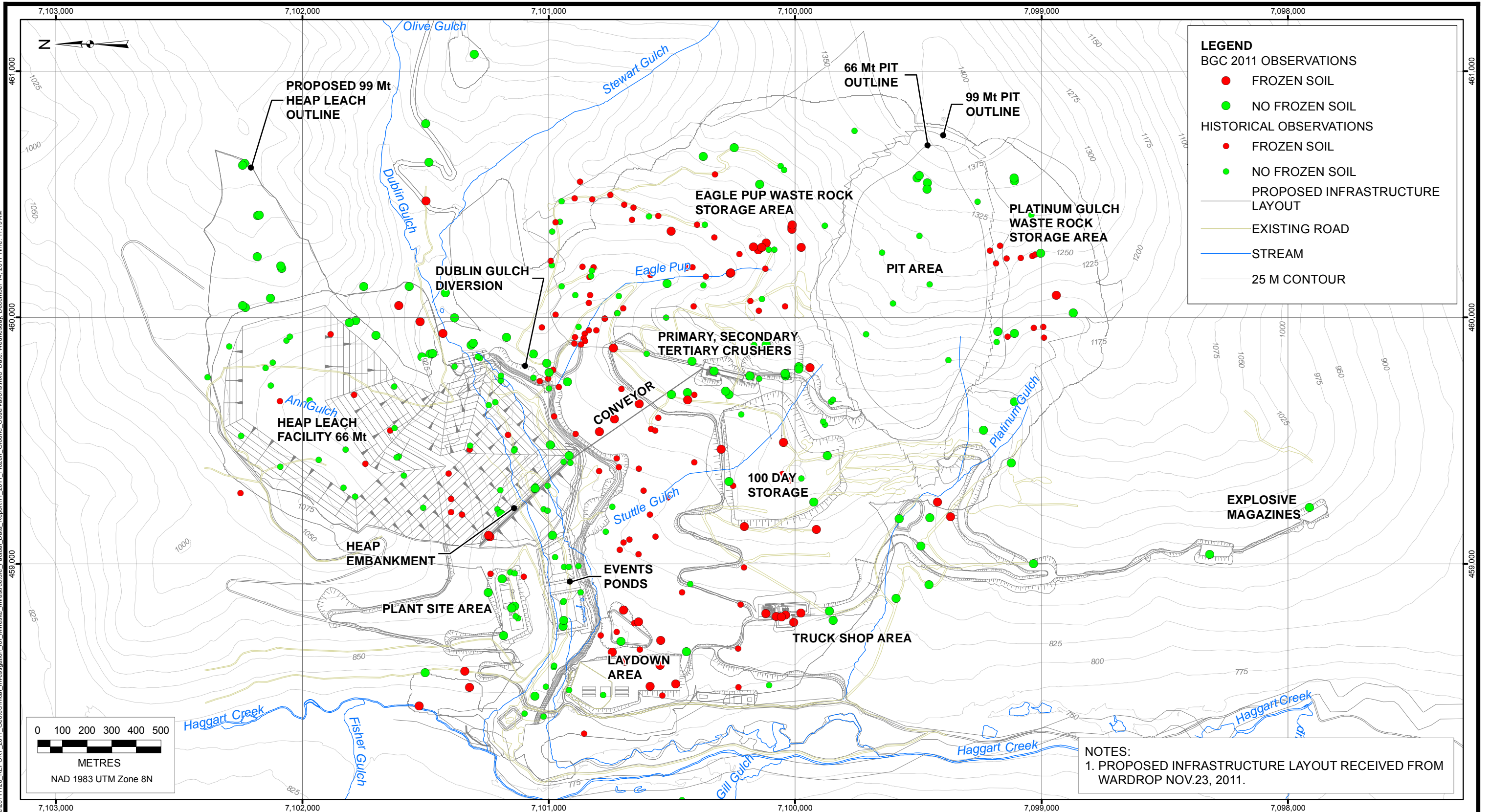
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TITLE: PLACER TAILINGS VISUAL CLASSIFICATION OF SURFICIAL MATERIALS	PROJECT NO.: 0792-006
DWG NO.: 10	REV.: 0



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TITLE: 2011 FROZEN GROUND OBSERVATIONS		
PROJECT No.:	DWG No.:	REV.:
0792-006	11	0

APPENDIX A

TERMINOLOGY FOR OUTCROP MAPPING AND TEST HOLE LOGS

TERMINOLOGY FOR TEST HOLE AND OUTCROP MAPPING LOGS

1.0 INTRODUCTION

This appendix provides references used for describing the properties of soil and rock as they appear in the test hole and outcrop mapping logs included in Appendices B, C and E.

2.0 SOILS

The following section presents the references used for the classification of soils as described in the logs. The presented descriptors have been used for the logging of the soil portion of diamond drill holes, auger holes, test-pits and the overburden part of the outcrops/cut slopes. References in this section have been organized so as to follow typical order of description used in BGC’s standard practice. It should be noted that descriptors for fine and coarse grained soil material have been combined and presented in their general order of appearance so as to avoid redundancy. Material is described as a soil if it is a transported soil, or if it is rock weathered in place with weathering grade greater than highly weathered (W4, Table 19) and with an extremely weak Unconfined Compressive Strength (UCS) (R0, Table 18).

Table 1. USCS Classification

Major Divisions			Group Symbol	Group Name
Coarse grained soils more than 50% retained on No.200 (0.075 mm) sieve	Gravel > 50% of coarse fraction retained on No.4 (4.75 mm) sieve	Clean gravel < 5% smaller than #200 sieve	GW	Well graded gravel, fine to coarse gravel
			GP	Poorly graded gravel
		Gravel with > 12% fines	GM	Silty gravel
			GC	Clayey gravel
	Sand ≥ 50% of coarse fraction passes No.4 sieve	Clean sand	SW	Well graded sand, fine to coarse sand
			SP	Poorly-graded sand
		Sand with >12% fines	SM	Silty sand
			SC	Clayey sand
Fine grained soils more than 50% passes No.200 sieve	Silt and clay liquid limit ≥ 50	Inorganic	ML	Silt
			CL	Clay
		Organic	OL	Organic silt, organic clay
	Silt and clay liquid limit ≥ 50	Inorganic	MH	Silt of high plasticity
			CH	Clay of high plasticity
		Organic	OH	Organic clay, organic silt
Highly organic soils			Pt	Peat

Table 2. Classification by Particle Size

Classification by Particle Size			
Name	Size Range		
	(mm)	US Standard Sieve	
		Retained	Passing
Boulders	> 200	8 inch	-
Cobbles	75-200	3 inch	8 inch
Gravel:			
Coarse	19 - 75	0.75 inch	3 inch
Fine	4.75 - 19	No. 4	0.75 inch
Sand:			
Coarse	2 - 4.75	No. 10	No. 4
Medium	0.43 - 2	No. 40	No. 10
Fine	0.075 – 0.43	No. 200	No. 40
Fines (silt or clay)	< 0.075	-	No. 200

Table 3. Proportion of Minor Components by Weight

Proportion of Minor Components by Weight	
“and”	40% - 50%
“y/ey”	25% - 40%
“some”	10% - 25%
“trace”	0% - 10%

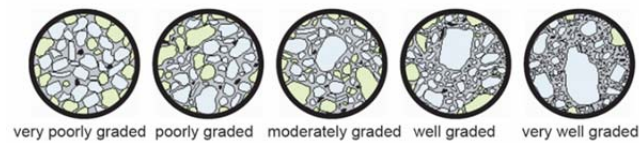


Figure 1. Gradation Classification

Table 4. Classification for Density of Granular Soils

Density of Granular Soils		
Description	SPT “N”	Field Test/Identification
V. Loose	0 – 4	None
Loose	4 – 10	Easily penetrated by 13 mm rod pushed by hand
Compact	10 – 30	Easily penetrated by 13 mm rod driven by hammer
Dense	30 – 50	Penetrated by 13 mm rod driven by hammer
V. Dense	> 50	Penetrated few cm's by 13 mm rod driven by hammer

Table 5. Classification for Consistency of Fine Soils

Strength of Cohesive Soils				
Grade	Description	Field Identification	SPT “N”	Undrained Shear Strength (kPa)
S1	Very soft	Easily penetrated several inches by fist	< 2	< 12
S2	Soft	Easily penetrated several inches by thumb	2 – 4	12 – 25
S3	Firm	Can be penetrated by thumb with moderate effort	4 – 8	25 – 50
S4	Stiff	Can be penetrated by thumb but penetrated only with great effort	8 – 15	50 – 100
S5	Very stiff	Readily indented by thumbnail	15 – 30	100 – 200
S6	Hard	Indented with difficulty by thumbnail	> 30	> 200

Table 6. Classification for Angularity of Coarse Grained Particles

Angularity of Coarse-Grained Particles	
Description	Criteria
Angular	Particles have sharp edges and relatively plane sides with unpolished surfaces
Subangular	Particles are similar to angular description but have rounded edges
Subrounded	Particles have nearly plane sides but have well-rounded corners and edges
Rounded	Particles have smoothly curved sides and no edges

Table 7. Classification for Particle shape

Particle Shape	
Flat	Particle width/thickness > 3
Elongated	Particles with length/width > 3
Flat and elongated	Particles meet criteria for both flat and elongated

Table 8. Classification for Plasticity of Soils

Plasticity of Cohesive Soils			
Description	Silt	Clay	Criteria
High	$W_L > 50$	$W_L > 50$	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit
Medium	-	$30 < W_L < 50$	The thread is easy to roll and not much time is required to reach the plastic limit. The lump crumbles when drier than the plastic limit.
Low	$W_L < 50$	$W_L < 30$	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.
Non-Plastic	NP	-	A 1/8 inch (3mm) thread cannot be rolled at any water content.

Where: W_L = Liquid Limit (%)

Table 9. Classification for Cementation of Soils

Cementation	
Description	Criteria
Weak	Crumbles or breaks with handling or little finger pressure
Moderate	Crumbles or breaks with considerable finger pressure
Strong	Will not crumble or break with finger pressure

Table 10. Classification for Soil Structure

Description	Criteria
Stratified	Alternating layers of varying material or color with layers at least 6 mm thick; note thickness
Laminated	Alternating layers of varying material or color with the layers less than 6 mm thick; note thickness
Fissured	Breaks along definite planes of fracture with little resistance to fracturing
Slickensided	Fracture planes appear polished or glossy, sometimes striated
Blocky	Cohesive soil that can be broken down into small angular lumps which resist further breakdown
Lensed	Inclusion of small pockets of different soils, such as small lenses of sand scattered through a mass of clay; note thickness
Homogeneous	Same color and appearance throughout

Table 11. Classification for Description of Dry Strength in Soils

None	Dry Specimen crumbles upon handling
Low	Crumbles into powder with some finger pressure
Medium	Crumbles with considerable finger pressure
High	Can't be broken with fingers; will break between thumb and hard surface
Very High	Cannot be broken between thumb and hard surface

Note: Assessed on natural soil pieces or moulded balls about 12 mm diameter that have been air-dried, or oven dried at less than 60 °C

Table 12. Classification for Dilatancy in Soil

Description	Criteria
None	No visible change in the specimen
Slow	Water appears slowly on the surface of the specimen during shaking and does not disappear or disappears slowly upon squeezing
Rapid	Water appears quickly on the surface of the specimen during shaking and disappears quickly upon squeezing

Table 13. Ice Description

Criteria	Descriptors
Colour	Colourless, white, stained, etc.
Clarity	Clear, transparent, cloudy, opaque, etc.
Bubble Content	Visually estimate percent
Bubble Size	In mm
Bubble Shape	Round, elongate, tubular Note: If the elongated bubbles have a preferential direction compared to the ground surface
Sediment inclusions or lenses	Visually estimate percent and thickness of lenses
Any other inclusions	Such as organics

Note: Massive ice description (ice greater than 1" or 2.5 cm)

Table 14. Classification for Description of Frozen Ground

ICE NOT VISIBLE

GROUP SYMBOL	SUBGROUP		
	SYMBOLS	DESCRIPTION	
N	Nf	Poorly bonded or friable	
	Nbn	No excess ice, well bonded	
	Nbe	Excess ice, well bonded	

VISIBLE ICE LESS THAN 2.5cm THICK

V	Vx	Individual ice crystals or inclusions	
	Vc	Ice coatings on particles	
	Vr	Random or irregularly oriented ice formations	
	Vs	Stratified or distinctly oriented ice formations	

VISIBLE ICE GREATER THAN 2.5cm THICK

ICE	ICE + soil type	Ice with soil inclusions	
	ICE	Ice without soil inclusions	

(Modified from Guide to Field Description of Permafrost for Engineering Purposes, NRC. Technical Memorandum 79)

NOTE 1) Dual symbols are used to indicate borderline or mixed ice classifications
 2) Visual estimates of ice content as indicated on borehole logs are $\pm 5\%$

LEGEND or

Source: American Society for Testing and Materials. Standard D4083: Standard Practice for Description of Frozen Soils (Visual-Manual Procedure). Developed by Sub-Committee D18.03. West Conshohocken, PA. 6

Table 15 Symbols for Augering Techniques

Technique Acronym	Description
SSA	Solid Stem Auger
HSA	Hollow Stem Auger
CRREL	Coring barrel compatible with auger rigs originally designed by the Cold Regions Research and Engineering Laboratory for the purpose of coring and sampling frozen ground

3.0 ROCK

Rock logs were generated to present collected information for any untransported rock material that was described as highly weathered or less (<W5, Table 19) and of an Unconfined Compressive Strength (UCS) greater than extremely weak (R0, Table 18). Although not applicable in every case, the following rock descriptors were used for the diamond drillholes core logging, for the bottom of various test-pits and for the outcrops/cut slopes.

The general organization of this section follows the BGC standard logging order for lithological descriptions and is completed with details of measurements and criteria that apply to the rock structural mass description and cut slope logging.

Table 16. Classification for Grain Size: describes the grains size(s) present in the rock

Term	Particle Size	Examples
Very Coarse	> 60 mm	Porphyries – w/ measureable grains
Coarse	2 – 60 mm	Breccia, Gneiss – w/ measurable grains
Medium	0.06 – 2 mm	Sandstone, Granite, Schist – having clearly visible grains
Fine	0.002 – 0.06 mm	Tuff, Siltstone, Claystone, Mudstone, Basalt.
Very Fine	< 0.002 mm	

Table 17. Classification for Rock Structure

Igneous	Sedimentary	Metamorphic
Massive	Massive	Massive
Banded	Clast-Supported	Banded
Disturbed / Seamy	Matrix-Supported	Foliated
Disintegrated	Bedded	Disturbed / Seamy
	Laminated	Disintegrated
	Disturbed / Seamy	
	Disintegrated	

Table 18. Unconfined Compressive Strength / Hardness Classification for Rock (after CFEM 2006)

	Grade	Description	Field Identification	UCS (MPa)	Point Load Index (MPa)
Rock Hardness	R6	Extremely Strong	Specimen can only be chipped with flat end geological hammer.	> 250	> 10
	R5	Very Strong	Specimen requires many blows with flat end geological hammer to fracture.	100-250	4-10
	R4	Strong	Specimen requires more than one blow of flat end geological hammer to fracture.	50-100	2-4
	R3	Medium Strong	Cannot be scraped or peeled with pocket knife; can be fractured with single firm blow of flat end geological hammer.	25-50	1-2
	R2	Weak	Can be peeled by a pocket knife with difficulty; shallow indentation made by firm blow with point geologic hammer.	5-25	-
	R1	Very Weak	Crumbles under firm blows with point of geological hammer.	1-5	-
	R0	Extremely Weak	Indented by thumbnail.	< 1	-

Table 19. Classification for Weathering Grades of Rock

Grade	Description	Field Identification
A1 / W1	Fresh and Unweathered	Parent rock showing no discoloration, loss of strength or any other weathering effects.
		Strength may be increased by some alteration types.
A2 / W2	Slightly Weathered or Altered	Rock may be slightly discolored, particularly adjacent to discontinuities, which may be open and will have slightly discolored surfaces; the intact rock is may be weaker than the fresh rock.
A3 / W3	Moderately Weathered or Altered	Rock is discolored; discontinuities may be open and have discolored surfaces with alteration starting to penetrate inwards; intact rock is noticeably weaker than W1/A1 rock of the same unit.
A4 / W4	Highly Weathered or Altered	Rock is discolored; discontinuities may be open and have discolored surfaces, and the original fabric of the rock near the discontinuities may be altered; alteration penetrates deeply inwards. The ratio of original rock to weathered rock should be estimated where possible.
A5 / W5	Completely Weathered or Altered	Rock is discolored and decomposed/ friable or changed completely to a soil, but original fabric is visible. The properties of the soil depend in part on the nature of the parent rock.
A6 / W6	Residual Soil	Original rock fabric is completely destroyed.

Table 20. Classification for Description of Discontinuity Aperture in Rock

Aperture	Description	
< 0.1 mm 0.1 – 0.25 mm 0.25 – 0.5 mm	Very tight Tight Partly open	“Closed” features
0.5 – 2.5 mm 2.5 – 10 mm > 10 mm	Open Moderately wide Wide	“Gapped” features
1 – 10 cm 10 – 100 cm 1 m	Very wide Extremely wide Cavernous	“Open” features

Table 21. Codification for Discontinuity Infill Description in Rock

Code	Type	Description
BX	Broken Rock	Fractures \leq 50 mm to 10 mm, rock fragments maintain intact rock strength
CX	Crushed Rock	Rock has been disintegrated
DX	Decomposed Rock	Clay altered and chemically weathered zones
CL	Clay	Very finely grained, cannot be brushed off hands when dry, no dilatancy, describe in comments
SM	Silt	Fine grained, can be brushed off hands when dry, dilates rapidly, estimate plasticity in comment form
SA	Sand	Non-cohesive infill; visible particles > 0.06 to 2 mm in diameter
FB	Fault Breccia	Clast supported, in matrix of gouge or mineral infill. Can be partially or completely healed
FG	Fault Gouge	Unconsolidated, matrix supported clay, sand and gravel sized rock fragments
ST	Surface Staining	* Record color
FE	Iron Oxides/ Staining	Red/orange/brown; can be scratched with a knife, may be slightly magnetic
TC	Talc	Typically white, slippery feel, lowest hardness (1); can be scratched with fingernail
CH	Chlorite	Green; low hardness (2-2.5) can be scratched with fingernail; non elastic; vitreous; streak colorless
SE	Sericite	White/grey/pale green, fine grained mica; variety of muscovite; occurs in small scales/flakes; silky luster
GY	Gypsum	Low hardness (2); can be scratched with fingernail; cleaves perfectly in one plane
CA	Calcite	Low hardness (3); cannot be scratched by fingernail, but can be scratched with a penny; 3 perfect cleavages; reacts strongly to acid without crushing
PY	Pyrite	White to yellow, shiny, may have block/cubic crystals
AN	Anhydrite	Semi hard (3-3.5); white, greyish, bluish, vitreous, rectangular cleavage fragments
KF	Potassium Feldspar	Light pink to red colour, associated with Potassic alteration, moderate hardness, can be scratched with steel knife with difficulty
QZ	Quartz	White to dark grey, glassy luster; moderate to high hardness (7); cannot be scratched with steel knife (hardness of 6.5); conchoidal fracture

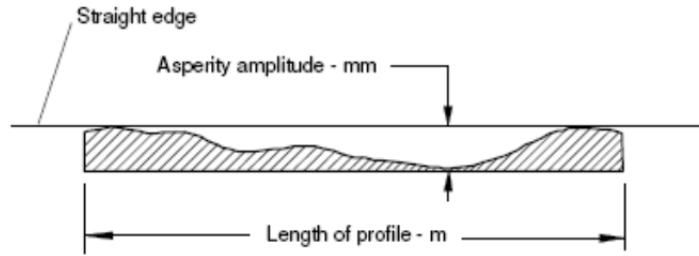


Figure 2. Descriptive Criteria for Joint Roughness Coefficient (JRC)

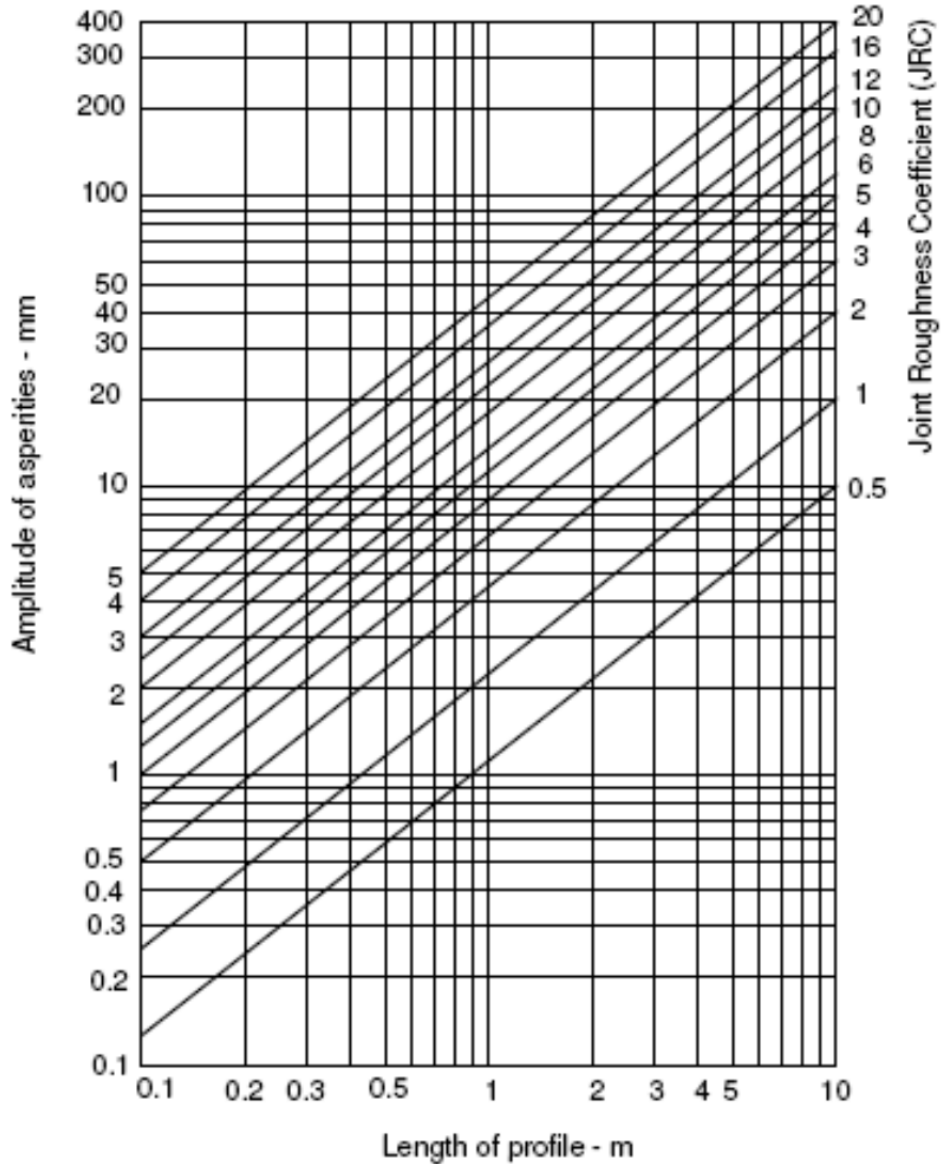


Figure 3. Chart for Determination of the Joint Roughness Coefficient (JRC) in Rock (after Barton et al., 1974)

Table 22. Classification for Discontinuity Spacing in Rock

Description	Discontinuity Spacing
Extremely Close Spacing	< 20 mm
Very Close Spacing	20 – 60 mm
Close Spacing	60 – 200 mm
Moderate Spacing	200 – 600 mm
Wide Spacing	600 – 2000 mm
Very Wide Spacing	2000 – 6000 mm
Extremely Wide Spacing	> 6000 mm

Table 23. Classification for Persistence of Structural Set in the Rock Mass

Description	Discontinuity Persistence
Very Low Persistence	< 1 m
Low Persistence	1 – 3 m
Medium Persistence	3 – 10 m
High Persistence	10 – 20 m
Very High Persistence	> 20 m

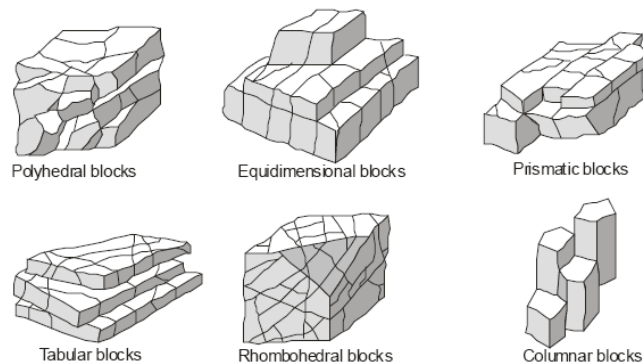


Figure 4. Classification for Block Shape in the Rock Mass Description

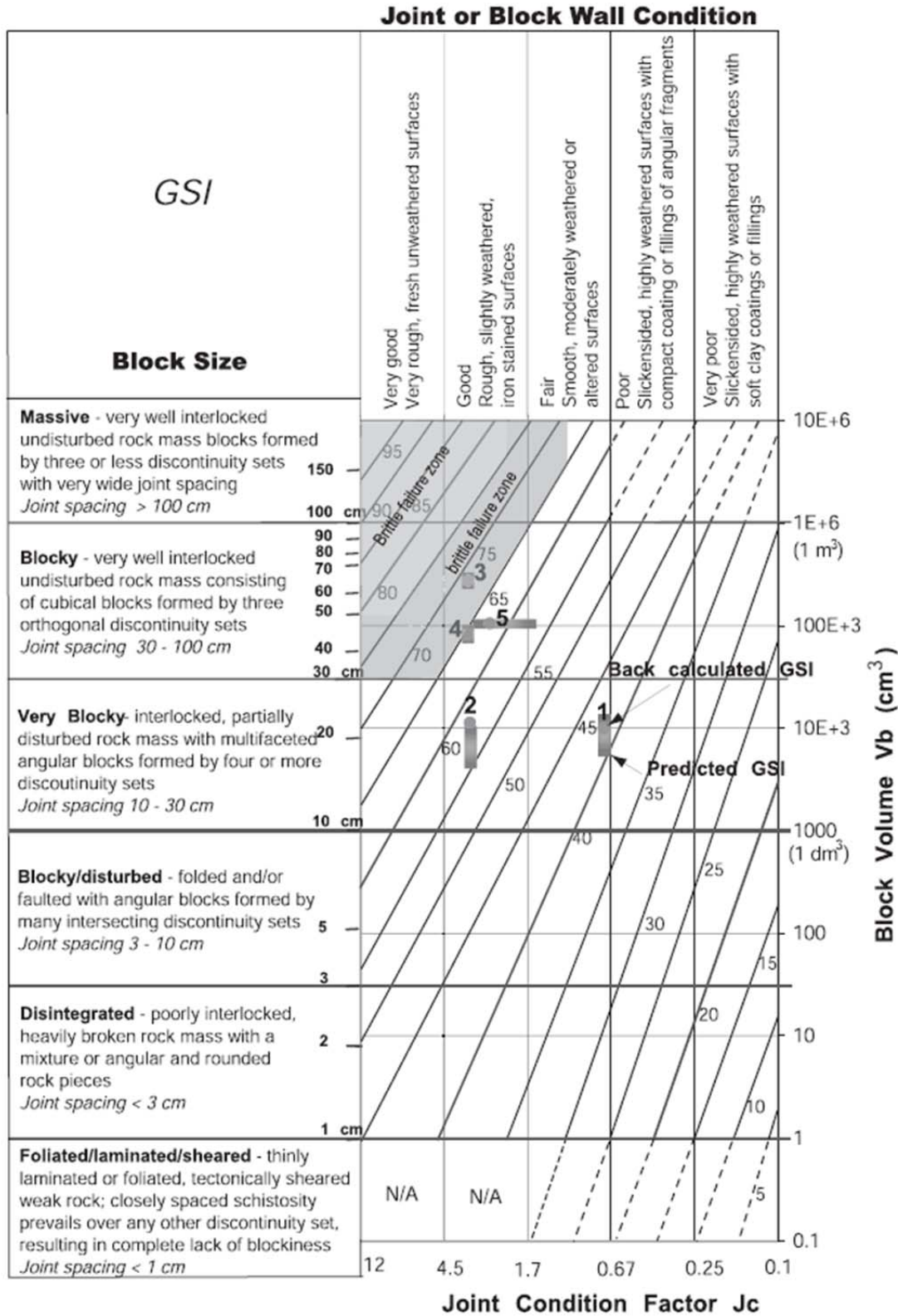


Figure 5. Chart for Geological Strength Index (GSI) Determination (after M. Cai, P.K. Kaiser, H. Uno, Y. Tasaka, M. Minami, International Journal of Rock Mechanics & Mining Sciences 41, 2004, p3-19)

Joint Wall Compressive Strength: Measure of how the strength of the joint wall compares to the intact strength of the rock and is based on the UCS intact hardness grade.

Rock Quality Designation (RQD):

$$RQD = \frac{\sum \text{Length of core pieces} > 10 \text{ cm}}{\text{Total length of core run}} \times 100\%$$

Fracture Intercept (m): Fracture intercept is the average length between fractures in meters.

Natural Discontinuity No.: Number of discontinuities per drilling interval. When fault infill or broken rock zones were encountered, 1 extra discontinuity for each 10mm was counted. When intervals were found to have closely spaced discontinuities too numerous to count, (discs of core or rubble) the length of the zone was divided by the average particle size to obtain the number of discontinuities, for example a 30 cm zone with average 6 cm diameter rock fragments would add 5.

Rock Mass rating (RMR, Bieniawski, 1976):

The processing of the data highlighted two issues arising within the fractured and weathered rock intervals were the drilling process affected the recoveries: the fracture count became less consistent with the rest of the data and Rock Mass Strength values were being assigned to drilling intervals that were partly unknown. To support the reporting accuracy, RMR values were only calculated for drilling intervals with a core recovery greater than 50%.

Cut Slopes and Outcrops:

Slope Angle: average inclination angle to a horizontal plane.

Slope Direction: average dipping direction of the face of the cut slope or outcrop.

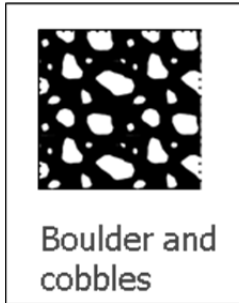
Dip: inclination angle of a rock structural set along the steepest line to a horizontal plane.

Dip direction: azimuth of the dip line, clockwise from North, local declination taken into account (23.5°E).

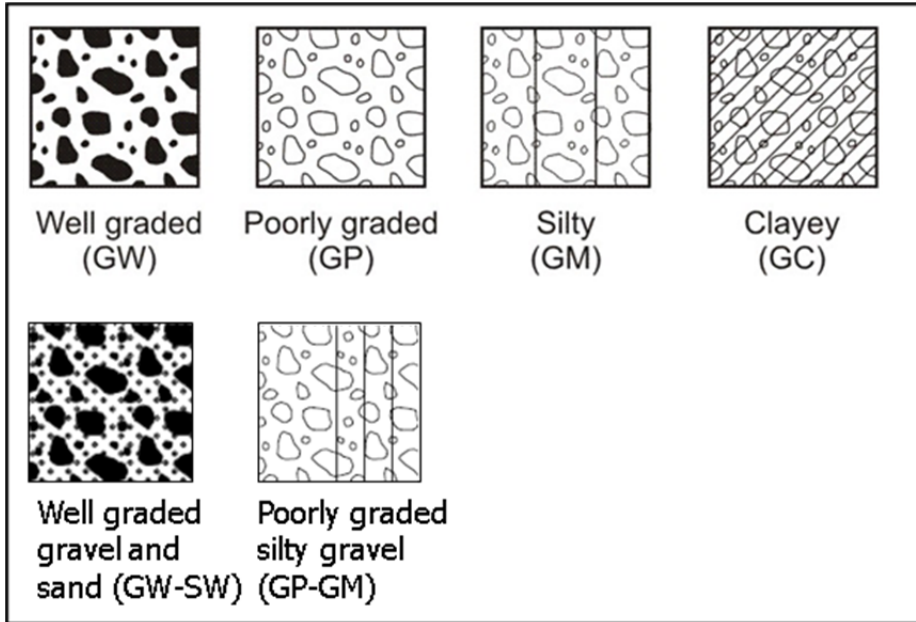
GRAPHIC SYMBOLS LEGEND FOR TEST HOLE LOGS

SOIL TYPES – COARSE-GRAINED

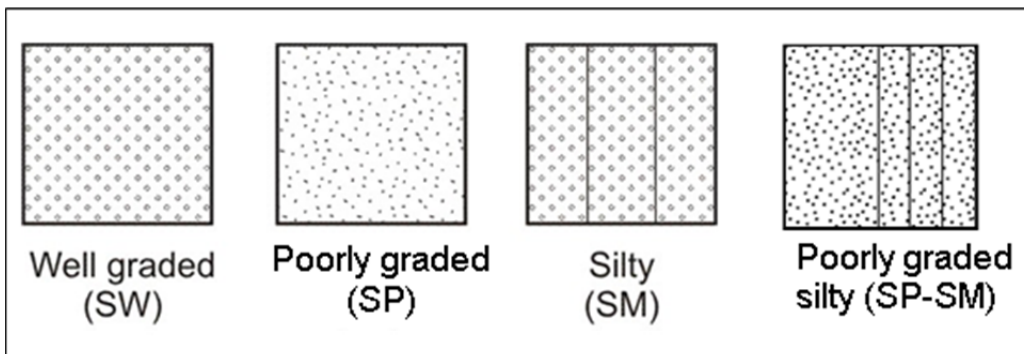
Boulders and cobbles



Gravel

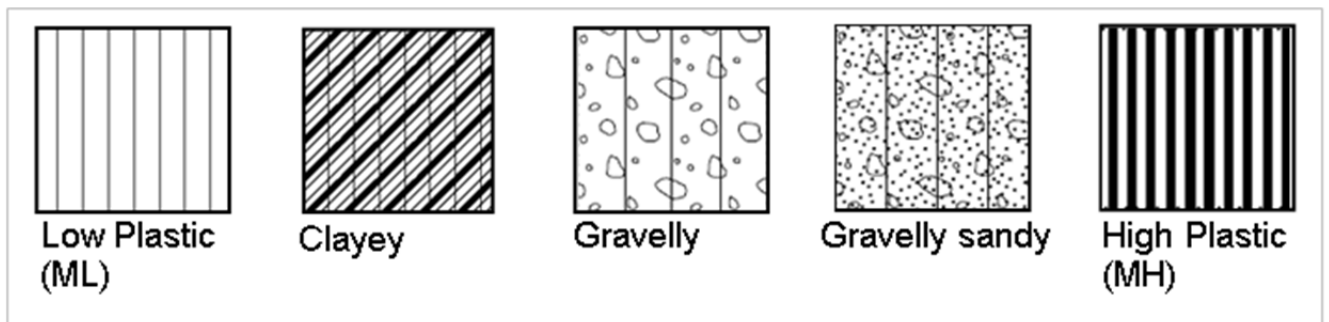


Sand

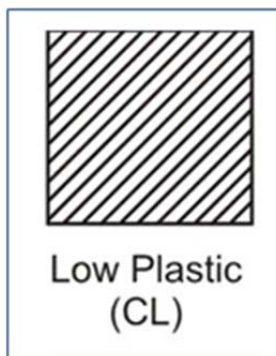


SOIL TYPES – FINE-GRAINED

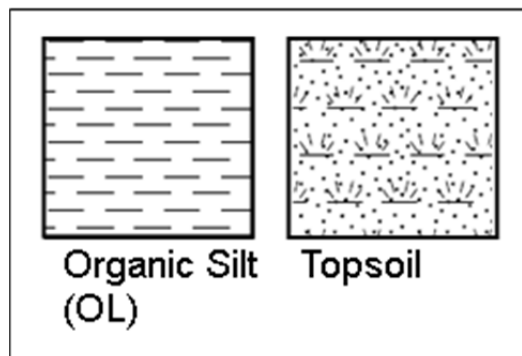
Silt



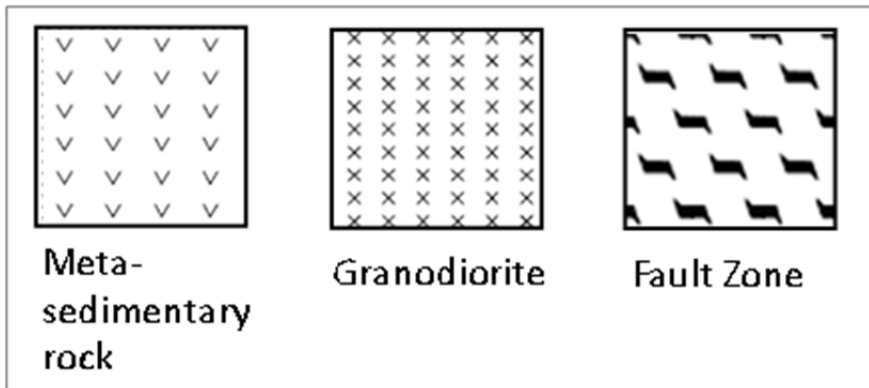
Clay



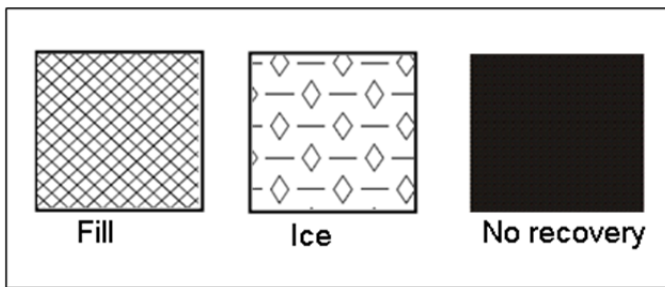
Organics



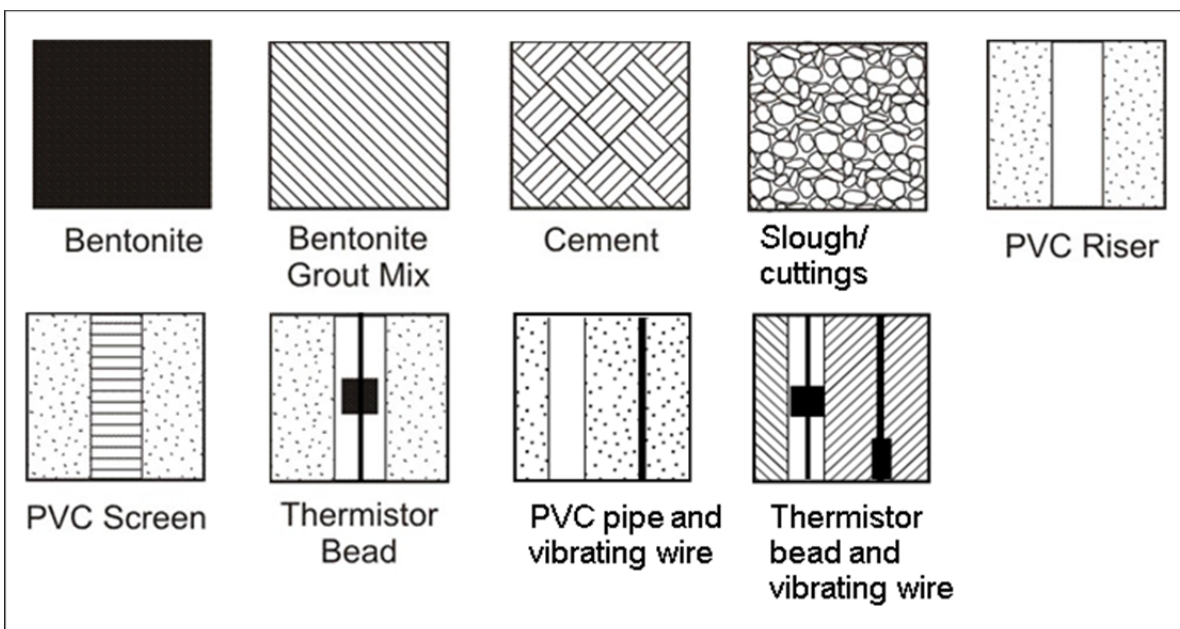
ROCK TYPES



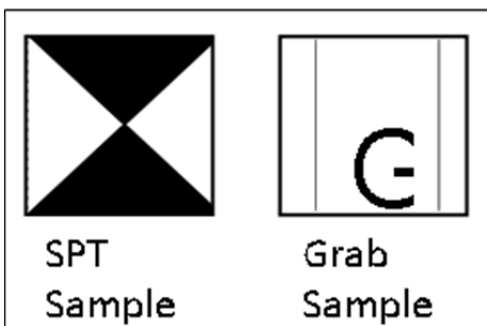
OTHERS



INSTALLATIONS



SAMPLES



APPENDIX B OUTCROP MAPPING

OUTCROP MAPPING

Table B- 1: Summary of Outcrop Maps

Outcrop	Northing (m)	Easting (m)	Elevation (masl)	RockType	UCS Rock Strength (R) ³	Weathering Grade (W) ⁴	Avgerage Block Size (m) ⁴	Average GSI ⁵
OC-BGC11-01	7099238	459344	954	Intrusive	3	2.5	0.3	45
OC-BGC11-02	7099143	459342	1067	Intrusive	3.5	2	0.2	45
OC-BGC11-03	7099041	459553	1047	Quartzite	4.5	1.5	0.1	40
OC-BGC11-04	7099026	459019	1036	Quartzite	4	1.5	0.5	50
OC-BGC11-05	7099094	459678	1053	Intrusive	4.5	2	0.5	65
OC-BGC11-06	7099075	459709	1054	Quartzite	4	2	0.3	55
OC-BGC11-07	7102198	460361	1192	Quartzite	4	2	0.07	40
OC-BGC11-08	7102198	460361	1191	Quartzite	3.5	2	0.07	40
OC-BGC11-09	7102205	460436	1200	Quartzite	--	--	0.2	50
OC-BGC11-10	7099875	460024	1146	Quartzite	2	2	0.2	45
OC-BGC11-11	7099815	460157	1198	Intrusive	4.5	1.5	0.3	60
OC-BGC11-12	7099828	460151	1188	Intrusive	4.5	1.5	0.2	55
OC-BGC11-13	7099824	460176	1195	Intrusive	3	2	0.4	60
OC-BGC11-14	7101532	461754	1206	Intrusive	4.5	2	0.25	55
OC-BGC11-15	7101570	461526	1151	Quartzite	2	4	0.05	28.5
OC-BGC11-16	7099341	460701	1406	Quartzite	5	2	0.6	65
OC-BGC11-17	7099403	460865	1403	Quartzite	5	2	0.4	65

Outcrop	Northing (m)	Easting (m)	Elevation (masl)	RockType	UCS Rock Strength (R) ³	Weathering Grade (W) ⁴	Average Block Size (m) ⁴	Average GSI ⁵
OC-BGC11-18	7099473	460332	1303	Quartzite	4.5	2	0.25	50
OC-BGC11-19	7099996	459794	1068	Quartzite	3	3	0.15	37.5
OC-BGC11-20	7100213	460060	1105	Quartzite	3	3	0.1	37.5
OC-BGC11-21	7099879	459852	1091	Quartzite	4	2	0.15	45
OC-BGC11-22	7099590	459880	1160	Intrusive	6	2	0.15	55
OC-BGC11-23	7100289	460110	1111	Quartzite	3.5	1.5	0.08	40
OC-BGC11-24	7100089	460221	1069	Intrusive	3	2	0.5	55
OC-BGC11-25	7100350	460142	1033	Quartzite	4.5	2	0.2	45
OC-BGC11-26	7099104	459656	1045	Quartzite	3	2	0.1	40
OC-BGC11-27	7099534	459343	971	Quartzite	3	2	0.07	42.5
OC-BGC11-28	7099610	459302	948	Quartzite	3.5	2	0.1	45
				Quartzite	2	2.5	0.03	35
OC-BGC11-29	7099512	459444	983	Quartzite	2.5	2.5	0.03	32
OC-BGC11-30	7099467	459387	947	Intrusive	4	2	0.2	55
				Intrusive	4	2	0.2	55
OC-BGC11-31	7099768	459788	1087	Quartzite	3.5	1.5	0.1	50
OC-BGC11-32	7099753	460179	1176	Intrusive	4.5	1.5	0.3	55
OC-BGC11-33	7099961	460143	1160	Quartzite	3.5	2	0.08	40
				Intrusive	3.5	2	0.15	55
OC-BGC11-34A	7099344	460193	1266	Quartzite	4.5	1.5	0.2	55
OC-BGC11-34B	7099344	460193	1266	Quartzite	4.5	1.5	0.2	55
OC-BGC11-35	7100175	459768	1023	Quartzite	3.5	2	0.15	50

Outcrop	Northing (m)	Easting (m)	Elevation (masl)	RockType	UCS Rock Strength (R) ³	Weathering Grade (W) ⁴	Average Block Size (m) ⁴	Average GSI ⁵
OC-BGC11-36	7100272	459698	1009	Quartzite	3	2	0.1	45
OC-BGC11-37	7100383	460067	1052	Quartzite	3.5	2	0.25	52
OC-BGC11-38	7100968	460124	997	Quartzite	4	1.5	0.2	55
OC-BGC11-39	7100395	460382	1057	Quartzite	4.5	2	0.2	55
OC-BGC11-40	7101809	459981	1051	Phyllite	1	2.5	0.005	20
				Quartzite	2	2	0.05	35
OC-BGC11-41	7102064	459915	1048	Phyllite	0.5	3	0.005	15
				Quartzite	3	2	0.1	40
OC-BGC11-42	7101893	459188	1072	Phyllite	1	2	0.02	15
OC-BGC11-43	7101403	459686	913	Phyllite	2	2.5	0.05	30
OC-BGC11-44	7101327	459852	895	Quartzite	2.5	2	0.07	35
OC-BGC11-45	7101136	459318	848	Quartzite	3.5	2	0.07	43
OC-BGC11-46	7101078	459173	839	Quartzite	4	1.5	0.1	50
OC-BGC11-47	7100272	459725	993	Quartzite	2	3	0.05	30
OC-BGC11-48A	7101009	458635	807	Quartzite	3	2	0.05	35
OC-BGC11-48B	7101022	458616	805	Quartzite	3	2	0.1	45
OC-BGC11-49	7101020	459452	859	Soil	--	--	--	--
OC-BGC11-50	7101140	458828	859	Phyllite	2	2.5	0.02	25
OC-BGC11-51	7101187	458932	885	Quartzite	0.5	4	0.005	--
OC-BGC11-52	7101039	459097	849	Phyllite	2	3	0.05	25
OC-BGC11-53	7099996	459359	1009	Quartzite	4	1.5	0.15	55

Outcrop	Northing (m)	Easting (m)	Elevation (masl)	RockType	UCS Rock Strength (R) ³	Weathering Grade (W) ⁴	Avgerage Block Size (m) ⁴	Average GSI ⁵
OC-BGC11-54	7099871	460694	1288	Intrusive	5.5	1.5	0.7	65
OC-BGC11-55A	7100389	460728	1248	Intrusive	4.5	1.5	0.5	60
OC-BGC11-55B	7100373	460719	1239	Intrusive	4.5	1.5	0.4	60

NOTES:

1. Location from handheld GPS measurements.
2. Coordinates in UTM NAD 83, Zone 8N.
3. Unconfined Compressive Strength / Hardness Classification for Rock (after CFEM 2006) - Appendix A
4. See Appendix A.
5. Geological Strength Index - See Appendix A.

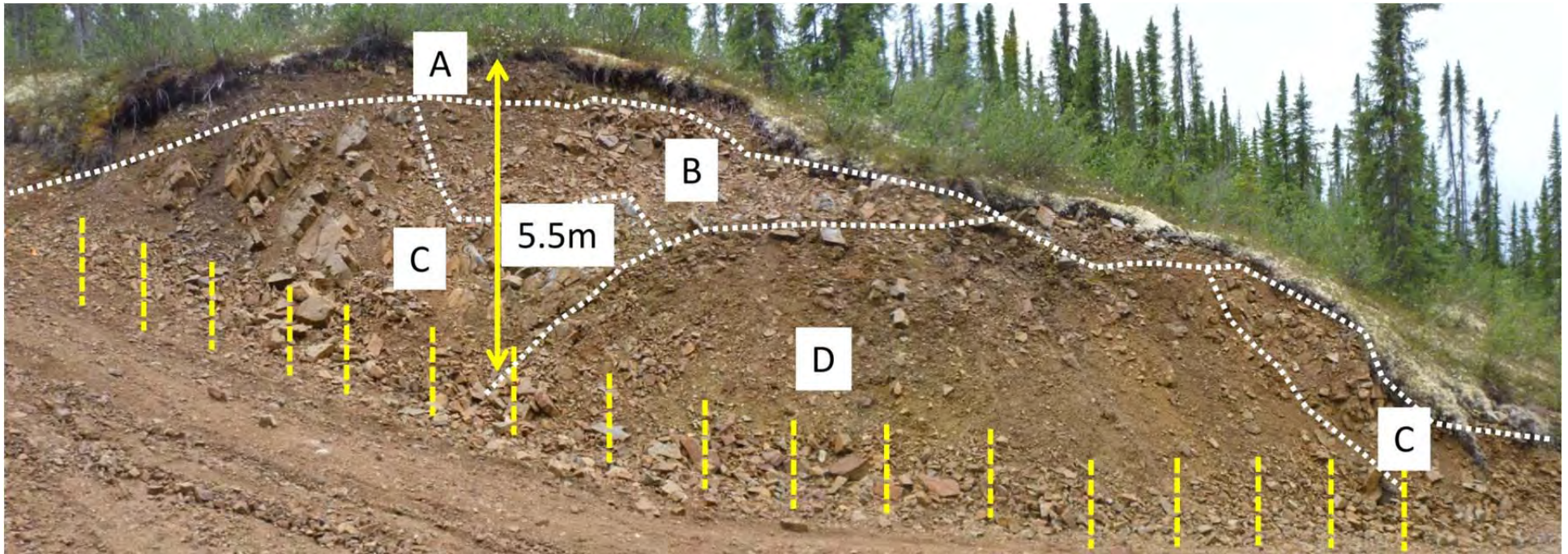
Outcrop OC-BGC11-01

Location: Platinum Gulch
Facility: Platinum WRSA
Outcrop type: Man-made (Road cut)

Northing: 7099238
Easting: 459344
Elevation: 954 m
Survey type: Handheld GPS

Slope Angle:
Slope Direction:
Date logged: 6/26/2011
Logged by: LGT/SP

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: GRAVEL: Sandy, some cobbles and trace silt (GW). Brown, moist, loose, nonplastic, and homogeneous. Clasts are granodiorite, R2-R3, angular, equidimensional to elongated, max particle size = 120 mm. [COLLUVIUM]

B: Pinkish brown, medium- to coarse-grained GRANODIORITE. Slightly to moderately weathered and iron-stained (W2.5), medium strong (R3). Blocks are polyhedral. GSI structure = blocky/disturbed, GSI surface = Good to Fair, GSI range = 40-50.

C: Pinkish grey, medium- to coarse-grained GRANODIORITE. Faintly weathered and iron-stained (W1.5), medium strong (R3). Blocks are rhombohedral.

D: Outcrop obscured by ravelled colluvium.

Outcrop OC-BGC11-01

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT C

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	34	232	1		6-15		0.15	2	J
2	48	82	1		6-15		0.15	2	J
3	50-75	314-344	1		6-15		0.15	2	J

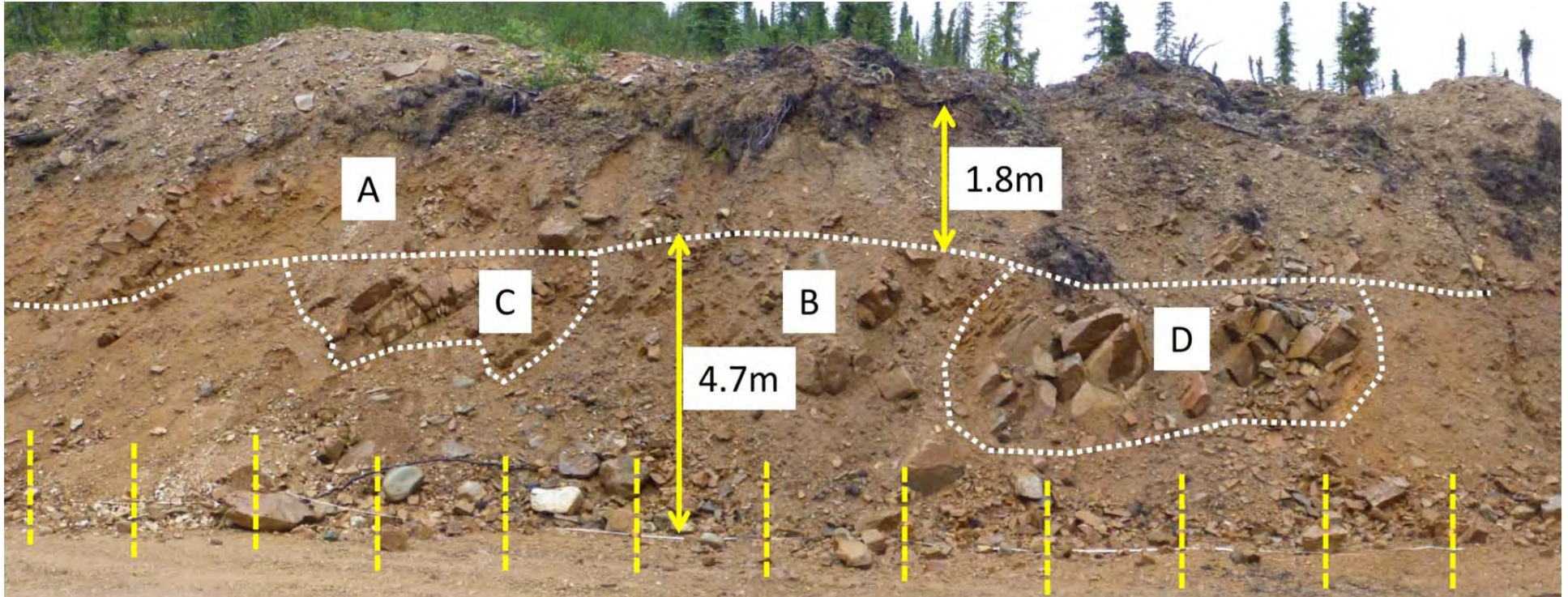
Outcrop OC-BGC11-02

Location: Platinum Gulch
Facility: Platinum WRSA
Outcrop type: Man-made (Road cut)

Northing: 7099143
Easting: 459342
Elevation: 1067 m
Survey type: Handheld GPS

Slope Angle:
Slope Direction:
Date logged: 6/27/2011
Logged by: SP/LGT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: GRAVEL: Sandy, some cobbles, well-graded (GP-SW). Non-plastic, brown, moist, loose, homogeneous. Sand is well-graded, fine to medium. Clasts are granodiorite and metasediments, R3-R4, angular, equidimensional to elongated, max particle size = 80 mm. [COLLUVIUM]

B: Ravelled COLLUVIUM covering face of outcrop.

C: Brownish grey medium- to coarse-grained GRANODIORITE. Slightly to moderately weathered (W2-2.5), weak to medium strong (R2-R3). Blocks are rhombohedral. GSI structure = Blocky/disturbed, GSI surface = Good, GSI range = 35-45.

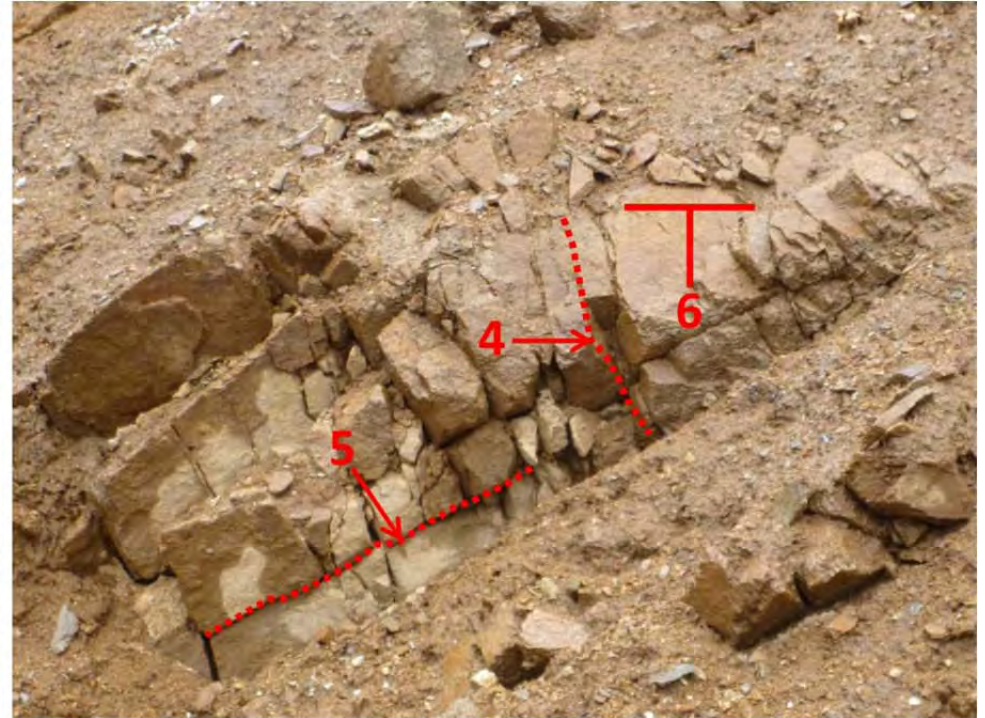
D: Medium- to coarse-grained orangish grey GRANODIORITE. Slightly weathered (W2), strong (R4). Blocks are rhombohedral, ~40 cm. GSI structure = Blocky, GSI surface = Good, GSI range = 50-60.

Outcrop OC-BGC11-02

PHOTO OF STRUCTURE – UNIT D



PHOTO OF STRUCTURE – UNIT C



DISCONTINUITY TABLE – UNITS C/D

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	51	34	2	SM-SA	4	3	0.3	>1.5m	J
2	42	59	2	SM-SA	4	3	0.4	>3.5m	J
3	68	144	2	SM-SA	5	3	0.4	>1.5m	J
4	29	76	2	SM-SA	14	3	0.1	>1m	J
5	88	333	3	SM-SA	14	3	0.1	>4m	J
6	54	280	3	SM-SA	14	3	0.1	>0.4m	J

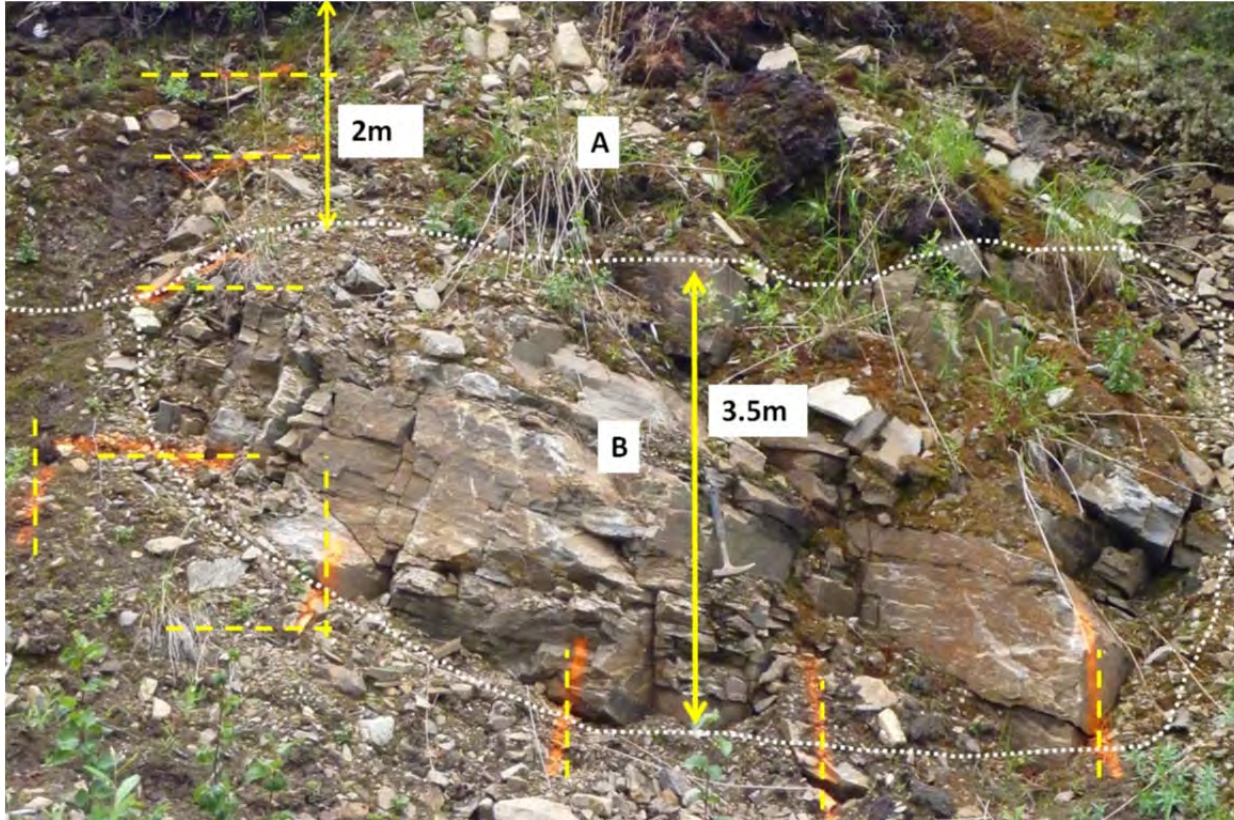
Outcrop OC-BGC11-03

Location: Platinum Gulch
Facility: Platinum WRSA
Outcrop type: Man-made (old road cut)

Northing: 7099041
Easting: 459553
Elevation: 1047 m
Survey type: Handheld GPS

Slope Angle:
Slope Direction:
Date logged: 6/28/2011
Logged by: LGT/SP

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: SAND: Gravelly, some cobbles, trace silt, some boulders, well-graded (SW-GW). Non plastic, brown, moist, loose, homogeneous Clasts are metasedimentary with trace granodiorite, angular, elongated, medium strong (R3-R4); max particle size is 300 mm. [COLLUVIUM]

B: Brownish grey, fine to medium grained QUARTZITE. Foliated, faintly weathered (W1.5), strong to very strong (R4-5). Blocks are tabular to rhombohedral. GSI structure = Very Blocky to Blocky/disturbed, GSI surface = Fair, GSI range = 35-45.

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Outcrop OC-BGC11-03

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	20	238	5	SM-SA-FE	6	4	0.1	>3m	JO
2	65	30	1	NV	12	4	0.4	>1.5m	J
3	83	168	4	SM-SA-FE	6	4	0.45	>0.5m	J

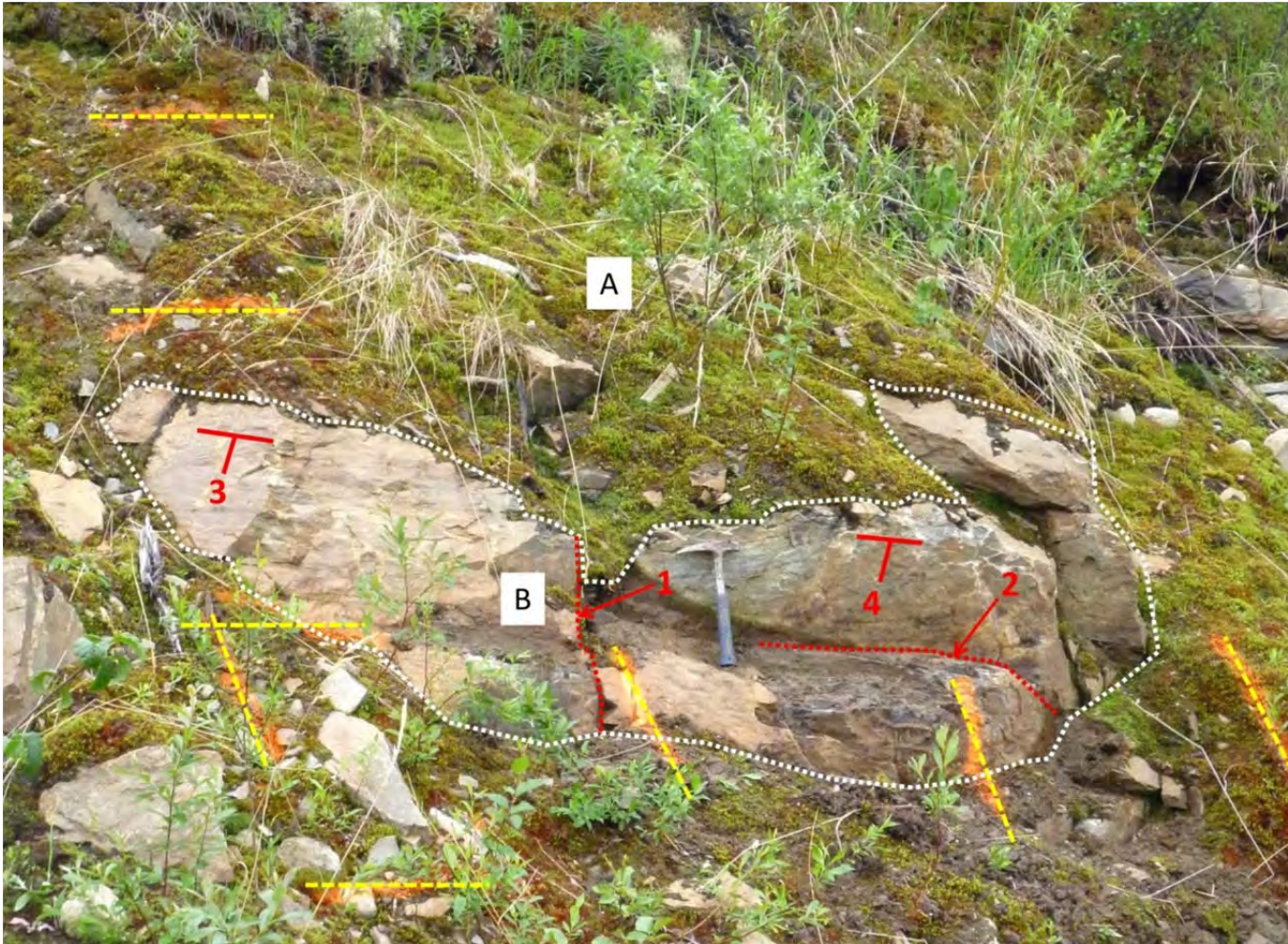
Outcrop OC-BGC11-04

Location: South of Platinum Gulch
Facility: Platinum WRSA
Outcrop type: Man-made (Old road cut)

Northing: 7099026
Easting: 459019
Elevation: 1036 m
Survey type: Handheld GPS

Slope Angle: 50°
Slope Direction: 225°
Date logged: 6/28/2011
Logged by: LGT/SP

PHOTO OF OUTCROP/STRUCTURE



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-04

DESCRIPTION OF MATERIALS

A: SAND: Gravelly, some cobbles, trace silt (GW-SW). Non plastic, brown, moist to wet, and homogeneous. Clasts are metasedimentary with trace granodiorite, angular, elongated, medium strong (R3-R4), slightly weathered (W2). [COLLUVIUM]

B: Brownish grey, fine- to medium-grained QUARTZITE. Strong (R4), faintly to slightly weathered (W1.5-W2). Discontinuities moderately to widely spaced. GSI structure = massive, GSI surface = Good to Fair, GSI range = 55-65.

DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	84	266	5	CX-SM-FE	14	4	1.2	>1m	J
2	10	308	NV	NV	8 (1m)	4	NV	>1m	J
3	50	10	NV	NV	8 (1m)	4	NV	>0.4m	J
4	88	317	0.5	SA-SM	6 (0.4m)	4	0.35	>0.7m	J

Outcrop OC-BGC11-05

Location: Platinum Gulch
Facility: Platinum WRSA
Outcrop type: Natural

Northing: 7099094
Easting: 459678
Elevation: 1053 m
Survey type: Handheld GPS

Slope Angle: 65°
Slope Direction: 215°
Date logged: 6/28/2011
Logged by: LGT/SP

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: GRAVEL: medium to coarse, sandy, some cobbles, trace silt, and trace boulders (GW). Sand is orange-brown, and moist. Clasts are angular, elongated, medium strong to strong (R3-R4), slightly weathered (W2). Max particle size= 350 mm. [COLLUVIUM]

B: Pinkish-brown, medium- to coarse-grained GRANODIORITE. Strong to very strong (R4-5), slightly to moderately weathered (W1.5-W2). Blocks are rhomboidal to tabular. GSI structure = Blocky, GSI surface = Good, GSI range = 60-70. Some evidence of contact with metasedimentary rock above the outcrop.

Outcrop OC-BGC11-05

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	26	3	50	FE	6 (1m)	4	0.5	>1.5	J
2	54	125	20	FE	6 (1m)	4	0.4	>1.5	J

Outcrop OC-BGC11-06

Location: Platinum Gulch
Facility: Platinum WRSA
Outcrop type: Natural

Northing: 7099075
Easting: 459709
Elevation: 1054 m
Survey type: Handheld GPS

Slope Angle: 46°
Slope Direction: 200°
Date logged: 6/28/2011
Logged by: SP/LGT

PHOTO OF OUTCROP



DESCRIPTION OF MATERIALS

A (3 to 10 m above base of outcrop): Grey to pinkish grey, fine- to medium-grained QUARTZITE. Faintly to slightly weathered (W1.5-W2), Strong (R4). Blocks are rhomboidal to tabular. GSI structure = Blocky, GSI surface = Good, GSI range = 55-65.

B (10 to 41 m above base of outcrop): Grey to pinkish grey, fine- to medium-grained QUARTZITE. Slightly weathered (W2), strong (R4). GSI structure = Very Blocky, GSI surface = Good to Fair, GSI range = 45-55.

Outcrop OC-BGC11-06

PHOTOS OF STRUCTURE, UNITS A/B (NUMBERS ABOVE PHOTOS GIVE THEIR LOCATION IN TERMS OF HEIGHT ABOVE OUTCROP BASE)

32 to 41 m



27 to 32 m



10-18 m



3-8 m



DISCONTINUITY TABLE

Height from base of outcrop (m)	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
41	83	170			10 to 20	4	NV		J
	65	260			2 to 16	4	NV		J
27	60	65			2 to 16	4	0.2 to 1	>4m	J
	60	70			2 to 16	4	0.2 to 2	>4m	J
18	82	302			10 to 20	4	1	>4m	J
	80	180			10 to 20	4	1	>6m	J
13	86	136			10 to 20	4	NV		J
	46	255			2 to 16	4	1 to 1.4		J
7	59	210			2 to 16	4	NV		J
	72	170	2 to 40	CX	10 to 20	4	0.25 to 1.2	>4m	J
	74	290			10 to 20	4	NV		J

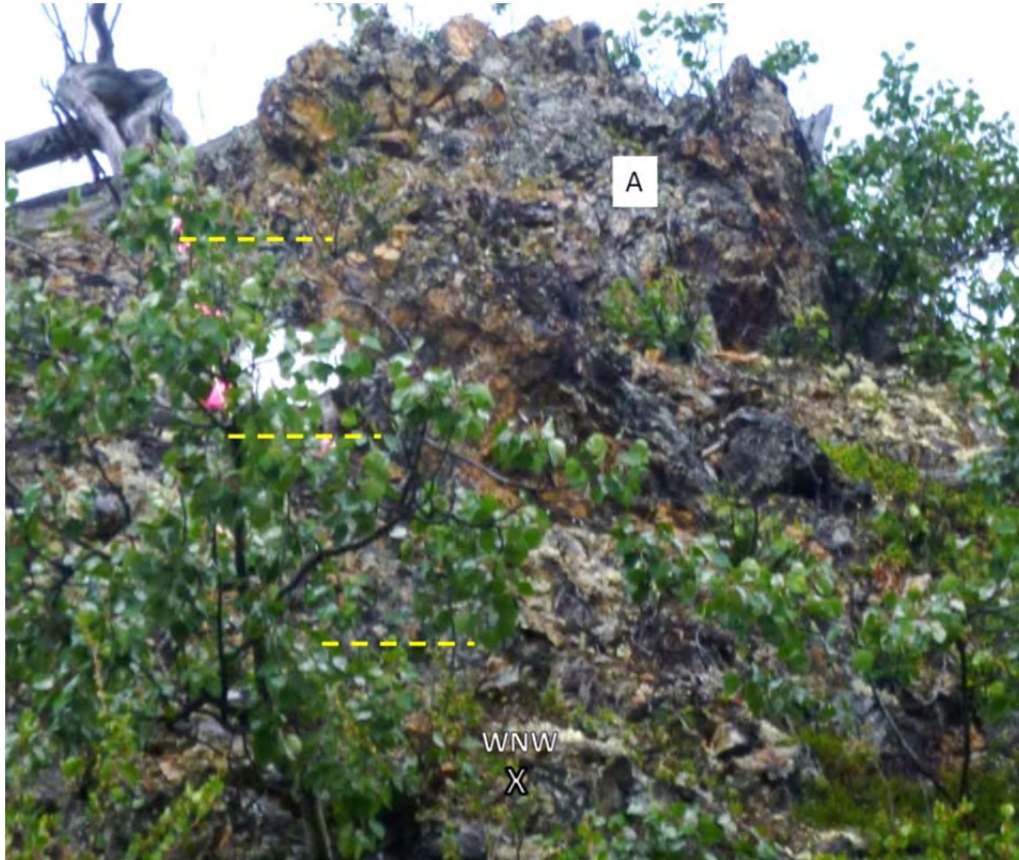
Outcrop OC-BGC11-07

Location: Ann Gulch
Facility: Heap leach pad
Outcrop type: Natural

Northing: 7102198
Easting: 460361
Elevation: 1192 m
Survey type: Handheld GPS

Slope Angle: 35°
Slope Direction: 145°
Date logged: 6/30/2011
Logged by: EB/SP/LGT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Brownish grey, fine- to medium-grained QUARTZITE. Slightly weathered (W2), moderately strong to strong (R4). Blocks are rhombohedral; discontinuities are very closely spaced. GSI structure = Blocky/disturbed, GSI surface = Good to Fair, GSI range = 35-45.

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Outcrop OC-BGC11-07

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	62	132	3-25	Bx-ML-SA	2-20 (1m)	4	0.1 to 0.4	1.5	J
2	88	280	2-10	Bx-ML-SA	2-20 (1m)	4		>1	J
3	58	210	0.2-0.5	Fe, Bx, SA	5 (0.1m)	4		>0.4	J
4	86	128	NV	NV		4			J

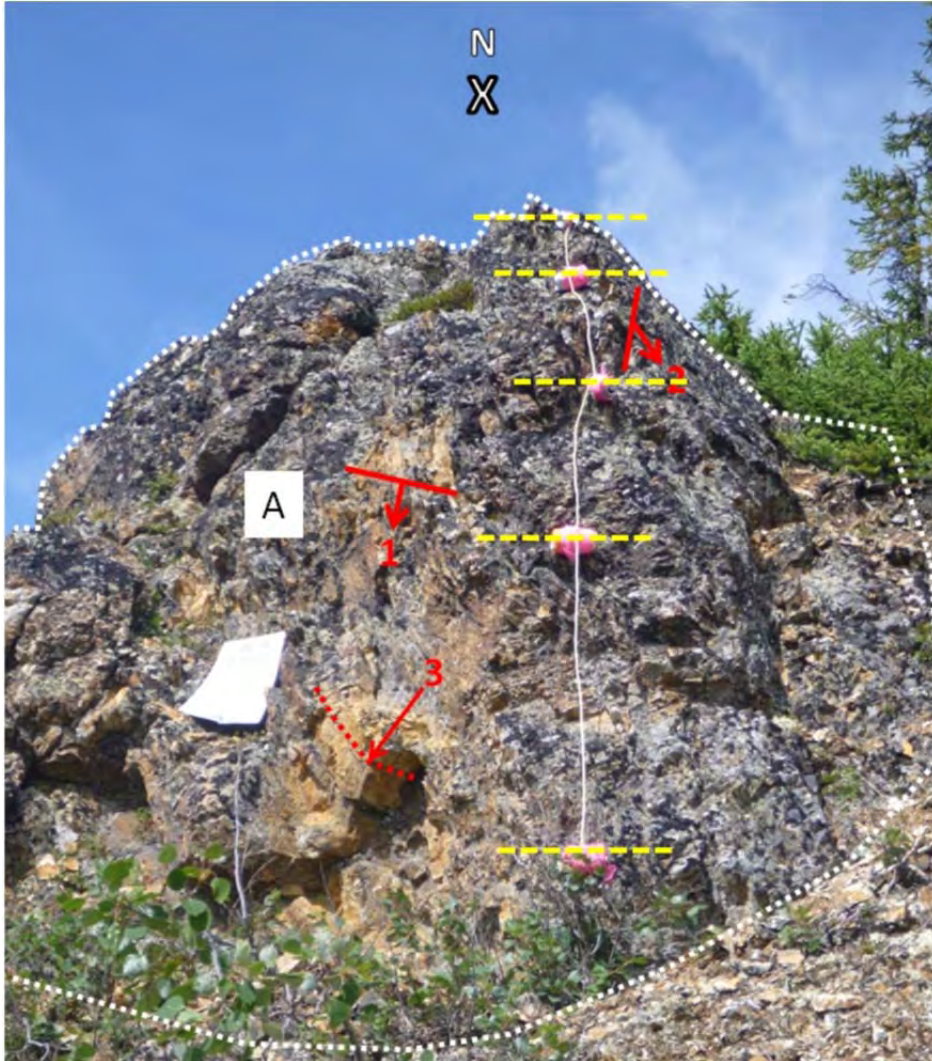
Outcrop OC-BGC11-08

Location: Ann Gulch
Facility: Heap leach pad
Outcrop type: Natural

Northing: 7102198
Easting: 460361
Elevation: 1191 m
Survey type: Handheld GPS

Slope Angle: 40°
Slope Direction: 145°
Date logged: 6/30/2011
Logged by: EB/SP/LGT

PHOTO OF OUTCROP/STRUCTURE



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-08

DESCRIPTION OF MATERIALS

A: Brownish grey, fine- to medium-grained QUARTZITE. Foliated, slightly weathered (W2), moderately strong to strong (R3 to R4). Blocks are polyhedral; discontinuities very closely spaced. GSI structure = Blocky/disturbed, GSI surface = Fair to Good, GSI range = 35-45.

DISCONTINUITY TABLE - UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	83	66	10	BX-FE	12	4	2	2.5	J
2	41	191	5	CX-FE	12	4	0.2	2	J
3	43	324	5	CX-ML-SA	10	4	0.2	0.5	J

Outcrop OC-BGC11-09

Location: Ann Gulch
 Facility: Heap leach pad
 Outcrop type: Natural

Northing: 7102205
 Easting: 460436
 Elevation: 1200 m
 Survey type: Handheld GPS

Slope Angle: 60°
 Slope Direction: 225°
 Date logged: 6/30/2011
 Logged by: EB/SP/LGT

PHOTO OF OUTCROP/STRUCTURE



1m spacing along outcrop surface between yellow dashed lines.

DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	80	202			10	4		6	J
2	25	279	15	FE-CX	4	4	0.3	1	J
3	37	346	15	FE-CX	4	4	0.3	1	J
4	85	92	90	FE-CX	10	4	0.6	3	J

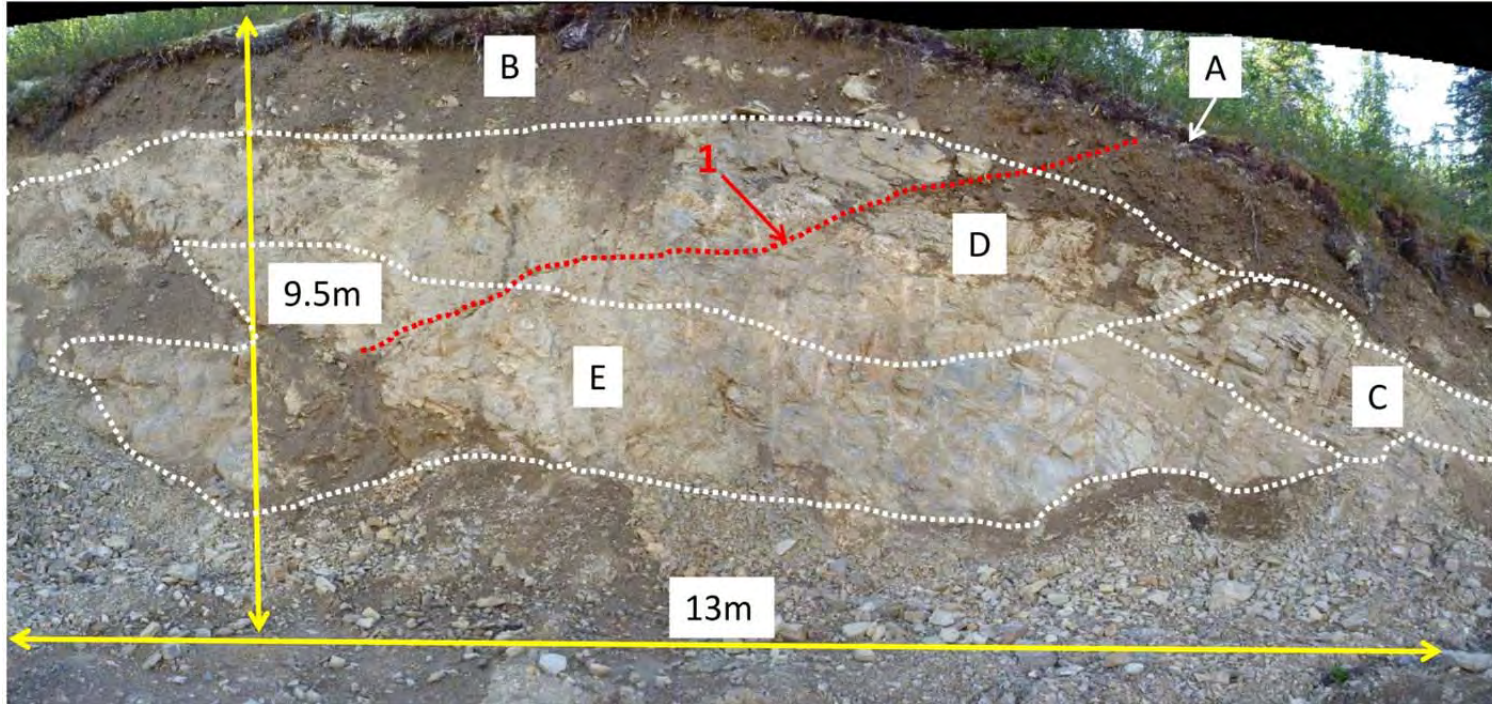
Outcrop OC-BGC11-10

Location: Pit area
Facility: Primary crusher
Outcrop type: Man-made (road cut)

Northing: 7099875
Easting: 460024
Elevation: 1146 m
Survey type: Handheld GPS

Slope Angle: 45-65°
Slope Direction: 285°
Date logged: 7/9/2011
Logged by: LGT

PHOTO OF OUTCROP



DESCRIPTION OF MATERIALS

A: Root mat and organic silt, 0.2 to 0.4m thick. [TOPSOIL]

B: SILT: Gravelly, some sand, trace cobbles and boulders (ML). Nonplastic, brown, moist, homogeneous. Maximum particle size is 300 mm. Clasts are elongated, angular, weak to strong (R2-R4). [COLLUVIUM]

C: Light brownish grey, fine-grained QUARTZITE. Faintly weathered (W1.5), very weak to weak (R1.5). Blocks are prismatic, with 3 main discontinuity sets (2,3 and 4). GSI structure = Very Blocky, GSI surface = Good to Fair, GSI range = 35-45.

D: Brownish-grey, fine-grained QUARTZITE. Slightly to moderately weathered (W2-W3), very weak (R1), extremely closely fractured. GSI structure = Blocky/disturbed, GSI surface = Good to Fair, GSI range = 30-40.

E: Light brownish-grey, fine-grained QUARTZITE. Lightly weathered (W2); moderately strong (R3). Blocks are prismatic with 4 main sets (5 to 8). GSI structure = Very Blocky, GSI surface = Good, GSI range = 45-55.

Outcrop OC-BGC11-10

PHOTO OF STRUCTURE – UNIT C



DISCONTINUITY TABLE – UNITS C/D/E

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	28	78	78	SD-CX	20	R2		7	J
2	75	73	73	CX-SM	2	R3	0.15	2	J
2	63	69	69	CX-SM-SA	4	R3	0.3	2	J
3	60	322	322	FE-SM-SA	20	R3	0.3	3	J
4	24	300	300	FE-SM-SA	16	R3	0.3	3	J
5	45	150	150	SA	20	R3	0.25	1.5	J
6	59	305	305	FE-CX-SA	6	R3	0.4	4	J
7	25	235	235	FE-SM-SA	16	R3	0.12	1.5	JO
8	65	85	85	FE-SM-SD	8	R3	0.05	2	J

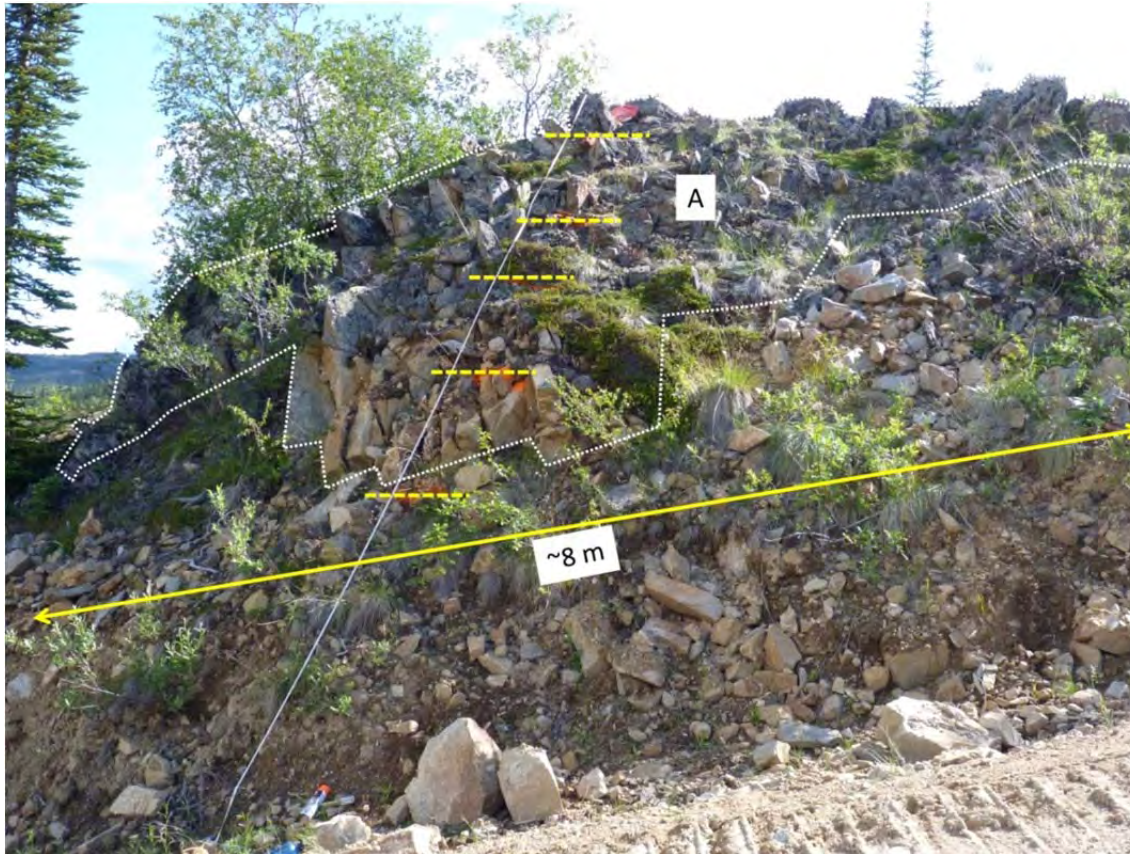
Outcrop OC-BGC11-11

Location: Pit area
Facility: Eagle Pup WRSA
Outcrop type: Natural, with road cut at its base

Northing: 7099815
Easting: 460157
Elevation: 1198 m
Survey type: Handheld GPS

Slope Angle: 35/65°
Slope Direction: 260-280°
Date logged: 7/10/2011
Logged by: LGT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Light grey, black-speckled, medium- to coarse-grained GRANODIORITE. Faintly weathered (W1.5); strong to very strong (R4 to R5), fracture spacing 5 to 40cm. Open apertures 1 to 30cm reflect dilation due to toppling. GSI structure = blocky, GSI surface = Poor, GSI range = 40-50.

Outcrop OC-BGC11-11

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	75	154	20	CX-SM-SD	8	R4	0.3	1.5	J
2	87	236	25	CX-SM-SD	12	R4	0.45	6	J
3	12	267	18	CX-SM-SD	16	R4	0.45	1.5	J
4	75	200	10	CX-SD	20	R4	0.2	7	J
5	70	128	35	CX-SM-SD	12	R4	0.3	3	J
6	222	112	20	CX-SM-SD	20	R4	0.5	0.6	J
7	52	160	30	OG	7	R4	0.2	1.5	J
8	83	303	15	CX-SM-SD	20	R4	0.3	3	J

Outcrop OC-BGC11-12

Location: Pit area
Facility: Eagle Pup WRSA
Outcrop type: Natural

Northing: 7099828
Easting: 460151
Elevation: 1188 m
Survey type: Handheld GPS

Slope Angle: 52°
Slope Direction: 305°
Date logged: 7/10/2011
Logged by: LGT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-12

DESCRIPTION OF MATERIALS

A: Light bluish grey medium- to coarse-grained GRANODIORITE. Slightly weathered (W1.5); strong to very strong (R4 to R5). Blocks are tabular, with closely to moderately spaced joints (1 main structural set). GSI structure = Very Blocky, GSI surface = Good, GSI range = 50-60.

DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	65	160	250	QZ	20	R4	0.15	3	J
2	78	29	12	BX-CX-SM-SA	16	R4	0.2	2	J
3	80	332	20	CX-SM-SA	8	R4	0.15	3	J
3	87	316	5	SM-SA-OG	4	R4	0.15	3	J
4	45	199		CX-SM-SA	16	R4	0.25	0.5	J

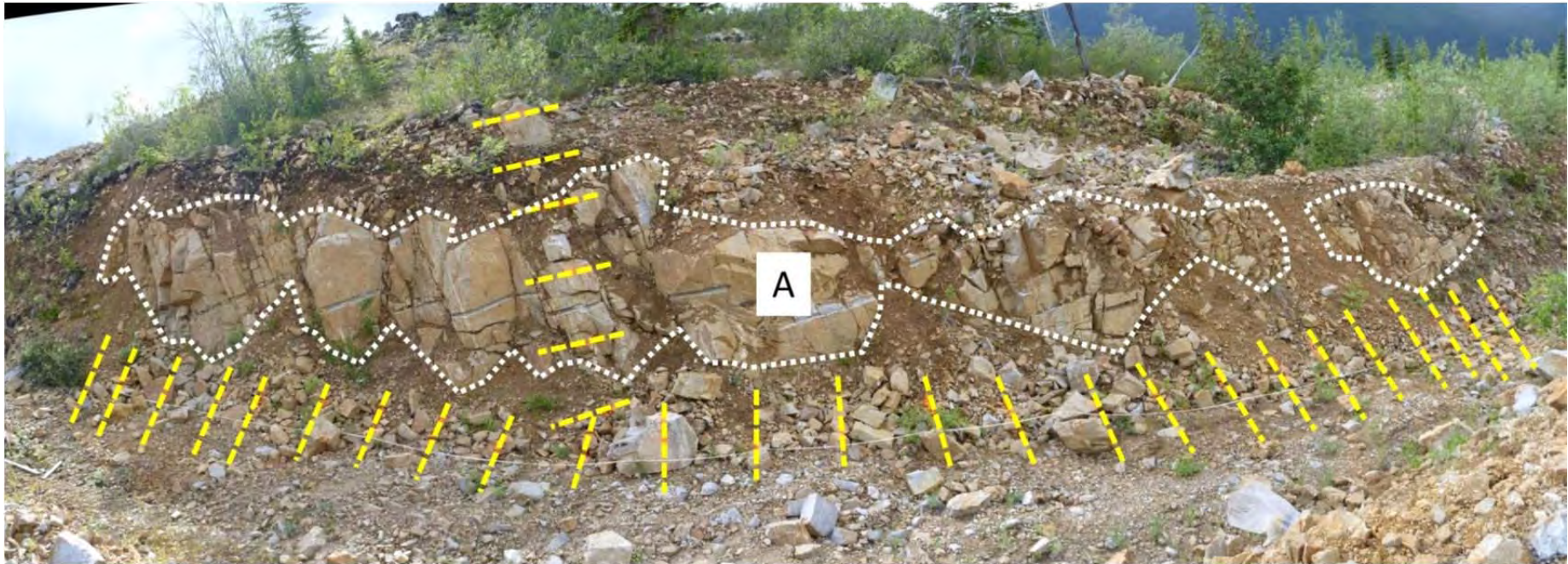
Outcrop OC-BGC11-13

Location: Pit area
 Facility: Eagle Pup WRSA
 Outcrop type: Man-made (exploration trench)

Northing: 7099824
 Easting: 460176
 Elevation: 1195 m
 Survey type: Handheld GPS

Slope Angle: 80°
 Slope Direction: 080°
 Date logged: 7/10/2011
 Logged by: LGT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Grey, medium- to coarse-grained GRANODIORITE. Slightly weathered (W2); medium strong (R3). Blocks are rhombohedral; ~ 50cm. GSI structure = Blocky, GSI surface = Good, GSI range = 55-65.

DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	80-88	230-236	4	FE-ML	4-8	R3	0.1-0.2	2.5	J
2	60-68	218-230	0-1	FE-ML	4-13	R3	0.14	0.8	J

Outcrop OC-BGC11-14

Location: Olive Gulch
Facility: Road/Explosive Storage area
Outcrop type: Man-made (old exploration road cut)

Northing: 7101532
Easting: 461754
Elevation: 1206 m
Survey type: Handheld GPS

Slope Angle: 57°
Slope Direction: 301°
Date logged: 7/21/2011
Logged by: EC/EB

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

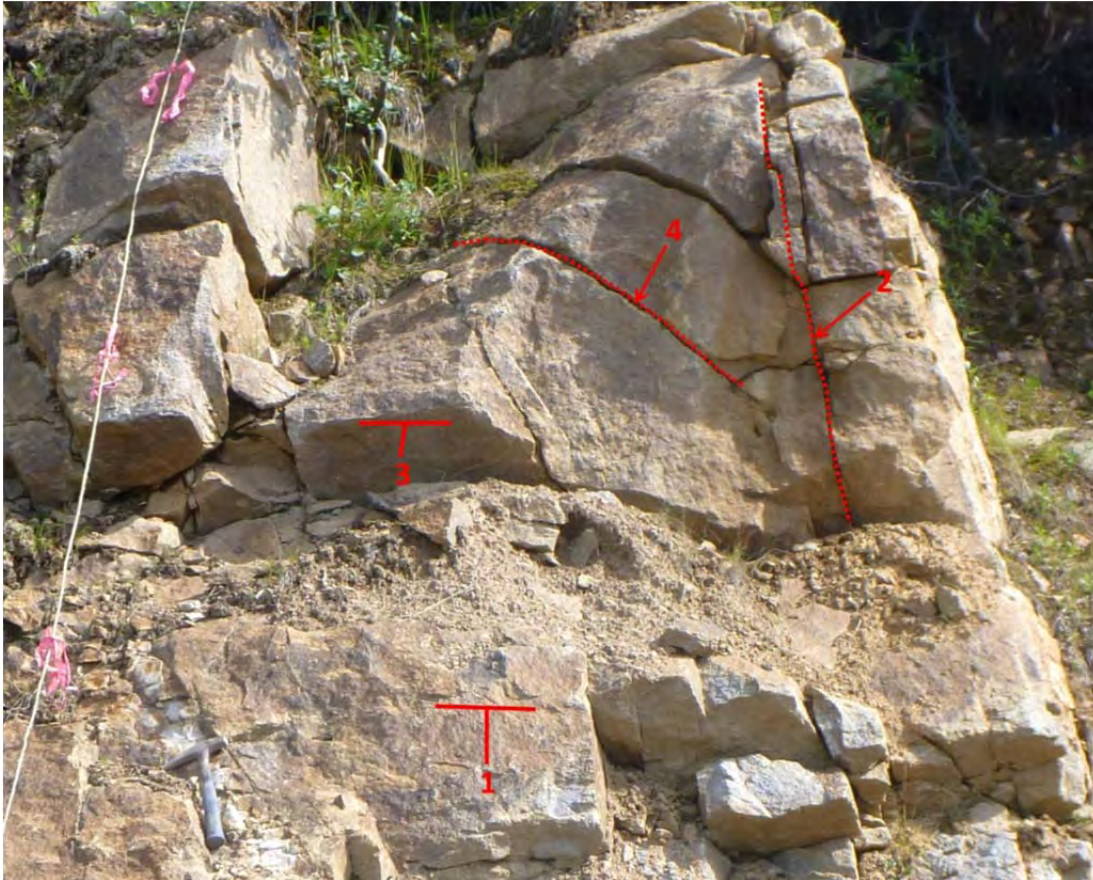
A: Root mat and organic SILT. [TOPSOIL]

B: Grey-white (black grains), coarse-grained GRANODIORITE. Lightly to moderately weathered (W1.5 - W2); strong to very strong (R4-R5). Blocks are polyhedral. GSI structure = Very Blocky, GSI surface = Good, GSI range = 50-60.

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Outcrop OC-BGC11-14

PHOTO OF STRUCTURE



DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	41-64	300-348	0.5-20	SA-GW-FE	14-18	R4	0.5	1.5	J
2	67-74	210-214	2-8	SA-GW-FE	8-16	R4	0.2	1.5	J
3	54-60	73-92	10-50	SA-GW-FE	12-16	R4	1	1.5	J
4	41-45	180-190	5-30	SA-GW-FE	6-10	R4	1	1.5	J

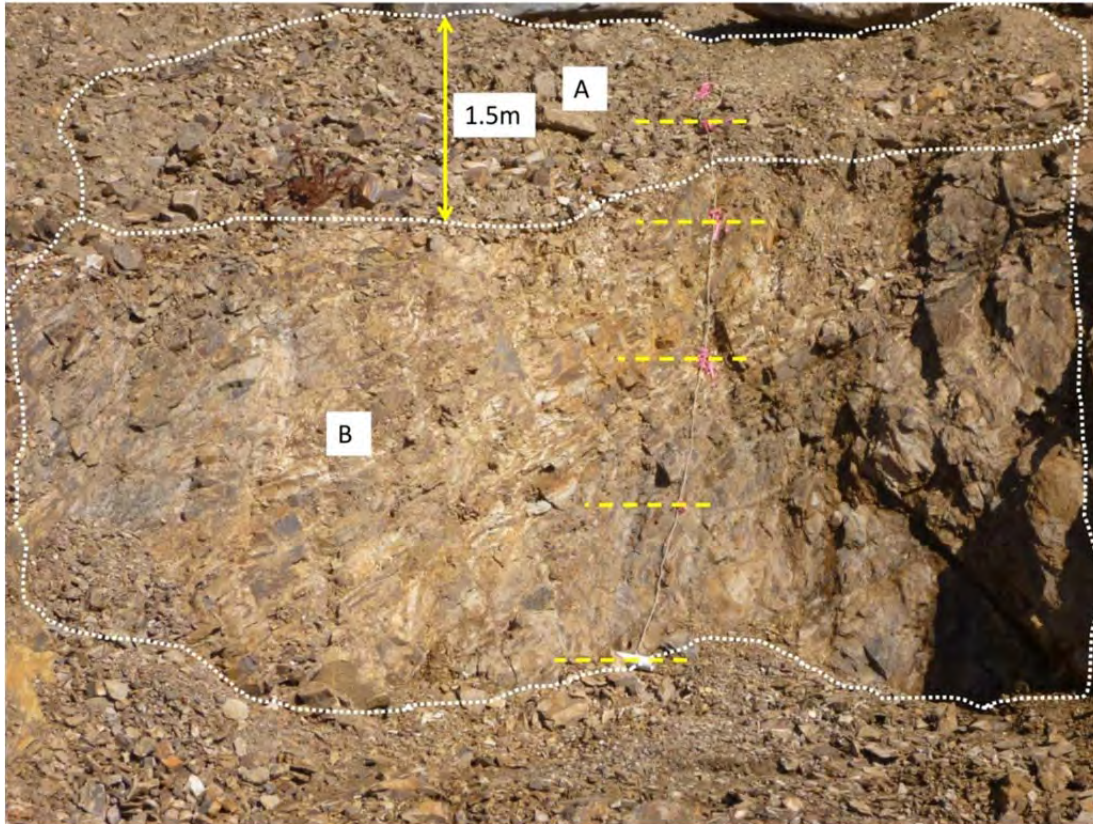
Outcrop OC-BGC11-15

Location: Olive Gulch
Facility: Road/Explosive Storage area
Outcrop type: Man-made (road cut)

Northing: 7101570
Easting: 461526
Elevation: 1151 m
Survey type: Handheld GPS

Slope Angle: 39°
Slope Direction: 054°
Date logged: 7/21/2011
Logged by: EC/EB

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: FILL, colluvium pushed over side for road. Not a natural feature.

B: Dark green to grey, fine-grained QUARTZITE. Foliated, highly weathered (W4) and heavily iron stained (some zones more so than others); weak(R1.5 to R2). Blocks equidimensional, ~ 0.05m. GSI structure = Blocky/Disturbed, GSI surface = Poor, GSI range = 22-35.

Outcrop OC-BGC11-15

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	32-42	308-325	5-10	FE-SA-SM	5-10	R1.5-R2	0.05	1	JO
2	51-67	177-182	1-3	FE-SA-SM	2-6	R1.5-R2	0.1	1	J
3	60-68	82-88	1-3	FE-SA-SM	2-6	R1.5-R2	0.15	1	J

Outcrop OC-BGC11-16

Location: Pit area
Facility: Open pit back wall
Outcrop type: Natural

Northing: 7099341
Easting: 460701
Elevation: 1406 m
Survey type: Handheld GPS

Slope Angle: 14°
Slope Direction: 217°
Date logged: 7/22/2011
Logged by: EC/EB

PHOTO OF OUTCROP



Outcrop OC-BGC11-16

DESCRIPTION OF MATERIALS

A: Sparse ORGANICS (<0.1m), mostly lichen, some moss, very dry.

B: Medium- to coarse-grained QUARTZITE. Heavily iron stained; colour is black with yellow-white lenses. Slightly weathered (W2); very strong to extremely strong (R5- R6). Blocks equidimensional, ~60 cm. GSI structure = Blocky, GSI surface = Good, GSI range = 55-75.

DISCONTINUITY TABLE – Unit B

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	16-18	253-264	NA	None	16-18	R5	0.4	>4	JO
2	72-83	251-260	15-20	SM-SA	12-16	R5	0.6	>1	J
3	79-85	157-170	20	SM	8-10	R5	0.6	>0.7	J

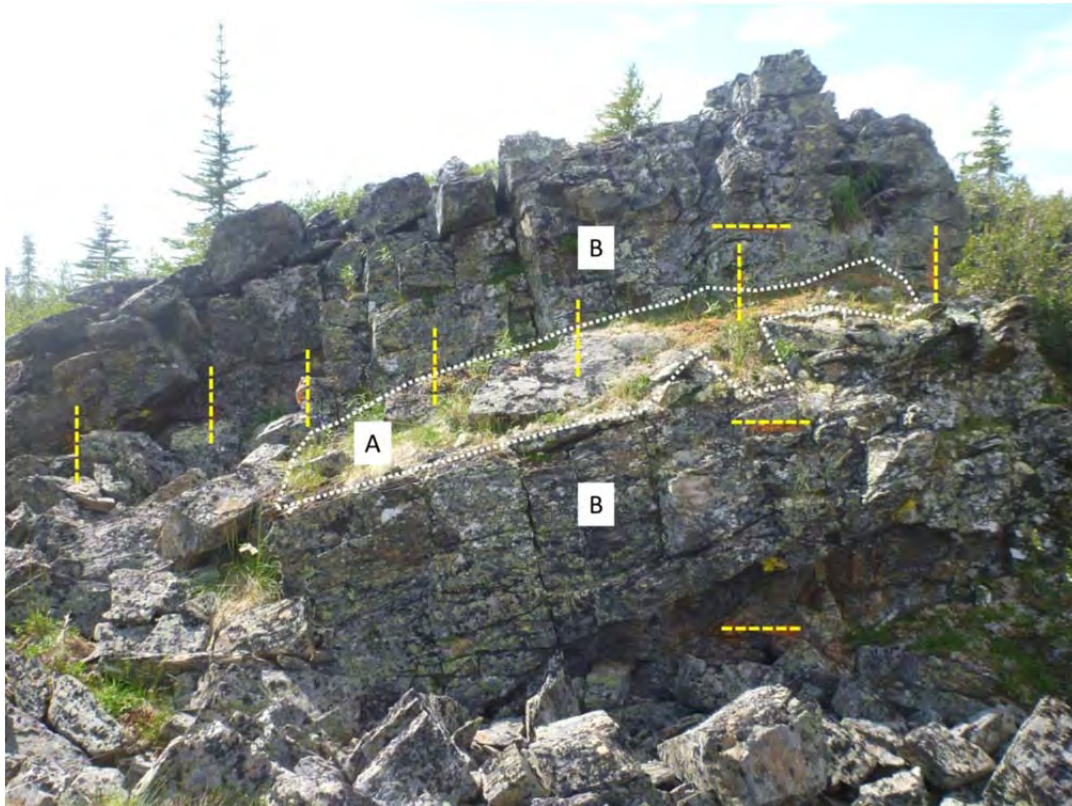
Outcrop OC-BGC11-17

Location: Eagle Pup
Facility: Open pit back wall
Outcrop type: Natural

Northing: 7099403
Easting: 460865
Elevation: 1403 m
Survey type: Handheld GPS

Slope Angle: 78°
Slope Direction: 082°
Date logged: 7/22/2011
Logged by: EC/EB

PHOTO OF OUTCROP



1m spacing between yellow dashed lines.

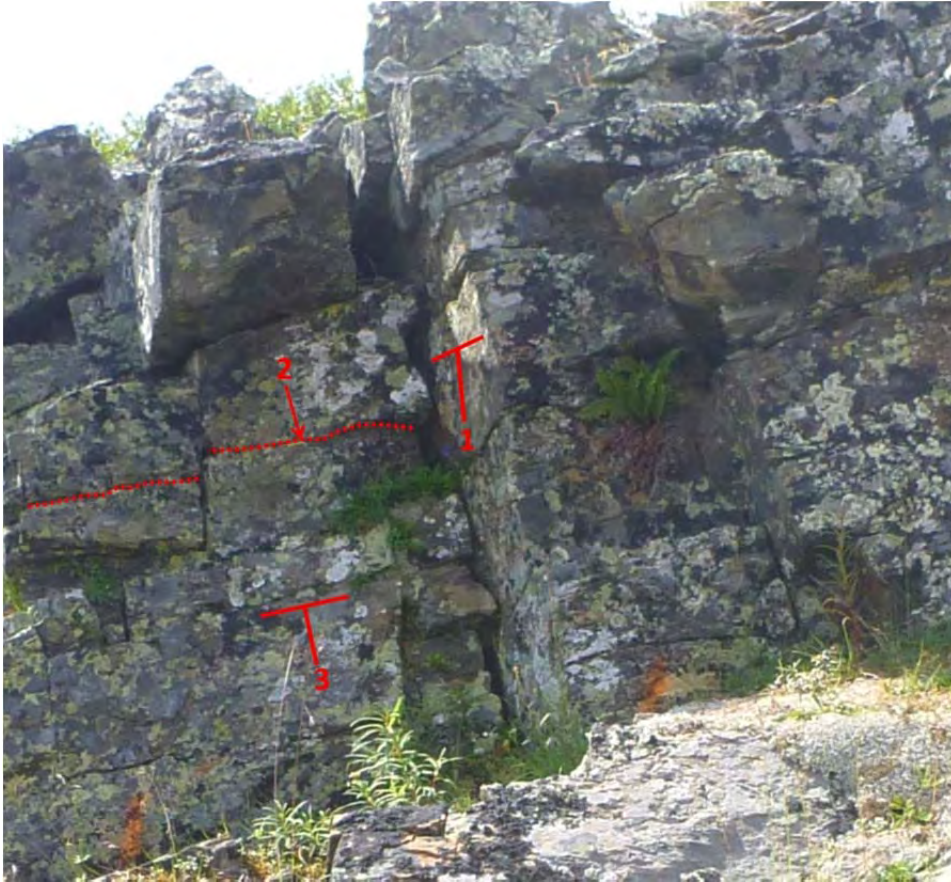
DESCRIPTION OF MATERIALS

A: Sparse ORGANICS, mostly lichen, some shrubs, some grass; very dry.

B: Medium to coarse-grained QUARTZITE. Heavily iron stained, coloured black with yellow-white lenses. Slightly weathered (W2); very strong to extremely strong (R5 to R6). Blocks equidimensional, ~ 40 cm. GSI structure = Blocky, GSI surface = Good, GSI range = 50-70.

Outcrop OC-BGC11-17

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	15-20	208-217	5-25	SM-SA	15-18	R5	0.4	3-10	JO
2	68-81	324-345	5-40	SA-SM	10-12	R5	0.3	>1	J
3	78-83	82-90	10-40	SA-SM	10-12	R5	0.4	NV	J

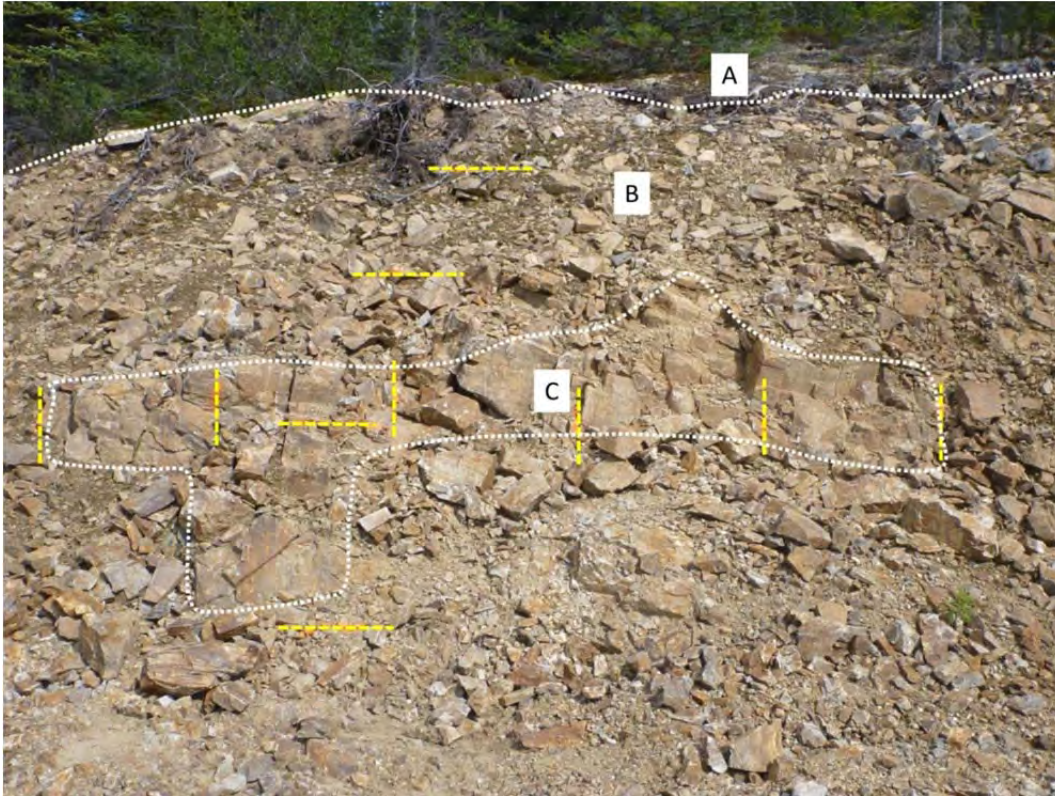
Outcrop OC-BGC11-18

Location: Pit area
Facility: Open pit
Outcrop type: Man-made (exploration drill pad cut)

Northing: 7099473
Easting: 460332
Elevation:
Survey type: Handheld GPS

Slope Angle: 34°
Slope Direction: 259°
Date logged: 7/22/2011
Logged by: EC/EB

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Vegetation, moss, root mat, 0.2 m thick. [TOPSOIL]

B: BOULDERS and COBBLES: Silty, some coarse gravel, greyish brown. Clasts equidimensional, angular, moderately weathered (W3), strong (R4); max size = 450 mm. [COLLUVIUM]

C: Grey, coarse-grained QUARTZITE. Slightly weathered (W2), strong to very strong (R4-R5). Blocks rhomboidal, ~ 25 cm. GSI structure = Very Blocky, GSI surface = Good, GSI range = 45-55.

Outcrop OC-BGC11-18

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT C

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	62-63	270-273	NV	NV	10-12	R4-R5		NA	J
2	22-35	213-226	3-15	SA-SM	8-10	R4-R5	0.2	NA	JO
3	77-81	069-080	2-20	SA-SM	8-10	R4-R5	0.3	NA	J
4	76-81	324-352	1-20	SA-SM	8-10	R4-R5	0.2	NA	J

Outcrop OC-BGC11-19

Location: Stuttle Gulch
Facility: Primary crusher
Outcrop type: Man-made (drill pad for BH-BGC11-50)

Northing: 7099996
Easting: 459794
Elevation: 1068
Survey type: Handheld GPS

Slope Angle: 38°
Slope Direction: 270°
Date logged: 7/23/2011
Logged by: EC

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Vegetation, moss, root mat. [TOPSOIL]

B: SILT (ML). Gravelly, low plasticity, low dry strength, no dilatancy, medium brown, moist.

C: Brownish-grey, fine-grained QUARTZITE. Moderately weathered (W3), weak to strong (R2-R4). Blocks are rhomboidal, ~ 15 cm. GSI structure = Very Blocky, GSI surface = Poor, GSI range = 30-45.

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Outcrop OC-BGC11-19

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT C

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	23-34	284-296	1-10	SA, SM	6-10	R3-R4	0.1	>0.5m	JO
2	85-86	154-162	1-5	SA, SM	2-4	R3-R4	0.2	>0.5m	J
3	66-71	088-090	1-10	SA, SM	4-8	R3-R4	0.3	>0.3m	J
4	72-78	219-222	1-10	SA, SM	14-16	R3-R4	0.3	>0.3m	J

Outcrop OC-BGC11-20

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Man-made (road cut)

Northing: 7100213
Easting: 460060
Elevation: 1105 m
Survey type: Handheld GPS

Slope Angle:
Slope Direction:
Date logged: 7/23/2011
Logged by: EC

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Vegetation, moss, root mat (0.05 m). [TOPSOIL]

B: Dark grey, fine- to medium-grained QUARTZITE. Moderately weathered (W3), medium strong to strong (R3 to R4). Blocks are tabular, ~ 0.1 m, with low fracture persistence. GSI structure = Blocky/disturbed, GSI surface = Fair, GSI range = 40-50.

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Outcrop OC-BGC11-20

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	78-83	84-93	0.5-5	SA, SM	10-14	R3-R4	0.2	>0.2m	J
2	78-80	174-188	NV	SA, SM	12-16	R3-R4	0.05	>1.0m	JO
3	36-37	312-318	0.5-2	SA, SM	10-14	R3-R4		>0.2m	J

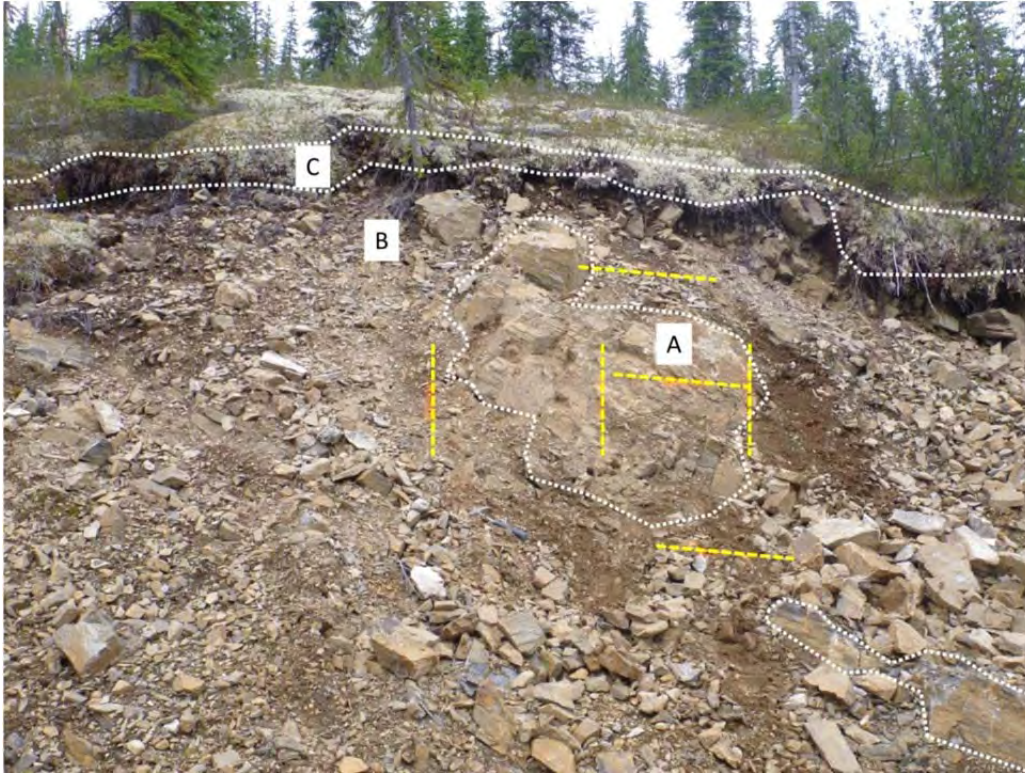
Outcrop OC-BGC11-21

Location: Stuttle Gulch
Facility: Primary crusher
Outcrop type: Man-made (Exploration drill pad cut)

Northing: 7099879
Easting: 459852
Elevation: 1091 m
Survey type: Handheld GPS

Slope Angle: 28°
Slope Direction: 332°
Date logged: 7/25/2011
Logged by: EB/EC

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Reddish-brown fine-grained QUARTZITE. Foliated and containing quartz veins, slightly to moderately weathered (W2-W3), strong (R4). Blocks rhomboidal, ~ 15 cm. GSI structure = Very Blocky, GSI surface = Fair, GSI range = 40-50.

B: GRAVEL (GW): Cobbly, some silt, trace sand, trace boulders, reddish-brown. Moist and heavily FE-stained. Clasts elongate, angular, medium strong (R3). Max particle size = 85 cm. [COLLUVIUM]

C: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-21

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	83-89	320-339	0.5	FE	8-12	R4	0.2	1m+	J
2	41-47	340-342	0.5-10	SA,SM	12-16	R4	0.1	1m+	J
3	68-72	114-125	0.5-2	SA,SM	8-12	R4	0.11	1m+	J
4	67-71	202-203	1	SA,SM	10-20	R4	0.15	1m+	J
5	30-37	238-250	0.5-2	SA,SM	12-14	R4	0.05	1m+	JO

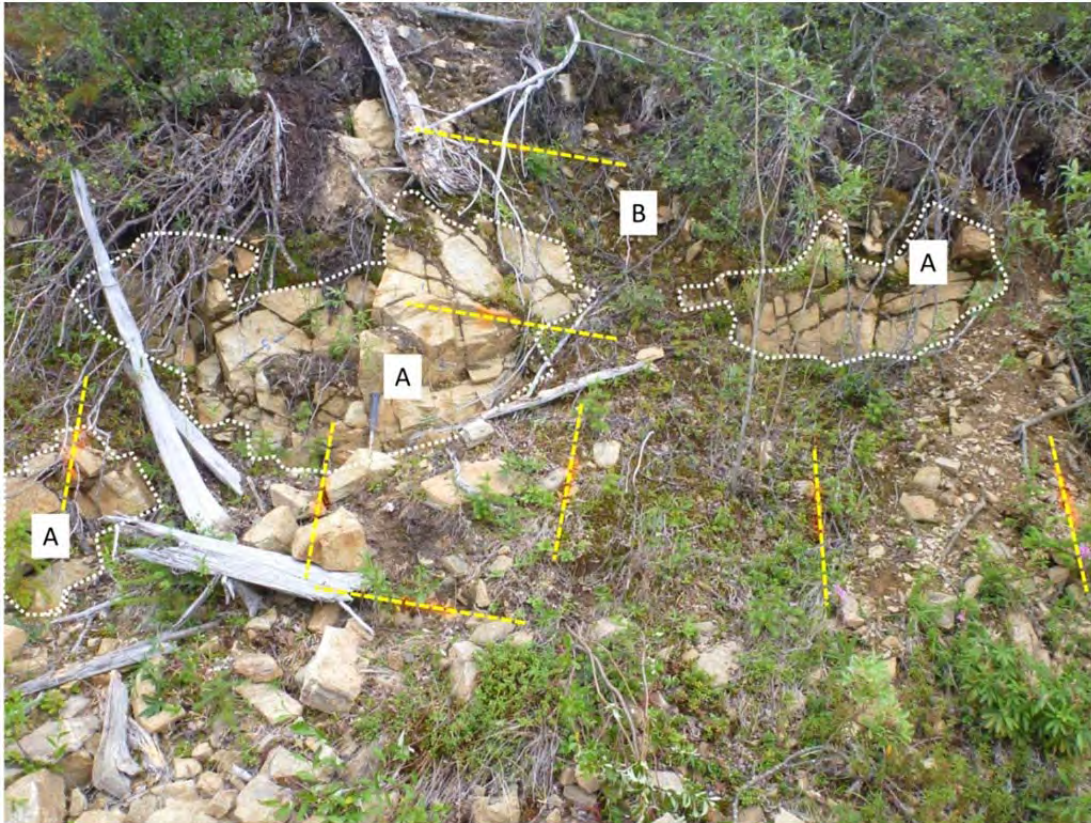
Outcrop OC-BGC11-22

Location: Pit area
Facility: Open pit
Outcrop type: Man-made (Exploration road cut)

Northing: 7099590
Easting: 459880
Elevation: 1160 m
Survey type: Handheld GPS

Slope Angle: 63°
Slope Direction: 252°
Date logged: 7/25/2011
Logged by: EB/EC

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Greyish-white, coarse-grained GRANODIORITE. Slightly weathered (W2); extremely strong (R6). Blocks ~ 15 cm; rhomboidal/polyhedral with low fracture persistence. GSI structure = Very Blocky, GSI surface = Good, GSI range = 50-60.

B: SILT (ML): Cobbly, bouldery, dark brown. Clasts equidimensional, angular to subangular, extremely strong (R6). Max particle size ~ 400 mm. Filled with organics/rootlets. [TOPSOIL]

Outcrop OC-BGC11-22

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	62-75	220-227	NV	FE	16-18	R6	NV	0.5m+	J
2	65-68	028-042	10	SA, SM	14-16	R6	0.2	NV	J
3	82-85	334-339	150	SA, SM	8-16	R6	0.25	NV	J
4	60-74	120-149	1-40	SA, SM	10-12	R6	0.12	NV	J
5	40-48	332-338	1-25	SA, SM	8-16	R6	0.12	NV	J

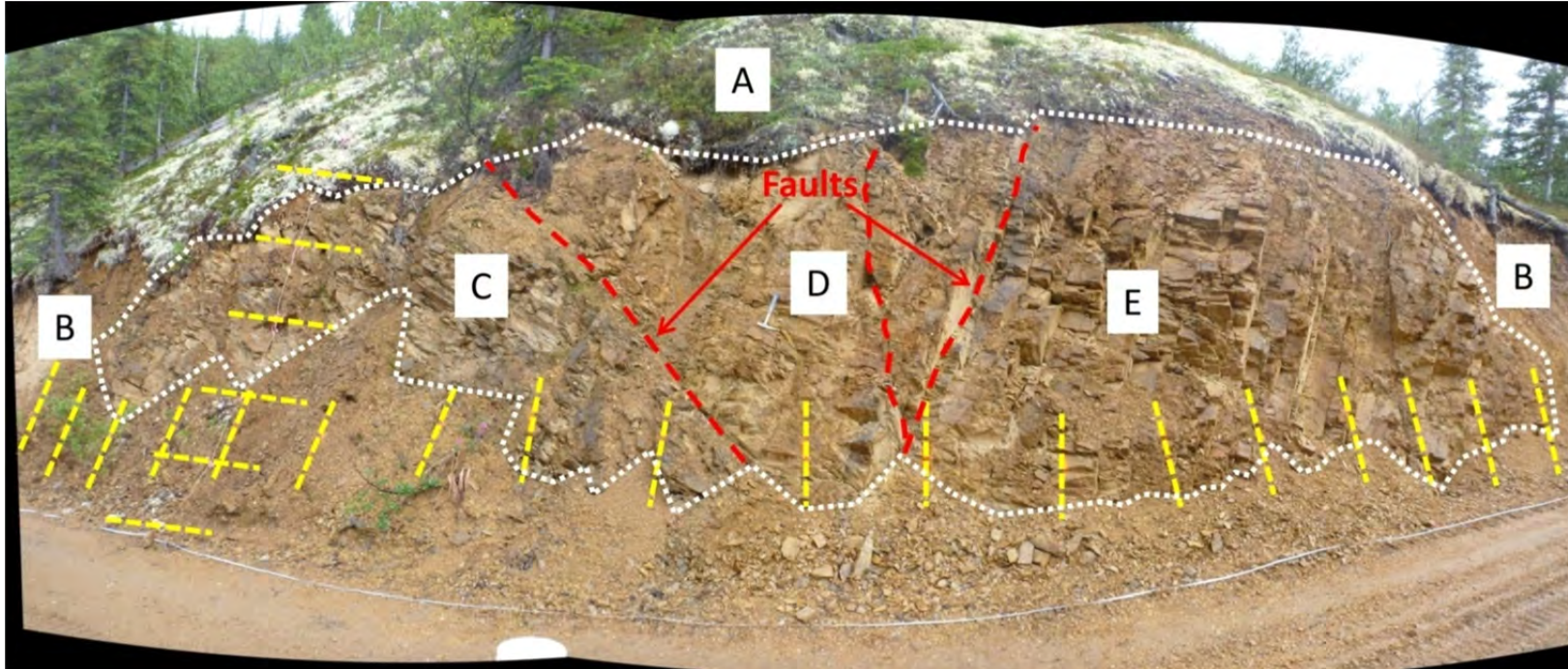
Outcrop OC-BGC11-23

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Man-made (road cut)

Northing: 7100289
Easting: 460110
Elevation: 1111 m
Survey type: Handheld GPS

Slope Angle: Natural 40° / Cut 80°
Slope Direction: 010°
Date logged: 8/1/2011
Logged by: LGT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Root mat and organic silt. [TOPSOIL]

B: GRAVEL (GW): Some sand, some, cobbles, well graded. Yellowish brown, moist, homogenous. Clasts are metasedimentary, flat & platy, angular; max particle size ~200 mm. [COMPLETELY WEATHERED METASEDIMENTARY ROCK]

C: Yellowish-grey, fine-grained QUARTZITE. Faintly weathered (W1.5); medium strong to strong (R3-R4). Four main discontinuity sets spaced very to extremely closely; blocks are tabular. GSI structure = Blocky/disturbed, GSI surface = Good to Fair, GSI range = 30-50.

D/E: Same as above but pinkish yellow colour. D: Joints are extremely closely spaced, GSI 30-35. E: Joints are very closely spaced, good joint condition, GSI 40-50.

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Outcrop OC-BGC11-23

PHOTO OF STRUCTURE, UNIT C



PHOTO OF STRUCTURE, UNIT D

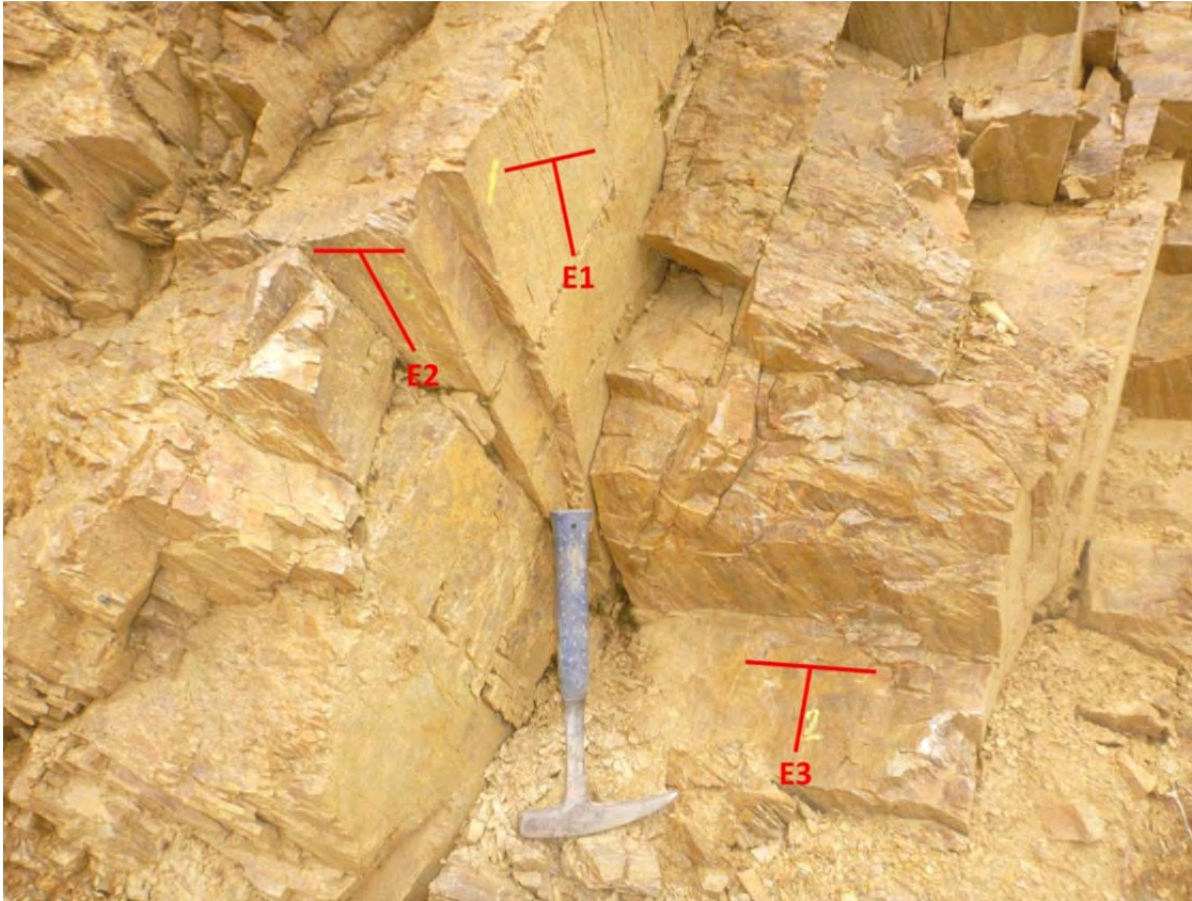


DISCONTINUITY TABLE, UNITS C/D

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
C1	25	260	0.1-15	CX, SA	20 (10 cm)	R3	0.1	>1m	JO
C2	59	093	1-2	CX, SA	6 (1m)	R4	0.3	1m	J
C3	75	061	NV	NV	20	R4		>1m	J
D1	71	231	NV	NV	20	R3		>4.9m	J
D2	76	170	0.2-4	CX	4	R4	0.2	1.5m	J
D3	74	335	5-15	CX	4	R4	0.3	>4m	J
D4	45	097	1	CX	16 (10 cm)	R4		>2.5m	J
D5	36	244	3-5	CX, SA	20 (10 cm)	R4	0.03	>1m	JO

Outcrop OC-BGC11-23

PHOTO OF STRUCTURE, UNIT E



DISCONTINUITY TABLE, UNIT E

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
E1	77	172	1-5	CX, None	4 (10 cm)	R4	0.2	>4.5m	J
E2	60	086	1-5	CX, SA	20 (10 cm)	R4	0.1	>2.5m	J
E3	38	267	0.5-1	CX	10 (10 cm)	R4	0.15	>1m	JO
E1 (Fault)	84	171	4	CX	1	R4		>4.5m	F

Outcrop OC-BGC11-24

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Man-made (road cut)

Northing: 7100089
Easting: 460221
Elevation: 1069 m
Survey type: Handheld GPS

Slope Angle: 48°
Slope Direction: 094°
Date logged: 7/31/2011
Logged by: SP

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Vegetation, moss, root mat (0.5 m). [TOPSOIL]

B: GRAVEL (GW). Well-graded. [COLLUVIUM]

C: Light brown to grey, medium- to coarse-grained GRANODIORITE. Lightly weathered and iron stained (W2); medium strong (R3). Blocks are rhomboidal, ~ 50 cm. GSI structure = Blocky, GSI surface = Good, GSI range = 55-65.

Outcrop OC-BGC11-24

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT C

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	78	261	0.1-5	CX, SA, FE	16 (1m)	R3		1m+	J
2	69	350	1-10	CX, BX, FE, SA	16 (10cm)	R3	0.5	0.5m+	J
3	32	156	10-100	BX, CX, SA, FE	12 (1m)	R3	0.6	1m+	J

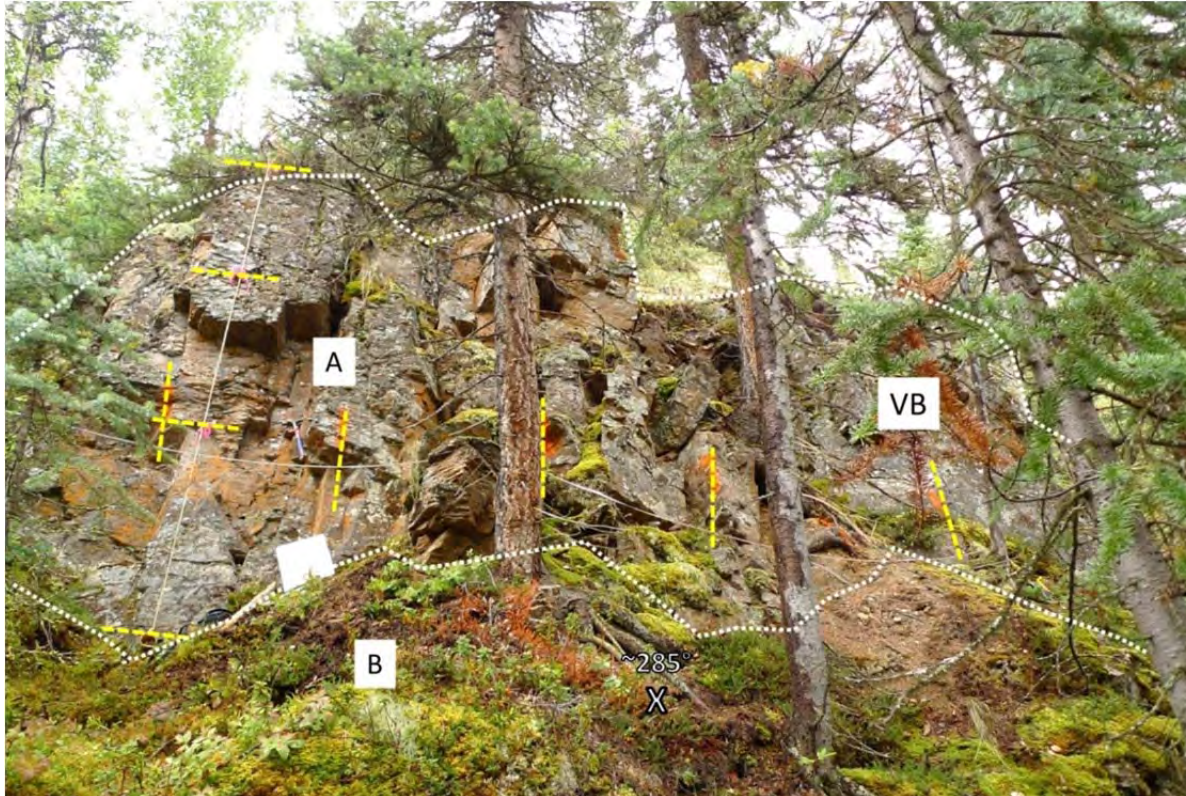
Outcrop OC-BGC11-25

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Natural

Northing: 7100350
Easting: 460142
Elevation: 1033 m
Survey type: Handheld GPS

Slope Angle: 60°
Slope Direction: 081°
Date logged: 8/13/2011
Logged by: GH/LGT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

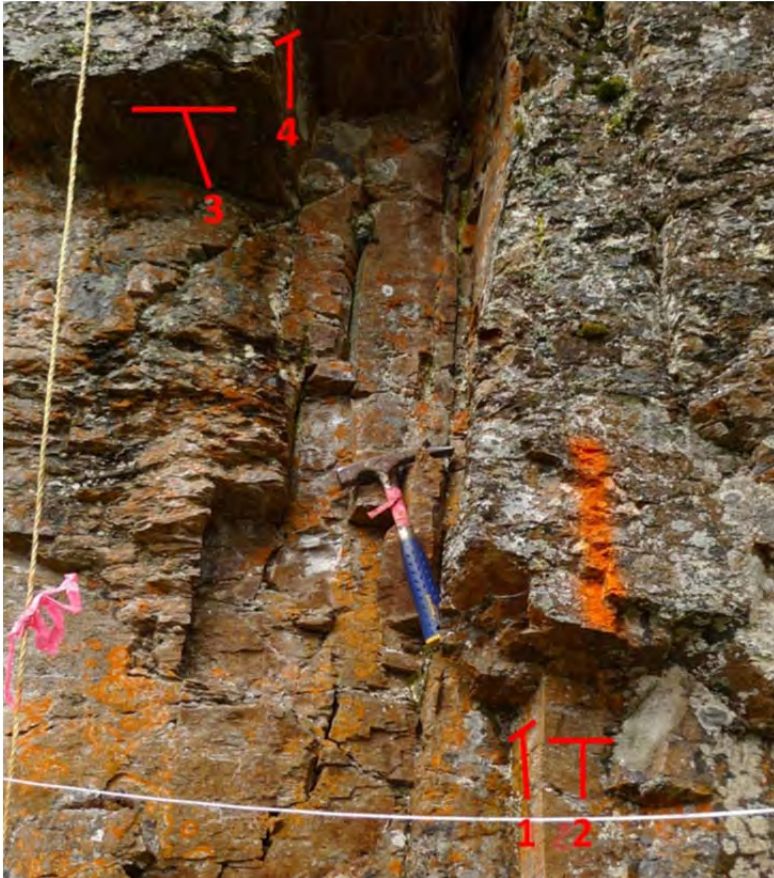
A: Light brown, fine-grained QUARTZITE. Strong to Very Strong (R4-R5); lightly weathered (W2). Blocks tabular, ~ 20 cm. GSI structure = Blocky/disturbed (area marked VB = Very Blocky). Breaks readily on foliation. GSI surface condition = Fair, GSI = 40-50.

B: GRAVEL and organic silt with some sand (GM). Nonplastic, dark brown, soft, homogenous with organic odour. Clasts angular, medium strong (R3.5), flat and elongated, 20-100 mm; max particle size ~ 100 mm (200 mm under Very Blocky part of outcrop, marked VB). [COLLUVIUM]

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Outcrop OC-BGC11-25

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	74-80	163-184	0-5mm	None	8 (1 m)	R3.5	0.2	3m+	J
2	77-85	077-091	2 mm	None	6-8 (1 m)	R3.5	0.2	3m+	J
3	32-36	265-283	0-1mm	Mica on	12-14 (0.1 m)	R3.5	0.15	1m	JO
4	86-89	199-222	0-1mm	None	8 (1 m)	R3.5	0.5	1m	J

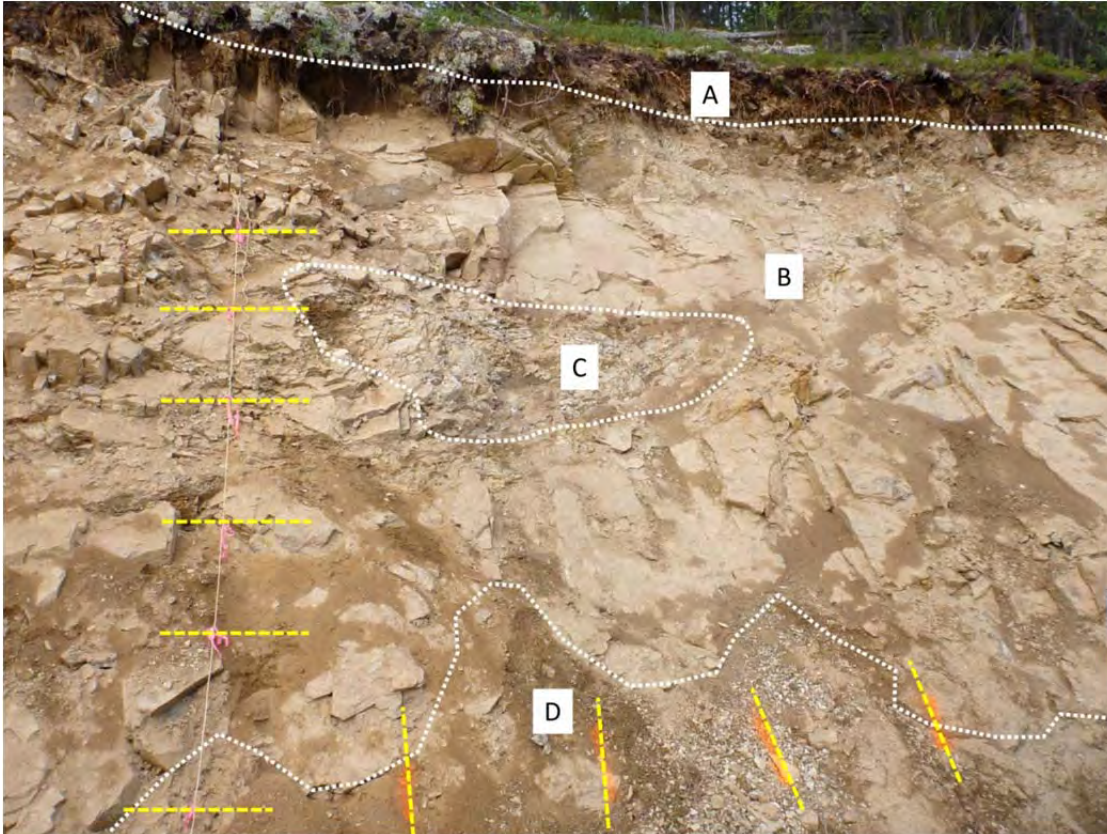
Outcrop OC-BGC11-26

Location: Platinum Gulch
Facility: Platinum Gulch WRSA
Outcrop type: Man-made (exploration road cut)

Northing: 7099104
Easting: 459656
Elevation: 1045 m
Survey type: Handheld GPS

Slope Angle:
Slope Direction:
Date logged: 8/2/2011
Logged by: SP

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Vegetation, moss, root mat. [TOPSOIL]

B: Light reddish brown QUARTZITE. Faintly to lightly weathered (W1.5-W2); medium strong (R3). Blocks rhombohedral; ~ 10 cm. GSI structure = Very Blocky, GSI surface = Fair, GSI range = 40-45.

C: Light grayish brown QUARTZITE. Moderately weathered (W3); weak (R2). Blocks rhomboidal; ~ 2 cm. GSI structure = Blocky/Distrubed to Disintegrated, GSI surface = Fair, GSI range = 25-35.

D: GRAVEL: Sandy, some cobbles, trace silt, well graded (GW). Nonplastic, dark reddish brown, moist, loose. Clasts angular to subangular, flat and elongated, medium strong (R2.5-R3); max particle size ~ 100 mm. [COLLUVIUM, SPALLED ROCK]

Outcrop OC-BGC11-26

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNITS B/C

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	53	259	1-10	SA, ML, FE	18	R3	0.3	1m+	JO
2	70	44	10-100	BX, ML, FE	12	R3	0.5	1m+	J
3	81	151	2-5	ML, SA, FE	10	R2.5-3	0.3	1m+	J

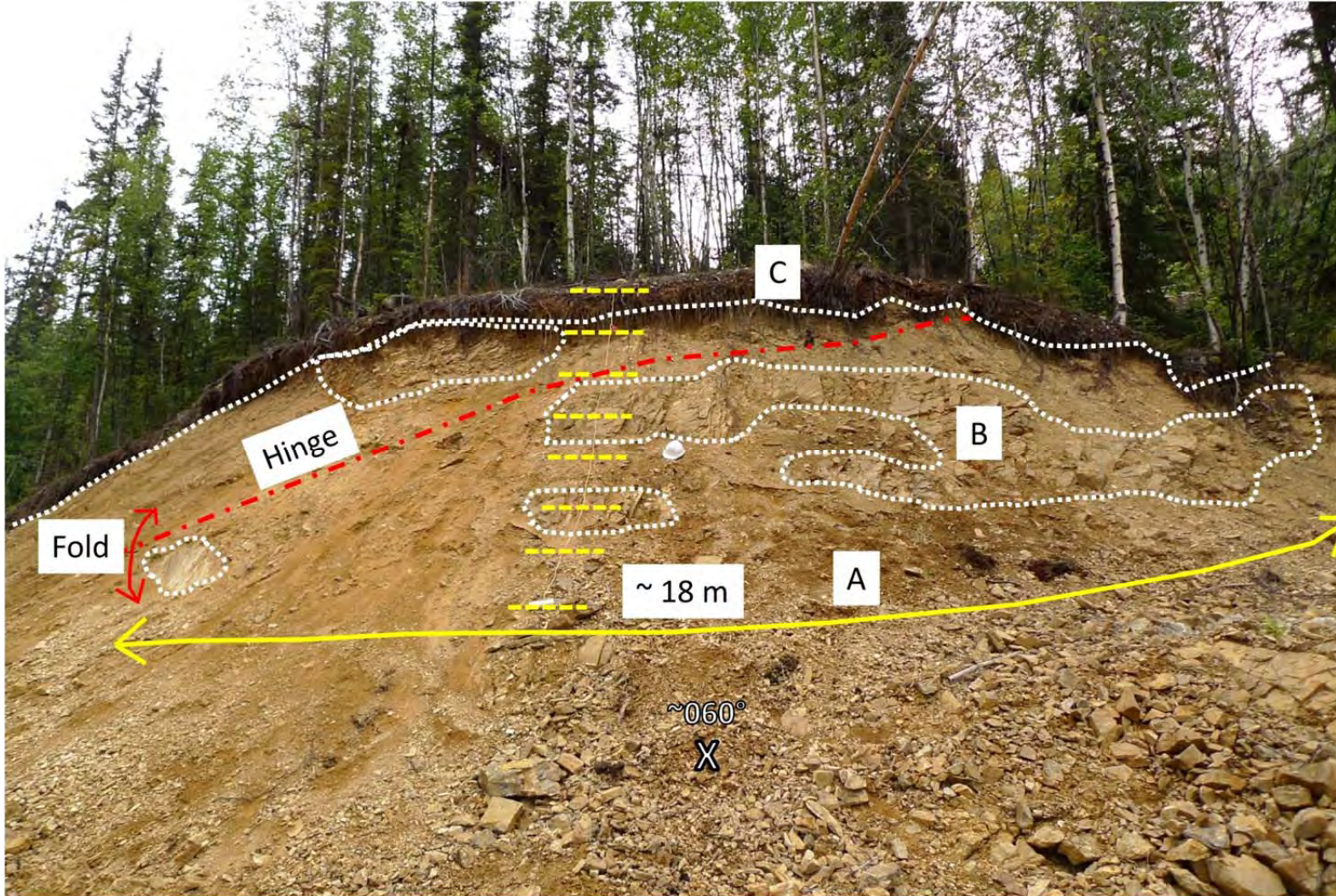
Outcrop OC-BGC11-27

Location: Platinum Gulch
Facility: Platinum Gulch WRSA
Outcrop type: Man-made (exploration drill pad cut)

Northing: 7099534
Easting: 459343
Elevation: 971 m
Survey type: Handheld GPS

Slope Angle: 41°
Slope Direction: 221-259°
Date logged: 8/13/2011
Logged by: GH/LT

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-27

PHOTO OF STRUCTURE – UNIT B (BELOW FOLD HINGE)



Outcrop OC-BGC11-27

DESCRIPTION OF MATERIALS

A: SAND and GRAVEL (SW). Well-graded, yellowish-brown, sand fine to medium. Clasts angular, flat/elongated, medium strong (R3). [COLLUVIUM]

B: Foliated, fissile fine-grained grey quartz schist, mostly weathered to yellowish brown.

	Above/near fold hinge:			Below fold hinge:		
Weathering	W3			W2		
Strength	R1			R3-R4		
Block size/shape	Crushed (5 cm to sand sized); tabular			5-10 cm; tabular		
GSI structure	Disintegrated			Lower Very Blocky		
GSI surface	Fair			Good-Fair		
GSI range	25-35			40-50		

C: Root mat and organic silt. [TOPSOIL]

DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	79-88	331-338	1-3	none	8 (0.1m)	R3	0.1	2m+	J
2	67-77	236-245		none	12 (1m)	R3	0.05	1m	JO
3	33-54	024-065		none	12 (0.1m)	R3	0.2	0.5m+	J
4	42-44	137-162		none	4-8 (1 m)	R3	1	2.5m	J
5	36-58	341-005		none	8 (1 m)	R3	0.5	1m+	J
6	32-47	021-040		none			0.03		JO

Outcrop OC-BGC11-28/28B

Location: Platinum Gulch

Northing: 7099610

Slope Angle: 50°

Facility: Platinum Gulch WRSA

Easting: 459302

Slope Direction: Curves 252-276°

Outcrop type: Man-made (exploration road/drill pad cut)

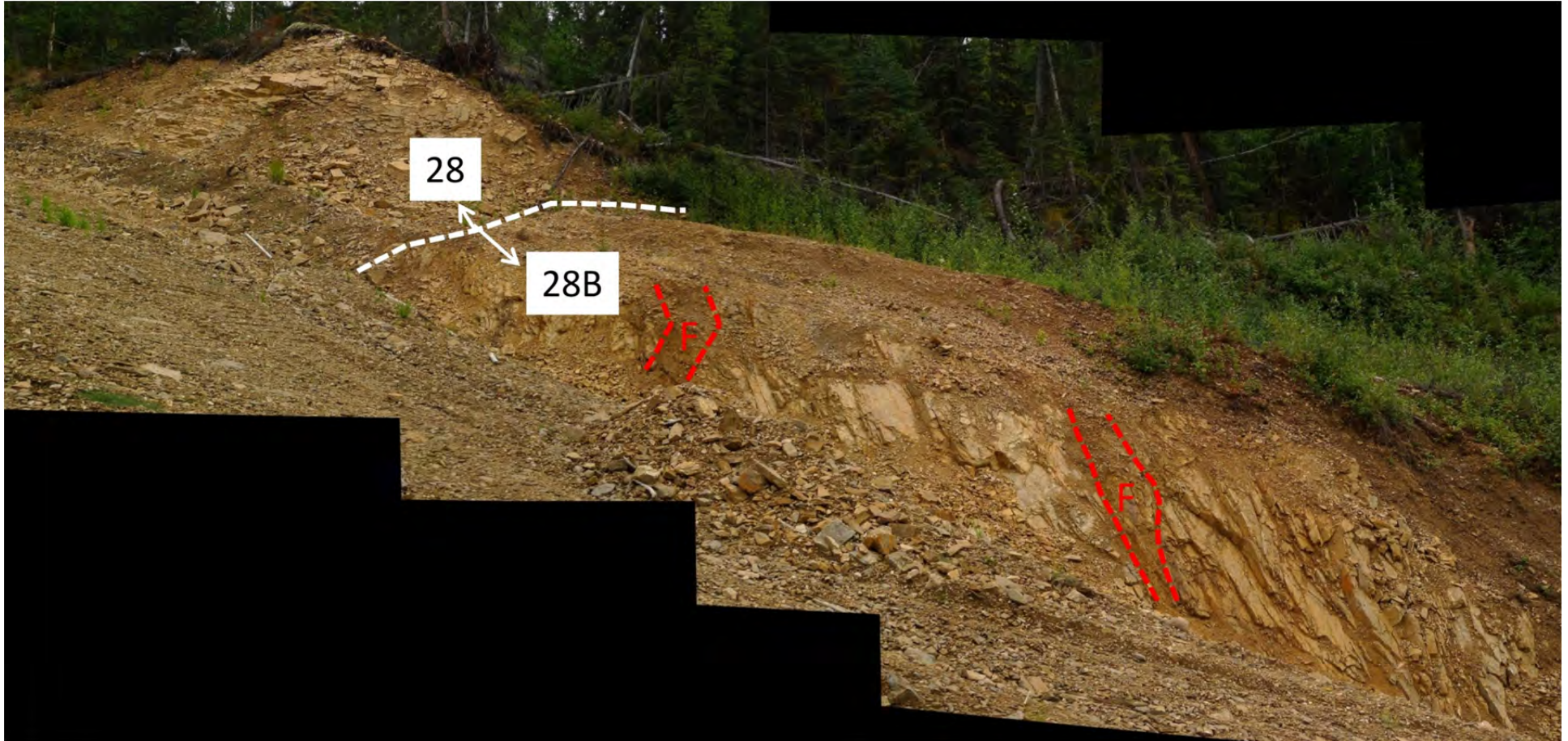
Elevation: 948 m

Date logged: 8/14/2011

Survey type: Handheld GPS

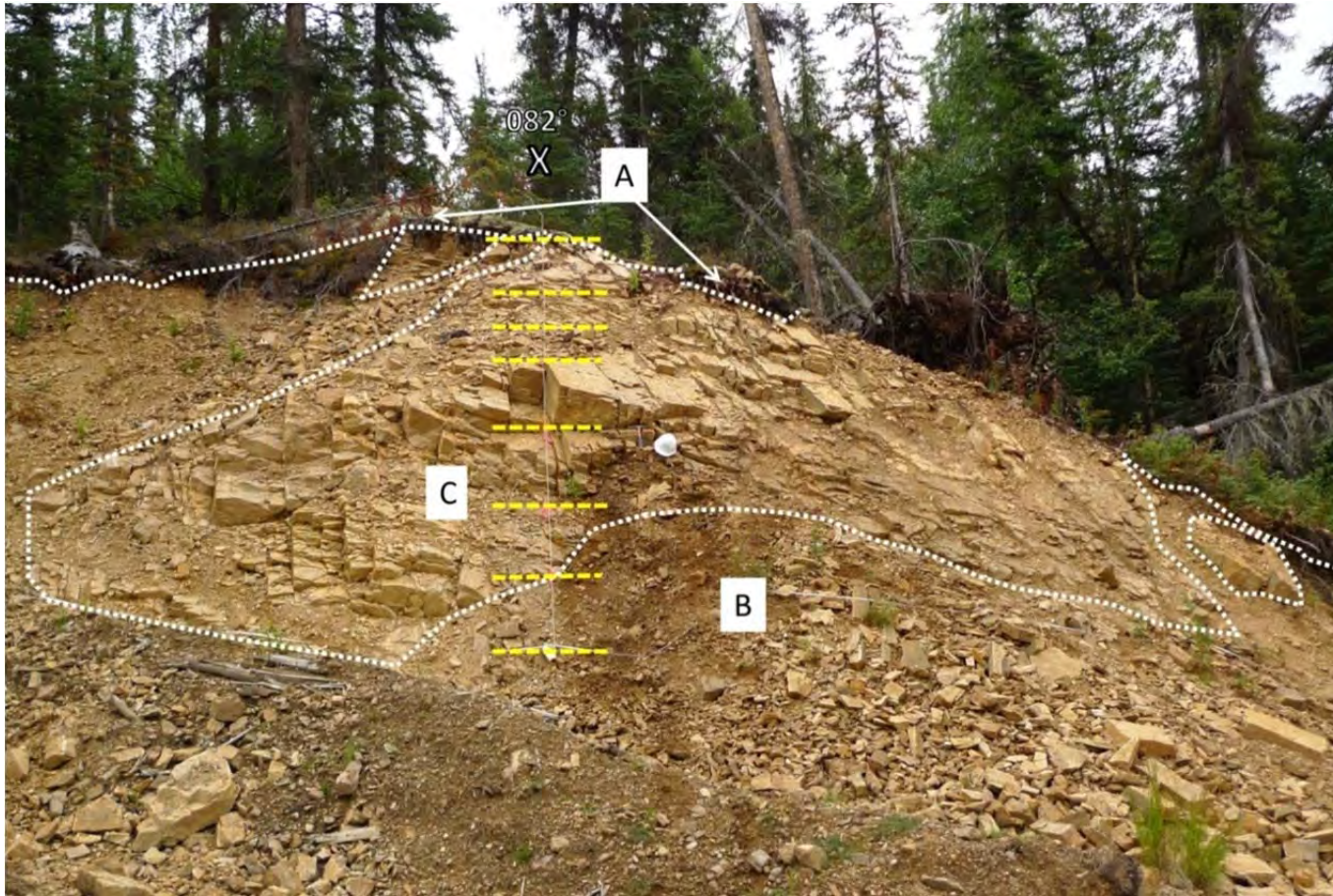
Logged by: GH

LARGE SCALE OF OUTCROP 28/28B, SHOWING LOCATION OF FAULTS



Outcrop OC-BGC11-28

PHOTO OF OUTCROP 28



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Moss, root mat, and organic silt [TOPSOIL]

B: GRAVEL: Sandy, brown, poorly-graded (GP). Clasts: Meta-sedimentary, angular, flat/elongated, medium strong (R3); max particle size ~ 150 mm. [COLLUVIUM] from local outcrop.

C: Brown, fine-grained QUARTZITE. Medium strong to strong (R3-R4) and lightly to moderately weathered (W2-W2.5), depending on biotite content. Blocks tabular, ~ 10 cm. GSI structure = Very blocky to Blocky/Disturbed, GSI surface = Good to Fair, GSI range = 40-50.

Outcrop OC-BGC11-28

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT C

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	40-43	239-251	0-3	None	8-10 (1m)	R2.5	0.1	2	JO
2	84-87	338-344	1-10	Qz, Fe	2-4(1m)	R2.5	0.15	5+	J
3	65-67	069-101	0-5	None	16(1m)	R3	0.25	1.5	J
4	61-70	110-118	5-10	Qz	6(1m)	R3	1.5	5+	J

Outcrop OC-BGC11-28B

Supplement to OC-BGC11-28

Small outcrop below OC-BGC11-28 with discontinuity sets 1, 2, and 4 from OC-BGC11-28 present. More highly fractured. Average strength = R2 (weak). Blocks 1-5 cm, GSI structure = Disintegrated, GSI surface = Fair, GSI range = 30-40. Contains 2 brittle sandy faults (see example below).

PHOTOGRAPH OF FAULT #2



DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	55	240	0-5	None	10 (1m)	R3	0.05	2m+	JO
Fault #1	75	202	400	CX		R2.5			F
Fault #2	76	175	250	CX		R3			F

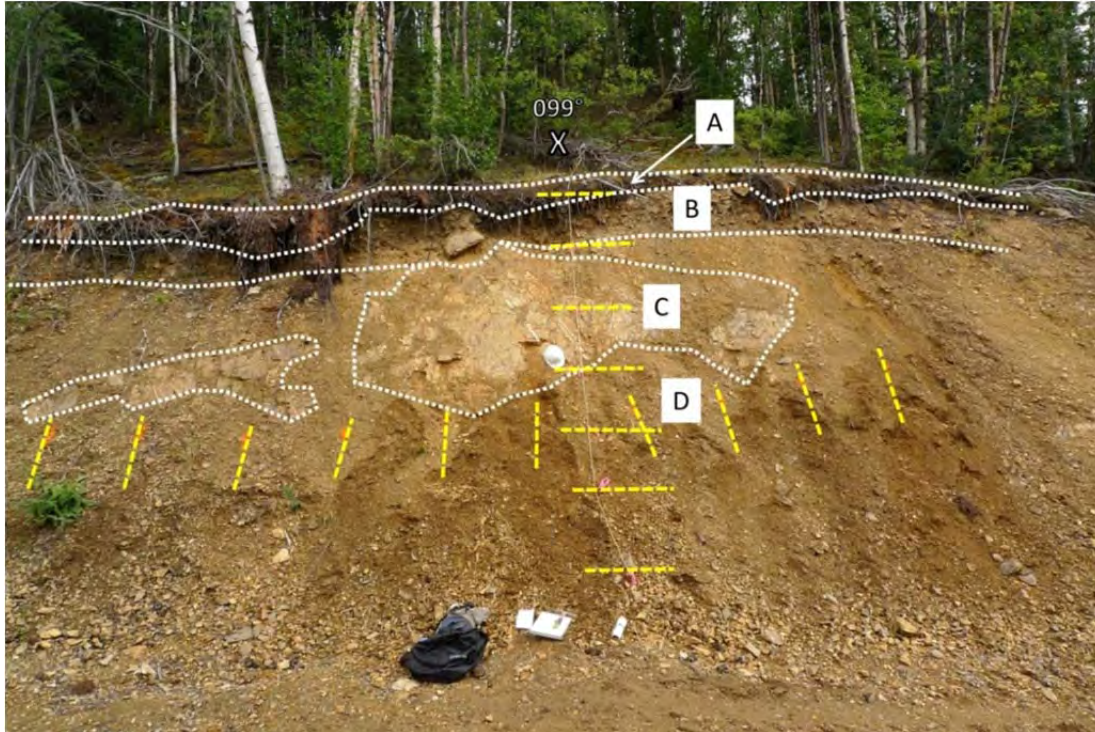
Outcrop OC-BGC11-29

Location: Pit area
Facility: Platinum Gulch WRSA
Outcrop type: Man-made (exploration road cut)

Northing: 7099512
Easting: 459444
Elevation: 983 m
Survey type: Handheld GPS

Slope Angle: 44°
Slope Direction: 270°
Date logged: 8/14/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Root mat organic silt. [TOPSOIL]

B: SAND: Gravelly/cobbly, some silt, trace boulders, trace rootlets (SW). Light brown, moist, homogenous. Clasts subangular to angular, equidimensional to elongated, medium strong to strong (R3-R4); max particle size ~ 400 mm. [COLLUVIUM]

C: Brown, fine-grained QUARTZITE. Slightly to moderately weathered (W2-W3); medium strong (R2-R3). Blocks tabular, 1-5 cm. GSI structure = Disintegrated, GSI surface = Fair, GSI range = 25-35.

D: Material ravelled from cut-slope. [COLLUVIUM]

Outcrop OC-BGC11-29

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT C

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	60-66	267-278	0-1	None	8-10 (0.1 m)	R2	0.03	2m+	JO
2	62-108	153-155	1-50	QZ	6-8 (1 m)	R2.5	0.1	2m+	J, V
3	36-39	078-089	0-2	None	10 (1 m)	R2.5	0.05	1m	J

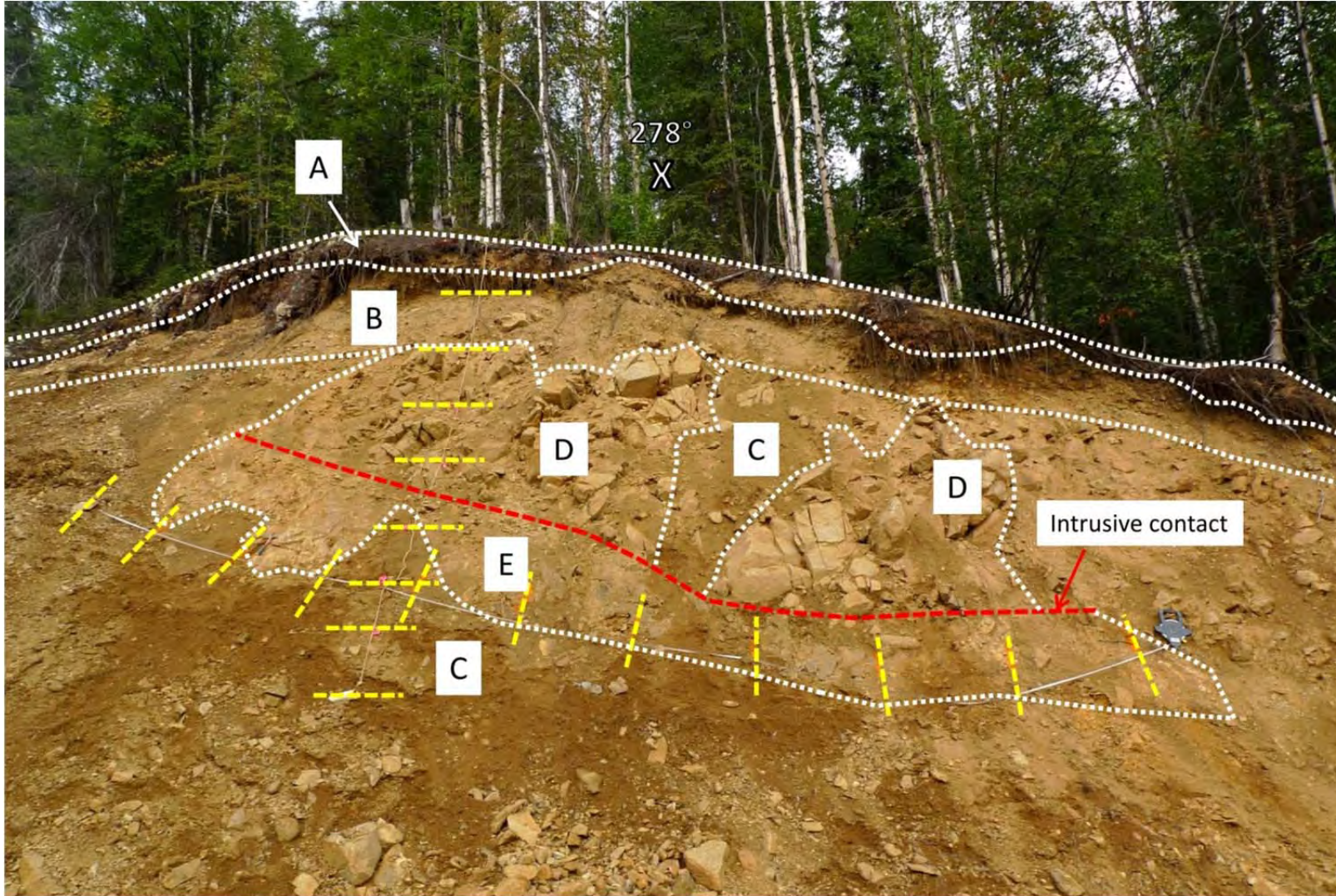
Outcrop OC-BGC11-30

Location: Pit area
Facility: Platinum Gulch WRSA
Outcrop type: Man-made (exploration road cut)

Northing: 7099467
Easting: 459387
Elevation: 947 m
Survey type: Handheld GPS

Slope Angle: 50°
Slope Direction: 259°
Date logged: 8/14/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-30

Photo of structure - Granodiorite

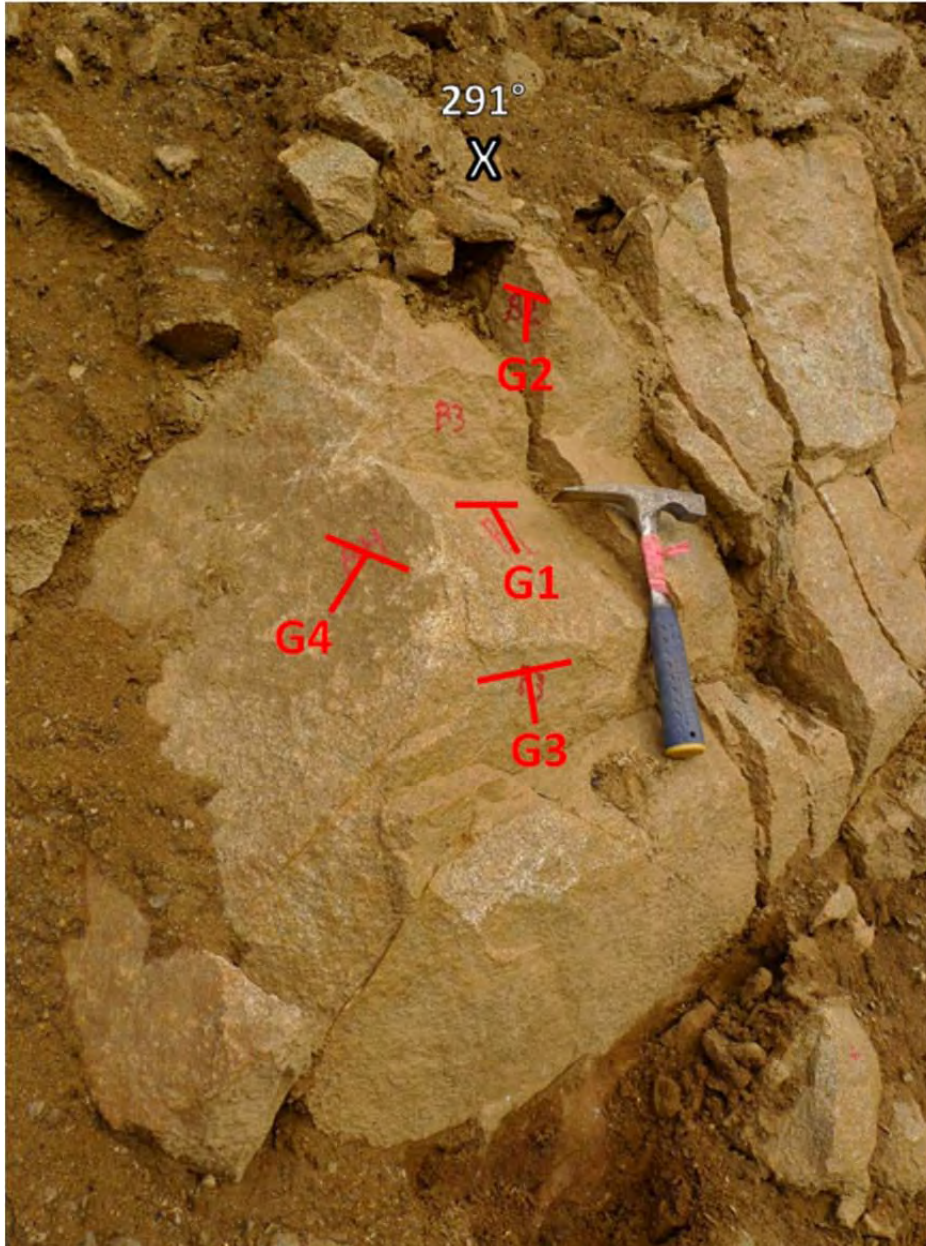
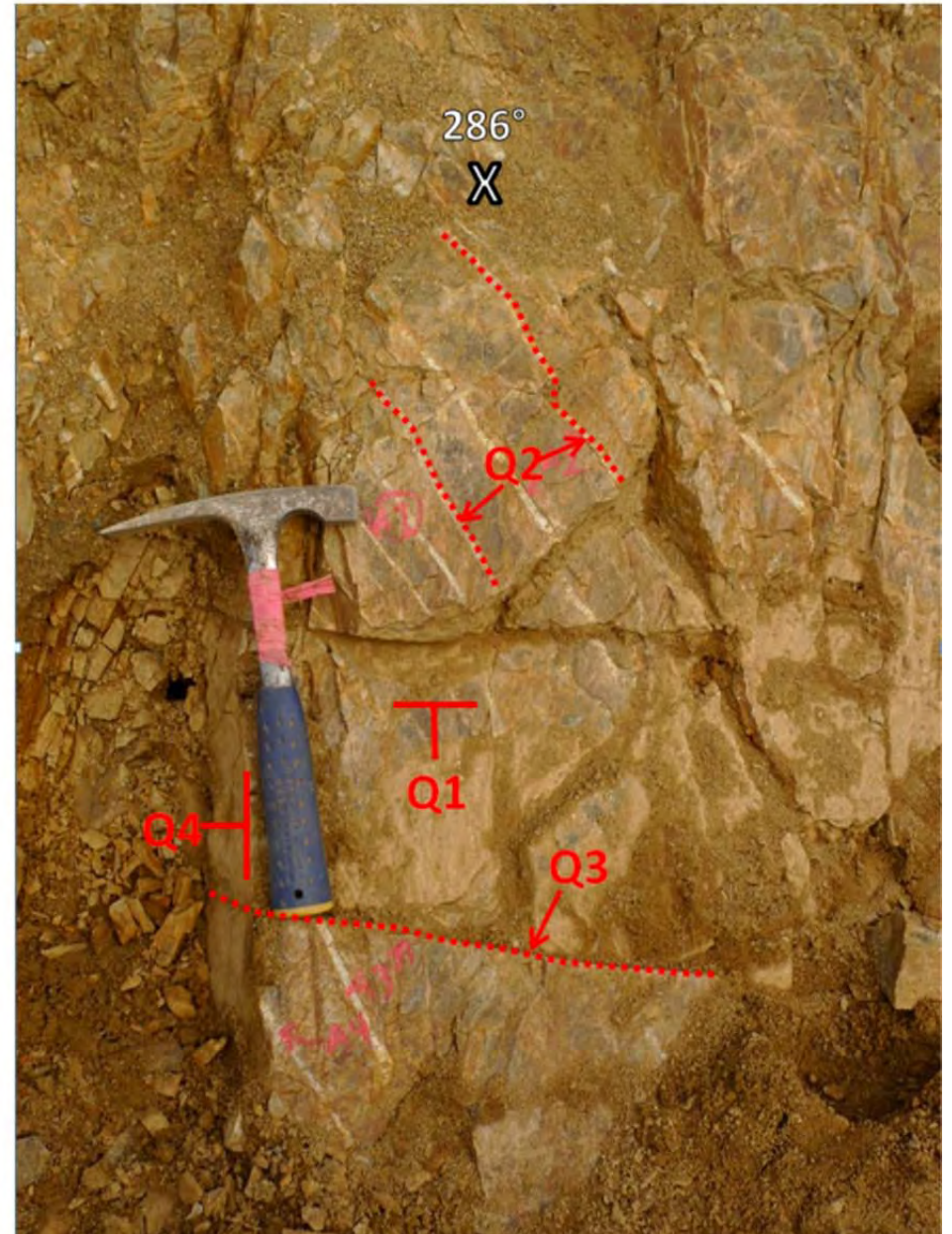


Photo of structure - Quartzite



Outcrop OC-BGC11-30

DESCRIPTION OF MATERIALS

A: Root mat and organic silt. [COLLUVIUM]

B: SAND with gravel and cobbles, well-graded (SW). Light brown, moist. Clasts granodiorite and meta-sedimentary, angular to sub-angular, equidimensional to tabular, strong (R3-R4); max particle size ~ 100 mm. [COLLUVIUM]

C: Ravelled material from outcrop.

D: Yellowish-grey medium-grained GRANODIORITE. Slightly weathered (W2); strong (R4). Blocks ~ 20cm, equidimensional to tabular. GSI structure = Very Blocky, GSI surface = Good, GSI range = 50-60.

E: Dark grey, fine-grained QUARTZITE. Lightly weathered (W1.5-W2); strong (R3-R4). Blocks ~ 5 cm; tabular. GSI structure = Blocky/Disturbed, GSI surface = Good to Fair, GSI range = 40-50.

Lithological contact: 0.3-m-thick zone of reddish-grey medium sandy decomposed rock (DX) derived from the granodiorite unit. Minor clay present from alteration of feldspars.

DISCONTINUITY TABLE – UNITS D/E

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
G1	45-61	216-249	0	None	16 (1m)	R4	0.5	0.7m	J
G2	81-85	138-162	0-30	SA	10-12(1m)	R4	0.4	1.5m+	J
G3	71-82	072-073	0-1	None	6-8(1m)	R4	0.4	3m+	J
G4	55-65	306-320	0-1	FE	6-8(1m)	R3	1	1m+	J
Q1	54-62	257-269	0	None	8 (1m)	R3	0.05	1m+	JO
Q2	No broken surfaces		1-10	QZ	10(1m)	R3.5	0.1	2m+	J, V
Q3	33-71	075-136	2	None	4-6(1m)	R3.5	0.15	1m	J
Q4	64-68	325-348	0.2	None	6-8(1m)	R3.5	0.08	1.5m+	J

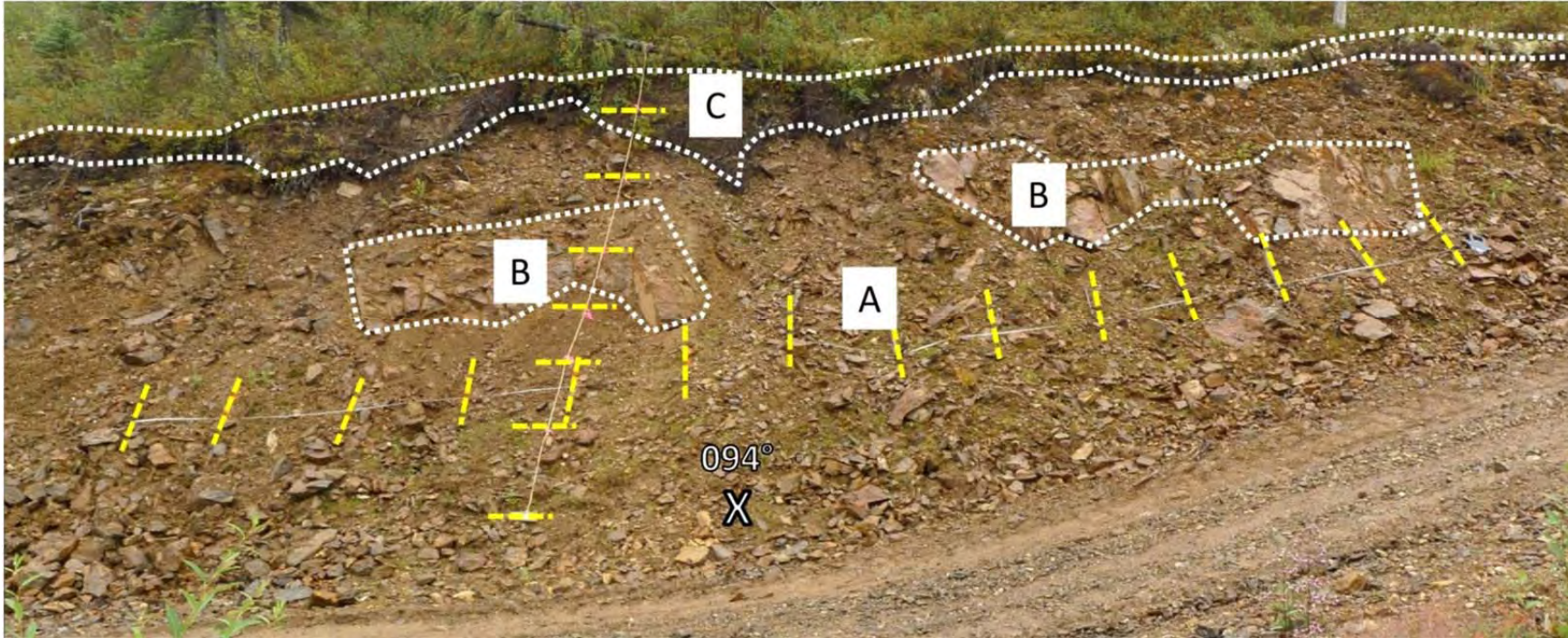
Outcrop OC-BGC11-31

Location: Pit area
Facility: Open pit
Outcrop type: Man-made (road cut)

Northing: 7099768
Easting: 459788
Elevation: 1087 m
Survey type: Handheld GPS

Slope Angle: 51°
Slope Direction: 287°
Date logged: 8/15/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: SILT: Gravelly, some sand (ML). Brown, soft, moist, and homogenous. Clasts: Meta-sedimentary, angular, flat/elongated, strong (R4). Fines have low plasticity, rapid dilatancy, low toughness. [COLLUVIUM]

B: Dark grey, fine-grained QUARTZITE. Faintly to lightly weathered (W1.5-W2); strong (R3-R4). Blocks ~ 10 cm; tabular. GSI structure = Blocky/Disturbed to Very Blocky, GSI surface = Good, GSI range = 45-55.

C: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-31

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	47-51	254-262	0	None	12-16	R3.5	0.1	1m+	JO
2	82-84	149-153	0-40	QZ	16	R4	0.3	1m+	J, V
3	33-50	087-118	0-2	None	12-20	R4	0.25	0.5m+	J
4	36-66	241-259	0-2	None	16	R4	0.4	1m+	J

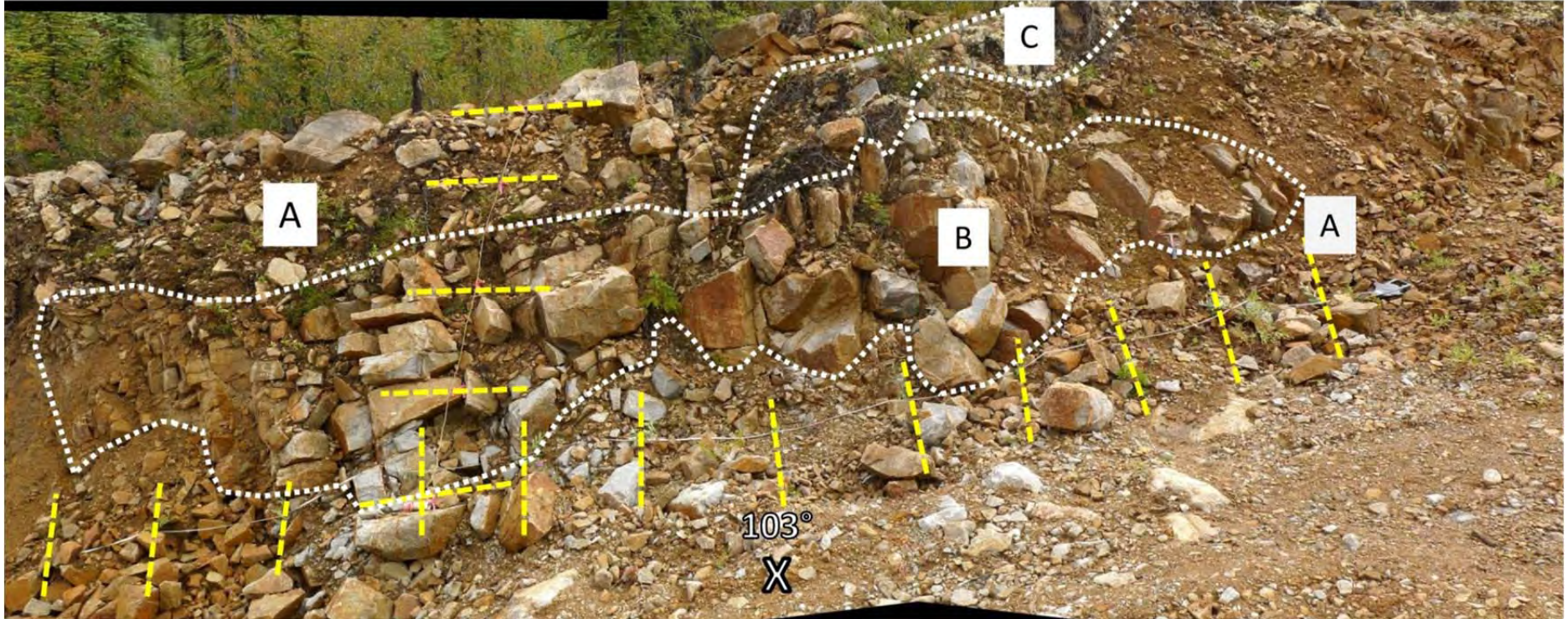
Outcrop OC-BGC11-32

Location: Pit area
Facility: Open pit
Outcrop type: Man-made (exploration trench)

Northing: 7099753
Easting: 460179
Elevation: 1176 m
Survey type: Handheld GPS

Slope Angle: 60°
Slope Direction: 272°
Date logged: 8/15/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: SAND: Some gravel, cobbles, and boulders, trace silt, well-graded (SW). Brown, moist, nonplastic. Clasts: Fine gravel to boulder-sized granodiorite; angular, equidimensional, strong (R4). [COLLUVIUM]

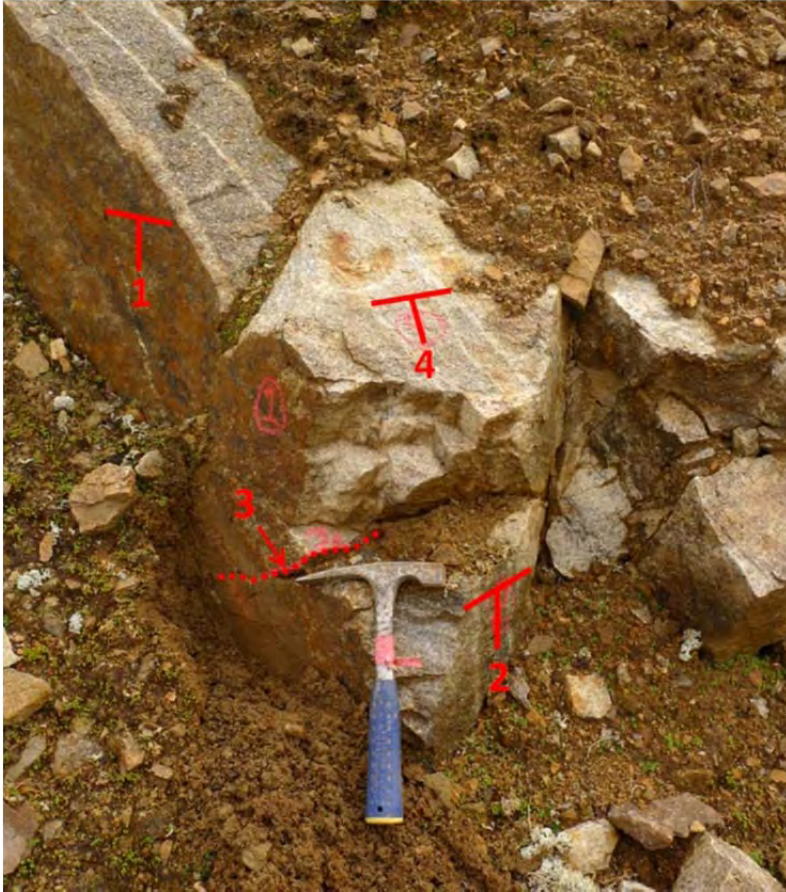
B: Grey, medium-grained GRANODIORITE. Faintly weathered (W1.5-W2); strength varies from R4-R5 with degree of weathering/alteration. Blocks ~ 30 cm; prismatic. Open fracture surfaces due to rock mass movement/dilation. GSI structure =Blocky to Very Blocky, GSI surface = Fair to Poor, GSI range = 45-55.

C: Root mat and organic silt. [TOPSOIL]

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Outcrop OC-BGC11-32

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	74-86	138-142	1-50	QZ	8(1m)	R4	0.3	2.5m+	J, V, S
2	78-89	231-238	2-20	None	10(1m)	R4.5	0.5	2.5m+	J
3	33-56	042-082	1-10	None	16(1m)	R4.5	0.4	1m+	J
4	38-51	260-270	NV	None	8(1m)	R4.5	0.4	1.5m+	J

Outcrop OC-BGC11-33

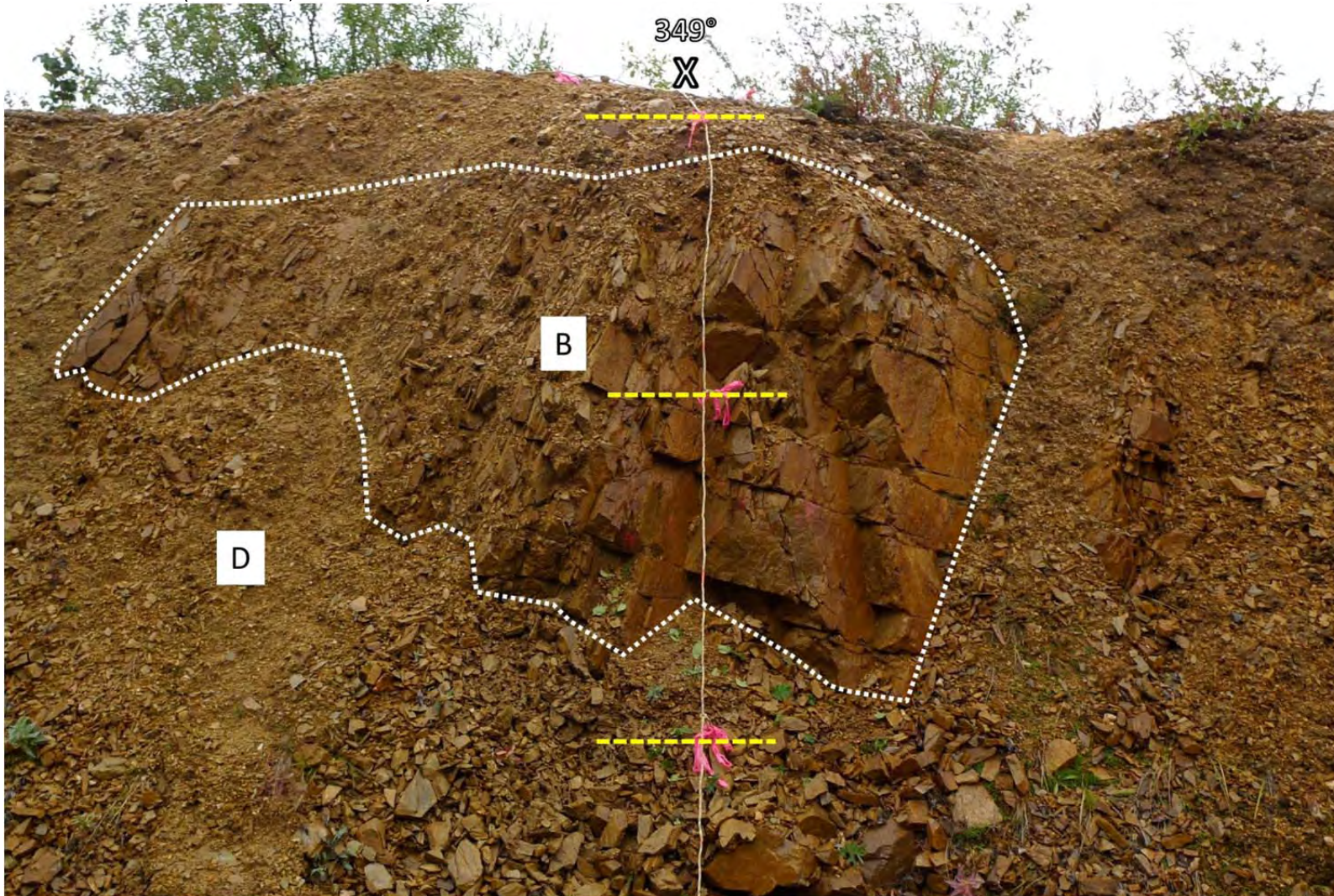
Location: Eagle Pup
Facility: Pit/Eagle Pup WRSA
Outcrop type: Man-made (exploration trench)

Northing: 7099961
Easting: 460143
Elevation: 1160 m
Survey type: Handheld GPS

Slope Angle: 70°
Slope Direction: 012°
Date logged: 8/15/2011
Logged by: GH

* Station comprises two outcrops (north and south) on either side of a deep ditch.

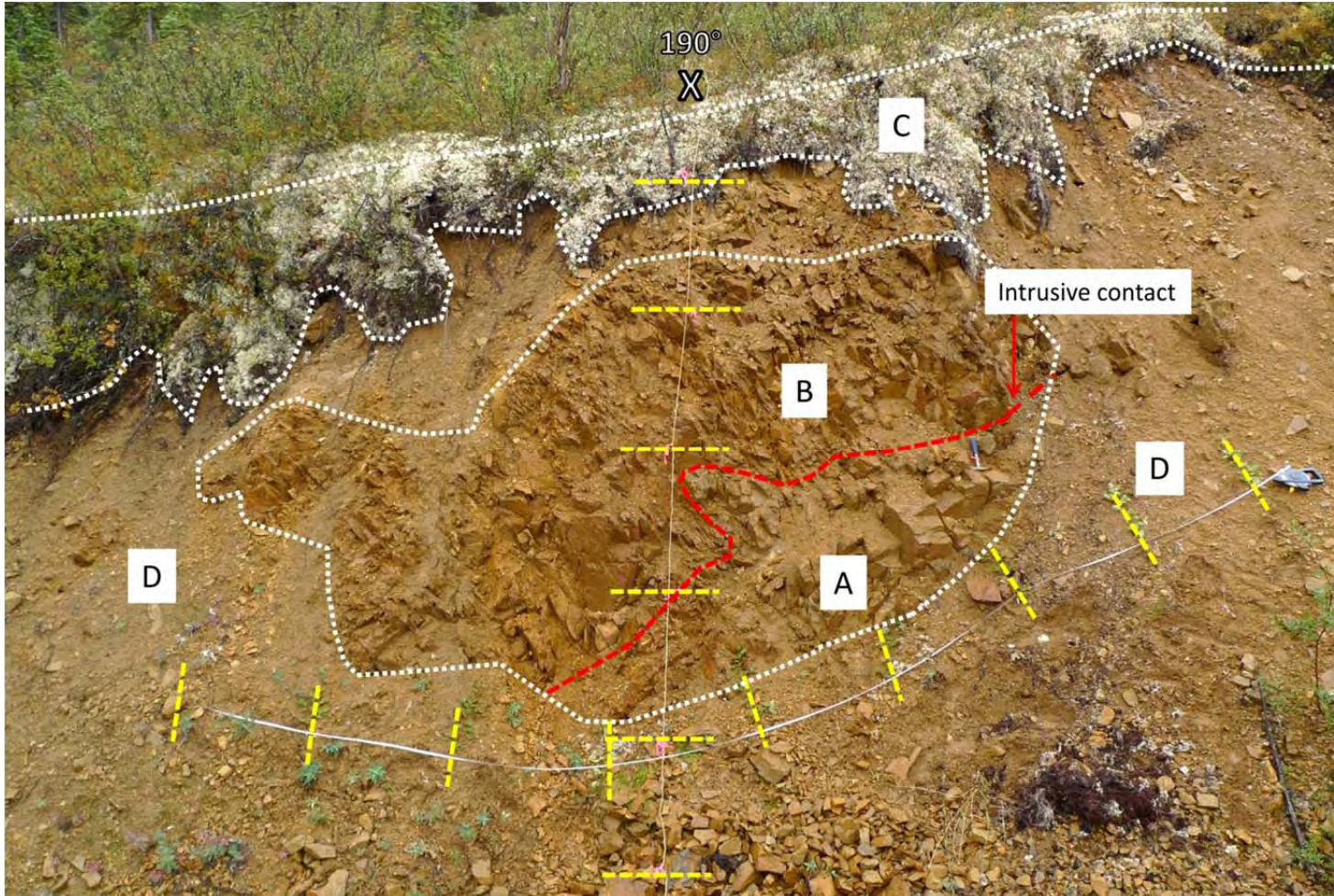
PHOTO OF OUTCROP (NORTH SIDE, FACING NORTH)



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-33

PHOTO OF OUTCROP (SOUTH SIDE, FACING SOUTH)



DESCRIPTION OF MATERIALS

A: Yellowish grey, medium-grained GRANODIORITE. Lightly weathered (W1.5-W2); strong (R3-R4). Blocks ~15 cm; equidimensional. GSI structure = Very Blocky, GSI surface = Good to Fair, GSI range = 45-55.

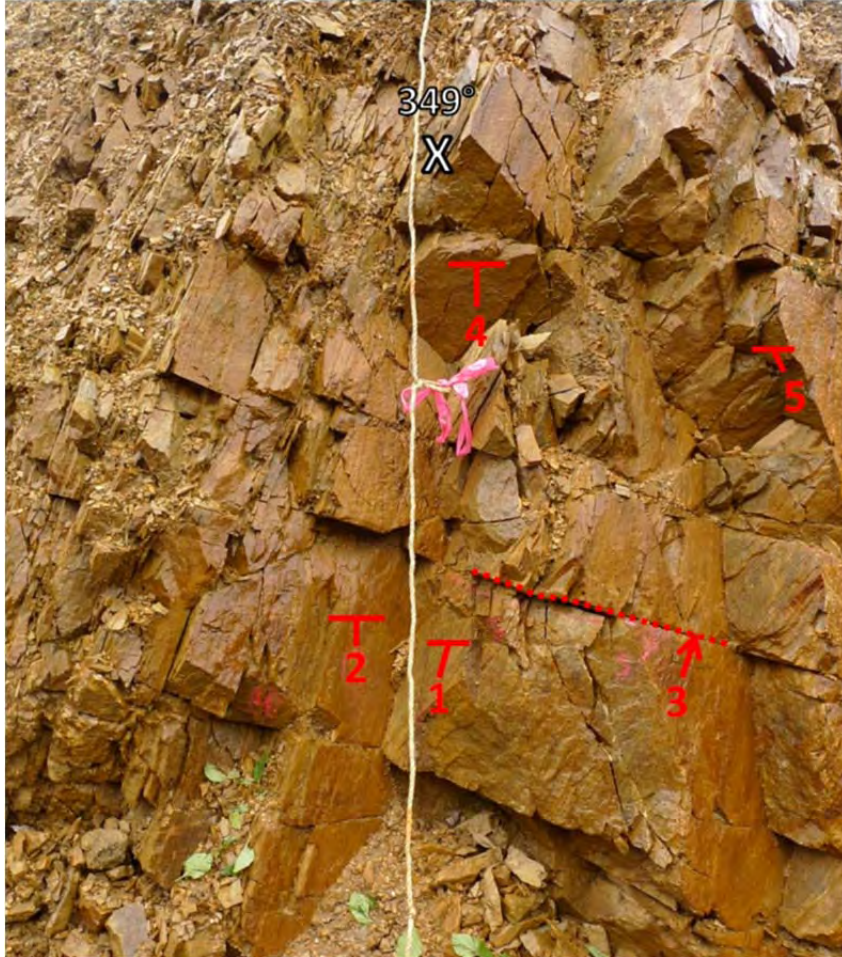
B: Light brown, fine-grained QUARTZITE. Lightly weathered (W1.5-W2); strong (R3-R4). Blocks ~ 8 cm; rhombohedral. GSI structure = Blocky/disturbed, GSI surface = Fair, GSI range = 35-45.

C: Root mat and organic silt. [TOPSOIL]

D: SAND: Fine to medium, some silt, some, gravel, well-graded (SW). Light brown, nonplastic, moist. Clasts composed of both granodiorite & metasedimentary rock, strong (R3-R4), angular. [COLLUVIUM]

Outcrop OC-BGC11-33

PHOTO OF STRUCTURE (NORTH)



DISCONTINUITY TABLE (NORTH OUTCROP, UNIT B)

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
N1	61-65	231-248	0-5	None	8 (10cm)	R3	0.05	1m+	JO
N2	82-84	132-134	1-3	None	4 (1m)	R3	0.1	2m+	J
N3	20-29	026-052	1-5	None	4 (1m)	R3	0.25	1.5m+	J
N4	58-71	347-006	1-2	None	8 (10cm)	R3	0.4	0.5m	J
N5	61-79	033-047	0	None	12 (10cm)	R3	0.5	0.3m	J

Outcrop OC-BGC11-33

PHOTO OF GRANODIORITE STRUCTURE (SOUTH OUTCROP)

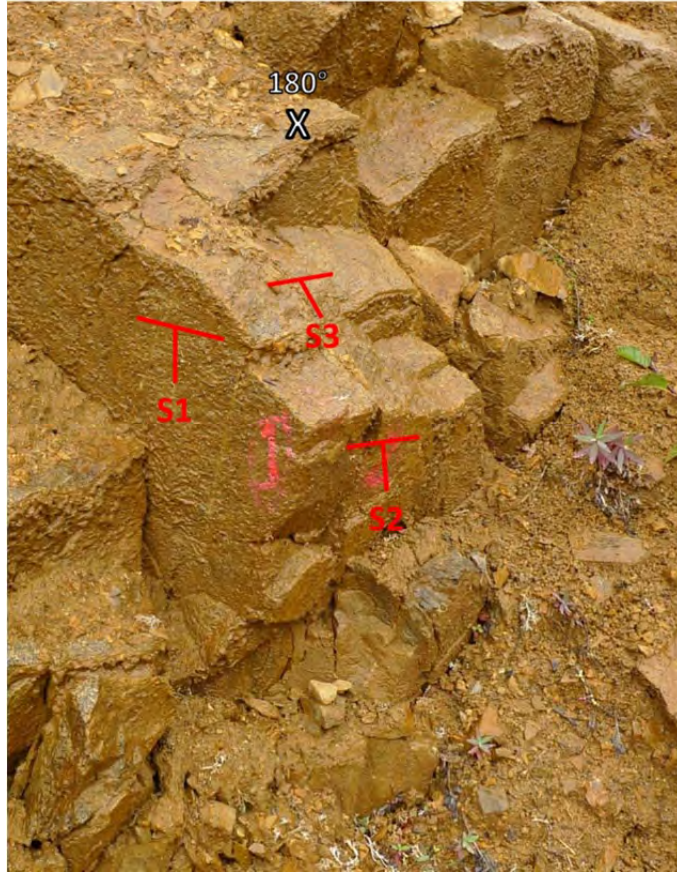
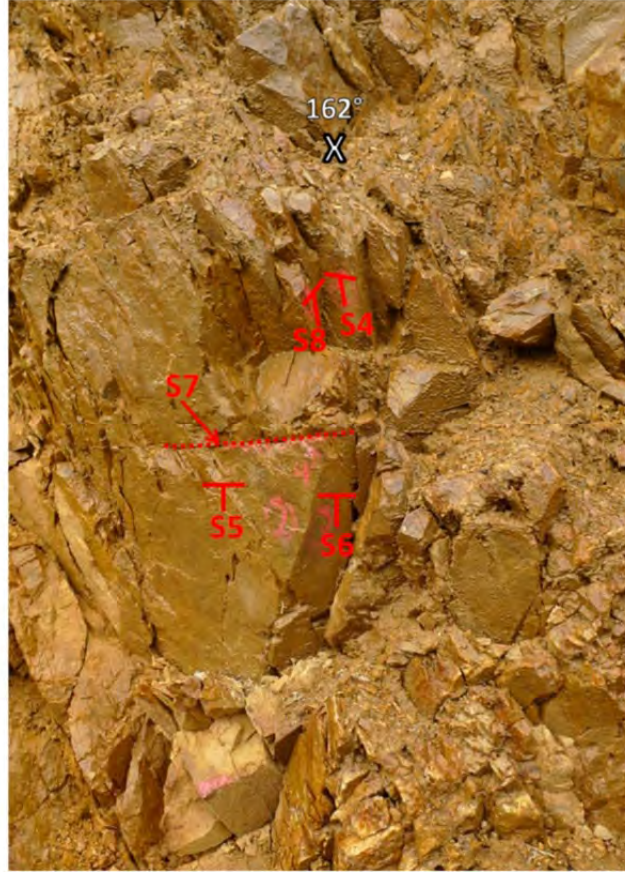


PHOTO OF QUARTZITE STRUCTURE (SOUTH OUTCROP)



DISCONTINUITY TABLE (SOUTH OUTCROP, UNITS B/A)

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
S1	69-76	225-228	2-30	BX, None	6-8(1m)	R4	0.2	1m+	J
S2	63-67	143-144	1-3	QZ	8-10(1m)	R3	0.15	1.5m+	J
S3	48	333-340	0	None	16(1m)	R4	0.2	0.5m	J
S4	46-65	194-238	0-2	None	4 (10cm)	R3	0.08	2m+	JO
S5	69-75	132-182	1	BX	12 (1m)	R3.5	NV	0.7m+	J
S6	64-66	133-138	2-5	None, BX	4 (1m)	R3.5	0.2	1m+	J
S7	16-30	240-269	0-2	None	4 (1m)	R3.5	0.3	1m+	J
S8	68-88	268-273	0.3	None	8 (10cm)	R3.5	NV	0.5m	J

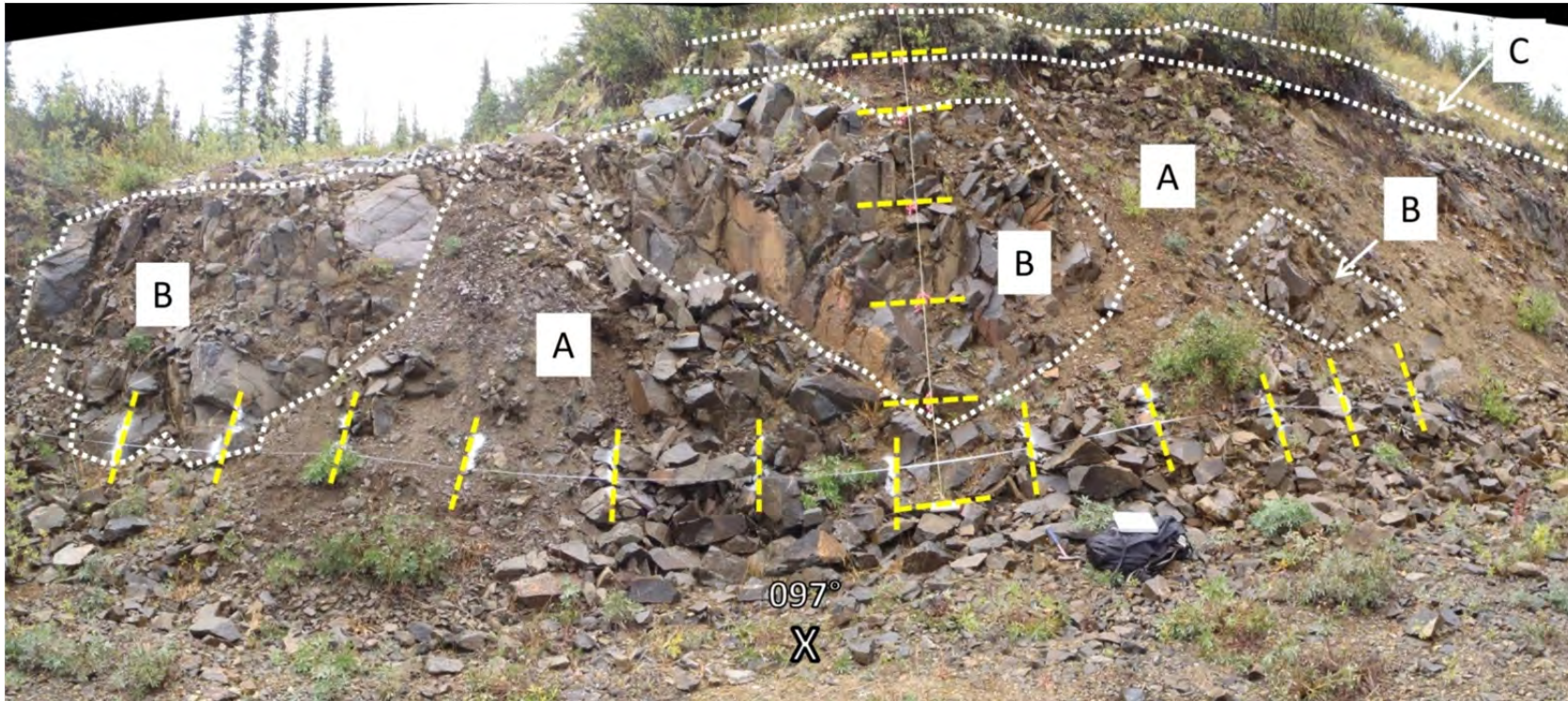
Outcrop OC-BGC11-34A

Location: Pit area
Facility: Open pit
Outcrop type: Man-made (exploration road cut)

Northing: 7099344
Easting: 460193
Elevation: 1266 m
Survey type: Handheld GPS

Slope Angle: 61°
Slope Direction: 275°
Date logged: 8/16/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: SAND: Gravelly, cobbly, some silt, well-graded (SW). Brown, nonplastic, moist, soft, homogenous. Clasts angular to sub-angular, equidimensional to flat, mostly intrusive lithology, weak to very strong (R2-R5, depending on degree of weathering). Max particle size ~ 200 mm. [COLLUVIUM]

B: Dark grey, fine-grained porphyritic igneous rock (possibly a DIKE). Groundmass of quartz & dark mafic minerals w/1-5mm plagioclase phenocrysts. Fresh with minor FE staining (W1-W1.5); very strong to extremely strong (R5-R6). Blocks ~ 30 cm; rhombohedral. GSI structure = Blocky to Very Blocky, GSI surface = Good, GSI range = 55-65.

C: Root mat and organic silt. [TOPSOIL]

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Page 1

Outcrop OC-BGC11-34A

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	86-87	130-145	1	None	2-4 (1m)	R3	0.7m	3m+	J, S
2	80-85	058-079	0	None	6-8 (1m)	R4.5	0.3m	2m+	J
3	45-70	309-326	0	None	8-10 (1m)	R5	0.3m	1m	J
4	43-58	212-236	0	None	12-16 (1m)	R5	0.4m	2m	J

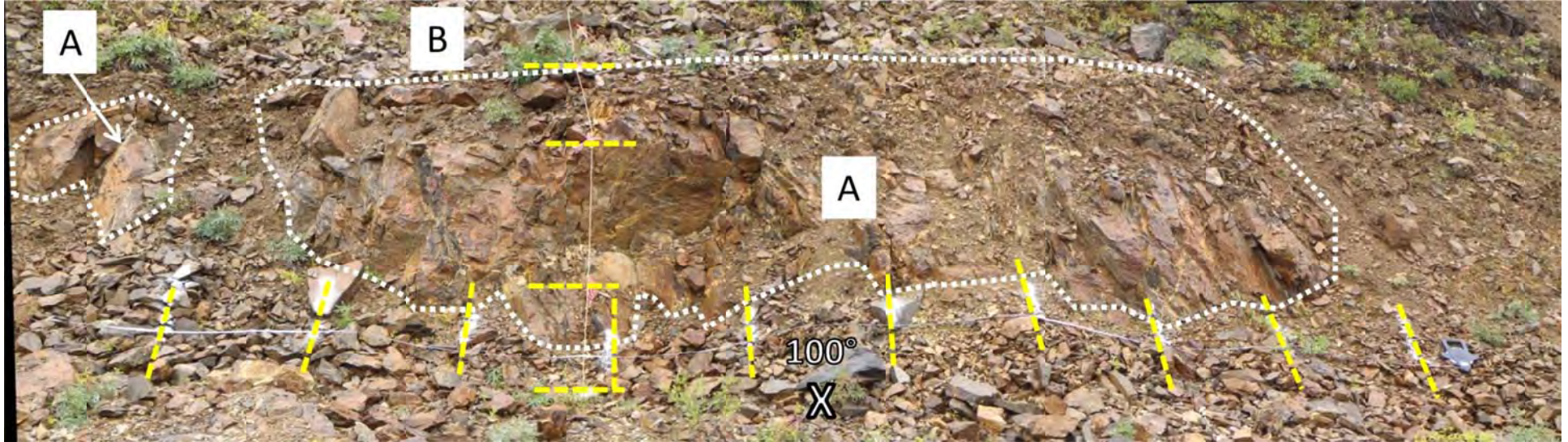
Outcrop OC-BGC11-34B

Location: Pit area
Facility: Open pit
Outcrop type: Man-made (exploration road cut)

Northing: 7099344
Easting: 460193
Elevation: 1266 m
Survey type: Handheld GPS

Slope Angle: 55°
Slope Direction: 283°
Date logged: 8/16/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

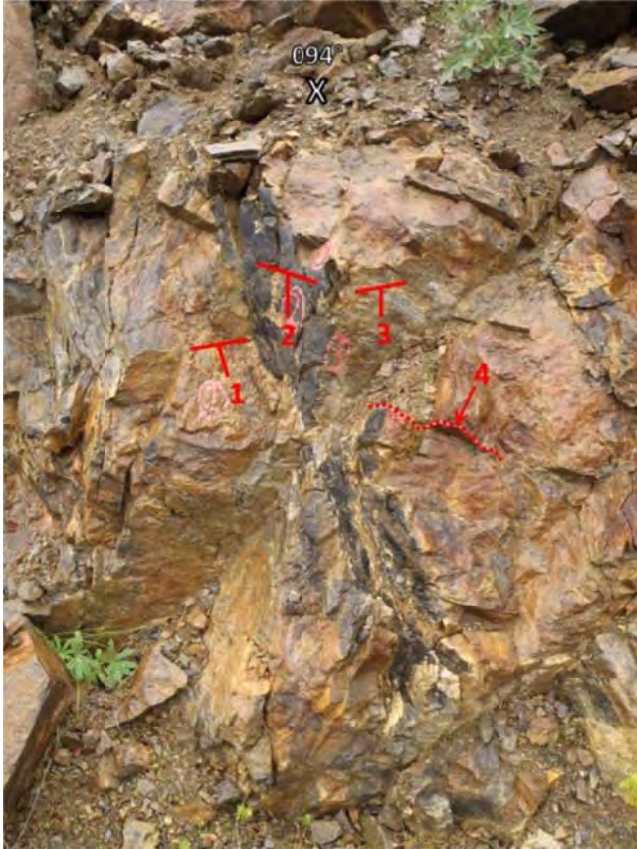
A: Dark grey, fine-grained QUARTZITE. Faintly weathered (W1.5); strong to very strong (R4-R5). Blocks rhomboidal; ~ 20 cm. GSI structure = Very Blocky, GSI surface = Good, GSI range = 50-60.

B: GRAVEL: Cobbly, well-graded (GW). Greyish-brown, nonplastic, moist, homogenous. ~60% clasts (Fine gravel to boulders, mostly intrusive lithology, angular, equidimensional to flat, very strong, max particle size ~ 300 mm); 40% matrix (30% sand, 10% silt). [COLLUVIUM]

C: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-34B

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	54-71	226-242	0-2	None	12-16 (1m)	R4	0.2	1m+	JO
2	85-88	310-320	0	None	12-16 (1m)	R4	0.3	1m+	J, V
3	70-82	076-098	1-3	SA	10-12 (1m)	R4	0.4	3m+	J
4	39-49	100-107	0-2	None	10-12 (1m)	R4	0.5	1m	J
5	59-79	334-010	0	None	15 (10cm)	R4	0.3	0.5m	J

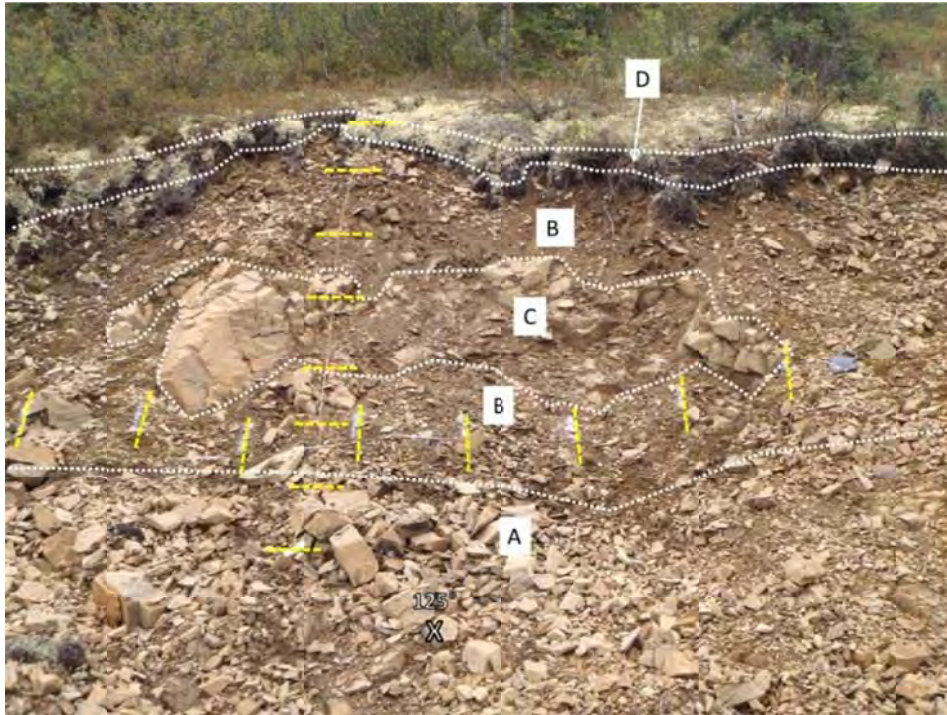
Outcrop OC-BGC11-35

Location: Stuttle Gulch
Facility: Secondary crusher
Outcrop type: Man-made (drill pad cut for BH-BGC11-37)

Northing: 7100175
Easting: 459768
Elevation: 1023 m
Survey type: Handheld GPS

Slope Angle: 50°
Slope Direction: 298°
Date logged: 8/16/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Blocky rubble derived from outcrop.

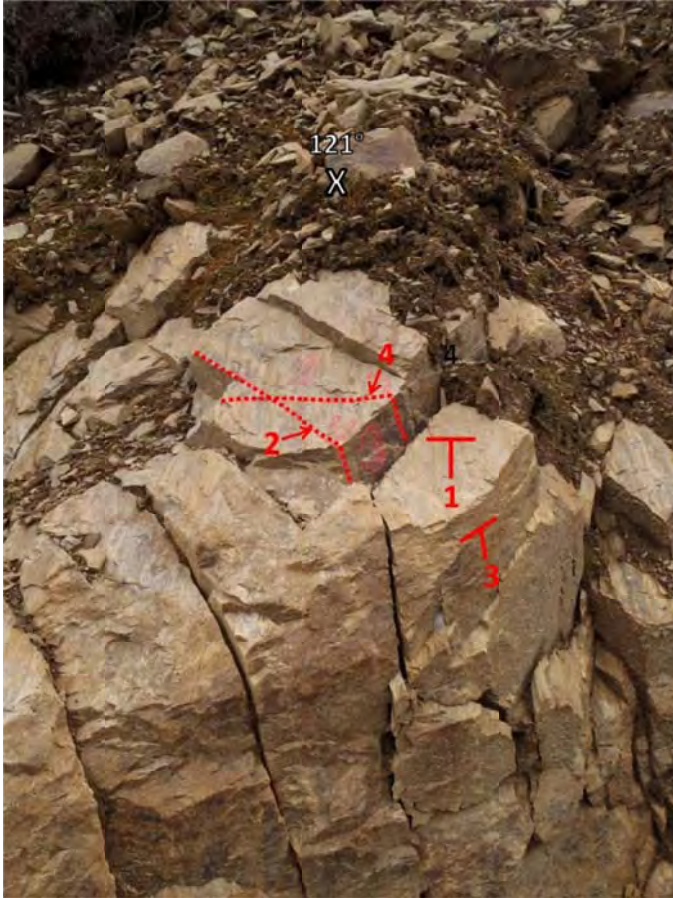
B: SAND: Fine to medium, gravelly, some silt, trace organic material, well-graded (SW-SM). Brown, nonplastic, moist, soft, and homogenous, with rapid dilatancy. Clasts are angular to sub-angular, meta-sedimentary, fine gravel to cobbles, flat and elongated, medium strong to strong (R3-R4). Max particle size ~ 200 mm. [COLLUVIUM]

C: Light brown, fine-grained QUARTZITE. Lightly weathered (W2); strong (R3-R4). Blocks ~ 15 cm; rhombohedral. GSI structure = Very Blocky, GSI surface = Good to Fair, GSI range = 45-55.

D: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-35

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT C

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	40-52	298-325	0	None	12-20 (1m)	R3	0.2	1m+	JO
2	62-80	142-160	0-5	None	4 (1m)	R3	0.3	1m+	J
3	76-89	264-298	0-50	None, SA	20 (1m)	R3	0.3	2m+	J
4	66-77	095-102	0-1	None	12 (10cm)	R3	0.5	0.3m	J

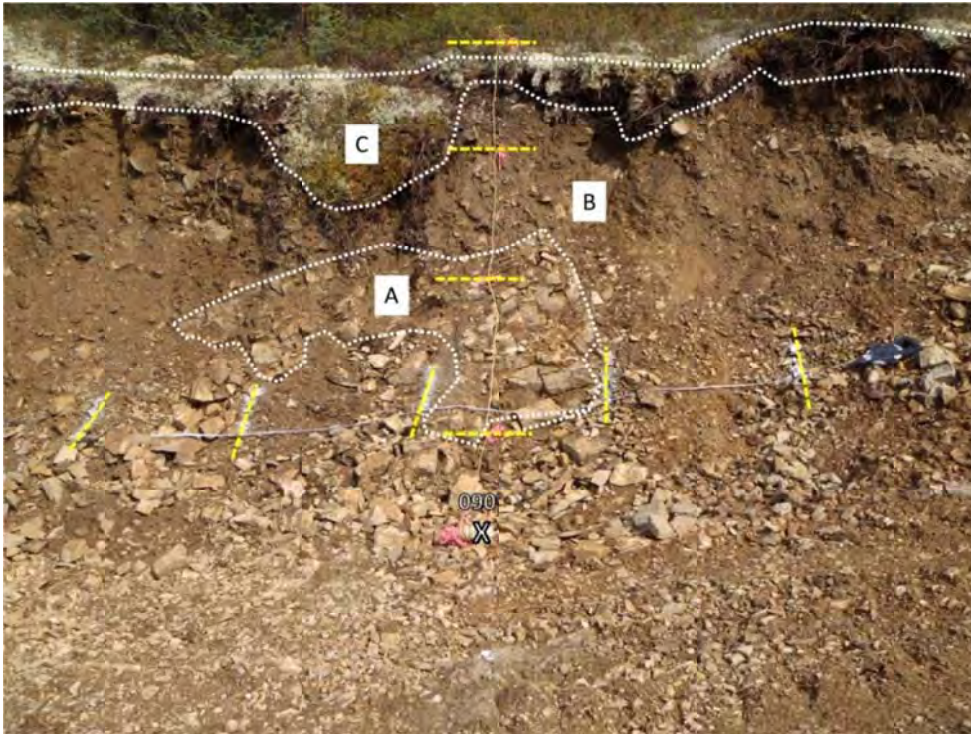
Outcrop OC-BGC11-36

Location: Stuttle Gulch
Facility: Secondary crusher
Outcrop type: Man-made (drill pad cut at BH-BGC11-36)

Northing: 7100272
Easting: 459698
Elevation: 1009 m
Survey type: Handheld GPS

Slope Angle: 60°
Slope Direction: 272°
Date logged: 8/16/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Yellowish-brown, fine-grained QUARTZITE. Lightly weathered (W2); medium strong (R3). Blocks ~ 10 cm; equidimensional. GSI structure = Very Blocky to Blocky/Disturbed, GSI surface = Good to Fair, GSI range = 40-50.

B: SAND: Silty, gravelly, trace cobbles, gap-graded (SM). Brown, nonplastic, no dry strength, rapid dilatancy, moist, firm. Silt content increases upwards; gravel increases downwards. Clasts: Metasedimentary lithology, angular to sub-angular, flat, medium strong (R3); max particle size ~ 100 mm. [COLLUVIUM]

C: Organic silt and root mat. [TOPSOIL]

Outcrop OC-BGC11-36

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	32-51	290-322	0-10	SA-SM	12 (10cm)	R3	0.1	0.5m+	JO
2	73-75	186-192	1-5	SA-SM	4 (10cm)	R3	0.1	1m+	J
3	62-65	091-098	1-10	SA-SM	16 (10cm)	R3	0.15	1.5m+	J

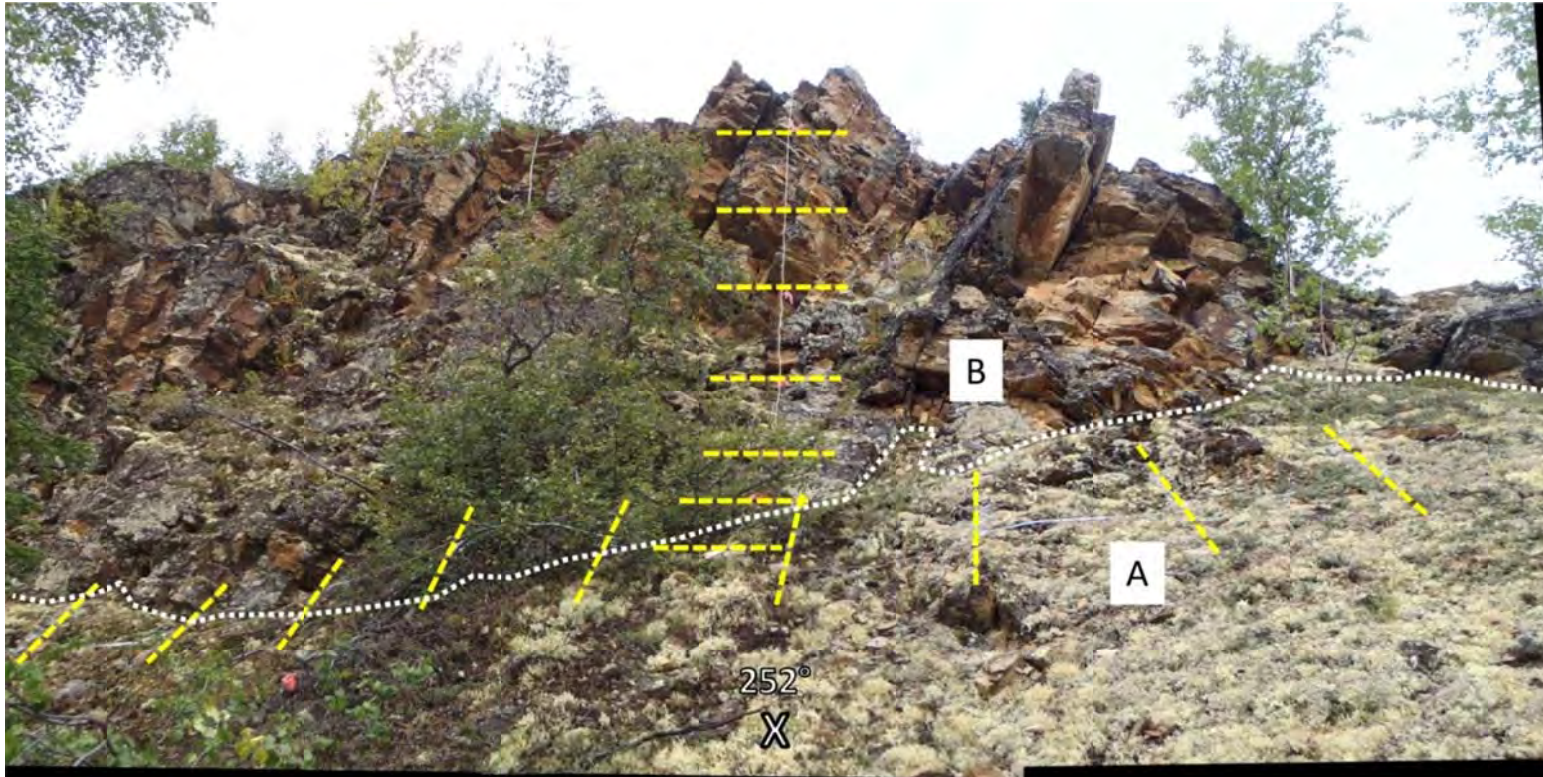
Outcrop OC-BGC11-37

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Natural

Northing: 7100383
Easting: 460067
Elevation: 1052 m
Survey type: Handheld GPS

Slope Angle: 65°
Slope Direction: 088°
Date logged: 8/17/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: GRAVEL: Some organic silt, some sand, trace cobbles, well-graded (GW-GM). Light brown, nonplastic, moist, firm to hard, and homogenous. Clasts are metasedimentary lithology derived from outcrop, flat, angular, medium strong (R3); max particle size ~ 200 mm. [COLLUVIUM]

B: Light brownish grey, fine-grained well-foliated QUARTZITE. Lightly weathered (W1.5-W2); strong (R3-R4; strength varies depending on pervasiveness of foliation planes & weathering). Blocks prismatic to rhombohedral; ~ 25 cm. GSI structure = Very Blocky, GSI surface = Good to Fair, GSI range = 45-55.

Outcrop OC-BGC11-37

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	27-32	279-315	0-20	None	12-16 (1m)	R3.5	0.2	3m	JO
2	63-67	164-172	1-40	BX, None	10-12 (1m)	R4	0.2	6m+	J
3	66-84	074-105	0-10	BX, None	12-20 (1m)	R4	0.3	1m	J
4	60-74	183-204	0-2	None	10-12 (1m)	R4	1+	1m	J

Outcrop OC-BGC11-38

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Man-made (road cut)

Northing: 7100968
Easting: 460124
Elevation: 997 m
Survey type: Handheld GPS

Slope Angle: 62°
Slope Direction: 218°
Date logged: 8/17/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: GRAVEL: Sandy, cobbly, trace silt, well-graded (GW). Brown, moist, soft, nonplastic and homogenous. Clasts are metasedimentary, sub-angular to angular, flat to elongate, strong (R4), and fine upwards from the base of the outcrop. Max particle size ~ 400 mm.

B: Grey, fine-grained QUARTZITE. Faintly weathered (W1.5); strong (R4). Blocks are prismatic; ~ 20 cm. GSI structure = Very Blocky, GSI surface = Good to Fair, GSI range = 50-60. Abundant small-scale (cm-scale) folds are present. A few blocks on outcrop surface are toppling outwards on discontinuity set 4.

C: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-38

PHOTO OF STRUCTURE



DISCONTINUITY TABLE - UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	29-35	252-330	0-5	None, SA-SM	10-20 (1m)	R3.5	0.2	4m+	JO
2	71-79	167	2-30	None, SA-SM	4 (1m)	R4	0.2	3m+	J
3	75-76	247-277	2-5	None, SA-SM	20 (1m)	R4	0.3	1m	J
4	71-73	059-076	0-100 (toppling)	None, SA-SM	20 (1m)	R4		1.5m	J

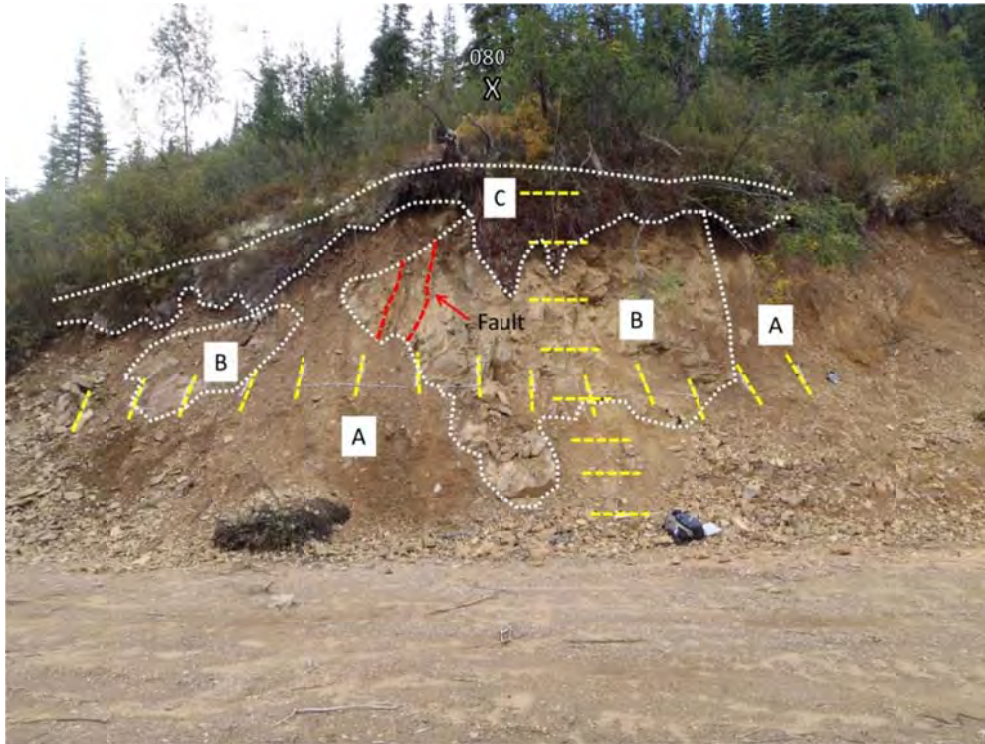
Outcrop OC-BGC11-39

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Man-made (drill pad cut)

Northing: 7100395
Easting: 460382
Elevation: 1057 m
Survey type: Handheld GPS

Slope Angle: 55°
Slope Direction: Curves 245-268° (convex)
Date logged: 8/17/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: SAND, gravelly, trace to some silt, trace cobbles, well-graded (SW-SM). Brown, nonplastic, moist, firm, and homogenous with rapid dilatancy and low dry strength. Clasts are meta-sedimentary, equidimensional to flat, angular to subangular, and medium strong to strong (R3-R4). Max particle size ~ 100 mm. [COLLUVIUM]

B: Dark grey, fine-grained QUARTZITE with veins of calcite. Lightly weathered (W1.5-W2); strong to very strong (R4-R5). Blocks ~ 20 cm; rhombohedral. GSI structure = Very Blocky, GSI surface = Good, GSI range = 50-60.

C: Root mat and organic silt. [TOPSOIL]

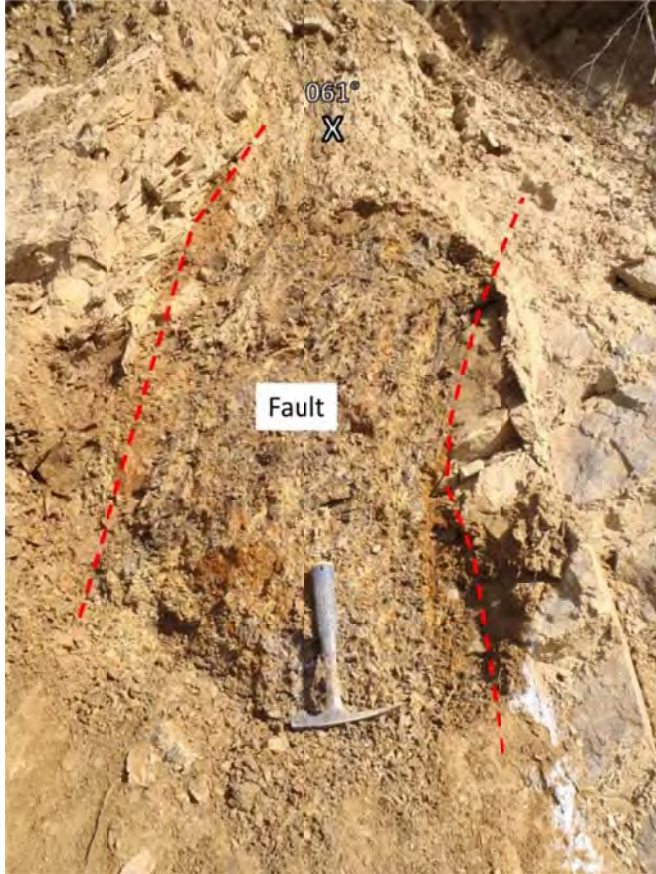
Outcrop OC-BGC11-39

PHOTO OF STRUCTURE



Outcrop OC-BGC11-39

PHOTO OF FAULT



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	52-59	293-311	0-4	None	4-20 (1m)	R4	0.2	2m+	JO
2	45-70	044-091	0-15	None	8 (1m)	R4	0.3	2m+	J
3	72-87	335-360	0	None	12 (1m)	R4	0.5	2.5m+	J
4	63-71	189-215	0-2	None	4 (1m)	R4	0.5	1m+	J
5	78-88	030-056	0	None	12 (1m)	R4	0.7	1m+	J

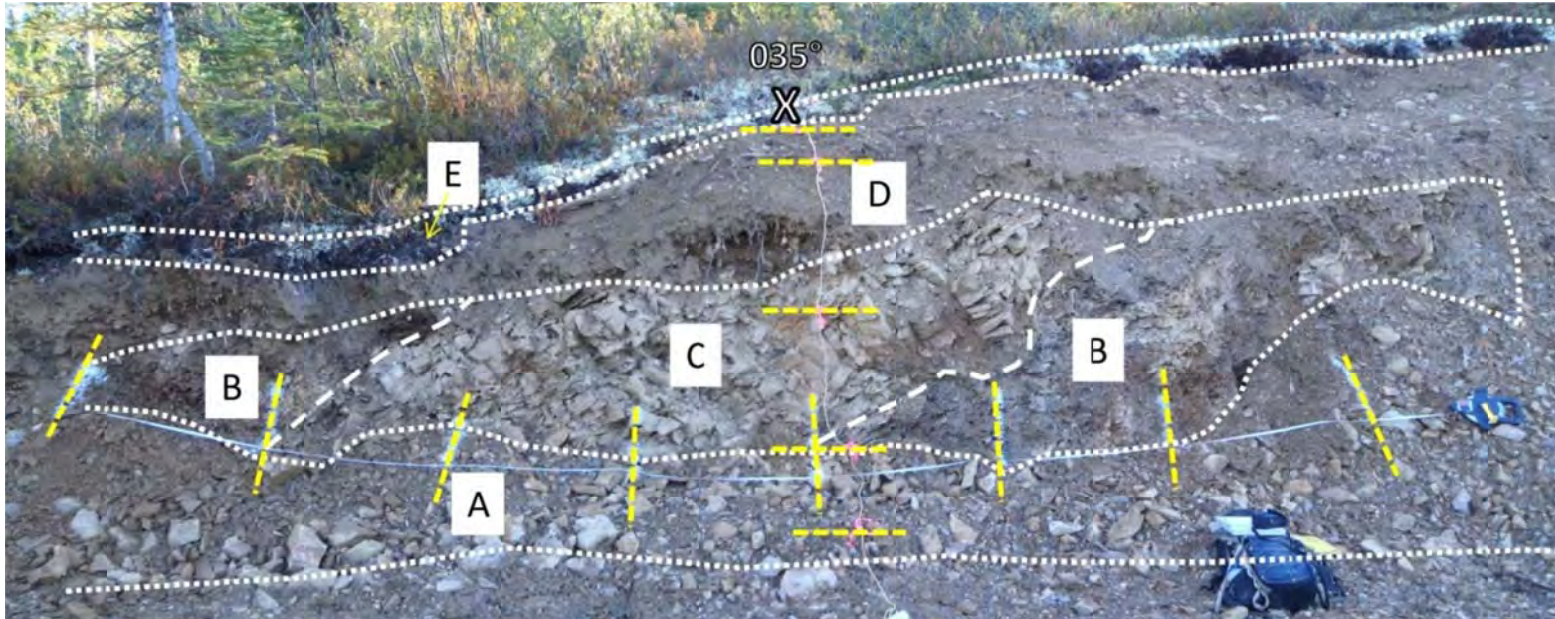
Outcrop OC-BGC11-40

Location: Ann Gulch
Facility: Heap leach pads
Outcrop type: Man-made (drill pad cut at BH-BGC11-29)

Northing: 7101809
Easting: 459981
Elevation: 1051 m
Survey type: Handheld GPS

Slope Angle: 60°
Slope Direction: 215°
Date logged: 8/18/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Fallen colluvium and rubble from outcrop.

B: Grey, fine-grained PHYLLITE interspersed with minor seams (~20%) of quartzite. Has brittly folded and crushed texture. Slightly to moderately weathered (W2-W3); very weak (R1). Blocks <1cm; tabular. GSI structure = Foliated/Laminated/Sheared, GSI surface = Fair to Very Poor, GSI range <20. NOTE: Rock mass quality possibly below the limit of GSI applicability.

C: Light brown, fine-grained QUARTZITE interspersed with micaceous phyllitic seams (~30%). Slightly weathered (W2); very weak to medium strong (R1-R3). Blocks ~ 5 cm; equidimensional to tabular. GSI structure = Blocky/Disturbed, GSI surface = Fair, GSI range = 30-40.

D: SILT: Some sand, trace fine to medium gravel, trace organics (ML). Brown, very soft, moist; low plasticity, rapid dilatancy, low toughness, no dry strength. Clasts angular to subangular, metasedimentary, flat, medium strong (R3). Max particle size ~ 5 cm. Grades into highly weathered bedrock at its base. [COLLUVIUM]

E: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-40

PHOTO OF STRUCTURE - UNIT C



DISCONTINUITY TABLE – UNITS B/C

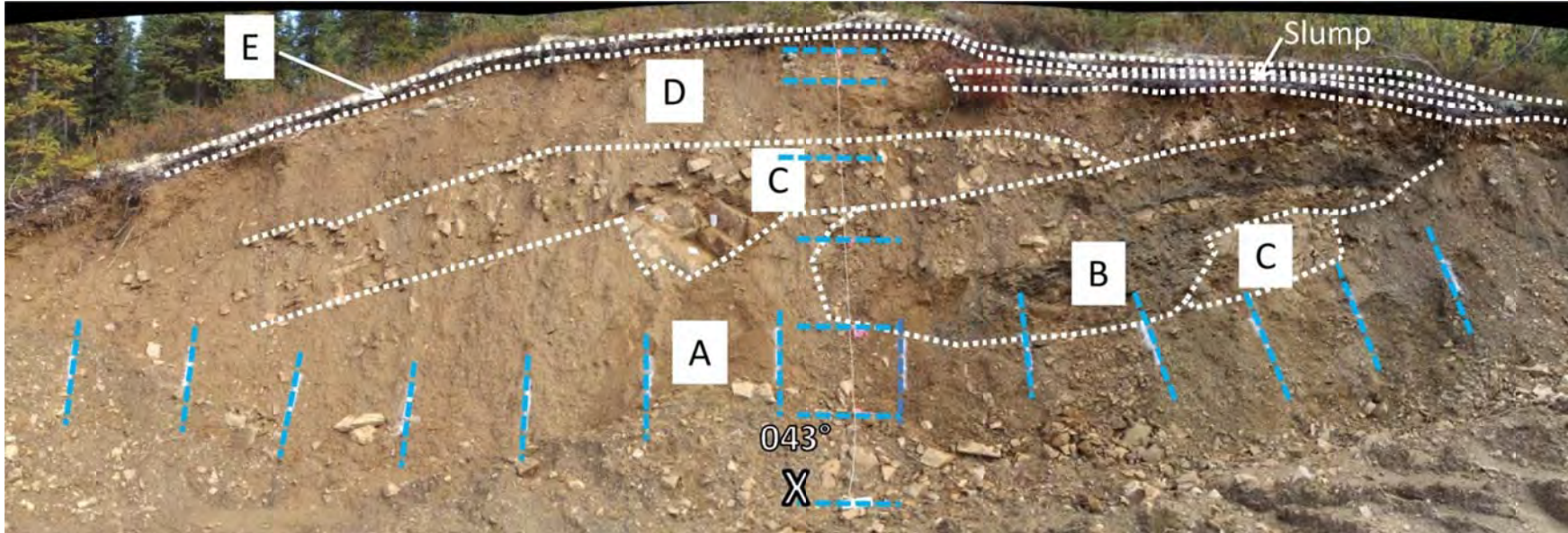
Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	29-33	278-316	1-3	None	8 (10cm)	R2	0.03	0.3m+	JO
2	55-72	158-167	1-3	None	8 (10cm)	R3	0.1	0.5m+	J
3	64-73	055-072	1-40	None, SM-SA	12 (10cm)	R3	0.08	0.3m+	J

Outcrop OC-BGC11-41

Location: Ann Gulch
Facility: Heap leach pad
Northing: 7102064
Easting: 459915
Elevation: 1048 m

Slope Angle: 50°
Slope Direction: Curves 200-241° (concave)
Date logged: 8/18/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing between blue dashed lines.

DESCRIPTION OF MATERIALS

A: Ravelled rock and colluvium draping outcrop.

B: Grey, fine-grained PHYLLITE with seams/blocks of interspersed quartzite (30%). Is sheared/weathered to a silty material in places. Lightly to highly weathered (W2-W4); extremely weak to very weak (R0-R1). Blocks tabular, <1 cm in size. GSI structure = Foliated/Laminated/Sheared, GSI surface = Poor, GSI range = <20. NOTE: rock mass quality below the limit of GSI applicability.

C: Brown, fine-grained QUARTZITE with micaceous foliation planes. Lightly weathered (W2); medium strong (R3). Blocks ~ 10 cm; rhombohedral. GSI structure = Blocky/Disturbed, GSI surface = Fair, GSI range = 35-45.

D: SILT: Sandy, trace gravel, trace organics (ML). Brown, moist, soft; low plasticity, rapid dilatancy, low toughness, no dry strength. Clasts meta-sedimentary, angular to subangular, medium strong (R3), flat. Max particle size ~ 7 cm. Coarsens downwards and grades into weathered bedrock.[COLLUVIUM]

E: Organic silt and root mat. [TOPSOIL]

Outcrop OC-BGC11-41

PHOTO OF STRUCTURE



DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	37-44	358-012	0-5	None, CX, DX	13 (10cm)	R3		1m+	JO
2	31-54	162-188	0	None, SM-SA	12 (1m)	R3		1m	J
3	85-89	071-095	0-100	None, SM-SA	20 (1m)	R3		1m	J
4	NOT A SET (IS FOLIATION WHERE FOLDED)								
5	65-70	260-267	0-100	None, SM-SA	8 (10cm)	R3		0.3m+	J

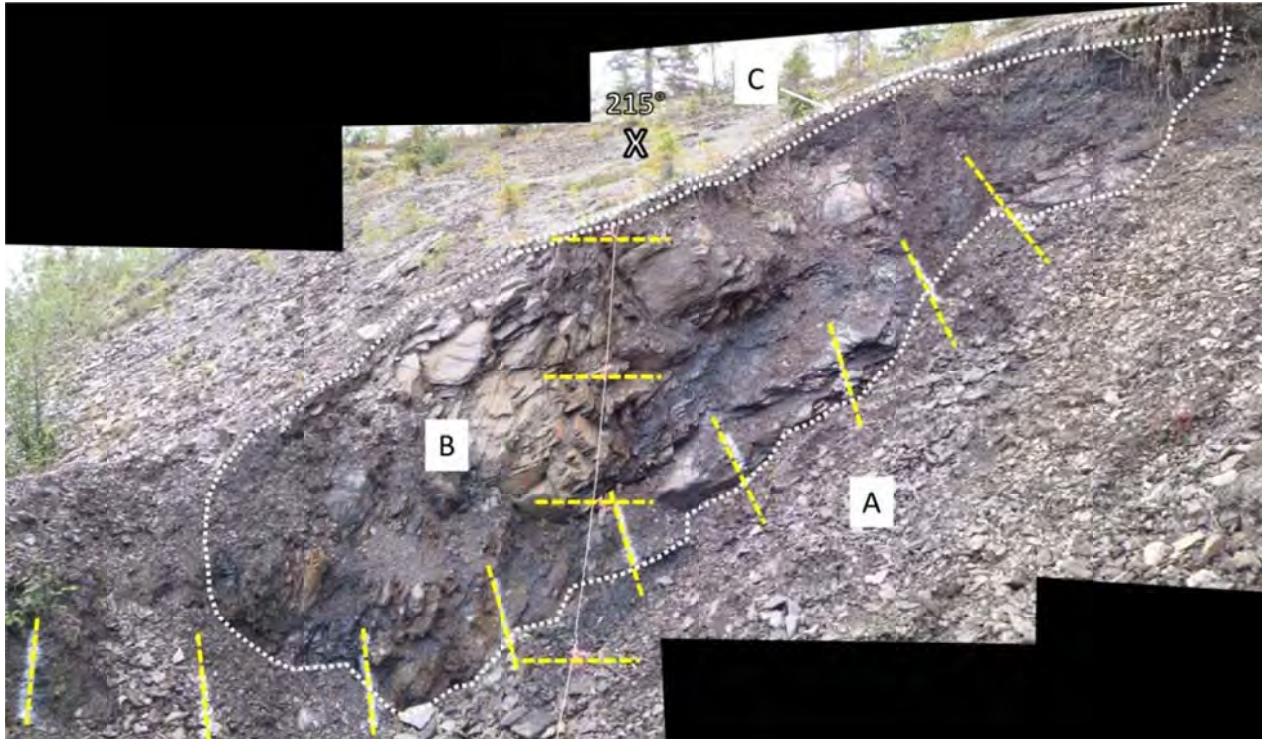
Outcrop OC-BGC11-42

Location: Ann Gulch
Facility: Heap leach pads
Outcrop type: Man-made (test pit wall)

Northing: 7101893
Easting: 459188
Elevation: 1072 m
Survey type: Handheld GPS

Slope Angle: 75°
Slope Direction: 041°
Date logged: 8/18/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing between blue dashed lines.

DESCRIPTION OF MATERIALS

A: Rubble from excavation.

B: Dark grey, fine-grained PHYLITE. Contains a few foliation-parallel 5-10 cm seams of graphitic material that are sheared and weathered to soil consistency. Lightly weathered (W2); extremely weak to weak (R0-R2). Blocks <1cm to ~ 10 cm where not sheared. GSI structure = Foliated/Laminated/Sheared, GSI surface = Poor, GSI range <20. NOTE: Rock mass quality below the limit of GSI applicability.

C: Colluvium, root mat and topsoil (~ 10 cm thick). [TOPSOIL]

Outcrop OC-BGC11-42

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	42-52	078-112	0-100	CX, SM, GP	20 (1m) - wavy	R1.5	0.01	3m+	JO
2	57-68	342-351	0.5-5	None	10 (1m) - wavy	R2	0.3	1m	J
3	50-62	193-222	0-3	None	20 (1m) - wavy	R2	0.3	1m	J

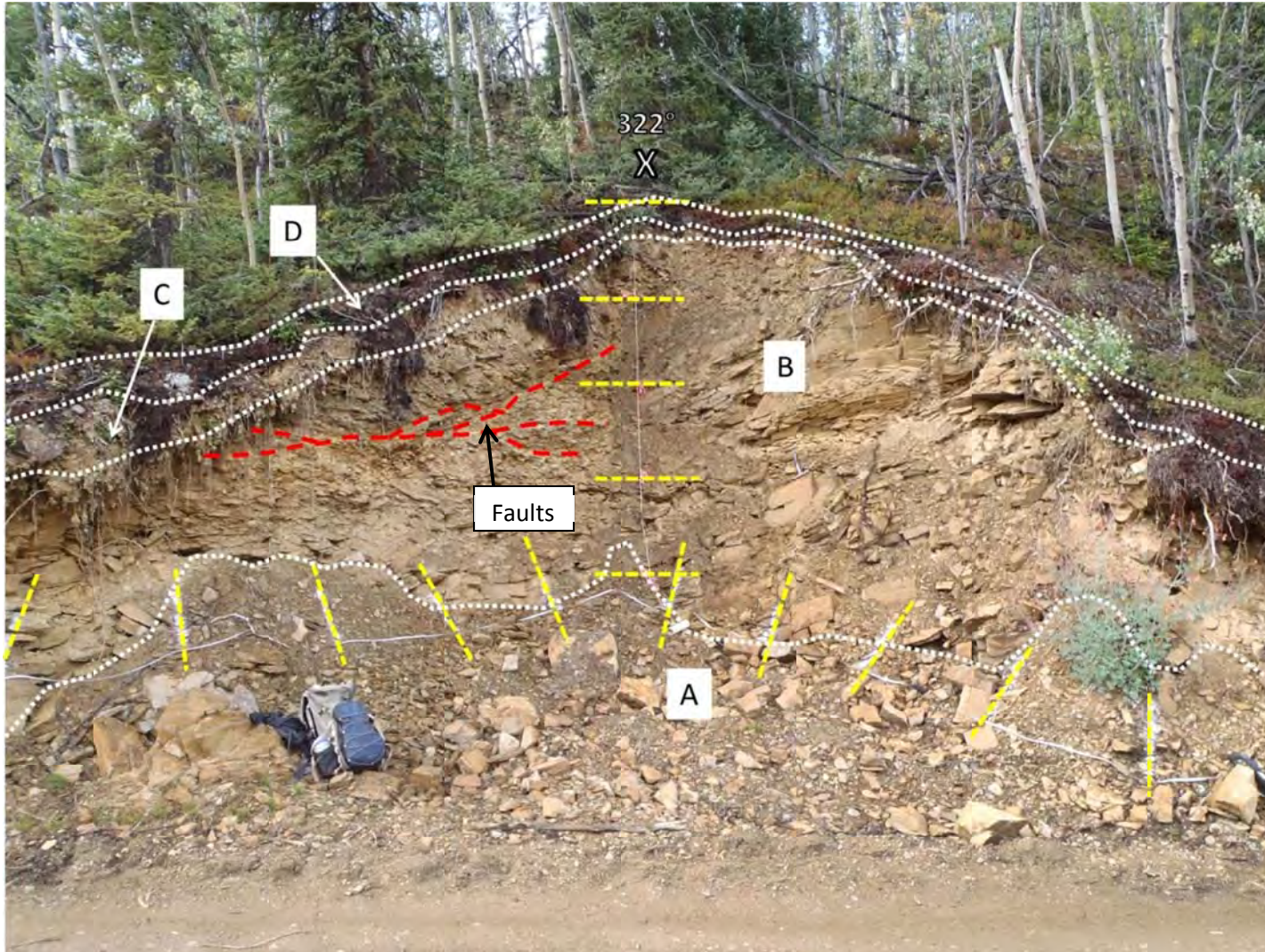
Outcrop OC-BGC11-43

Location: Middle reach Dublin Gulch
Facility: Heap leach pads
Outcrop type: Man-made (borrow cut next to road)

Northing: 7101403
Easting: 459686
Elevation: 913 m
Survey type: Handheld GPS

Slope Angle: 70-90°
Slope Direction: 095/181° (concave square corner cut into slope).
Date logged: 8/18/2011
Logged by: GH

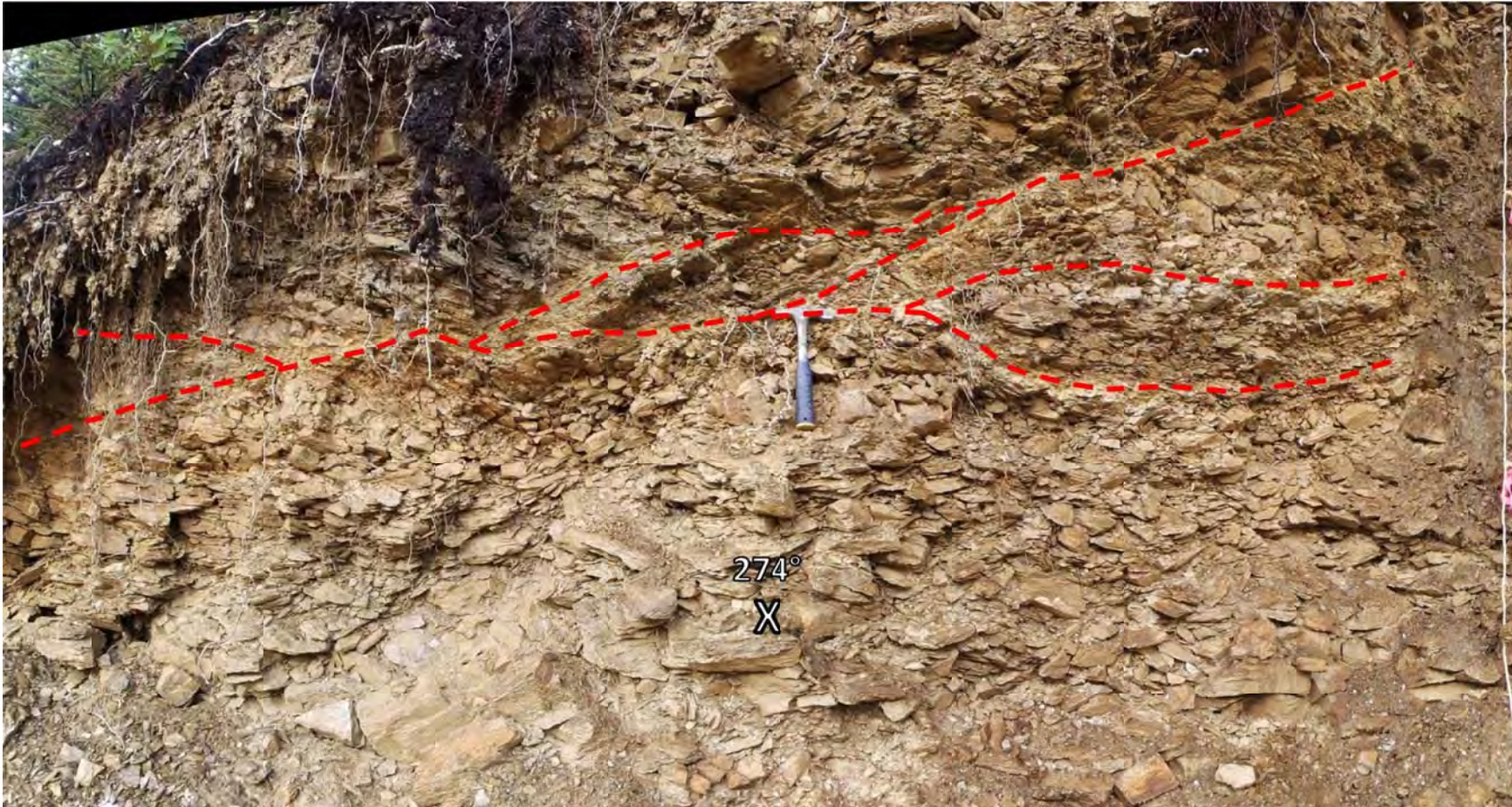
PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-43

PHOTO OF FAULTS



DESCRIPTION OF MATERIALS

A: Ravelled soil and blocks derived from the outcrop.

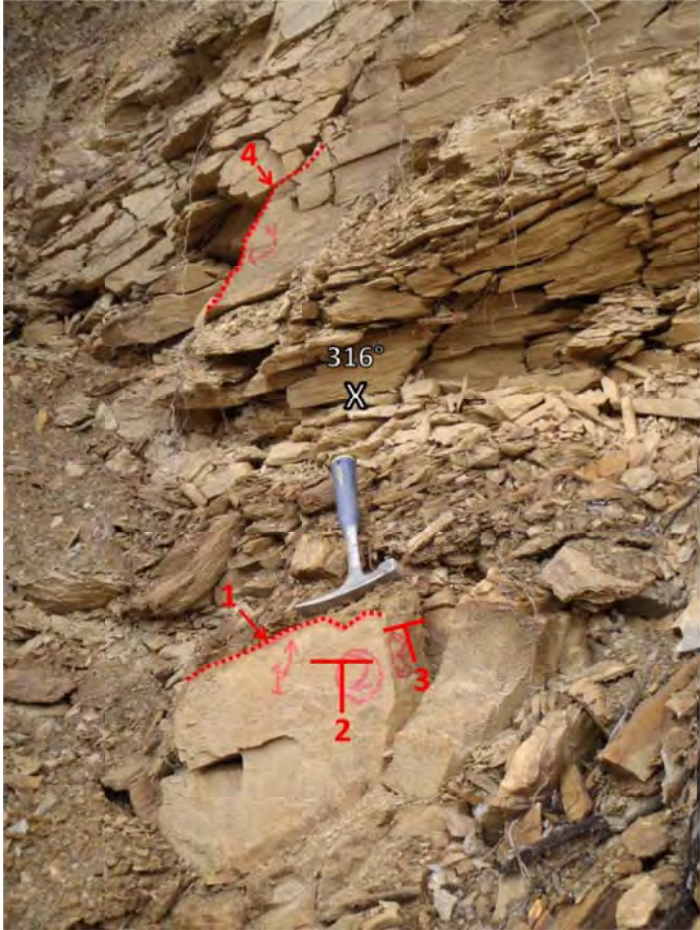
B: Brown to grey, fine-grained PHYLLITE (blockier material at base of outcrop = quartzite). Lightly to moderately weathered (W2-W3); very weak to medium strong (R1-R3), very dilated. Blocks ~ 5 cm (except 15 cm at base of outcrop); prismatic to tabular. GSI structure = Blocky/Disturbed to disintegrated, GSI surface = Poor, GSI range <25.

C: SAND: Silty, some gravel, some organics, well-graded (SM). Brown, dry, very soft, homogenous; low plasticity, rapid dilatancy, low toughness, and no dry strength. Clasts are metasedimentary, angular to sub-angular, flat, weak to medium strong (R2-R3). Max particle size ~ 100 mm. The lower contact grades downwards into weathered bedrock. [COLLUVIUM]

D: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-43

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	17-25	265-311	0-10	None, CX	6-10 (1m)	R2		2m+	JO
2	83-88	167-175	1-30	None, SA-SM	12-16 (1m)	R2.5		1m+	J
3	74-87	081-105	1-50	None, SA-SM	12 (10cm)	R2.5		1.5m+	J
4	49-72	218-244	2-15	None, SA-SM	20 (1m)	R2.5		1.5m	J
5	30-40	265-280	10-70	FB, CX	20 (1m)	R1		6m+	F

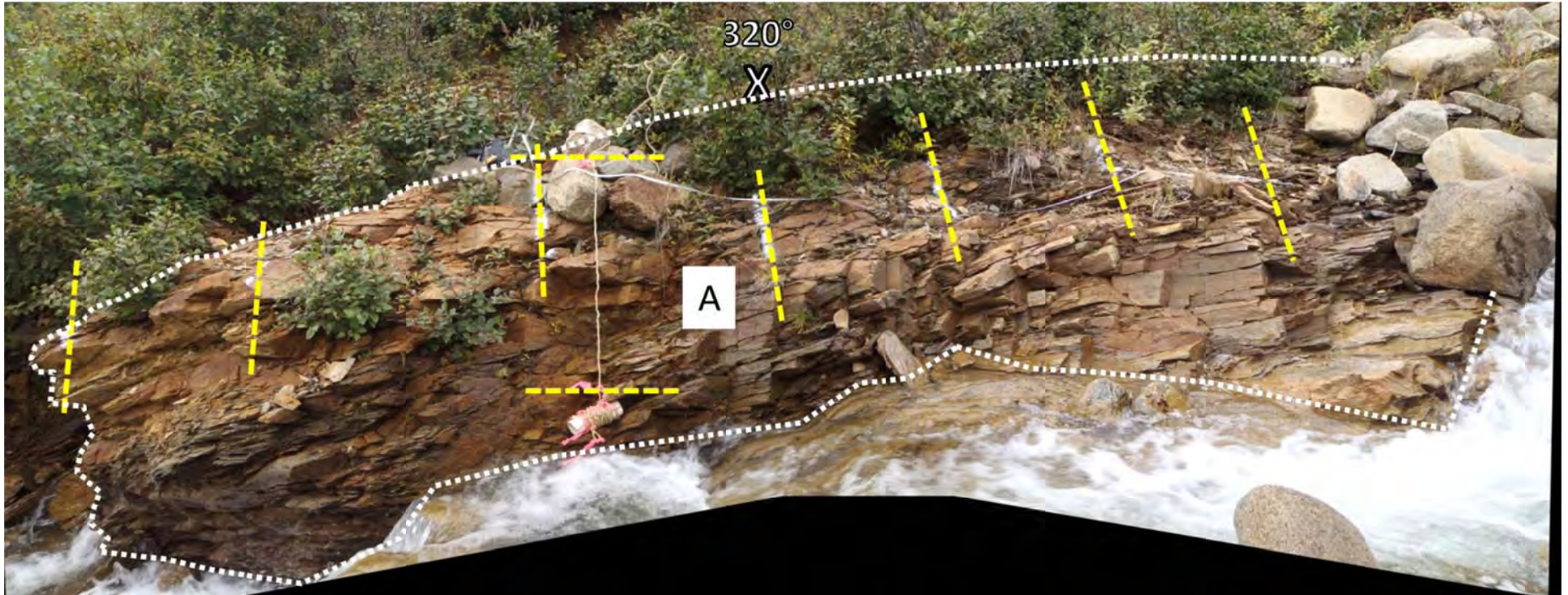
Outcrop OC-BGC11-44

Location: Middle reach Dublin Gulch
Facility: Heap leach pads
Outcrop type: Natural (stream channel)

Northing: 7101327
Easting: 459852
Elevation: 895 m
Survey type: Handheld GPS

Slope Angle: 60°
Slope Direction: 121°
Date logged: 8/19/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Brown, fine-grained QUARTZITE (60%) interbedded with seams of soft grey PHYLLITE (40%). Abundant small-scale tight folding apparent (cm to dm-scale) with axial planes oriented parallel to foliation. Lightly weathered (W2); weak (R2 - phyllite) to medium strong (R3 - quartzite). Blocks 5-10 cm; tabular (phyllite) to equidimensional (quartzite). GSI structure = Blocky/Disturbed, GSI surface = Fair, GSI range = 30-40.

Outcrop OC-BGC11-44

PHOTO OF STRUCTURE



DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	18-33	283-306	0-5	None	20 (1m)	R2.5	0.05	4m+	JO
2	72-79	080-097	0-2	None	8 (10cm)	R2.5	0.1	1m	J
3	62-69	165-169	0-3	None	4 (1m)	R2.5	0.2	1m	J
4	23-38	112-130	NV	NV	12 (10cm)	R2.5	0.8	0.6m	J

Outcrop OC-BGC11-45

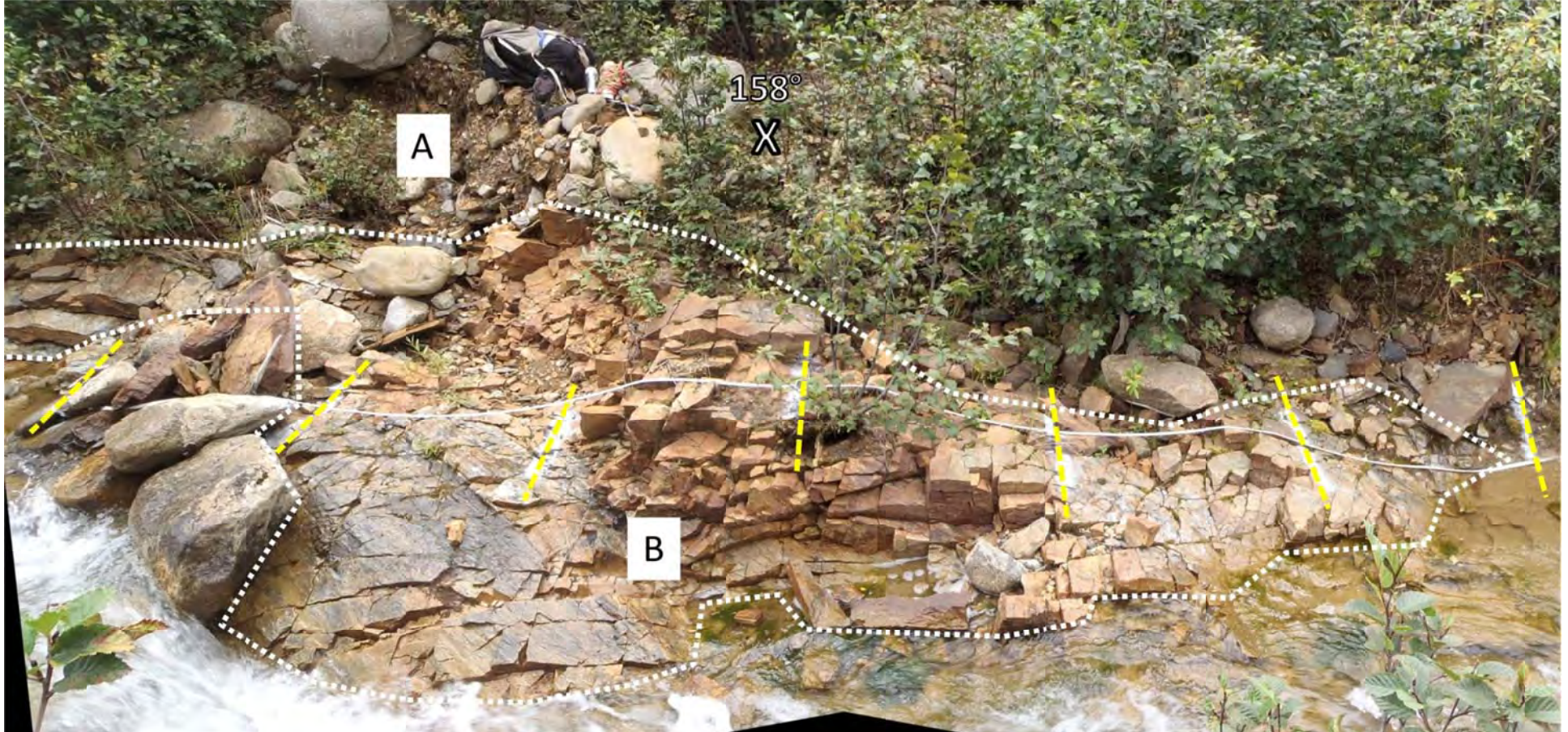
Location: Dublin Gulch valley bottom
Facility: Heap leach pads
Outcrop type: Natural (stream channel)

Northing: 7101136
Easting: 459318
Elevation: 848 m
Survey type: Handheld GPS

Slope Angle: 70°
Slope Direction: 245°
Date logged: 8/19/2011
Logged by: GH

* Station comprises two small outcrops (45 South and 45 North) on opposite sides of Dublin Creek.

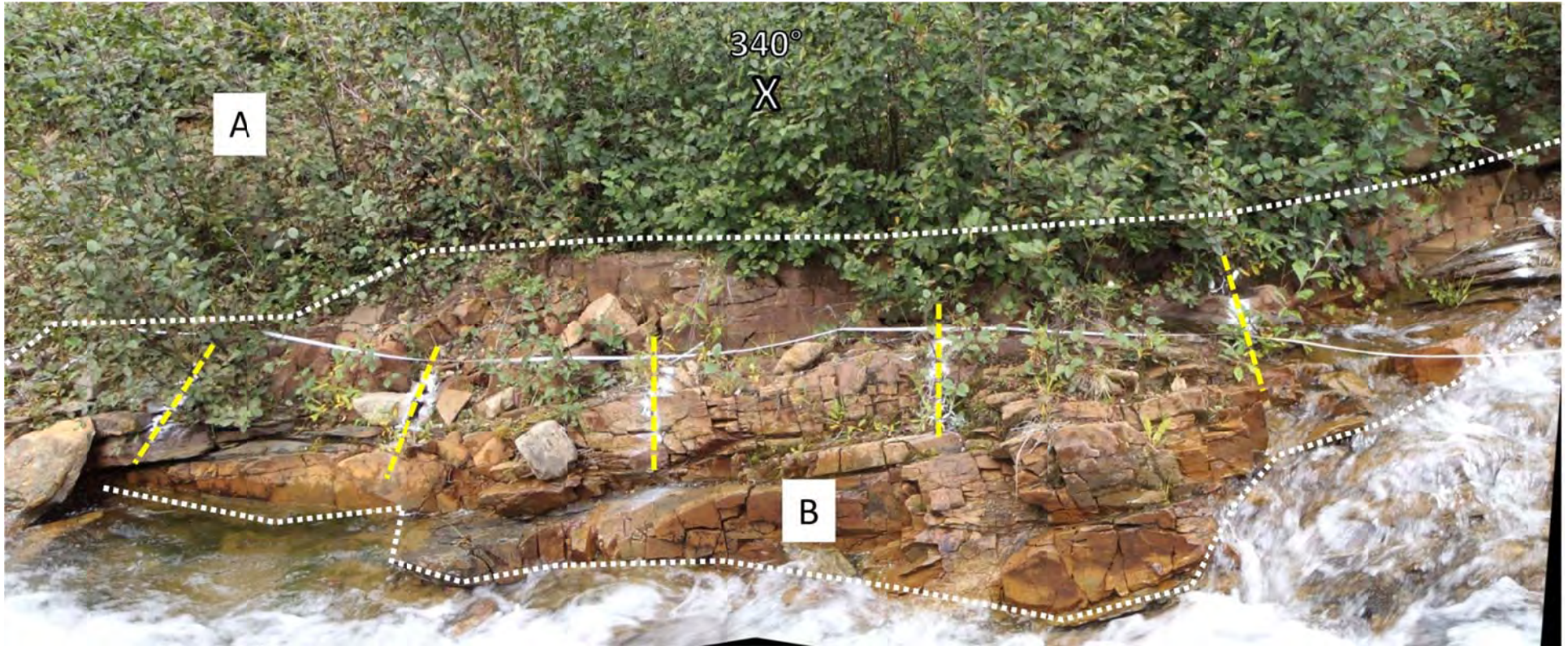
PHOTO OF OUTCROP 45 SOUTH



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-45

PHOTO OF OUTCROP 45 NORTH



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: SAND: Gravelly, some cobbles and boulders, well-graded (SW). Brown, nonplastic, moist, soft, and homogenous; No dilatancy, toughness or dry strength. Clasts are various lithologies, angular to rounded, all shapes, and max particle size ~500 mm. [PLACER TAILINGS]

B: Grey, fine-grained QUARTZITE with minor micaceous seams of PHYLLITE (~20%). Lightly weathered (W2); medium strong to strong (R3-R4). Blocks 5-10 cm; equidimensional to prismatic. GSI structure = Blocky/Disturbed, GSI surface = Good to Fair, GSI range = 40-50.

Outcrop OC-BGC11-45

PHOTO OF STRUCTURE – 45 SOUTH



DISCONTINUITY TABLE – 45 SOUTH

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
S1	28	261	0-10	None	20 (1m)	R3	0.05	5m+	JO
S2	72-82	138-150	1-5	None, SU	16 (1m)	R3	0.07	4m+	J, V
S3	81-82	234-248	1-3	None	20 (1m)	R3	0.15	1m	J

Outcrop OC-BGC11-45

PHOTO OF STRUCTURE – 45 NORTH



DISCONTINUITY TABLE – 45 NORTH

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
N1	25-29	297-337	0-10	None	20 (1m)	R3	0.05	5m+	JO
N2	84-89	154-162	1-5	None, SU	16 (1m)	R3	0.07	4m+	J, V
N3	80-86	064-089	1-3	None	20 (1m)	R3	0.15	1m	J

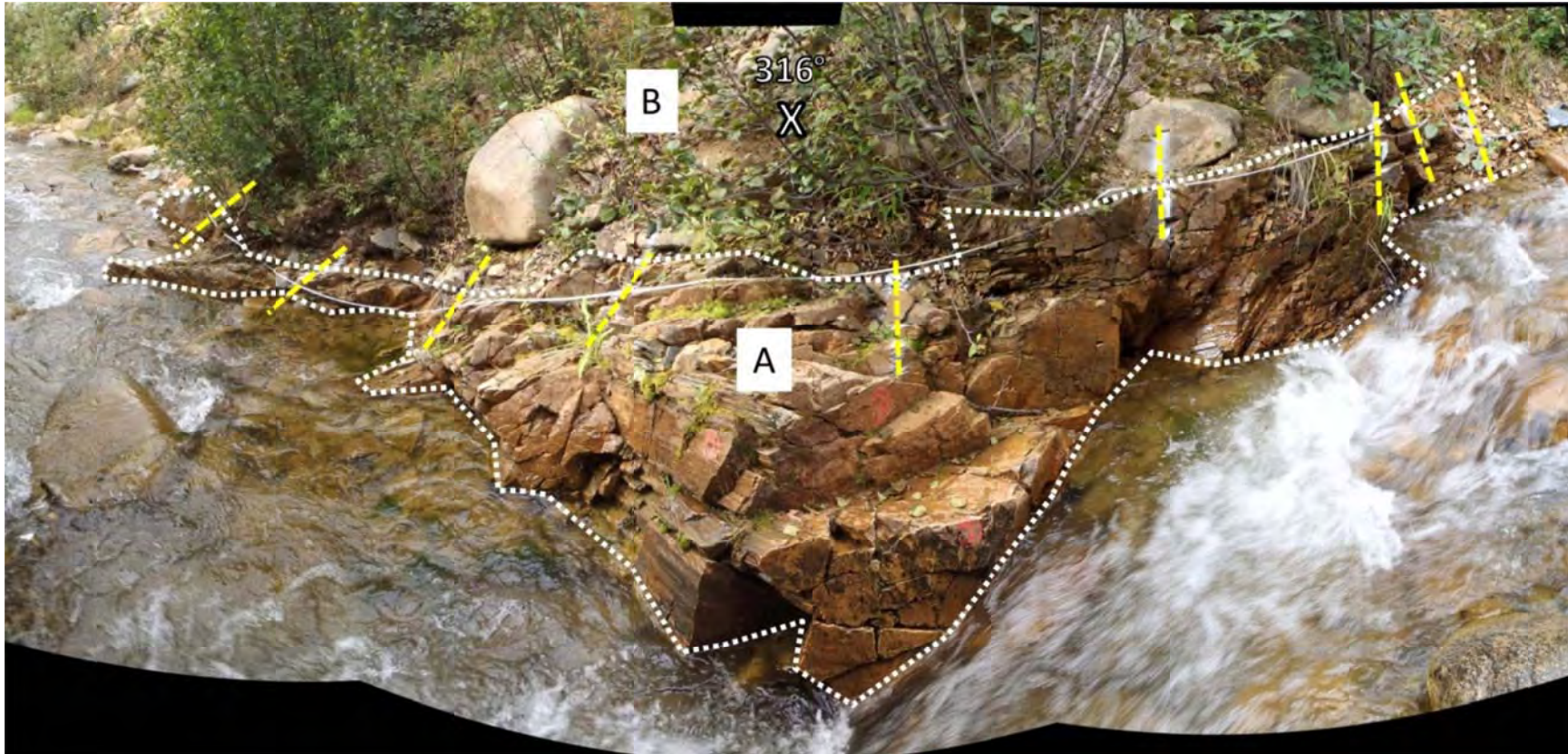
Outcrop OC-BGC11-46

Location: Dublin Gulch valley bottom
Facility: Heap leach pads
Outcrop type: Natural (stream channel)

Northing: 7101078
Easting: 459173
Elevation: 839 m
Survey type: Handheld GPS

Slope Angle: 45°
Slope Direction: Curves 158-177° (convex)
Date logged: 8/19/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Brown, fine-grained QUARTZITE. Faintly to lightly weathered (W1.5-W2); strong (R4). Blocks ~ 10 cm; prismatic. GSI structure = Very Blocky to Blocky/Disturbed, GSI surface = Good, GSI range = 45-55.

B: SAND: Gravelly, some cobbles and boulders, well-graded (SW). Brown, nonplastic firm, moist, and homogenous; no dilatancy or dry strength. Clasts are various lithologies, medium strong to very strong (R3-R5), angular to rounded, all shapes. Max particle size ~700 mm. [PLACER TAILINGS]

N:\BGC\Projects\0792 Victoria Gold\006 EG Infrastructure 2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix B Outcrop Mapping\OUTCROPLOGS\OC-BGC11-46 20120120.docxPage 1

Outcrop OC-BGC11-46

PHOTO OF STRUCTURE



DISCONTINUITY TABLE (UNIT A)

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	27-45	309-348	0-5	None	20 (1m) - wavy	R4	0.1	1m+	JO
2	79-85	143-157	0-3	None	12 (1m)	R4	0.3	1.5m+	J
3	78-90	067-089	0-5	None	20 (1m)	R4	0.15	1.5m	J
4	64-79	198-212	NV	NV	10 (1m)	R4	NV	1m	J

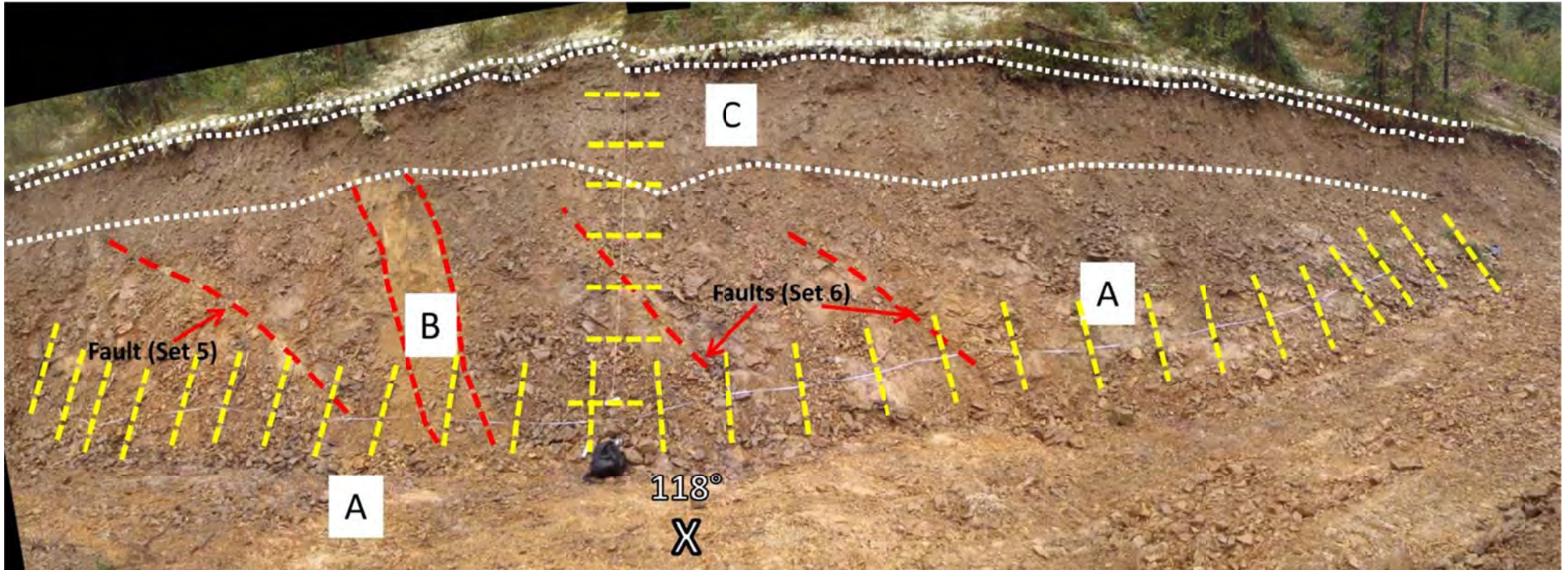
Outcrop OC-BGC11-47

Location: Stuttle Gulch
Facility: Secondary crusher
Outcrop type: Man-made (plate load test excavation cut)

Northing: 7100272
Easting: 459725
Elevation: 993 m
Survey type: Handheld GPS

Slope Angle: 50°
Slope Direction: 302°
Date logged: 8/20/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Fine-grained meta-sedimentary rock, mostly yellowish-brown, but altered to a purple-red color near the middle of the outcrop (QUARTZITE?). Moderately weathered (W3); weak to medium strong (R2-R3), except extremely to very weak (R0-R1) near faults and at sandy dike. Blocks ~ 0.05 m; tabular. GSI surface = Poor, GSI structure = Disturbed, GSI range = 25-35.

B: Medium-grained yellow SAPROLITE (intrusive dike). Completely weathered (W5); extremely to very weak (R0-R1). Effectively medium sand with silt and clay (SM). Nonplastic. ~ 1 m wide.

C: [COLLUVIUM]. See test pit log for TP-BGC11-50.

D: Root mat and organic silt. [TOPSOIL] See test pit log for TP-BGC11-50.

Outcrop OC-BGC11-47

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	40-45	294-308	0-5	SA-SM	16 (1m)	R2	0.05	1.5m	JO
2	52-66	163-187	0-5	CX	12 (1m)	R2.5	0.15	2m+	J
3	54-71	072-094	0-5	SA-SM	8 (10cm)	R2.5	0.1	2m+	J
4	59-72	261-285	NV	SA-SM	12 (10cm)	R2.5	NV	0.5m+	J
5	41	247	100	FG	16 (1m)	R1		6m+	F
6	43-62	178-205	30	CX/DX	12 (1m)	R1		5m+	F

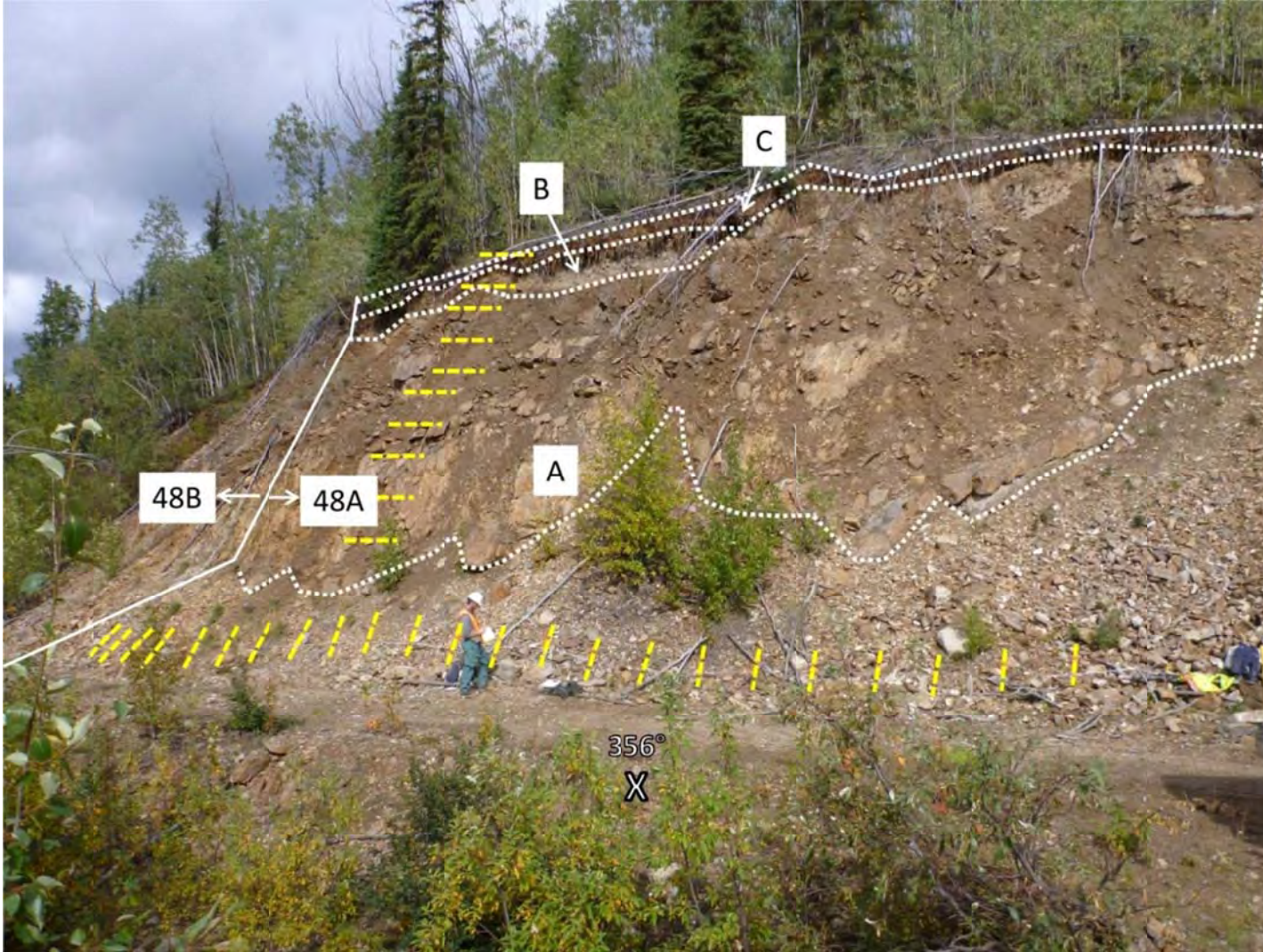
Outcrop OC-BGC11-48A

Location: Dublin Gulch valley bottom
Facility: Plant site
Outcrop type: Man-made (road cut)

Northing: 7101009
Easting: 458635
Elevation: 807 m
Survey type: Handheld GPS

Slope Angle: 68°
Slope Direction: Curves 192-219° (convex)
Date logged: 8/21/2011
Logged by: GH/LGT

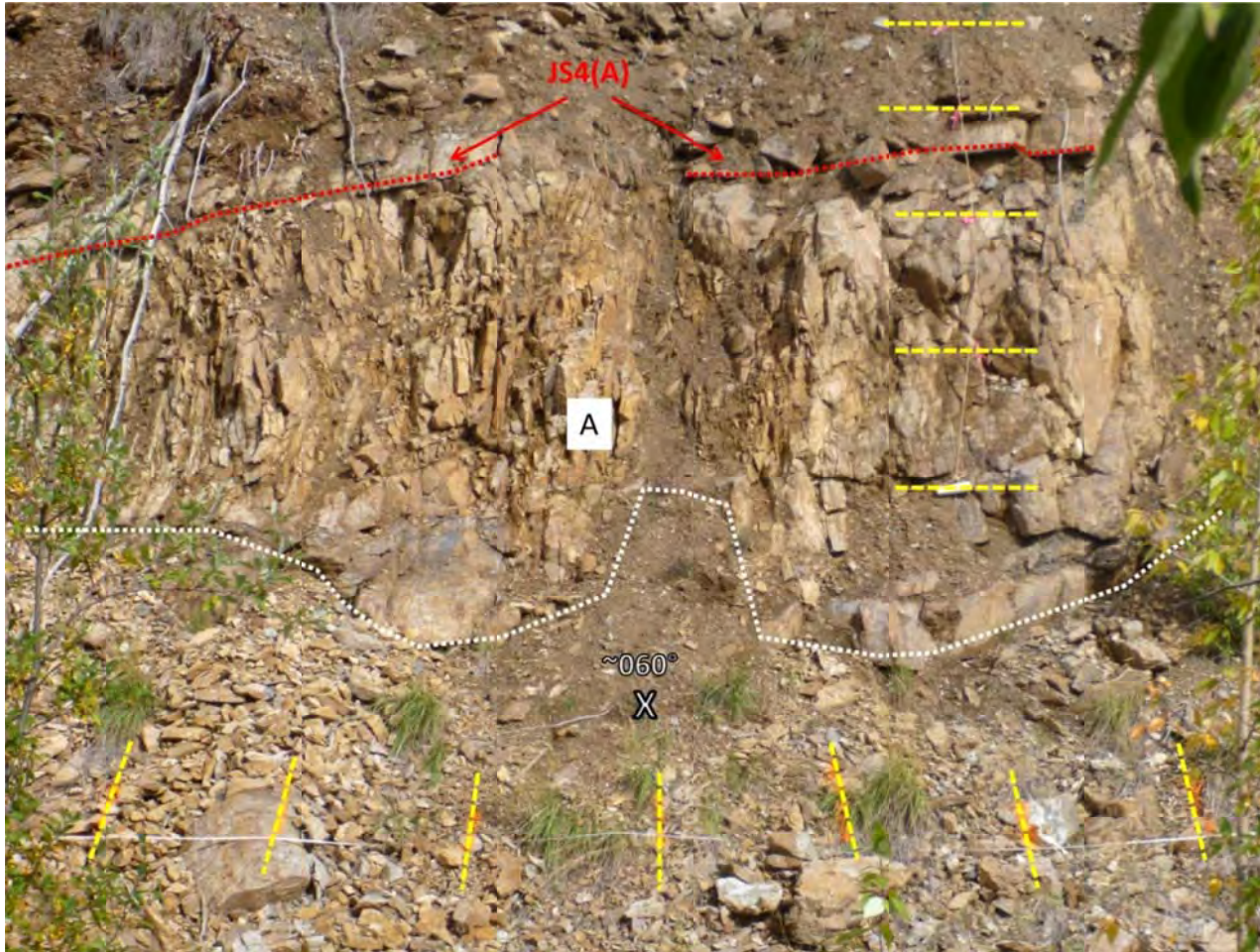
PHOTO #1 OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-48A

PHOTO #2 OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

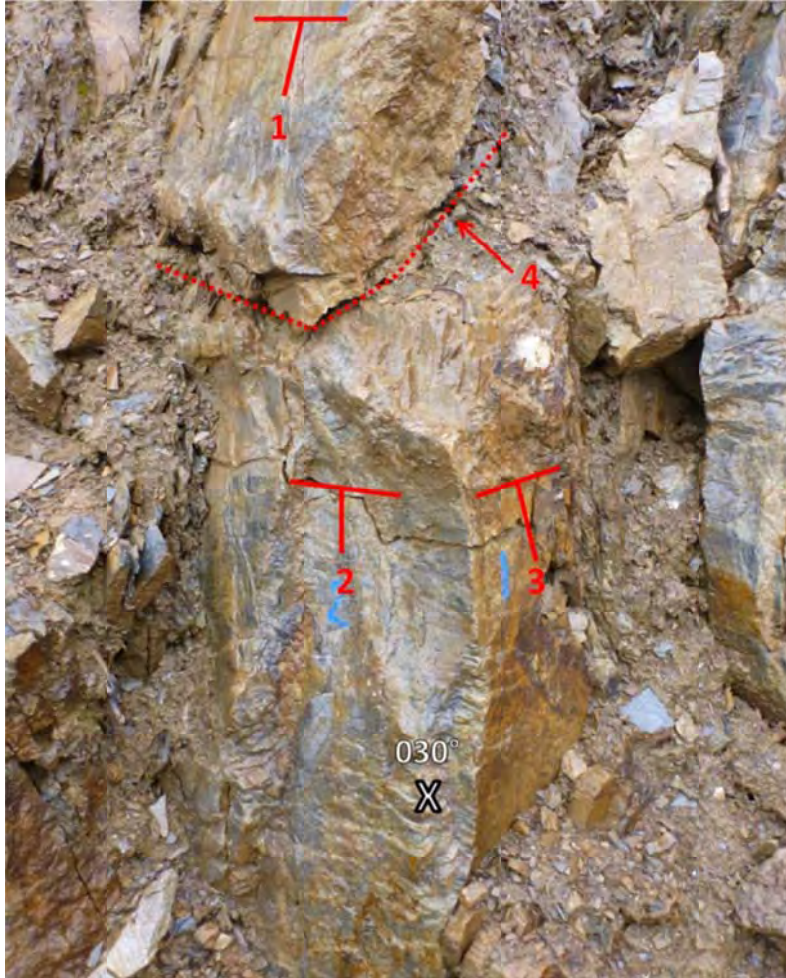
A: Reddish-brown, fine-grained QUARTZITE. Highly fractured at cm-scale - usually shatters when struck with a geological hammer. Lightly weathered (W2); medium strong (R3). Blocks rhombohedral; ~ 5 cm. GSI structure = Disturbed/Seamy, GSI surface = Fair, GSI range = 30-40.

B: GRAVEL: Some sand, trace silt, trace boulders, trace rootlets, well-graded (GW). Brown, dry to moist, homogenous, non-cemented. Clasts are flat to elongate, angular, metasedimentary, medium strong to strong (R3-R4). Max particle size 400 mm. [HIGHLY WEATHERED ROCK]

C: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-48A

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	72-86	140-170	0-10	None	4 (0.3m)	R3	0.05	3m+	J
2	46-70	243-251	NV	None	12 (0.3m)	R3	1	1m	J
3	57-76	235-252	0-10	None	12 (0.3m)	R3	0.2	1m	JO
4	28-35	236-241	100	BX	16 (1m)	R3	NV	8m+	J, F

Outcrop OC-BGC11-48B

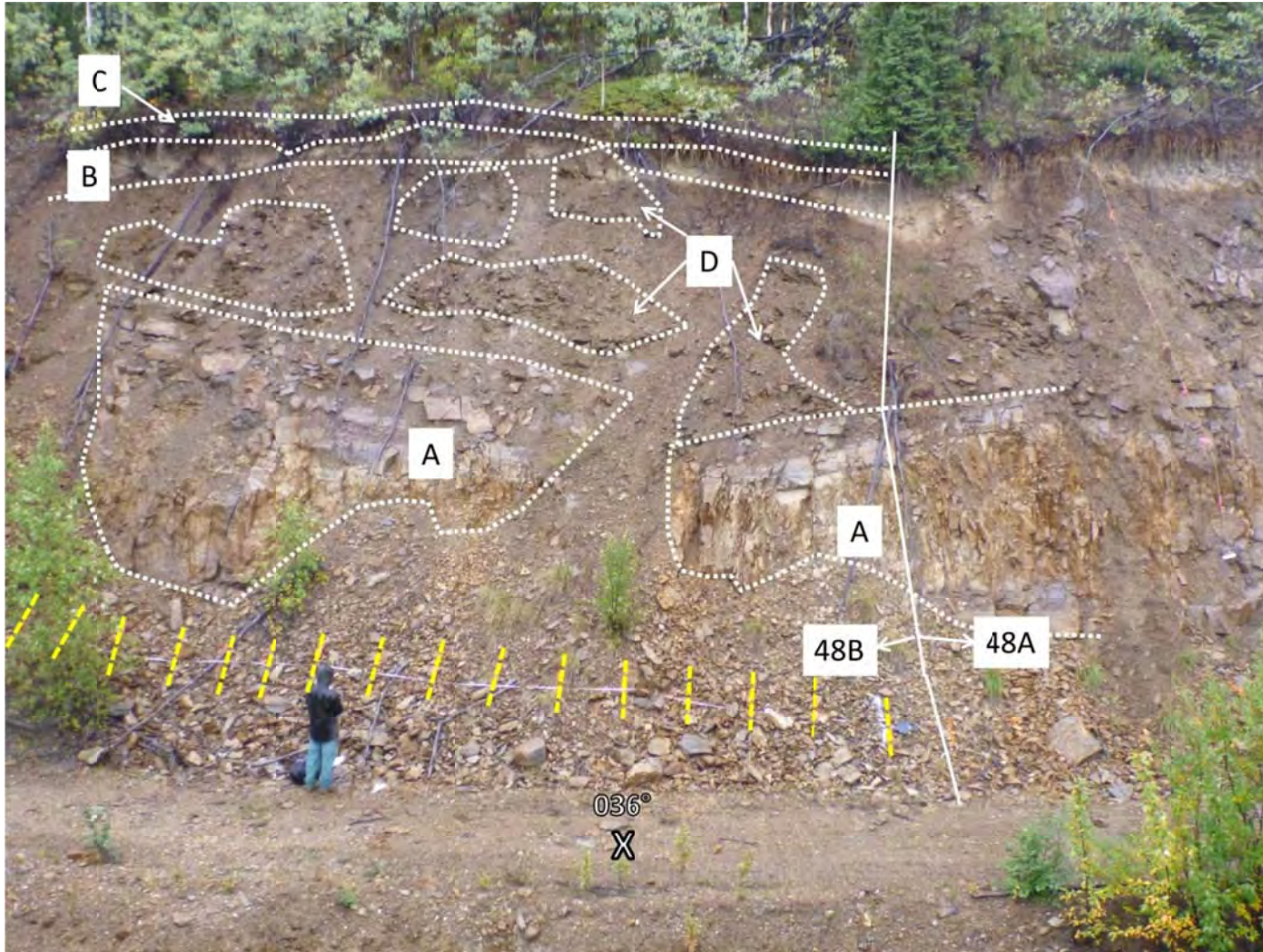
Location: Dublin Gulch valley bottom
Facility: Plant site
Outcrop type: Man-made (road cut)

Northing: 7101022
Easting: 458616
Elevation: 805 m
Survey type: Handheld GPS

Slope Angle: 55°
Slope Direction: 239°
Date logged: 8/21/2011
Logged by: GH/LGT

* Outcrop is immediately adjacent (west) of OC-BGC11-48A.

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-48B

DESCRIPTION OF MATERIALS

A: Yellowish-brown, fine-grained QAUARTZITE. Foliated, faintly to lightly weathered (W1.5-W2); medium strong (R3). Blocks are prismatic. Discontinuities are extremely close to closely spaced and are medium persistent. GSI structure = Blocky/Disturbed, GSI surface condition = Good, GSI range = 40-50.

B: GRAVEL: Some sand, trace cobbles and boulders, trace silt, well-graded (GW). Brown, dry to moist, homogenous, non-cemented. Clasts are flat to elongate, angular, metasedimentary, highly weathered (W4); medium strong to strong (R3-R4). Maximum particle size 400 mm. [HIGHLY WEATHERED ROCK]

C: Root mat and organic silt. [TOPSOIL]

D: Greyish-brown, fine-grained QUARTZITE. Moderately weathered (W3); medium strong (R3). 3 main discontinuity sets with loose apertures (rock mass is well-dialated) and infilled with CX or iron stained. GSI structure = disintegrated, GSI surface = Poor, GSI range <25.

DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	82-87	231-247	0-5	None	20 (1m)	R3	0.05	2m+	JO
2	71-90	320-327	0-30	QZ, FE	12 (1m)	R3	0.1	4m+	J, S
3	47-54	215-233	3-100	BX, CX	20 (1m)	R3	0.5	10m	J, F
4	67-70	321-336	0-3	None	8 (10 cm)	R3	0.2	1m	J

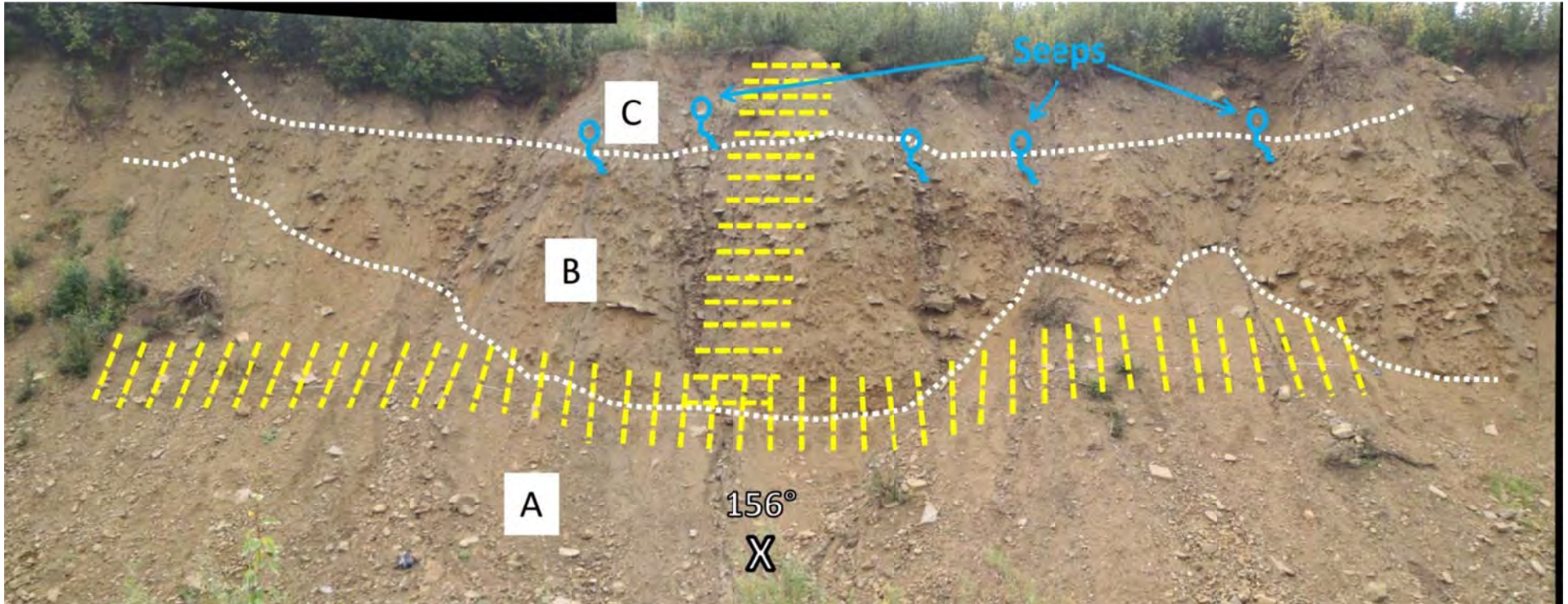
Outcrop OC-BGC11-49

Location: Dublin Gulch valley bottom
Facility: Heap leach pads
Outcrop type: Man-made (excavated by placer miners)

Northing: 7101020
Easting: 459452
Elevation: 859 m
Survey type: Handheld GPS

Slope Angle: 66° (unit B); 41° (unit A)
Slope Direction: 336°
Date logged: 8/22/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Ravelled material from scarp.

B: GRAVEL: Sandy (medium to coarse), cobbly, trace boulders, well-graded (GW). Brown, nonplastic, firm, moist, homogenous (massive), and poorly consolidated. Fines notably absent. Clasts angular to subrounded, many lithologies (various types of metasedimentary or intrusive rock), weak to very strong (R2-R5), all shapes. Max particle size ~ 500 mm. Deposit contains layers of aligned cobbles dipping gradually to the west. This is suggestive of being the distal part of a debris fan originating further east along Dublin Gulch, where larger debris is found. This interpretation remains speculative for the time being. Other plausible interpretations include placer tailings (least likely) and till (lateral moraine, believed to be less likely than debris flow hypothesis).

C: SAND: Silty, some gravel, trace cobbles, well-graded (SM). Lensed with pockets of sandy silt 1-5 cm thick. Brown, moist, soft, nonplastic, with low dry strength, rapid dilatancy, low toughness. Clasts are angular to subrounded, all shapes, various lithologies, weak to strong (R2-R4). [FILL]

Outcrop OC-BGC11-49

PHOTO OF UNIT B



DISCONTINUITIES N/A (NO ROCK UNITS)

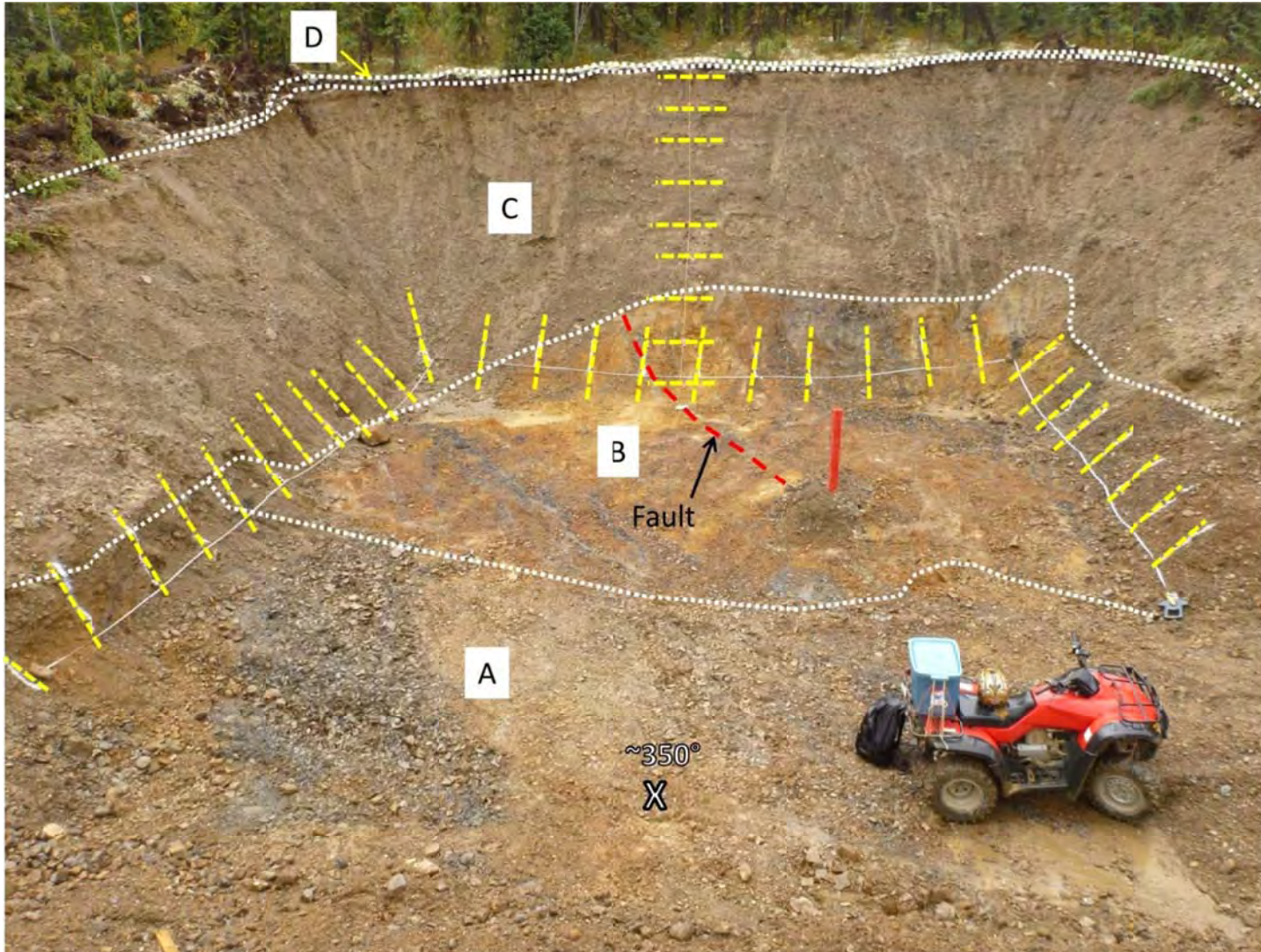
Outcrop OC-BGC11-50

Location: West flank Tin Dome
Facility: Plant site
Outcrop type: Man-made (plate load test excavation cut)

Northing: 7101140
Easting: 458828
Elevation: 859 m
Survey type: Handheld GPS

Slope Angle: 46°
Slope Direction: 172°
Date logged: 8/22/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

Outcrop OC-BGC11-50

PHOTO OF UNIT B, INCLUDING CLAYEY FAULT INDICATED ON OUTCROP PHOTO

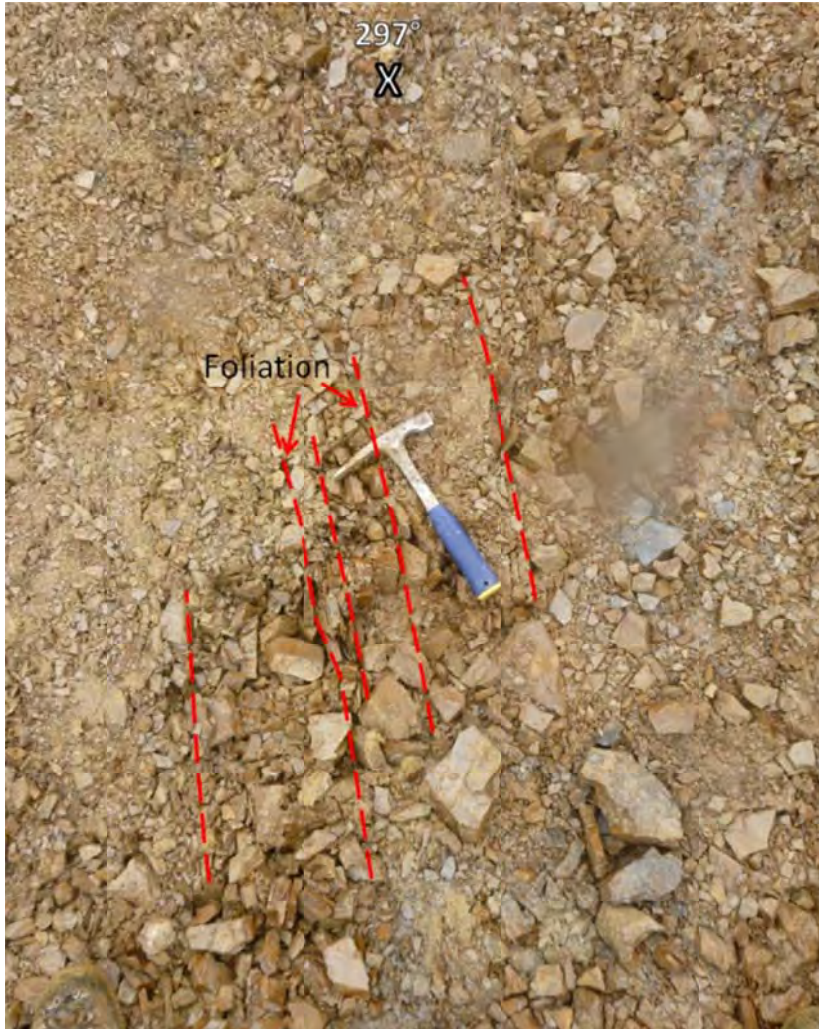


DESCRIPTION OF MATERIALS

- A: Fine-grained brown/black SLATE/PHYLLITE and QUARTZITE. Lightly to moderately weathered (W2-W3); weak to medium strong (R2-R3). Blocks average 1-3 cm; are equidimensional to tabular. GSI structure = disintegrated, GSI surface = Fair to Poor, GSI range = 20-30. [LIGHTLY TO MODERATELY WEATHERED BEDROCK]
- B: Highly weathered fine-grained yellow/black SLATE/PHYLLITE and QUARTZITE. Moderately to completely weathered (W3-W5); extremely to very weak (R0-R1). No significant blockiness; GSI not applicable. Quartzite weathered towards a sandy soil; slate /phyllite towards a slippery silty/clayey micaceous material. [MODERATELY TO COMPLETELY WEATHERED BEDROCK]
- C: Gravelly sand - see test pit log for TP-BGC11-103. [TILL]
- D: Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-50

PHOTO OF STRUCTURE – UNIT A



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
Foliation	72-83	206-223	0-100	None, CX, FG	8 (10cm)	R2.5	0.02	1m+	JO, F

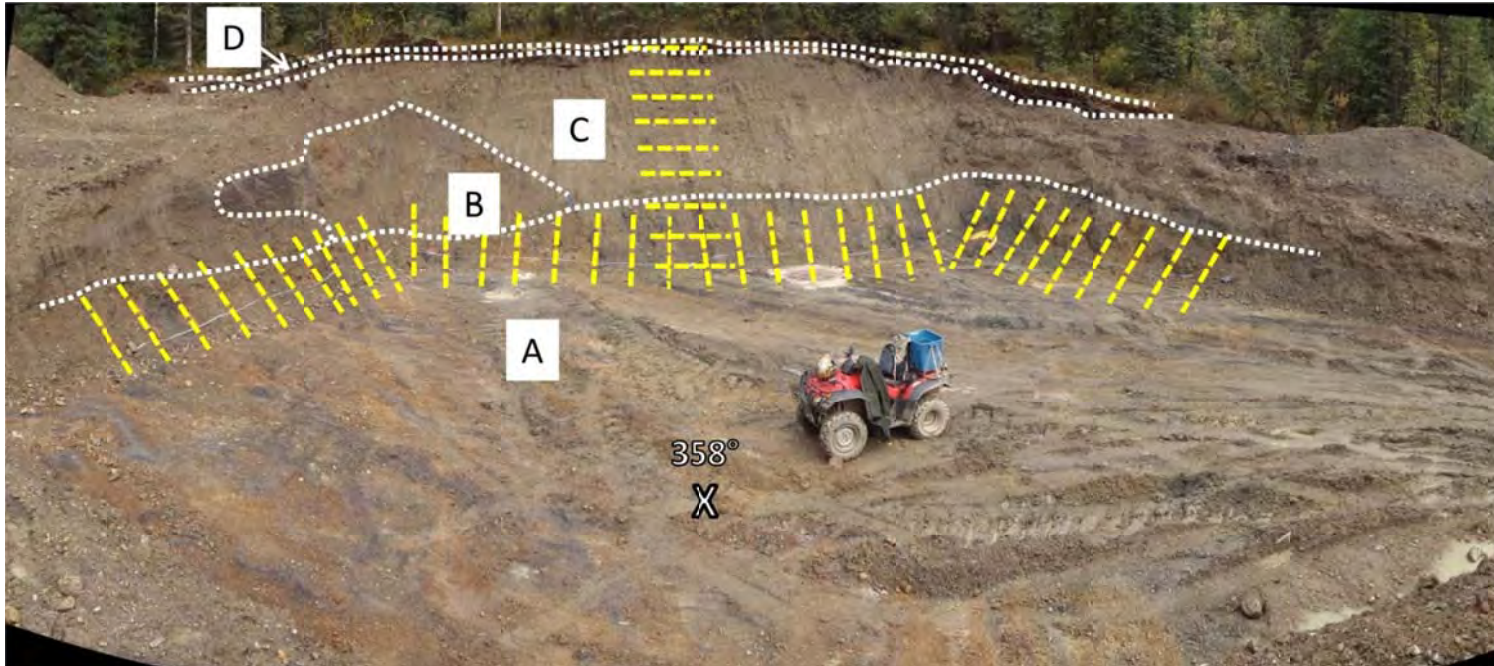
Outcrop OC-BGC11-51

Location: West flank Tin Dome
Facility: Plant site
Outcrop type: Man-made (plate load test excavation cut)

Northing: 7101187
Easting: 458932
Elevation: 885 m
Survey type: Handheld GPS

Slope Angle: 58°
Slope Direction: 165°
Date logged: 8/23/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

- A:** Fine-grained, brown to black meta-sedimentary rock. Mostly QUARTZITE (60%) with seams of PHYLLITE (35%) and quartz veins (5%). Moderately to completely weathered (W3-W5); extremely to very weak (R0-R1) except some small "floating" blocky zones. Weathering generally decreases with depth, but varies laterally as well. Phyllite is silty/clayey and slippery; quartzite mostly sandy/silty. Avg block size <1cm; GSI not applicable. [HIGHLY WEATHERED BEDROCK]
- B:** Highly dilated, rotated and displaced weathered bedrock with intermixed silty sand (SM) from soil above. Clasts moderately weathered (W2.5) and weak (R2.5); 1-5 cm in size. GSI not applicable. [DILATED, COMPLETELY WEATHERED BEDROCK AND SOIL]
- C:** [COLLUVIUM/TILL]. See testpit log for TP-BGC11-105.
- D:** Root mat and organic silt. [TOPSOIL]

Outcrop OC-BGC11-51

PHOTO OF UNIT B



Outcrop OC-BGC11-51

PHOTO OF UNIT A



DISCONTINUITY TABLE – UNIT A

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
Foliation	37-87	158-215	0-100	None, CX, FG	12 (10cm)	R0-R3	0.005-0.05	1m+	JO, F

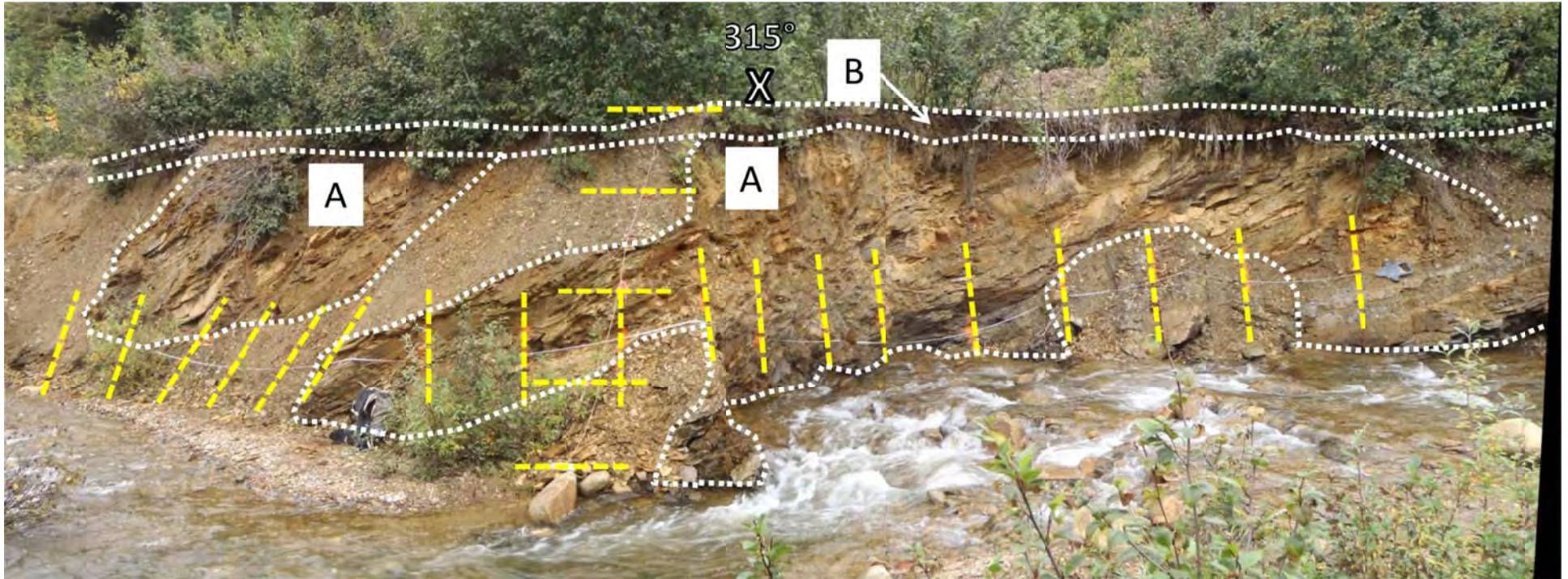
Outcrop OC-BGC11-52

Location: Dublin Gulch valley bottom
Facility: Heap leach pads / processing ponds
Outcrop type: Natural (stream channel)

Northing: 7101039
Easting: 459097
Elevation: 849 m
Survey type: Handheld GPS

Slope Angle: 65°
Slope Direction: Curves 116-172° (convex)
Date logged: 8/23/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: Dark grey, fine-grained QUARTZITE (60%) interbedded with schistose seams of PHYLLITE (40%). Phyllite weathered to clayey residual soil in upper ~1m of outcrop. Slightly weathered to highly weathered (W2-W4); very weak (R1 - phyllite) to medium strong (R3 - quartzite). Blocks ~ 5 cm, equidimensional. GSI structure = Disturbed, GSI surface = Poor, GSI range = 20-30. Some beds in this outcrop may be calcareous, as indicated by (1) the presence of minor calcite precipitate on the outcrop surface, and (2) some of the blocky “quartzite” seams being softer than typical quartz according to a field hardness test.

B: SAND: Silty, some fine to medium gravel, well-graded (SM). Contains abundant rootlets. Brown, moist, loose, homogenous; nonplastic, no dry strength, rapid dilatancy. Clasts angular to subangular, weak to medium strong (R2-R3), flat, mostly metasedimentary. [COLLUVIUM with TOPSOIL].

Outcrop OC-BGC11-52

PHOTO OF STRUCTURE



DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	24-30	211-250	0-10	DX	20 (1m)	R2	0.05 (marble), <0.01 (phyllite)	5m+	JO
2	74-81	335-344	1-10	DX, CA	6 (1m)	R2.5	0.1	4m+	J
3	72-88	076-099	1-5	DX, CA	8 (10cm)	R2.5	0.1	0.5m	J
4	56-65	180-197	NV	CA	12 (1m)	R2.5	0.5	1m+	J

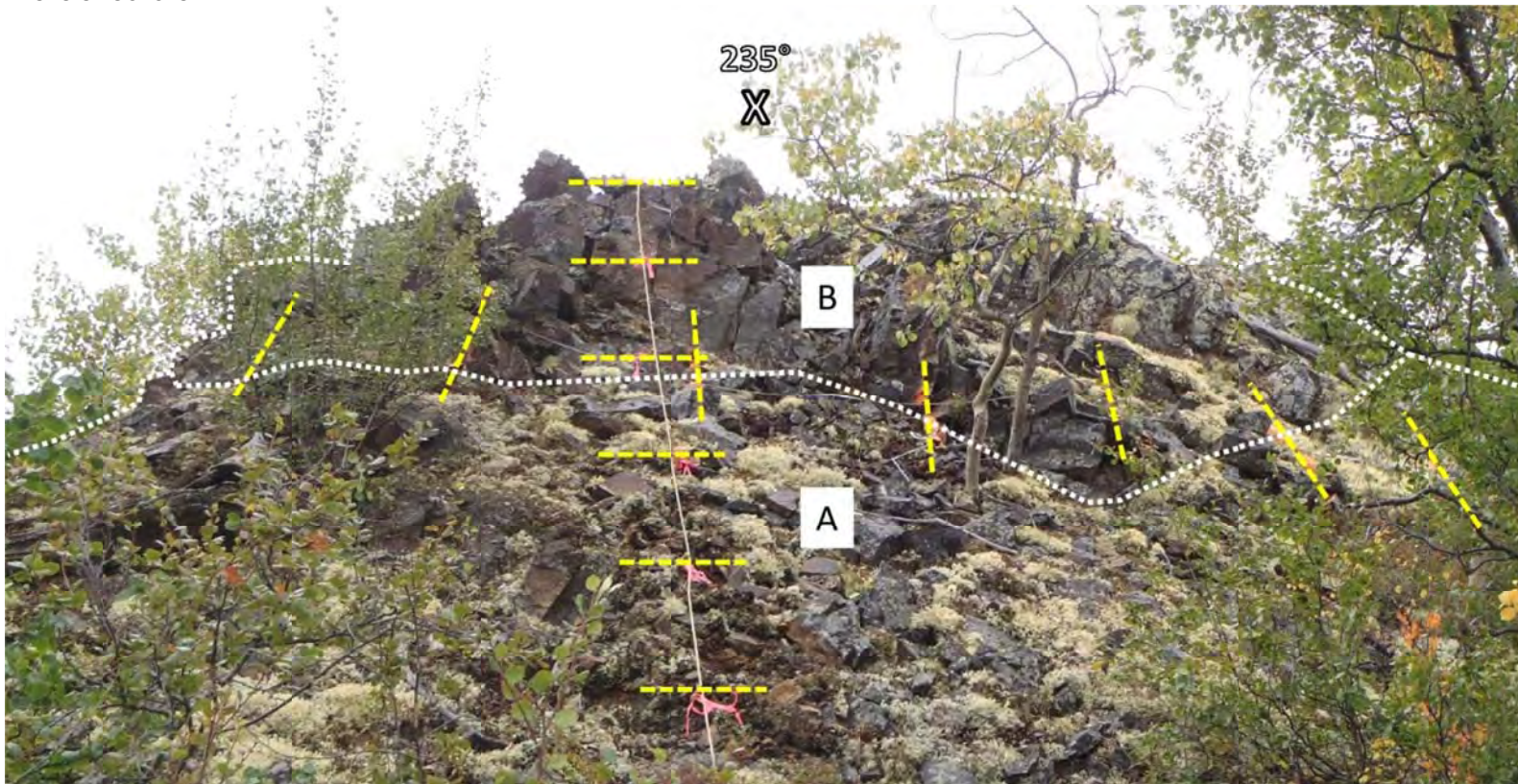
Outcrop OC-BGC11-53

Location: Stuttle Gulch
Facility: 100-day storage
Outcrop type: Natural

Northing: 7099996
Easting: 459359
Elevation: 1009 m
Survey type: Handheld GPS

Slope Angle: 45°
Slope Direction: 044°
Date logged: 8/23/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: GRAVEL: Cobbly, some sand, some organic silt, well-graded (GW). Fines moist, very soft; have low plasticity, rapid dilatancy, and no dry strength. Clasts are metasedimentary, angular, flat, and medium strong to strong (R3-R4). [COLLUVIUM] with thin cap of [TOPSOIL].

B: Dark grey, fine-grained QUARTZITE. Faintly weathered (W1.5); strong (R4). Blocks ~ 15 cm.; equidimensional. GSI structure = Very Blocky, GSI surface = Good, GSI range = 50-60.

Outcrop OC-BGC11-53

PHOTO OF STRUCTURE



DISCONTINUITY TABLE

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	29-32	271-294	0-5	None	8 (1m)	R4	0.15	1m+	JO
2	78-86	139-161	1-20	None, OG	8 (1m)	R4	0.15	2m+	J
3	69-86	060-089	1-10	None, OG	16 (1m)	R4	0.15	2m+	J

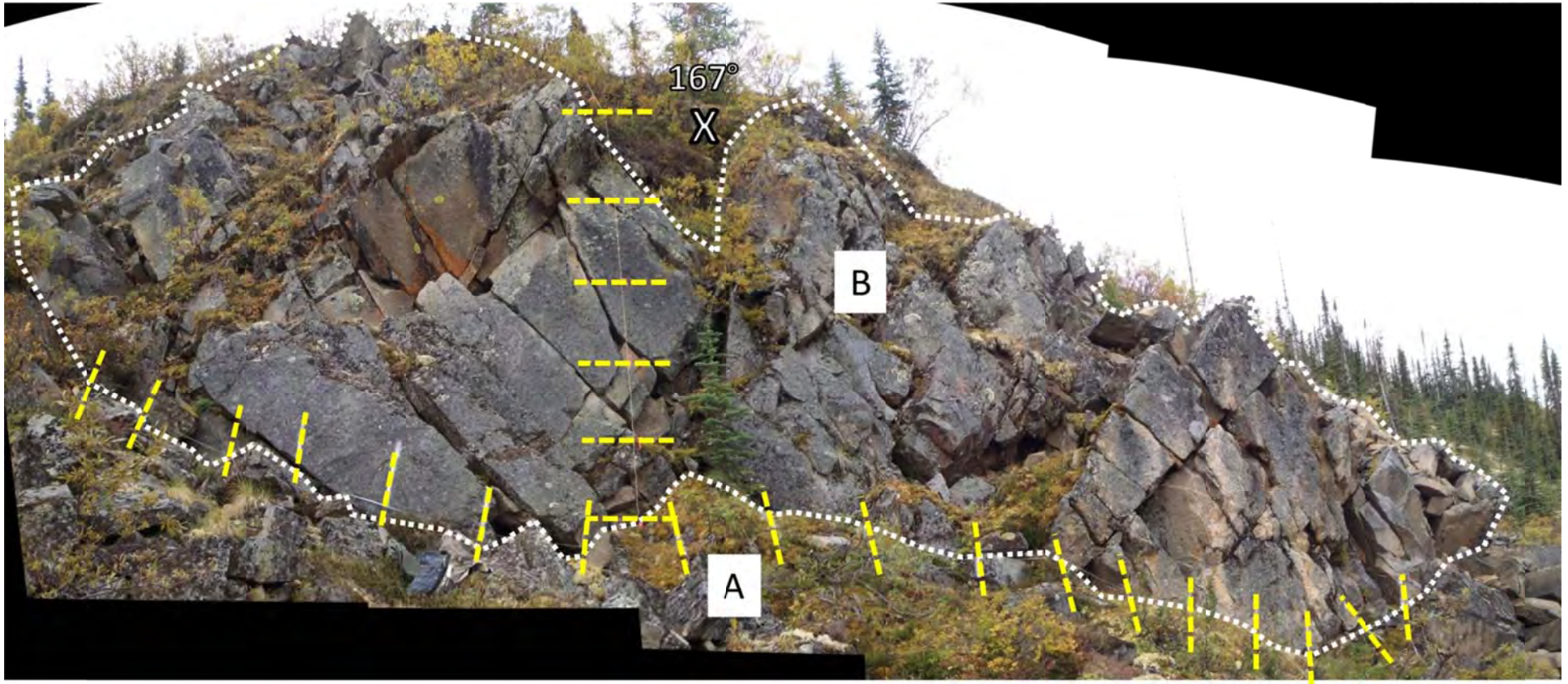
Outcrop OC-BGC11-54

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Natural

Northing: 7099871
Easting: 460694
Elevation: 1288 m
Survey type: Handheld GPS

Slope Angle: 75°
Slope Direction: 142°
Date logged: 8/24/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

- A: BOULDERS: Some cobbles, some gravel, minor organics. Clasts angular, equidimensional to elongate, granodiorite, very strong to extremely strong (R5-R6). [FROST-SHATTERED BEDROCK]
- B: Medium-grained grey GRANODIORITE. Faintly weathered (W1.5); very strong to extremely strong (R5-R6). Blocks ~ 0.7 m; rhombohedral. GSI structure = Blocky, GSI surface = Good, GSI range = 60-70.

Outcrop OC-BGC11-54

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	82-87	143-172	1-100	None	12 (1m)	R5	0.6	5m+	J
2	53-70	211-247	1-100	None	16 (1m)	R5	0.7	6m+	J
3	59-83	035-054	1-50	None	16 (1m)	R5	2	4m+	J
4	41-57	105-128	1-100	None	12 (1m)	R5	2	5m+	J

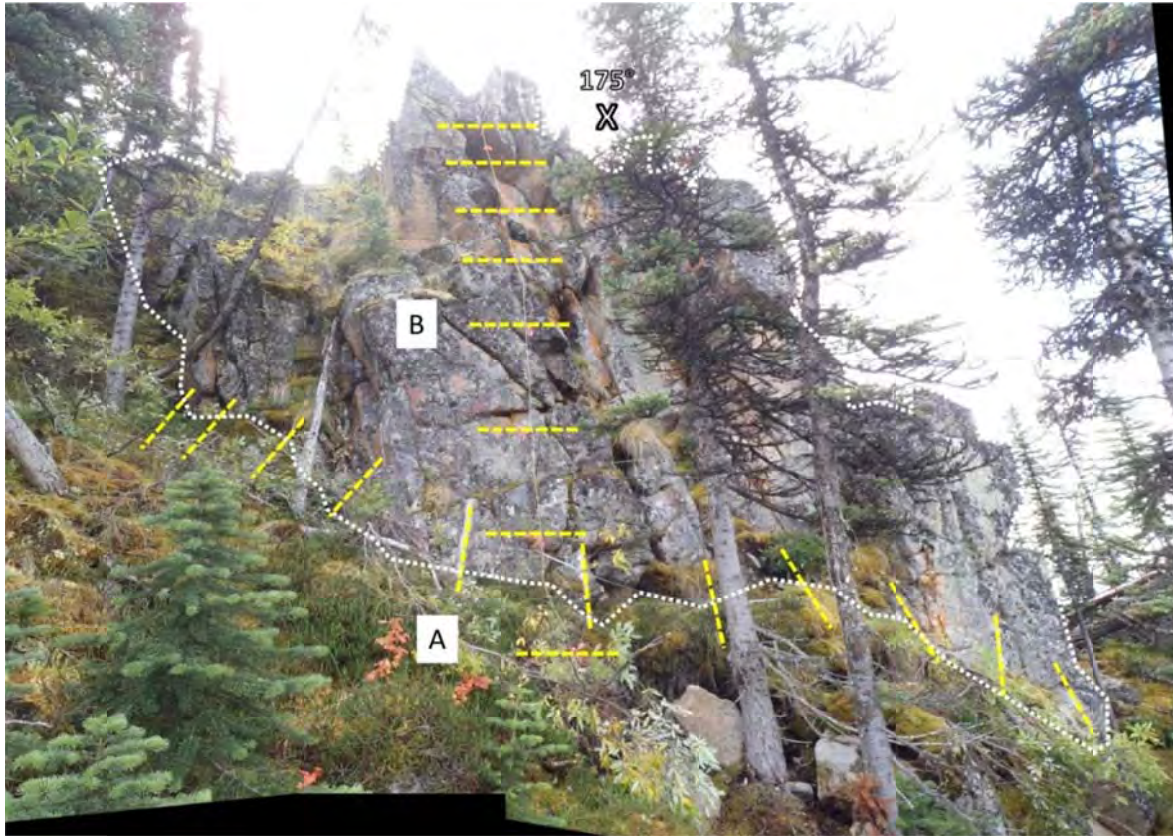
Outcrop OC-BGC11-55A

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Natural

Northing: 7100389
Easting: 460728
Elevation: 1248 m
Survey type: Handheld GPS

Slope Angle: 78°
Slope Direction: 349°
Date logged: 8/24/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: BOULDERS and COBBLES covered by ~ 10 cm of TOPSOIL (moss/heather root mat and some organic silt). Clasts angular, equidimensional to elongate, granodiorite. [FROST-SHATTERED BEDROCK]

B: Grey, coarse-grained GRANODIORITE. Faintly weathered (W1.5); strong to very strong (R4-R5). Blocks ~ 0.5m; rhombohedral. GSI structure = Blocky, GSI surface = Good to Fair, GSI range = 55-65.

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Outcrop OC-BGC11-55A

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	78-89	354-000	0-50	None	8 (1m)	R4	0.6	3.5m	J
2	80-82	252-280	0-50	None	12-16 (1m)	R4	0.7	3m	J
3	24-42	070-285	0-5	None	12 (1m)	R4	0.7	2m	J
4	52-62	252-275	0-40	None	12 (1m)	R4	1	2m	J

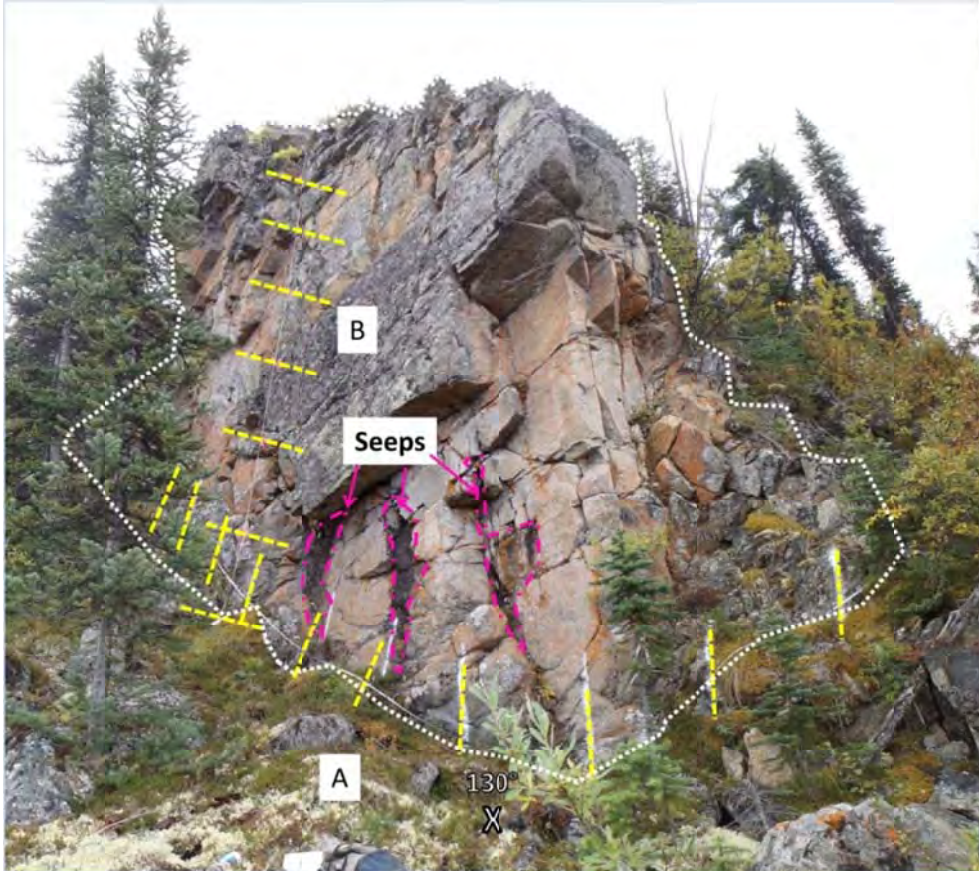
Outcrop OC-BGC11-55B

Location: Eagle Pup
Facility: Eagle Pup WRSA
Outcrop type: Natural

Northing: 7100373
Easting: 460719
Elevation: 1239 m
Survey type: Handheld GPS

Slope Angle: 85°
Slope Direction: 353°
Date logged: 8/24/2011
Logged by: GH

PHOTO OF OUTCROP



1m spacing along outcrop surface between yellow dashed lines.

DESCRIPTION OF MATERIALS

A: BOULDERS and COBBLES covered by ~ 10 cm of TOPSOIL (moss/heather/root mat and some organic silt). Angular, equidimensional to elongate, granodiorite. [FROST-SHATTERED BEDROCK]

B: Grey, coarse-grained GRANODIORITE. Faintly to lightly weathered (W1.5-W2); strong to very strong (R4-R5). Some zones of moderately weathered R3 rock near water seeps. Blocks ~ 0.4 m; rhombohedral. GSI structure = Blocky, GSI surface = Good to Fair, GSI range = 55-65.

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Outcrop OC-BGC11-55B

PHOTO OF STRUCTURE



DISCONTINUITY TABLE – UNIT B

Set	Orientation range		Aperture (mm)	Infill	JRC (scale)	JWCS	Spacing (m)	Persistence (m)	Type
	Dip°	DD°							
1	83-89	352-357	0-3	None	10 (1m)	R4	0.4	8m+	J
2	78-90	252-286	0-40	None	16 (1m)	R4	0.5	4m	J
3	18-32	036-065	0-3	CA	20 (1m)	R4	0.4	2.5m	J
4	50-70	260-294	0-10	None	20 (1m)	R4	0.8	2m	J
5	57-70	160-165	1-3	CA	4-8 (1m)	R4	1	7m+	J

APPENDIX C TEST PIT LOGS

TEST PIT LOGS

Table C- 1: Summary of Test Pits

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Final Length (m)
TP-BGC11-50	7100283	459702	1011	5.8
TP-BGC11-51	7100436	459665	972	4.7
TP-BGC11-52	7101809	459978	1051	5.0
TP-BGC11-53	7102242	460049	1103	4.0
TP-BGC11-54	7102175	460416	1178	2.0
TP-BGC11-55	7102242	460619	1209	2.0
TP-BGC11-56	7102183	460247	1158	2.0
TP-BGC11-57	7102087	460209	1144	5.0
TP-BGC11-58	7102129	460079	1118	5.0
TP-BGC11-59	7099985	459791	1065	2.6
TP-BGC11-60	7099940	459795	1067	2.7
TP-BGC11-61	7099870	459439	1008	5.0
TP-BGC11-62	7099924	459251	975	6.2
TP-BGC11-63	7099914	459139	940	4.6
TP-BGC11-64	7100206	459151	945	4.7
TP-BGC11-65	7099236	459542	986	8.0
TP-BGC11-66	7099178	459943	1134	7.5
TP-BGC11-67	7099861	458808	857	6.5
TP-BGC11-68	7099590	458859	849	6.5
TP-BGC11-69	7099457	458915	875	6.5
TP-BGC11-70	7099123	459409	983	3.0
TP-BGC11-71	7101244	459115	885	2.2
TP-BGC11-72	7101194	459202	874	4.3
TP-BGC11-73	7101498	460472	984	3.0
TP-BGC11-74	7101487	460629	1014	5.5
TP-BGC11-75	7101499	460786	1040	4.2
TP-BGC11-76	7101302	461068	1125	5.4
TP-BGC11-77	7101655	461364	1099	5.5
TP-BGC11-78	7101628	461728	1186	5.5
TP-BGC11-79	7101806	461903	1166	3.8
TP-BGC11-80	7101551	461753	1210	6.7
TP-BGC11-81	7101480	461711	1207	3.9
TP-BGC11-82	7100691	458604	815	7.5
TP-BGC11-83	7100077	458786	863	1.3
TP-BGC11-84	7100006	458763	863	2.6
TP-BGC11-85	7100055	458783	865	2.5
TP-BGC11-86	7101751	460124	894	7.5

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Final Length (m)
TP-BGC11-87	7100247	460690	1194	3.5
TP-BGC11-88	7101008	459813	922	6.0
TP-BGC11-89	7100737	459876	985	6.5
TP-BGC11-90	7100502	459686	981	6.5
TP-BGC11-91	7100633	459649	969	2.1
TP-BGC11-92	7100733	459587	933	1.7
TP-BGC11-93	7100794	459536	917	1.9
TP-BGC11-94	7101522	459983	930	5.0
TP-BGC11-95	7100135	460283	1109	3.2
TP-BGC11-96	7100170	460287	1103	2.0
TP-BGC11-97	7099463	460546	1353	4.0
TP-BGC11-98	7099465	460521	1342	2.2
TP-BGC11-99	7099496	460576	1352	2.5
TP-BGC11-100	7099505	460565	1345	3.5
TP-BGC11-101	7098940	460089	1180	2.4
TP-BGC11-102	7099111	460565	1361	4.6
TP-BGC11-103	7101150	458820	865	6.0
TP-BGC11-104	7100924	459738	832	5.6
TP-BGC11-105	7101190	458939	878	3.0
TP-BGC11-106	7100012	460375	1155	3.0
TP-BGC11-107	7100261	460182	1044	3.2
TP-BGC11-108	7099975	460285	1145	1.5
TP-BGC11-109	7100520	460137	977	5.3
TP-BGC11-110	7101062	459851	942	5.0
TP-BGC11-111	7100373	460653	1185	2.0
TP-BGC11-112	7100504	460350	1033	2.5
TP-BGC11-113	7099490	459071	878	2.5
TP-BGC11-114	7099369	459190	914	3.2
TP-BGC11-115	7099453	459187	899	5.7
TP-BGC11-116	7099423	459251	905	2.3
TP-BGC11-117	7099578	459184	896	8.0
TP-BGC11-118	7100268	459335	951	5.3
TP-BGC11-119	7100548	458587	824	2.1
TP-BGC11-120	7100484	458511	816	4.4
TP-BGC11-121	7100588	458502	815	3.3
TP-BGC11-122	7100707	458684	825	5.8
TP-BGC11-123	7101322	458498	808	2.4
TP-BGC11-124	7101502	458559	825	6.5
TP-BGC11-125	7100047	459492	995	2.5

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Final Length (m)
TP-BGC11-126	7100300	459464	946	3.1
TP-BGC11-127	7100117	459889	1096	2.5
TP-BGC11-128	7101526	458423	803	3.0
TP-BGC11-129	7101340	458563	822	6.8
TP-BGC11-130	7101183	458709	865	6.0
TP-BGC11-131	7101430	459934	921	3.5
TP-BGC11-132	7101515	459840	922	5.3
TP-BGC11-133	7101608	460048	984	4.4
TP-BGC11-134	7101419	460101	945	4.2
TP-BGC11-135	7101383	459998	931	3.5
TP-BGC11-136	7101306	459896	910	4.8
TP-BGC11-137	7101171	459920	943	5.0
TP-BGC11-138	7100041	459771	1050	6.3
TP-BGC11-139	7099033	459001	935	3.0
TP-BGC11-140	7097912	459228	954	4.5
TP-BGC11-141	7098871	460019	1154	5.0
TP-BGC11-142	7098318	459037	913	4.5
TP-BGC11-143	7100441	458644	838	4.0
TP-BGC11-144	7100144	460540	1158	1.0
TP-BGC11-145	7101471	459853	959	5.1

NOTES:

1. Location for BGC holes from handheld GPS measurements.
2. Coordinates in UTM NAD 83, Zone 8N.

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-50

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459702E, 7100283N
 GROUND ELEVATION (m): 1011
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 29 Jun 11
 FINISH DATE: 29 Jun 11
 FINAL DEPTH OF PIT (m): 5.8
 LOGGED BY: EBLGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	×	W _l %	○
						20	40	60	80
0					ORGANICS Lichen, moss, rootmat. [TOPSOIL]				
0.65					SILT (ML) Gravelly, some clay, trace sand, low to medium plasticity, firm, yellowish brown, moist to damp, metasedimentary clasts are angular to subrounded, elongated, maximum particle size = 70 mm, weak to strong (R2 to R4). [COLLUVIUM]				
1.5					GRAVEL (GW) Trace sand, well graded, angular, elongated, maximum particle size = 250 mm, weak to strong (R2 to R4), yellowy brown, moist, metasedimentary clasts. [COLLUVIUM]				
2.5					GRAVEL (GM) Silty, trace sand, trace cobbles, trace boulders, well graded, angular, elongated, maximum particle size = 350 mm, weak to strong (R2 to R4), low plastic matrix, yellowish brown, moist, metasedimentary clasts. [COLLUVIUM]				
2.8	G	1			Below 0.65 m - Increasing silt and cobble content to the top of bedrock. 2.8 - 3.0 m - Sample 1, grain size analysis, 84% gravel, 11% sand, 5% fines. Specific gravity analysis, average specific gravity 2.8.				
3.3	G	2			3.3 - 3.6 m - Sample 2, grain size analysis, 67% gravel, 23% sand, 10% fines.				
4.0					SAND (SM) Silty, some gravel, trace cobbles, trace boulders, well graded, subangular to angular, elongated, maximum particle size = 300 mm, metasedimentary clasts, weak (R2), yellow brown, moist. Silt content varies throughout the unit, forms lenses. Iron staining observed. [HIGHLY TO COMPLETELY WEATHERED BEDROCK]				
5.8			4.5		End of test pit at 5.8 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Logged from cut walls for drill pad for BH-BGC11-36 and excavated below pad surface.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-51

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459665E, 7100436N
 GROUND ELEVATION (m): 972
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 29 Jun 11
 FINISH DATE: 29 Jun 11
 FINAL DEPTH OF PIT (m): 4.7
 LOGGED BY: EBLGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	▲
						REMOLD	◇	□	△
						★ % Fines		Pocket Pen /2	
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Lichen, moss, rootmat, wet. [TOPSOIL]				
1	G	1			GRAVEL (GM) Silty, trace to some sand, metasedimentary clasts are angular to subrounded, elongated, maximum particle size = 140 mm, weak (R2). Fines: low plasticity, hard, rapid dilatancy, yellow brown, white mottling and iron staining, wet when thawed. Frozen: Nbn. [COLLUVIUM] 0.2 - 1.44 m - Sample 1, grain size analysis, 32% gravel, 24% sand, 34% silt, 10% clay.	○	×	★	
2	G	2			SAND (SM) Some gravel, some silt, trace cobbles well graded, angular to subangular elongated, maximum particle size = 140 mm, yellow brown, moist, metasedimentary clasts, weak to medium strong (R2 to R3). [COLLUVIUM] 2.4 - 2.8 m - Sample 2, grain size analysis, 50% gravel, 23% sand, 27% fines. Sulphate analysis, 0.13 % total sulphate ion content.	○	★		
3					SAND AND GRAVEL (GM) Silty, trace cobbles, trace boulders, well graded, angular to subrounded, elongated, maximum particle size = 400 mm, yellowish brown, dry to moist, low plastic matrix, metasedimentary clasts, weak to strong (R2 to R4). [COLLUVIUM]				
4	G	3			4.2 - 4.5 m - Sample 3, grain size analysis, 60% gravel, 34% sand, 6% fines.	★			
5					End of test pit at 4.7 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered from 0.2-1.44 m. 5) Encountered resistance on boulders (weathered bedrock?) at bottom of excavation. 6) Logged from cut walls for drill pad for BH-BGC11-35 and excavated below pad surface.				
6									
7									
8									

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-52

LOCATION: ANN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459978E, 7101809N
 GROUND ELEVATION (m): 1051
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 02 Jul 11
 FINISH DATE: 02 Jul 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.5					GRAVEL (GM) Some silt, trace sand, trace cobbles, well graded, loose, maximum particle size = 200 mm, flat and elongated, angular, brown, moist, clasts: medium strong (R3), slightly weathered (W2). [COLLUVIUM]				
2.0					METASEDIMENTARY ROCK Brown, iron staining, extremely to highly fractured, medium strong (R3), slightly weathered (W2). [SLIGHTLY WEATHERED BEDROCK]				
3.3 - 3.5	G	1	2		3.3 - 3.5 m - Sample 1, grain size analysis, 30% cobbles, 63% gravel, 6% sand, 1% fines.				
5.0					End of test pit at 5.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated at drill pad for BH-BGC11-29.				

EGP (TESTPIT)_EGP_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460049E, 7102242N
 GROUND ELEVATION (m): 1103
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 02 Jul 11
 FINISH DATE: 02 Jul 11
 FINAL DEPTH OF PIT (m): 4.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.5					SILT (ML) Trace sand, trace gravel, trace cobbles, low plasticity, firm to stiff, brown, moist. [COLLUVIUM] PP/2 = 100 kPa			△	
1.5					GRAVEL (GM) Some silt, trace sand, trace cobbles, well graded, maximum particle size = 200 mm, flat and elongated, angular, iron staining. [MODERATELY WEATHERED METASEDIMENTARY BEDROCK]				
2.0	G	1			2.0 - 2.2 m - Sample 1, grain size analysis, 79% gravel, 18% sand, 3% fines.	★	○		
4.0					End of test pit at 4.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated at drill pad for BH-BGC11-27.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460416E, 7102175N
 GROUND ELEVATION (m): 1178
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 02 Jul 11
 FINISH DATE: 02 Jul 11
 FINAL DEPTH OF PIT (m): 2.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0	G	1			ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.4 - 0.6					GRAVEL (GM) Silty, some sand, trace cobbles, trace boulders, well graded, loose, maximum particle size = 250 mm, angular, iron staining. Fines: low plastic, firm to stiff, brown, moist, heterogeneous. [COLLUVIUM] 0.4 - 0.6 m - Sample 1, grain size analysis, 57% gravel, 18% sand, 19% silt, 6% clay.				
0.6 - 2.0			2		METASEDIMENTARY ROCK Iron staining, highly fractured, blocky, angular, strong (R4), slightly weathered (W2). [SLIGHTLY WEATHERED BEDROCK]				
2.0					End of test pit at 2.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated at drill pad for BH-BGC11-25.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-55

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460619E, 7102242N
 GROUND ELEVATION (m): 1209
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 02 Jul 11
 FINISH DATE: 02 Jul 11
 FINAL DEPTH OF PIT (m): 2.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.5	G	1			GRAVEL (GM) Silty, trace sand, trace cobbles. Fines: low plastic, firm, brown, moist, heterogeneous. [COLLUVIUM] 0.4 - 0.6 m - Sample 1, grain size analysis, 57% gravel, 19% sand, 20% silt, 4% clay.				
1.5			3		METASEDIMENTARY ROCK Excavated as silty gravel, trace sand, trace cobbles, well graded, maximum particle size = 250 mm, angular, iron staining, moist. [MODERATELY WEATHERED METASEDIMENTARY BEDROCK] 1.9 - 2.0 m - Sample 2, grain size analysis, 80% gravel, 13% sand, 7% fines.				
2.0	G	2			End of test pit at 2.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated at drill pad for BH-BGC11-24.				
3									
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-56

LOCATION: ANN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460247E, 7102183N
 GROUND ELEVATION (m): 1158
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 04 Jul 11
 FINISH DATE: 04 Jul 11
 FINAL DEPTH OF PIT (m): 2.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa							
						40		80		120		160	
						VANE	FIELD	LAB	UC/2				
						PEAK	◇	■	▲	Pocket Pen /2			
			REMOLD			◇	□	△					
						★ % Fines							
						Moisture Content							
						W _p %	W _o %		W _l %		X		
						X	20	40	60	80	X		
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]								
1			3.5		METASEDIMENTARY ROCK Blocky to very blocky, poor joint wall condition, medium strong (R3), moderately to highly weathered (W3 to W4). [MODERATELY TO HIGHLY WEATHERED BEDROCK]								
2			2		METASEDIMENTARY ROCK As above but slightly weathered. [SLIGHTLY WEATHERED BEDROCK]								
3					End of test pit at 2.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) No sample taken.								
4													
5													
6													
7													
8													

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-57

LOCATION: ANN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460209E, 7102087N
 GROUND ELEVATION (m): 1144
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 04 Jul 11
 FINISH DATE: 04 Jul 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa					
						40		80		120	160
						VANE	FIELD	LAB	▲ UC/2		
						PEAK	◆	■	△ Pocket Pen /2		
						Moisture Content					
						W _p %	W ₁ %		W ₂ %		W _L %
						×	—	○	—	×	
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]						
1	G	1			GRAVEL (GM) Silty, some sand, trace cobbles, angular, iron staining, size increasing with depth. Fines: low plasticity, firm to very stiff, brown, moist. [COLLUVIUM] 0.5 - 1.1 m - Sample 1, grain size analysis, 50% gravel, 18% sand, 27% silt, 5% clay.	○	××★				
3			3		METASEDIMENTARY ROCK Gravel and silt present when excavated, blocky, iron stained, maximum particle size = 300 mm, subangular to angular, medium strong (R3), moderately weathered (W3). [MODERATELY WEATHERED METASEDIMENTARY BEDROCK]						
5					End of test pit at 5.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Cut for drill pad for BH-BGC11-26 logged as part of test pit.						

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-58

LOCATION: ANN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460079E, 7102129N
 GROUND ELEVATION (m): 1118
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 04 Jul 11
 FINISH DATE: 04 Jul 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.7		1			GRAVEL AND SAND (GM/SM) Silty, trace cobbles, trace clay. Fines: low plastic, firm, brown, moist, slow dilatancy. [COLLUVIUM] At 0.7 m - Sample 1, grain size analysis, 36% gravel, 37% sand, 23% silt, 4% clay.	○	✕		
2.0		2			GRAVEL (GW) Fine to coarse, trace sand, trace silt, well graded, compact, maximum particle size = 50 mm, flat and elongated, angular, iron staining, moist, laminated, very weak (R1). [COLLUVIUM]	⊗			
2.0 - 5.0			4		METASEDIMENTARY ROCK Blocky, iron stained, 300 mm blocks, angular to sub-angular, highly weathered (W4), medium strong (R3). Excavated as cobbles, gravel, and silt. [HIGHLY WEATHERED BEDROCK] At 2.0 m - Sample 2, grain size analysis, 69% gravel, 25% sand, 6% fines. Specific gravity analysis, 2.84 (average).				
5.0					End of test pit at 5.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				

EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-59

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459791E, 7099985N
 GROUND ELEVATION (m): 1065
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 07 Jul 11
 FINISH DATE: 07 Jul 11
 FINAL DEPTH OF PIT (m): 2.6
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0	G	1			ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.4 - 0.6					GRAVEL (GM) Fine to medium gravel, silty, some sand, some cobbles, trace boulders, elongated, angular, maximum particle size = 350 mm. Fines: non-plastic, soft, brown, homogeneous, moist. [COLLUVIUM] 0.4 - 0.6 m - Sample 1, grain size analysis, 67% gravel, 20% sand, 12% silt, 1% clay.				
1.85 - 2.6			3.5		METASEDIMENTARY ROCK Orange grey, very weak to weak (R1.5 to R2), moderately to highly weathered (W3 to W4), fine grained, disturbed, polyhedral structure, when excavated: gravel, with silt and sand. JS 1 - Dip/DipDir: 38/124 Ap: 30-150mm JRC: 14/0.3m JS 2 - Dip/DipDir: 85/194 Ap: 30-150mm JRC: 2-16/0.3m JS 3 - Dip/DipDir: 89/266 Ap: 30-150mm JRC: 16/0.1m [MODERATELY TO HIGHLY WEATHERED METASEDIMENTARY BEDROCK] At 1.85m - Block size increases, slightly to moderately weathered (W2.5), medium strong to strong (R3 to R4).				
2.6					End of test pit at 2.6 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated at drill pad for BH-BGC11-50.				

EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-60

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459795E, 7099940N
 GROUND ELEVATION (m): 1067
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 07 Jul 11
 FINISH DATE: 07 Jul 11
 FINAL DEPTH OF PIT (m): 2.7
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
REMOLD	◇	□	△	Pocket Pen /2						
★ % Fines										
Moisture Content										
W _p %	W ₁ %			W ₂ %						
×	20	40	60	80	×					
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]					
1	G	1			SILT (ML) Some sand, some gravel, trace cobbles, non-plastic, moist, brown-grey, soft, homogeneous, gravel and cobbles are angular and elongated. [COLLUVIUM] 0.65 - 1.0 m - Frozen: Vx, Vr (30%). 0.7 - 0.9 m - Sample 1, grain size analysis, 20% gravel, 25% sand, 53% silt, 2% clay. Sulphate analysis, 0.07% total sulphate ion content. 1.0 - 1.5 m - Frozen: Vx (5-10%).	⊗		★		
2					SILT AND GRAVEL (ML/GM) Some sand. Frozen: Vx (10%). [COLLUVIUM]					
3					ORGANICS Dark black. Frozen: Nbn. [ORGANICS]					
3					End of test pit at 2.7 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.65 m to bottom of excavation. 5) Test pit consists of road cut and 1 m of excavation.					
4										
5										
6										
7										
8										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

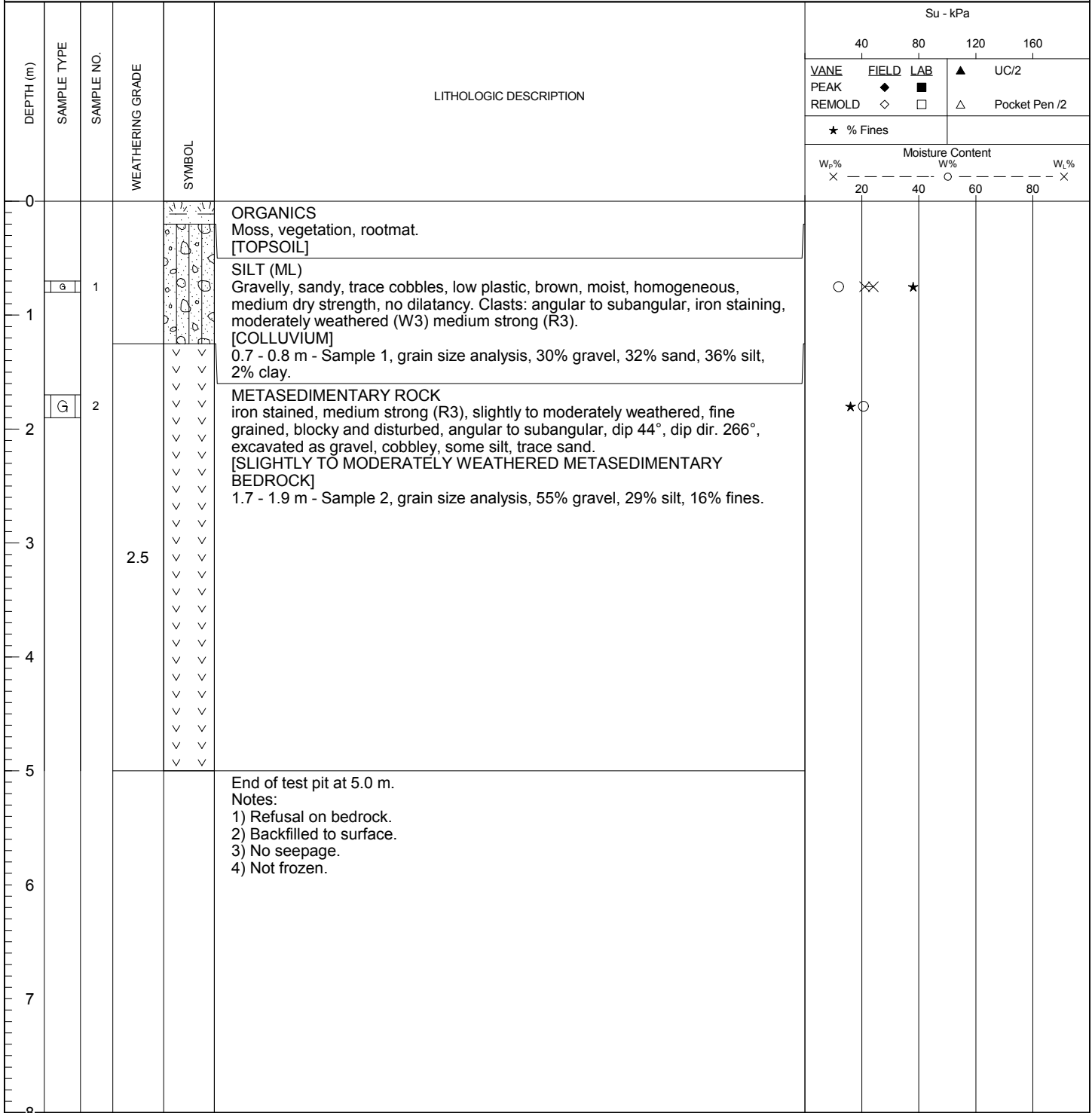
TEST PIT # TP-BGC11-61

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459439E, 7099870N
 GROUND ELEVATION (m): 1008
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 07 Jul 11
 FINISH DATE: 07 Jul 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW



EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-62

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459251E, 7099924N
 GROUND ELEVATION (m): 975
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 07 Jul 11
 FINISH DATE: 07 Jul 11
 FINAL DEPTH OF PIT (m): 6.2
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						VANE	FIELD	LAB	UC/2	
						PEAK	◆	■	▲	
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]					
1		1			SILT (ML) Cobbly, gravelly, sandy, low plastic, soft, brown, moist, homogeneous, moderate dry strength, no dilatancy. [COLLUVIUM] 0.9 - 1.0 m - Sample 1, grain size analysis, 33% gravel, 23% sand, 40% silt, 4% clay.		○	★		
2					SAND (SM) Silty, trace gravel, trace cobbles, compact to dense, brown, moist, homogeneous. [COLLUVIUM]					
3.5 - 3.7		2			3.5 - 3.7 m - Sample 2, grain size analysis, 33% gravel, 39% sand, 22% silt, 6% clay.		○	×	×	★
6.2					End of test pit at 6.2 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.					

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-63

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459139E, 7099914N
 GROUND ELEVATION (m): 940
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 07 Jul 11
 FINISH DATE: 07 Jul 11
 FINAL DEPTH OF PIT (m): 4.6
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.5 - 0.7	G	1	4		GRAVEL (GM) Silty, trace cobbles (angular, maximum particle size = 250 mm), trace sand, brown, moist, homogeneous. Fines are low plastic, firm, medium dry strength, rapid dilatency. [COLLUVIUM] 0.5 - 0.7 m - Sample 1, grain size analysis, 53% gravel, 23% sand, 20% silt, 4% clay.	○	★		
1.5 - 2.0					METASEDIMENTARY ROCK Disintegrated, iron stained, highly weathered (W4), weak (R2). [HIGHLY WEATHERED BEDROCK]				
2.0 - 4.6			3		METASEDIMENTARY ROCK Iron stained, slightly to moderately weathered (W2.5 to W3), medium strong (R3), very blocky, polyhedral, excavated as cobbles, some gravel, some silt. [MODERATELY WEATHERED BEDROCK] 1.1 - 1.5 m - Frozen: Vx (1-2%).				
4.6					End of test pit at 4.6 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered from 1.1-1.5 m. 5) Substantial sloughing.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459151E, 7100206N
 GROUND ELEVATION (m): 945
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 07 Jul 11
 FINISH DATE: 07 Jul 11
 FINAL DEPTH OF PIT (m): 4.7
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						VANE	FIELD	LAB	UC/2
						40	80	120	160
						PEAK	◆	■	▲
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.3					SILT (SM) Some gravel, some sand, trace cobbles, non plastic, greyish brown. Frozen: Nf. [COLLUVIUM]				
1.5		1			SAND (SP) Trace silt, some gravel, trace cobbles, fine to medium, poorly graded, angular, greyish brown. Frozen: Nf. [COLLUVIUM] 1.5 - 1.6 m - Sample 1, grain size analysis, 26% gravel, 60% sand, 14% fines.	○	★		
2.5			5		METASEDIMENTARY ROCK Disintegrated, orange brown, fine grained, completely weathered (W5), very weak (R1), excavated as gravel and sand. [COMPLETELY WEATHERED BEDROCK]				
3.5			3		METASEDIMENTARY ROCK Very blocky, dark orange brown, angular to sub-angular, fine to coarse, moderately weathered (W3), medium strong to strong (R3 to R4), excavated as cobbles, sandy, gravelly. Visible set dipping downslope. [MODERATELY WEATHERED BEDROCK]				
4.7					End of test pit at 4.7 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered from 0.3-2.0 m.				

EGR (TESTPIT)_EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-65

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459542E, 7099236N
 GROUND ELEVATION (m): 986
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 09 Jul 11
 FINISH DATE: 09 Jul 11
 FINAL DEPTH OF PIT (m): 8.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	▲
						REMOLD	◇	□	△
						★ % Fines			Pocket Pen /2
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0		1			ORGANICS Moss, vegetation, rootmat. [TOPSOIL]		○	★	
0.4 - 0.5			4.5		SAND (SW) Gravelly, trace cobbles, trace silt, well graded, compact, maximum particle size = 150 mm, sub-angular to angular, light brown, moist, homogeneous. [COLLUVIUM] 0.4 - 0.5 m - Sample 1, grain size analysis, 49% gravel, 36% sand, 15% fines.				
0.5 - 2.0					GRAVEL (GM) Silty, some sand, some cobbles, dark grey to brown. Clasts: metasedimentary, medium strong to strong (R3 to R4). [HIGHLY TO COMPLETELY WEATHERED METASEDIMENTARY BEDROCK]				
2.0 - 6.0			3		METASEDIMENTARY ROCK Iron stained, medium strong to strong (R3 to R4), moderately weathered (W3), very blocky, poor to fair joint condition, rhombohedral, iron stained, excavated as cobbles, some gravel, trace silt, trace sand. [MODERATELY WEATHERED BEDROCK]				
6.0 - 7.0			4		METASEDIMENTARY ROCK Light brown, medium strong (R3), highly weathered (W4), Blocky/disturbed, poor to fair joint condition, oxidized, excavated as cobbles (100 mm), sandy, some gravel. [HIGHLY WEATHERED BEDROCK]				
7.0 - 8.0			3		METASEDIMENTARY ROCK Very blocky, poor to fair joint condition, rhombohedral, fine to coarse, angular, moderately weathered (W3), medium strong to strong (R3 to R4), excavated as cobbles, some gravel, trace sand. [MODERATELY WEATHERED BEDROCK]				

(Continued on next page)

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-65

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459542E, 7099236N
 GROUND ELEVATION (m): 986
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 09 Jul 11
 FINISH DATE: 09 Jul 11
 FINAL DEPTH OF PIT (m): 8.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
REMOLD	◇	□	△	Pocket Pen /2						
★ % Fines										
Moisture Content										
W _p %	W ₁ %			W ₁ %	W ₁ %					
×	20	40	60	80	×					
8					End of test pit at 8.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Excavation on side of steep slope along existing road cut. Approximately 5 m exposure above road surface.					
9										
10										
11										
12										
13										
14										
15										
16										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459943E, 7099178N
 GROUND ELEVATION (m): 1134
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 09 Jul 11
 FINISH DATE: 09 Jul 11
 FINAL DEPTH OF PIT (m): 7.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	▲
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat [TOPSOIL]				
0.5					GRAVEL (GM) Silty, trace cobbles, compact, maximum particle size = 150 mm, subangular to angular, light brown, moist, homogeneous. [COLLUVIUM]				
1.5		1			SAND (SM) Silty, some gravel, fine to medium, well graded, dense, maximum particle size = 35 mm, angular to sub-angular, light to orangish brown, stratified. [COLLUVIUM]	○	★		
2.0		2			At 1.3 m - Sample 1, grain size analysis, 12% gravel, 63% sand, 25% fines. [COLLUVIUM]	△	○	×	
2.5			4.5		SILT (ML) Sandy, some clay, trace gravel, low plastic, soft, light brown to grey, moist, homogeneous. [COLLUVIUM] PP/2 = 25 kPa At 1.8 m - Sample 2, grain size analysis, 2% gravel, 32% sand, 51% silt, 15% clay.				★
3.5			3		SAND AND GRAVEL (SW-GW) Iron staining. Clasts: metasedimentary, very weak to weak (R1 to R2). [HIGHLY TO COMPLETELY WEATHERED METASEDIMENTARY BEDROCK]				
4.0					METASEDIMENTARY ROCK Iron staining, medium strong (R3), moderately weathered (W3), very blocky, fair to poor joint condition, excavated as cobbles and gravel. [MODERATELY WEATHERED BEDROCK]				
5.0					METASEDIMENTARY ROCK Iron staining, medium strong to strong (R3 to R4), slightly weathered (W2), blocky, fair joint condition, excavated as cobbles and gravel. [SLIGHTLY WEATHERED BEDROCK]				
6.0			2						
7.5					End of test pit at 7.5 m Notes: 1) Refusal on bedrock.				

(Continued on next page)

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-66

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459943E, 7099178N
 GROUND ELEVATION (m): 1134
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 09 Jul 11
 FINISH DATE: 09 Jul 11
 FINAL DEPTH OF PIT (m): 7.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	UC/2
						VANE	FIELD	LAB		
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _o %	W _l %	W _l %	W _l %
						×	○	○	○	×
						20	40	60	80	
8					2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated at old drill pad cut slope.					
9										
10										
11										
12										
13										
14										
15										
16										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-67

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458808E, 7099861N
 GROUND ELEVATION (m): 857
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 10 Jul 11
 FINISH DATE: 10 Jul 11
 FINAL DEPTH OF PIT (m): 6.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	▲
						REMOLD	◇	□	△
						★ % Fines		Pocket Pen /2	
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1	G	1			GRAVEL and SAND (GM/SM) Silty, trace cobbles, compact, maximum particle size = 200 mm, medium brown, moist, homogeneous. Clasts: angular to subangular. [COLLUVIUM] 0.7 - 0.9 m - Sample 1, grain size analysis, 45% gravel, 19% sand, 28% silt, 8% clay. Specific gravity analysis, 2.72 (average).	○	★		
2					GRAVEL and SAND (GM/SM) As above, some cobbles, trace boulders, maximum particle size = 500 mm, dense. Cobbles and boulders very strong to extremely strong (R5 to R6). [COLLUVIUM/TILL?]				
3									
4									
5									
6									
7					End of test pit at 6.5 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-68

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458859E, 7099590N
 GROUND ELEVATION (m): 849
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 10 Jul 11
 FINISH DATE: 10 Jul 11
 FINAL DEPTH OF PIT (m): 6.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

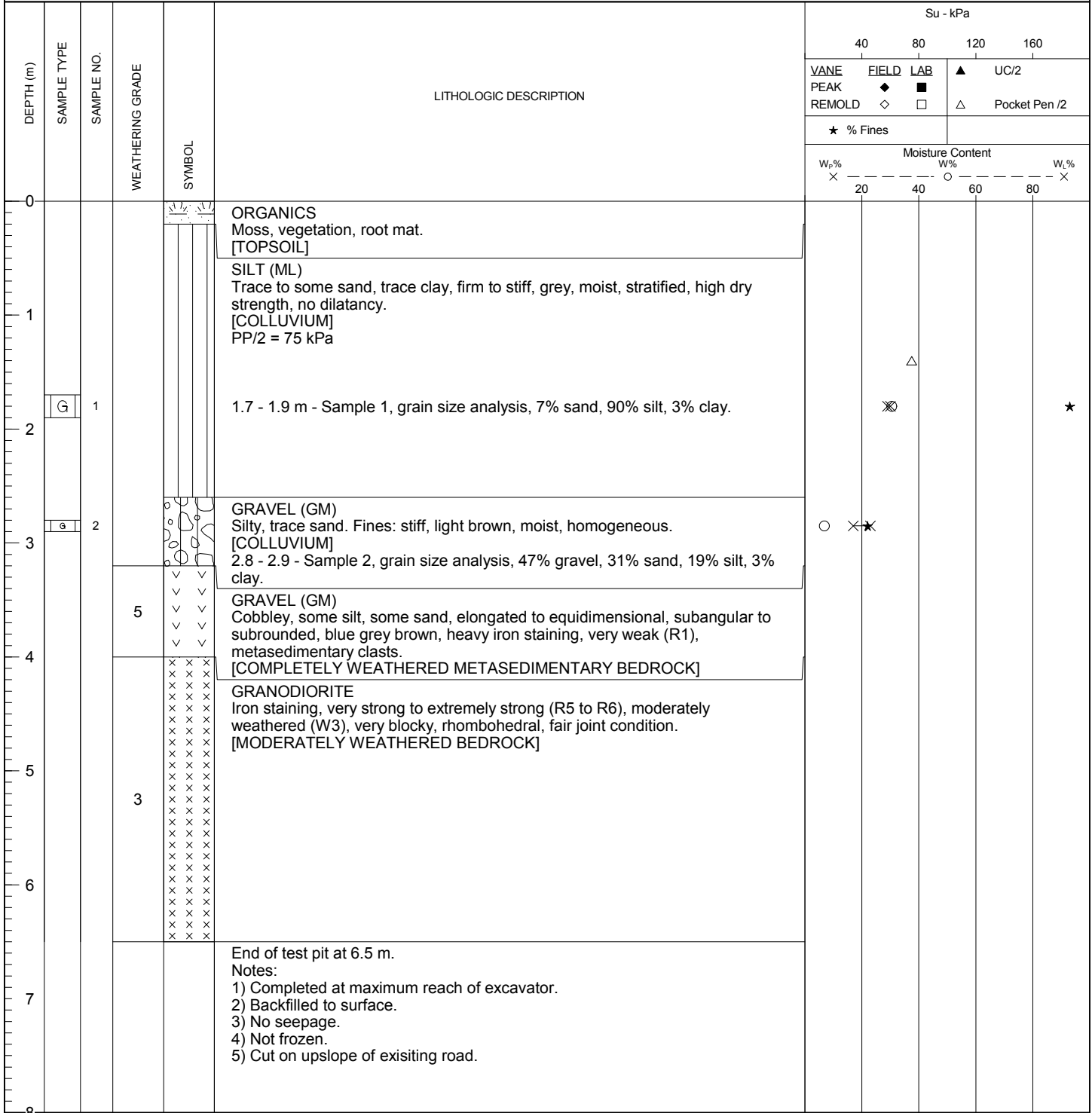
DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1					GRAVEL (GM) Silty, some fine sand, trace cobbles, trace boulders, non plastic, compact, medium brown, moist, homogeneous. Clasts: angular to sub-angular, maximum particle size = 270 mm. [COLLUVIUM]				
2	G	1			1.6 - 1.8 m - Sample 1, grain size analysis, 44% gravel, 38% sand, 18% fines.	○	★		
3					SAND (SM) Silty, cobbly, gravelly, trace boulders, dense, moist, medium brown, homogeneous, sub-angular to sub-rounded, maximum particle size = 400 mm, average cobble size = 100-150 mm, non plastic matrix. [COLLUVIUM]				
4									
5									
6									
7					End of test pit at 6.5 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458915E, 7099457N
 GROUND ELEVATION (m): 875
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 10 Jul 11
 FINISH DATE: 10 Jul 11
 FINAL DEPTH OF PIT (m): 6.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW



EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-70

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459409E, 7099123N
 GROUND ELEVATION (m): 983
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 10 Jul 11
 FINISH DATE: 10 Jul 11
 FINAL DEPTH OF PIT (m): 3.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

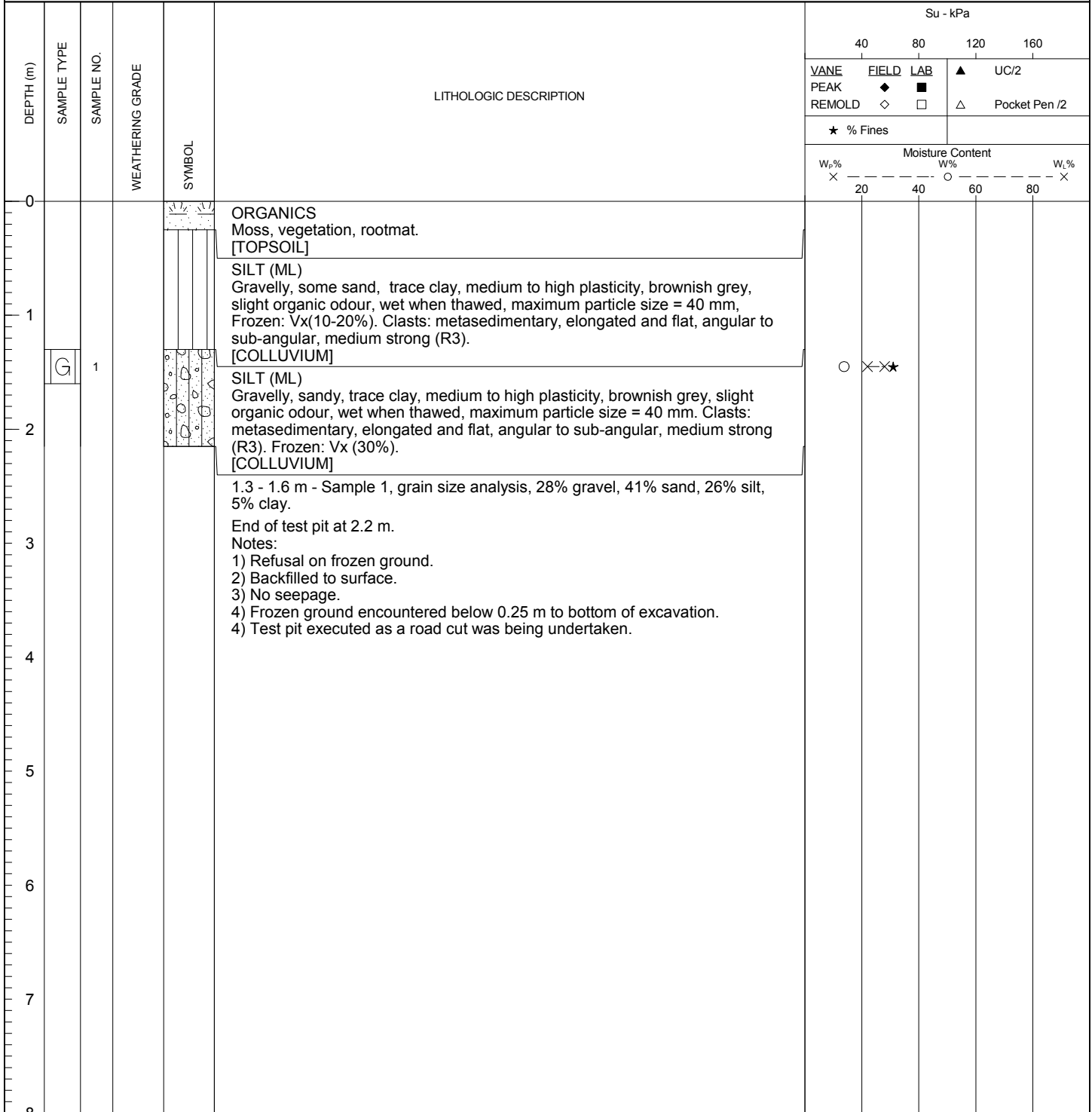
DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	
						×	○	○	×	
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]					
0.5			2		GRANODIORITE Iron staining, very strong to extremely strong, slightly weathered, blocky, rhombohedral, sand and silt infilling. [SLIGHTLY WEATHERED BEDROCK]					
3.0					End of test pit at 3.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated adjacent to existing road cut.					

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459115E, 7101244N
 GROUND ELEVATION (m): 885
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 11 Jul 11
 FINISH DATE: 11 Jul 11
 FINAL DEPTH OF PIT (m): 2.2
 LOGGED BY: EC/LGT
 REVIEWED BY: PQ/DW



EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-72

LOCATION: ANN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459202E, 7101194N
 GROUND ELEVATION (m): 874
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 11 Jul 11
 FINISH DATE: 11 Jul 11
 FINAL DEPTH OF PIT (m): 4.3
 LOGGED BY: EC/LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1	G	2			SILT (ML) Some gravel, some sand, low plastic, soft, dark grey brown, moist, homogeneous, slow dilatancy. Clasts: fine to medium grained, equidimensional to elongated, angular, metasedimentary. [COLLUVIUM]	○	★	×	×
2					GRAVEL (GM) Fine to medium gravel, silty, trace sand, trace clay, equidimensional to elongated, angular, metasedimentary clasts. Fines: low plastic, soft, light brown, moist, homogeneous, slow dilatancy. [COLLUVIUM] 0.8 - 1.0 m - Sample 2, grain size analysis, 35% gravel, 46% sand, 12% silt, 7% clay.				
3	G	1	3		METASEDIMENTARY ROCK Yellowish brown, fine grained, foliated, phyllitic, slightly to moderately weathered (W2.5), intact rock strength varies from very weak (R1) for small phyllitic clasts to strong (R4) for boulder size siltstone clasts. One main set observed (foliation): Dip 15°, dip direction 300°. [MODERATELY WEATHERED BEDROCK] 2.4 - 2.6 m - Sample 1, grain size analysis, 68% gravel, 25% sand, 7% fines.	★			
4.3					End of test pit at 4.3 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Excavated as road cut was being undertaken.				

EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-73

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460472E, 7101498N
 GROUND ELEVATION (m): 984
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 11 Jul 11
 FINISH DATE: 11 Jul 11
 FINAL DEPTH OF PIT (m): 3.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	
						×	○	○	×	
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]					
1	G	1			SILT (ML) Sandy, some boulders, some cobbles, some gravel, non-plastic, medium brown, homogeneous, maximum particle size = 800 mm. Frozen 0.3-1.0m: Vx (<1%). Clasts: subangular to subrounded, strong to very strong (R4 to R5). [COLLUVIUM] 0.8 - 1.0 m - Sample 1, grain size analysis, 17% gravel, 50% sand, 31% silt, 2% clay.	○	★			
2					BOULDERS AND COBBLES Silty, some gravel, trace sand, rounded to subrounded, moist, strong to very strong (R4 to R5) clasts, maximum particle size = 1200 mm. [COLLUVIUM]					
3					End of test pit at 3.0 m. Notes: 1) Refusal on boulders. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered from 0.3-2.1 m.					
4										
5										
6										
7										
8										

EGR (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-74

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460629E, 7101487N
 GROUND ELEVATION (m): 1014
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 11 Jul 11
 FINISH DATE: 11 Jul 11
 FINAL DEPTH OF PIT (m): 5.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1	G	1			GRAVEL (GM) Silty, trace sand, trace cobbles, dense, grey to medium brown, moist, homogeneous, maximum particle size = 150 mm, angular to subangular, matrix non plastic. [COLLUVIUM] 1.2 - 1.4 m - Sample 1, grain size analysis, 39% gravel, 26% sand, 28% silt, 7% clay.	○	××★		
3	g	2			SILT (ML) Trace gravel, trace sand, trace clay, low plastic, soft to firm, grey, moist to wet, homogeneous, low dry strength, rapid dilatancy. [COLLUVIUM] At 3.0 m - Sample 2, grain size analysis, 3% sand, 83% silt, 14% clay.		××○		★
4	g	3			SAND (SM) Silty, some gravel, trace cobbles, trace clay, light brown, moist, angular, moderately weathered, iron staining, maximum particle size = 200 mm, low plastic matrix. [COLLUVIUM] At 4.5 m - Sample 3, grain size analysis, 20% gravel, 30% sand, 44% silt, 6% clay.	○	××		★
5.5					End of test pit at 5.5 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage (wet layer at 2.5-3.0 m). 4) Not frozen. 5) Minor sloughing of pit walls.				

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-75

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460786E, 7101499N
 GROUND ELEVATION (m): 1040
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 12 Jul 11
 FINISH DATE: 12 Jul 11
 FINAL DEPTH OF PIT (m): 4.2
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1					GRAVEL (GM) Silty, some cobbles, trace sand, well graded, loose, maximum particle size = 200 mm, elongated and flat, angular, light brown, moist, stratified. [COLLUVIUM]				
2			3		METASEDIMENTARY ROCK Iron staining, medium strong (R3), moderately weathered (W3), blocky, disturbed, disintegrated, tabular blocks, flat, poor joint condition, excavated as cobbles and gravel, some silt. Dominant joint set has dip/dir of 050°/118°, average spacing 80-150 mm, maximum block size = 300 mm, average joint roughness coefficient (JRC) of 5 mm/25 cm. [MODERATELY WEATHERED BEDROCK]				
4.2					End of test pit at 4.2 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) No samples taken.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-76

LOCATION: STEWART GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 461068E, 7101302N
 GROUND ELEVATION (m): 1125
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 12 Jul 11
 FINISH DATE: 12 Jul 11
 FINAL DEPTH OF PIT (m): 5.4
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa					
						40	80	120	160		
0	G	1	3		ORGANICS Moss, vegetation, rootlets. [TOPSOIL] 0.4 - 0.6 m - Sample 1, grain size analysis, 18% gravel, 20% sand, 54% silt, 8% clay.						
0.4 - 0.6											
0.6 - 1.0											
1.0 - 2.0					SILT (ML) Some fine to coarse gravel, trace sand, trace clay, non plastic, soft, medium brown, moist, homogeneous, no dilatancy. [COLLUVIUM]						
2.0 - 5.4					METASEDIMENTARY ROCK Medium strong (R3), moderately weathered (W3), disintegrated, polyhedral blocks, fair joint condition, non stained. Excavated as cobbles and gravels, silty, trace boulders, maximum particle size = 450 mm, average particle size: 50-200 mm. [MODERATELY WEATHERED BEDROCK]						
5.4 - 5.4					End of test pit at 5.4 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Sloughing at 5.4 m. 6) Easy excavating.						

VANE	FIELD	LAB	▲	UC/2
PEAK	◆	■		
REMOLD	◇	□	△	Pocket Pen /2
★ % Fines				
Moisture Content				
W _p %	W ₁ %		W ₂ %	
×	○	○	×	×

EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-77

LOCATION: OLIVE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 461364E, 7101655N
 GROUND ELEVATION (m): 1099
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 12 Jul 11
 FINISH DATE: 12 Jul 11
 FINAL DEPTH OF PIT (m): 5.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0		1			ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.4 - 0.5					GRAVEL (GM) Silty, trace cobbles, trace sand, well graded, compact, maximum particle size = 150 mm, angular to subangular, medium to dark brown, moist, homogeneous. [COLLUVIUM] 0.4 - 0.5 m - Sample 1, grain size analysis, 22% gravel, 34% sand, 31% silt, 13% clay.				
0.5 - 2.0			4		METASEDIMENTARY ROCK Dark brown to grey (with iron staining), highly weathered (W4), weak to medium strong (R2 to R3). Disintegrated - excavated as silty gravel, some cobbles, trace boulders. [HIGHLY WEATHERED BEDROCK]				
2.0 - 5.5			3.5		METASEDIMENTARY ROCK Iron stained, weak to medium strong (R2 to R3), moderately to highly weathered (some pockets of completely weathered orangish yellow material), blocky/disturbed, tabular blocks, excavated as gravel and cobbles, silty, trace boulders, maximum particle size = 350 mm, average size 50-100 mm. [MODERATELY TO HIGHLY WEATHERED BEDROCK]				
5.5					End of test pit at 5.5 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 461903E, 7101806N
 GROUND ELEVATION (m): 1166
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 12 Jul 11
 FINISH DATE: 12 Jul 11
 FINAL DEPTH OF PIT (m): 3.8
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	▲
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.5	G	1			SAND (SW-SM) Some silt, some cobbles, some gravel, trace boulders (granodiorite), subangular to subrounded, maximum particle size = 300 mm. Fines: low plastic, brownish grey, wet, homogeneous. Frozen: Vs (30-40%), lenses approximately 4 cm thick. [COLLUVIUM?]	★	×		
1.5	g	2			0.4 - 0.6 m - Sample 1, grain size analysis, 41% gravel, 48% sand, 8% silt, 3% clay.		○	★	
2.5			5.5		METASEDIMENTARY ROCK Orange to yellow, extremely weak (R0), residual soil to completely weathered (W6 to W5), fine grained, saturated, excavated as clay/silt/sand. Frozen: Nbn. [COMPLETELY WEATHERED BEDROCK] 1.2 - 1.3 m - Sample 2, grain size analysis, 1% gravel, 72% sand, 27% fines. 1.3 - 1.5 m - Seepage from southern corner of pit.				
3.8					End of test pit at 3.8 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) Seepage from 1.3-1.5 m. 4) Frozen ground encountered below 0.2 m to bottom of excavation. 3) Slouging due to seepage.				

EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-80

LOCATION: OLIVE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 461753E, 7101551N
 GROUND ELEVATION (m): 1210
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 13 Jul 11
 FINISH DATE: 13 Jul 11
 FINAL DEPTH OF PIT (m): 6.7
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Topsoil, rootmat. [TOPSOIL]				
0.5					SILT (ML) Sandy, some gravel, trace clay, trace cobbles, trace boulders, low plastic, rapid dilatancy, brown, no odour, moist, soft, homogeneous. Clasts: elongated, angular (metasedimentary gravel), maximum particle size = 500 mm, granodiorite boulders, fresh to slightly weathered (W1.5) and very strong (R5). [COLLUVIUM]				
1.5					GRAVEL (GW) Sandy, trace cobbles, well graded, maximum particle size = 55 mm, pinkish brown, moist. Clasts: metasedimentary, elongated, angular, weak (R2). [COLLUVIUM]				
2.5			2		METASEDIMENTARY ROCK Dark reddish grey, medium strong (R3), slightly weathered (W2), fine grained, foliated, dominant joint set parallel to foliation (40°/278°) with secondary sets: (86°/197°) and (85°/61°), joint roughness (JRC) along foliation 30mm/0.45 m = 20, 0.25-1 mm aperture, infill is iron staining and crushed rock. Other sets JRC=10, similar infill and apertures. Foliation very closely spaced (20-100 mm apart). [SLIGHTLY WEATHERED BEDROCK]				
6.7					End of test pit at 6.7 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) Light seepage at base of test pit. 4) Not frozen. 5) Test pit is at an existing road cut.				

EGR (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 461711E, 7101480N
 GROUND ELEVATION (m): 1207
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 13 Jul 11
 FINISH DATE: 13 Jul 11
 FINAL DEPTH OF PIT (m): 3.9
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Topsoil, rootmat [TOPSOIL]				
1	G	1			SAND AND GRAVEL (SW/GW) Some cobbles, trace silt, some cobbles, well graded, loose, maximum particle size = 250 mm, light pinkish brown, no odour, moist, homogeneous, non-cemented. Clasts: mainly metasedimentary, weak (R2), slightly to moderately weathered (W2.5), equidimensional to elongated, angular to subangular. [COLLUVIUM]	★	○		
2			2.5		GRAVEL (GW-GM) Sandy, some silt, trace clay, trace cobbles, trace boulders, equidimensional to elongated, angular to subangular, maximum particle size = 300 mm, Fines: low plastic, brown, moist, homogeneous, no odour, no cementation, rapid dilatency. [COLLUVIUM] 0.7 - 1.0 m - Sample 1, grain size analysis, 65% gravel, 24% sand, 10% silt, 1% clay. At 1.3 m - There is a 10 cm thick organic layer: roots, organic silt.				
3					METASEDIMENTARY ROCK Light yellowish brown, fine grained, extremely closely fractured, foliated, very weak (R1), slightly to moderately weathered (W2.5), 1 recognizable set parallel to foliation (with a dip/dip direction of 85/159), typical aperture 1-4mm, infill is crushed rock, sand and iron staining, JRC= 4-15, spacing between foliation 20-120 mm. Clasts: medium strong to strong (R3 to R4), slightly weathered (W2). Quartz veins present. [SLIGHTLY TO MODERATELY WEATHERED BEDROCK]				
4					End of test pit at 3.9 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Excavation was made as a refreshing of existing road cut.				
5									
6									
7									
8									

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458604E, 7100691N
 GROUND ELEVATION (m): 815
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 11 Jun 11
 FINISH DATE: 11 Jun 11
 FINAL DEPTH OF PIT (m): 7.5
 LOGGED BY: DW
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _L %	W _p %	W _L %	W _p %
						×	○	○	×	×
0					ORGANICS Organics, rootmat. [TOPSOIL]					
1	G	4			SILT (ML) Some medium to fine sand, trace gravel, compact, yellowish/beige, moist. Clasts: maximum particle size = 80 mm, subrounded elongated. [ALLUVIUM?] 0.7 - 1.1 m - Sample 4, grain size analysis, 20% sand, 80% fines.		⊗			★
2	G	5			SILT (ML) Sandy, low plasticity, firm to stiff, yellowish to brown, moist to wet. [ALLUVIUM] PP/2 = 75 kPa 1.9 - 2.1 m - Sample 5, grain size analysis, 25% sand, 75% fines.		⊗	△		★
3	G	2			SAND (SM) Medium to fine, silty, gravelly, compact to dense, maximum particle size = 100 mm, subrounded, brownish, reddish, moist, highly weathered gravel sized metasedimentary rock fragments. [ALLUVIUM] 3.0 - 3.3 m - Sample 2, grain size analysis, 7% gravel, 21% sand, 72% fines.	○	⊗			★
4					SILT (ML) Sandy, low plasticity, very stiff, brown, moist, some organics. [ALLUVIUM] PP/2 = 140-190 kPa					
5	G	1			4.7 - 4.9 m - Sample 1, grain size analysis, 5% sand, 90% silt, 5% clay.	○				△ ★
6					GRAVEL (GP) Some sand, gravel sized particles of highly weathered metasediments, maximum particle size = 170 mm. [ALLUVIUM]					
7	G	3			SILT (ML) Sandy, clayey, low plasticity, still to very stiff, brown to black, moist to wet when thawed, some organics, soft, organic smell. Frozen: Vs-Vx (5-10%). [ALLUVIUM] PP/2: 90-140 kPa At 6.5 m - Exposed massive ice (approximately 0.6 x 0.3 m). 6.7 - 7.0 m - Sample 3, grain size analysis, 2% gravel, 20% sand, 74% silt, 4% clay.		○			★
8					End of test pit at 7.5 m.					

(Continued on next page)

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-82

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458604E, 7100691N
 GROUND ELEVATION (m): 815
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 11 Jun 11
 FINISH DATE: 11 Jun 11
 FINAL DEPTH OF PIT (m): 7.5
 LOGGED BY: DW
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa					
							40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2	
						PEAK	◆	■			
		◇	□	△	Pocket Pen /2						
★ % Fines						Moisture Content					
		×			○	×					
		20	40	60	80						
8					<p>Notes:</p> <ol style="list-style-type: none"> 1) Completed at maximum reach of excavator. 2) Test pit corresponded with septic system excavation of 52 m x 53 m. 3) No seepage. 4) When excavation for septic system began June 8th, 2011, ground was frozen below 0.3 m. When test pit was excavated frozen ground encountered from 6.2 m to base of test pit. 						
9											
10											
11											
12											
13											
14											
15											
16											

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-83

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458786E, 7100077N
 GROUND ELEVATION (m): 863
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 13 Jul 11
 FINISH DATE: 13 Jul 11
 FINAL DEPTH OF PIT (m): 1.3
 LOGGED BY: EC/JS
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Rootmat, brown peat, vegetation, moss. [TOPSOIL]				
0.6 - 0.8	G	1			ORGANIC SILT Dark brown to black. Frozen: trace Vx. [ORGANICS]		○	★	×
0.6 - 0.8					SAND (SM) Fine sand, gravelly, silty, trace cobbles, trace boulders. Fines: inorganic silt, high plastic, brown to grey, wet. Frozen: trace Vx, Nbe, wet when thawed. [COLLUVIUM] 0.6 - 0.8 m - Sample 1, grain size analysis, 33% gravel, 29% sand, 31% silt, 7% clay.				
1.3					End of test pit at 1.3 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.3 m to bottom of excavation. 4) Difficult to excavate, excavator resting on saturated soils.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-84

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458763E, 7100006N
 GROUND ELEVATION (m): 863
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 13 Jul 11
 FINISH DATE: 13 Jul 11
 FINAL DEPTH OF PIT (m): 2.6
 LOGGED BY: EC/JS
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	UC/2	
						PEAK	◆	■	▲	
REMOLD	◇	□	△	Pocket Pen /2						
★ % Fines										
Moisture Content										
W _p %	W _o %		W _l %		W _u %					
×	20	40	60	80	×					
0				▨	ORGANICS Moss, vegetation, rootmat. [TOPSOIL]					
	G	1		▨	SILT (ML) Some sand, some gravel, trace cobbles, medium brown, moist, homogeneous. Clasts: angular to subangular clasts, loose. [COLLUVIUM] 0.4 - 0.6 m - Sample 1, grain size analysis, 20% sand, 34% silt, 46% clay. Sulphate analysis, 0.07% total sulphate ion content.	○				★
1				▨	SILT (ML) Sandy, fine, trace clay, mottled medium grey and orange brown. Frozen: Nbn, moist when thawed. Sand/gravel/cobble lenses until end of pit. [COLLUVIUM]					
	G	2		▨	2.3 - 2.5 m - Sample 2, grain size analysis, 2% gravel, 4% sand, 84% silt, 10% clay.	⊗				★
2				▨	End of test pit at 2.6 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) Slow seepage in north wall at 0.5 m. 4) Frozen ground encountered below 0.75 m to bottom of excavation. 5) Frozen material very difficult to excavate.					
3				▨						
4				▨						
5				▨						
6				▨						
7				▨						
8				▨						

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-85

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458783E, 7100055N
 GROUND ELEVATION (m): 865
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 13 Jul 11
 FINISH DATE: 13 Jul 11
 FINAL DEPTH OF PIT (m): 2.5
 LOGGED BY: EC/JS
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Rootmat, moss, vegetation. [TOPSOIL]				
1	G	1			ORGANIC SILT Dark brown to black. Frozen: trace Vx. [ORGANICS]	○ ×		★	
2					SILT (ML) Sandy (fine), some gravel, trace cobbles, trace boulders, low to non-plastic, brown to grey, wet, occasional stratification of tan brown layer, angular clasts. Frozen: Vs (50%). [COLLUVIUM] 0.7 - 1.1 m - Sample 1, grain size analysis, 7% gravel, 42% sand, 40% silt, 11% clay.				
3					SILT (ML) Sandy, fine, trace clay, non to low plastic. Frozen: Vs/Vr (20-25%), thin ice lenses, decreasing ice content with depth. [COLLUVIUM]				
4					End of test pit at 2.5 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.3 m to bottom of excavation. 5) Very difficult to excavate.				
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-86

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460124E, 7101751N
 GROUND ELEVATION (m): 894
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 13 Jul 11
 FINISH DATE: 13 Jul 11
 FINAL DEPTH OF PIT (m): 7.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1					SAND (SM) Fine to medium, gravelly, silty, well graded, compact, maximum particle size = 60 mm, angular to subangular, yellowish brown, moist, homogeneous, rootlets and trace organics. [COLLUVIUM]				
2					METASEDIMENTARY ROCK Foliated, yellowish brown, highly to completely weathered (W4 to W5), weak to medium strong (R2 to R3), maximum block size = 400 mm, excavated as gravel, some cobbles, some sand, trace silt, trace boulders, layered with different degrees of weathering and colour 0.5-1.5 m thick (yellow orange, grey, brown). [HIGHLY TO COMPLETELY WEATHERED BEDROCK]				
4			4.5						
5									
6									
7									
8					End of test pit at 7.5 m. Notes: 1) Completed at maximum reach of excavator.				

(Continued on next page)

EGR (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-86

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460124E, 7101751N
 GROUND ELEVATION (m): 894
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 13 Jul 11
 FINISH DATE: 13 Jul 11
 FINAL DEPTH OF PIT (m): 7.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa					
						40	80	120	160		
						VANE	FIELD	LAB	▲	UC/2	
						PEAK	◆	■			
REMOLD	◇	□	△	Pocket Pen /2							
★ % Fines											
						Moisture Content					
						W _p %	W ₁ %			W ₂ %	
						×	20	40	60	80	×
8					2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Easy excavating (weak rock).						
9											
10											
11											
12											
13											
14											
15											
16											

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-87

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460690E, 7100247N
 GROUND ELEVATION (m): 1194
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 14 Jul 11
 FINISH DATE: 14 Jul 11
 FINAL DEPTH OF PIT (m): 3.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa					
						40		80		120	160
						VANE	FIELD	LAB	▲ UC/2		
						PEAK	◆	■	Pocket Pen /2		
						★ % Fines					
						Moisture Content					
						W _p %	W _p %	W _p %	W _p %	W _p %	
						×	—	○	—	×	
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]						
1	G	1			ORGANIC SILT Gravelly, some sand, trace cobbles, non-plastic, very soft, dark brown to black, moist. [ORGANICS]	★○					
2					SAND (SP) Coarse, gravelly (fine), trace cobbles, poorly graded, very loose, largest particle size = 100 mm, stratified, angular to subangular, medium brown to brownish orange, moist to wet. [COLLUVIUM] 0.6 - 0.8 m - Sample 1, grain size analysis, 26% gravel, 68% sand, 6% fines. At 1.1 m - Light seepage.						
3			5		SAND (SW) Coarse sand, gravelly, some cobbles, angular, very dense, orangish brown, very weak to weak (R1 to R2), clasts are metasedimentary. Becomes cobbly and less weathered with depth. Cobbles: strong (R4). [COMPLETELY WEATHERED METASEDIMENTARY BEDROCK]						
3			3		METASEDIMENTARY ROCK Same as above but stronger (R4) and moderately weathered (W3). [MODERATELY WEATHERED BEDROCK]						
4					End of test pit at 3.5 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) Seepage observed, along back of road, at 1.1 m. 4) Not frozen. 5) Excavated on upslope of road cut. 6) Excavation became extremely difficult at 3.3 m. 7) Heavy rain overnight may be contributing to seepage.						
5											
6											
7											
8											

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459813E, 7101008N
 GROUND ELEVATION (m): 922
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 14 Jul 11
 FINISH DATE: 14 Jul 11
 FINAL DEPTH OF PIT (m): 6.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1					SAND (SM) Fine to coarse sand, silty, gravelly, trace cobbles, trace boulders, angular to subangular, maximum particle size = 500 mm. Fines: non-plastic, firm, light brown, moist, homogeneous. [COLLUVIUM]				
1.5 - 1.8 m	G	1			1.5 - 1.8 m - Sample 1, grain size analysis, 27% gravel, 39% sand, 23% silt, 11% clay.		○	×	×
2					SAND (SW) Some cobbles, some gravel, fine to coarse, trace boulders, trace silt, well graded, dense, maximum particle size = 800 mm, light brown, dry to moist, homogeneous, cobbles and boulders are slightly weathered (W2), strong to very strong (R4 to R5), metasedimentary and granodiorite. Clasts: subangular to subrounded. [COLLUVIUM]				
4					COBBLES AND BOULDERS Sandy, trace gravel, trace silt, maximum particle size = 1100 mm, subrounded to subangular, light brown, dry. Clasts: slightly weathered (W2), medium strong to strong (R4 to R5), metasedimentary and granodiorite. [COLLUVIUM]				
6					End of test pit at 6.0 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Required moderate effort to excavate.				

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-89

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459876E, 7100737N
 GROUND ELEVATION (m): 985
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 14 Jul 11
 FINISH DATE: 14 Jul 11
 FINAL DEPTH OF PIT (m): 6.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			Pocket Pen /2
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.4 - 0.6	G	1			GRAVEL (GW) Fine to medium, trace sand, trace cobbles, subangular to subrounded, well graded, very loose, maximum particle size = 150 mm, grey to brown, moist, homogeneous. [COLLUVIUM] 0.4 - 0.6 m - Sample 1, grain size analysis, 78% gravel, 21% sand, 1% fines.				
1.15 - 1.5	G	2			SAND (SM) Silty, some gravel, trace cobbles, trace boulders, firm, brown, moist, homogeneous. Clasts: subangular. 1.15 - 1.5 m - Frozen: Vx (<1%). [COLLUVIUM] 1.3 - 1.5 m - Sample 2, grain size analysis, 24% gravel, 32% sand, 36% silt, 8% clay.		×	○	★
5.0 - 6.5			5		METASEDIMENTARY ROCK Disintegrated to blocky/disturbed, grey to brown, completely weathered (W5), excavated as cobbles and gravel, sandy, trace silt, 30% of blocks weathered to clay. Extremely weak to weak (R0 to R2), some medium strong to strong clasts (R3 to R4). [COMPLETELY WEATHERED BEDROCK]				
6.5 - 7.0					End of test pit at 6.5 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered from 1.15 - 4.5 m.				

EGR (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

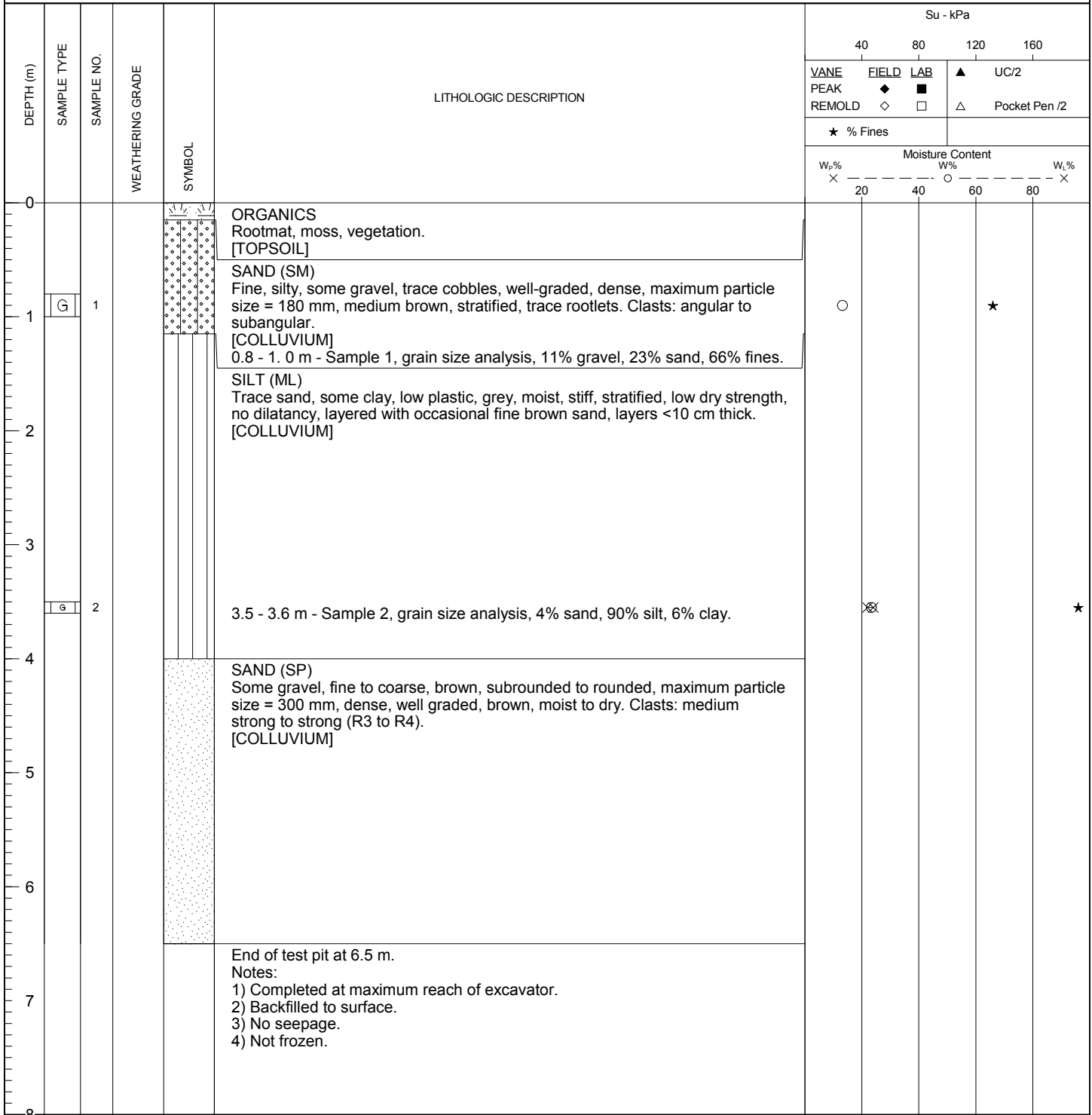
TEST PIT # TP-BGC11-90

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459686E, 7100502N
 GROUND ELEVATION (m): 981
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 14 Jul 11
 FINISH DATE: 14 Jul 11
 FINAL DEPTH OF PIT (m): 6.5
 LOGGED BY: EC
 REVIEWED BY: PQ/DW



EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-91

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459649E, 7100633N
 GROUND ELEVATION (m): 969
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 15 Jul 11
 FINISH DATE: 15 Jul 11
 FINAL DEPTH OF PIT (m): 2.1
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
1	G	1			SAND (SM) Silty, some fine to medium gravel, trace cobbles, well graded, loose, medium to brown, wet, stratified, trace rootlets. Clasts angular, maximum particle size = 250 mm. [COLLUVIUM]		×	×	○
2	G	2			SILT (ML) Some sand, trace cobbles (quartz), trace gravel, trace clay, grey, angular particles, wet when thawed, stratified. Frozen: Vx (20%). [COLLUVIUM] 0.8 - 1.0 m - Sample 1, grain size analysis, 7% gravel, 23% sand, 60% silt, 10% clay.		○	★	×
3					GRAVEL (GM) Silty, some sand, some cobbles, trace clay, subangular to subrounded, grey to brown, quartz clasts. Frozen: Vx (10%). [COLLUVIUM] 1.6 - 1.8 m - Sample 2, grain size analysis, 52% gravel, 33% sand, 12% silt, 3% clay.				
4					End of test pit at 2.05 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.55 m to bottom of excavation. 5) Extreme difficulty excavating (scraping).				
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-92

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459587E, 7100733N
 GROUND ELEVATION (m): 933
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 15 Jul 11
 FINISH DATE: 15 Jul 11
 FINAL DEPTH OF PIT (m): 1.7
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
REMOLD	◇	□	△	Pocket Pen /2						
★ % Fines										
Moisture Content										
W _p %	W ₁ %			W ₁ %						
×	20	40	60	80	×					
0					ORGANICS Vegetation, moss, rootmat. [TOPSOIL]					
1	G	1			SILT (ML) Some gravel, trace cobbles, trace sand, turns to slurry when thawed, grey. Clasts of quartz: angular to subangular. Frozen: Vx (20-50%). [COLLUVIUM] 0.25 - 0.5 m - Trace organic inclusions. 0.6 - 0.8 m - Sample 1, grain size analysis, 7% sand, 81% silt, 12% clay.		×	○		★
2					End of test pit at 1.7 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) Seepage from west corner at 0.25 m. 4) Encountered frozen ground below 0.25 m to bottom of excavation. 5) Extreme difficulty excavating.					
3										
4										
5										
6										
7										
8										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-93

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459536E, 7100794N
 GROUND ELEVATION (m): 917
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 15 Jul 11
 FINISH DATE: 15 Jul 11
 FINAL DEPTH OF PIT (m): 1.9
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa					
						40		80		120	160
						VANE	FIELD	LAB	▲ UC/2		
						PEAK	◇	■	△ Pocket Pen /2		
						Moisture Content					
						W _p %	W _p %		W _p %		W _p %
						×	—	○	—	×	
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]						
1					ORGANIC SILT (OL) Rootlets, brown/dark brown to black. [ORGANICS]						
1.9					ICE Thin layer of silt, trace sand, fine, clear, transparent, <1% bubbles.						
2					SILT AND ICE (ML) Some gravel, trace sand, trace cobbles, brown to grey, homogeneous, quartz clasts. Frozen: Vx/Vs (40-60%), <1% bubbles, trace nodules <100 mm, turns to slurry when thawed. [COLLUVIUM]						
1.9					End of test pit at 1.9 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) Light seepage in NE corner at 0.4 m. 4) Encountered frozen ground below 0.4 m to bottom of excavation. 5) Extremely difficult to excavate (scraping).						

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459983E, 7101522N
 GROUND ELEVATION (m): 930
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 15 Jul 11
 FINISH DATE: 15 Jul 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.5	G	1			SILT (ML) Some gravel, sandy, trace boulders (angular), low plastic, medium brown. Frozen: Vx, (5-10%), occasional ice lenses, soft and wet when thawed. [COLLUVIUM] 1.2 - 1.3 m - Sample 1, grain size analysis, 22% gravel, 34% sand, 34% silt, 10% clay.		×	★	
2.0	G	2			SAND (SP) Fine, some gravel, trace silt, trace cobbles, trace boulders, poorly graded, moist, homogeneous, medium to light brown. Clasts angular, maximum particle size = 200 mm. [COLLUVIUM] 2.3 - 2.5 m - Sample 2, grain size analysis, 21% gravel, 38% sand, 41% fines.		○	★	
4.2					At 4.2 m - Boulder content increases, subrounded to rounded.				
5.0					End of test pit at 5.0 m. Notes: 1) Refusal on boulders. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered from 0.2-2 m.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-95

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460283E, 7100135N
 GROUND ELEVATION (m): 1109
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 26 Jul 11
 FINISH DATE: 26 Jul 11
 FINAL DEPTH OF PIT (m): 3.2
 LOGGED BY: EC/SP
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0					ORGANICS Moss, vegetation, root mat. [TOPSOIL]				
1	G	1	5		SAND (SW) Fine to medium, trace gravel, trace silt, trace clay, compact, maximum particle size = 250 mm, angular, moist. Disintegrated granodiorite. [COMPLETELY WEATHERED GRANODIORITE ROCK] 0.1 - 0.3 m - Orangish brown. 0.3 - 3.2 m - Light grayish brown - white stratified (0.5-10 cm). 0.9 - 1.1 m - Sample 1, grain size analysis, 10% gravel, 65% sand, 25% fines.	○	★		
2	G	2	5		SAND (SW) Some clay, trace silt, trace gravel, maximum particle size = 250 mm, angular, light gray brown; disintegrated granodiorite. [COMPLETELY WEATHERED GRANODIORITE ROCK] 1.8 - 2.2 m - Frozen: Vx (<1%). 2.0 - 3.0 m - Sample 2, grain size analysis, 1% gravel, 75% sand, 24% fines. 2.2 - 3.2 m - Frozen: Vx (20-30%), white, granular.		★		
3.2					End of test pit at 3.2 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 5) Frozen ground encountered below 1.8 m to bottom of excavation.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460287E, 7100170N
 GROUND ELEVATION (m): 1103
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 26 Jul 11
 FINISH DATE: 26 Jul 11
 FINAL DEPTH OF PIT (m): 2.0
 LOGGED BY: EC/SP
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Moss, vegetation, rootmat. [TOPSOIL]				
0.7 - 1.0	G	1			SAND (SW) Some gravel, some cobbles, loose, maximum particle size = 100 mm, angular, moist, stratified, trace rootlets, medium brown - gray. Clasts: granodiorite. [COMPLETELY WEATHERED BEDROCK]	○	★		
1.0 - 1.8	G	2			SAND (SW) Fine to medium, trace gravel, trace silt, maximum particle size = 45 mm, angular, compact, moist, stratified; layers: 1-10 cm. Clasts: granodiorite. [COMPLETELY WEATHERED BEDROCK]	○	×		★
1.8 - 2.0	G	3			0.7 - 0.9 m - Sample 1, grain size analysis, 26% gravel, 60% sand, 14% fines.	○	★		
2.0 - 3.0					SILT (ML) Clayey, trace gravel, trace sand, low plastic, grey and brown, moist to wet, stratified brown - grey layers, slow dilatancy. Frozen: Vx (< 1%). [COMPLETELY WEATHERED BEDROCK] 1.3 - 1.5 m - Sample 2, grain size analysis, 3% gravel, 32% sand, 54% silt, 11% clay.				
3.0 - 4.0					SAND (SW) Some gravel, trace cobbles, trace silt, maximum particle size = 280 mm, angular, medium brown. Frozen: Vx (30-40%), cloudy, wet when thawed. [COMPLETELY WEATHERED BEDROCK] 1.8 - 2.0 m - Sample 3, grain size analysis, 36% gravel, 49% sand, 15% fines.				
4.0 - 8.0					End of test pit at 2.0 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 1.25 m to bottom of excavation. 5) Extreme difficulty excavating 1.6-2.0 m.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006


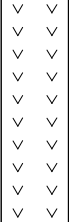
TEST PIT # TP-BGC11-97

LOCATION: PIT AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460546E, 7099463N
 GROUND ELEVATION (m): 1353
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 25 Jul 11
 FINISH DATE: 25 Jul 11
 FINAL DEPTH OF PIT (m): 4.0
 LOGGED BY: EC
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa							
						40		80		120		160	
						VANE	FIELD	LAB	UC/2				
						PEAK	◇	■	▲				
			REMOULD		◇	□	△	Pocket Pen /2					
						★ % Fines							
						Moisture Content							
W _p %		W ₁ %		W ₂ %		W ₃ %							
×		○		○		×							
20		40		60		80							
0					GRAVEL (GW) Cobbly, silty, sandy, trace boulders, maximum particle size = 500 mm, loose, damp, homogeneous. Clasts metasedimentary; angular. [COLLUVIUM]								
1	G	1											
2													
3			4		METASEDIMENTARY ROCK Highly weathered, blocky/disturbed, fair joint condition, light to dark brown, heavily iron stained, very weak to weak (R1 to R2), excavated as cobbles and gravel - some sand. Some clasts: quartz, medium strong to strong (R2 to R3). [HIGHLY WEATHERED BEDROCK]								
4	G	2			End of test pit at 4.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.								
5													
6													
7													
8													

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-98

LOCATION: PIT AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460521E, 7099465N
 GROUND ELEVATION (m): 1342
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 25 Jul 11
 FINISH DATE: 25 Jul 11
 FINAL DEPTH OF PIT (m): 2.2
 LOGGED BY: EC/PQ
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0									
1	G	1			GRAVEL AND COBBLES (GW) Silty, sandy, trace boulders, well-graded, loose, damp, homogeneous. Clasts metasedimentary, angular, medium to very strong (R3 to R5), maximum particle size = 500 mm. [COLLUVIUM]				
2	G	2	2.5	∇ ∇	METASEDIMENTARY ROCK Slightly to moderately weathered (W2 to W3), very blocky, angular, fair joint condition, brown to red to black, iron stained, medium to very strong (R3 to R5). [SLIGHTLY TO MODERATELY WEATHERED BEDROCK]				
3					End of test pit at 2.2 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-99

LOCATION: PIT AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460576E, 7099496N
 GROUND ELEVATION (m): 1352
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 25 Jul 11
 FINISH DATE: 25 Jul 11
 FINAL DEPTH OF PIT (m): 2.5
 LOGGED BY: EC/PQ
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa									
						40	80	120	160						
						VANE	FIELD	LAB	▲	UC/2					
						PEAK	◆	■							
						REMO	◇	□	△	Pocket Pen /2					
						★ % Fines									
						Moisture Content									
						W _p %	×	20	40	60	80	W _L %	○	×	
0					GRAVEL AND COBBLES (GW) Silty, sandy, trace boulders, well-graded, angular, medium to very strong (R3 to R5) metasedimentary clasts, maximum particle size = 500 mm, loose, damp. [COLLUVIUM]										
1					METASEDIMENTARY ROCK Highly weathered, some quartz alteration, very blocky, angular, fair joint condition, light to dark orangish brown, heavily iron stained, medium strong to strong (R3 to R4). [HIGHLY WEATHERED BEDROCK]										
2			4												
3		1			End of test pit at 2.5 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.										
4															
5															
6															
7															
8															

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-100

LOCATION: PIT AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460565E, 7099505N
 GROUND ELEVATION (m): 1345
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 25 Jul 11
 FINISH DATE: 25 Jul 11
 FINAL DEPTH OF PIT (m): 3.5
 LOGGED BY: EC/PQ
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _p %	W _p %	W _p %	W _p %
						×	○	○	○	×
0					GRAVEL AND COBBLES (GW) Silty, sandy, trace boulders, well-graded, angular, medium strong to very strong (R3 to R5), metasedimentary clasts, maximum particle size = 500 mm, loose, damp. [COLLUVIUM]					
1					METASEDIMENTARY ROCK Highly weathered, blocky/disturbed, angular, fair joint condition, light to dark brown, iron stained, very weak to weak (R1 to R2), trace strong to very strong material (R4 to R5). [HIGHLY WEATHERED BEDROCK]					
3.5	1		4		End of test pit at 3.5 m. Notes: 1) Refusal on Bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.					
4										
5										
6										
7										
8										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-101

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460089E, 7098940N
 GROUND ELEVATION (m): 1180
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 30 Jul 11
 FINISH DATE: 30 Jul 11
 FINAL DEPTH OF PIT (m): 2.4
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0	G	1			ORGANICS Rootmat. [TOPSOIL]				
1	G	2			SILT (ML) Trace sand, non plastic, brown to dark brown, slight organic odour, wet, stratified. Frozen: Vr (5%), ice lenses 2-30 mm thick. [COLLUVIUM]				
2					GRAVEL and SAND (GW/SW) Trace to some silt, trace cobbles and boulders, well graded, maximum particle size = 550 mm, brown. Clasts: metasedimentary, elongated, angular, medium strong to very strong (R3 to R5). Frozen: Vx (30-40%), wet when thawed. [COLLUVIUM]				
3					End of test pit at 2.4 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.2 m to bottom of excavation. 5) Excavated on side of pad for planned exploration drill hole.				
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-102

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460565E, 7099111N
 GROUND ELEVATION (m): 1361
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 01 Aug 11
 FINISH DATE: 01 Aug 11
 FINAL DEPTH OF PIT (m): 4.6
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0	G	1			ORGANICS Rootmat, organic silt. [TOPSOIL]				
0.5			4		GRAVEL (GW) Silty, cobbly some fine sand, trace boulders, well graded, compact, particles are flat; angular, metasedimentary, strong (R4), brown, moist, homogeneous, non-cemented, no odour. [COLLUVIUM]				
1.5			2		COBBLES AND BOULDERS Some silt, some sand, some gravel, brown, moist. Clasts: metasedimentary, flat, platy, angular, strong (R4). [HIGHLY WEATHERED METASEDIMENTARY ROCK]				
2.5					METASEDIMENTARY ROCK Dark grey discoloured reddish brown, fine grained, foliated, weak to strong (R2 to R4), slightly weathered (W2). 3 Joint Sets: 25°/248° (foliation), 48°/350°, 89°/077° Fault along foliation (22°/250° fault with 35 mm gouge, chlorite, crushed/decomposed rock). [SLIGHTLY WEATHERED BEDROCK]				
4.6					End of test pit at 4.6 m. Notes: 1) Refusal on Bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Test pit excavated on drill pad for BH-BGC11-45.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458820E, 7101150N
 GROUND ELEVATION (m): 865
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 05 Aug 11
 FINISH DATE: 05 Aug 11
 FINAL DEPTH OF PIT (m): 6.0
 LOGGED BY: AKU
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	
						×	○	○	×	
0					ORGANICS Organic silt and rootmat. [TOPSOIL]					
1					SILT (ML) Sandy, some gravel, trace cobbles, moist, low plastic, loose to compact, sub-angular to sub-rounded, flat, metasedimentary clasts, light brown. [COLLUVIUM]					
4					SAND AND GRAVEL (SW/GW) Trace silt, trace cobbles, well graded, maximum particle size = 250 mm, subrounded, metasedimentary and granodiorite clasts (generally metasedimentary), compact, light brown. [TILL]					
4.4 - 4.8	G	1			4.4 - 4.8 m - Sample 1, grain size analysis, 47% gravel, 48% sand, 5% fines.					★
6			4.5		METASEDIMENTARY ROCK Orange and grey, fine grained, foliated, highly fractured, highly weathered (W4.5), very weak (R1). [HIGHLY WEATHERED BEDROCK]					
6					End of test pit at 6.0 m. Notes: 1) Completed at target depth. 2) Test pit used for plate load testing. 3) No seepage. 4) Not frozen.					
7										
8										

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459738E, 7100924N
 GROUND ELEVATION (m): 832
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 23 Aug 11
 FINISH DATE: 23 Aug 11
 FINAL DEPTH OF PIT (m): 5.6
 LOGGED BY: LGT/DW
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat and organic silt. 0.1 - 0.3 m - Light seepage directly below rootmat. [TOPSOIL]				
1	G	1			SILT (ML) Gravelly, some sand, trace cobbles, non plastic, firm, brownish grey, no odour, moist, homogeneous, rapid dilatancy. Clasts: metasedimentary, equidimensional to flat, angular to sub-angular, medium strong to strong (R3 to R4), maximum particle size = 200 mm. [COLLUVIUM].				
2					GRAVEL (GW) Sandy, some cobbles, trace boulders, well graded, loose to compact, maximum particle size = 500 mm, particles are equidimensional to elongated, rounded to angular, mix of medium to very strong (R3 to R5), granodiorite and medium strong to strong (R3 to R4), metasedimentary rock (90% metased clasts), pinkish brown, moist, homogeneous, non cemented. [TILL] 1.2 - 1.6 m - Sample 1, grain size analysis, 60% gravel, 33% sand, 7% fines.				
4			4		SILT (ML) Sandy, gravelly, trace clay, low plastic, firm to stiff, yellowish orange, no odour, moist, laminated/foliated: phyllitic rock that crumbles down to silt when disturbed, micaceous. [COMPLETELY WEATHERED METASEDIMENTARY BEDROCK]				
5			4		GRAVEL (GW) Sandy, silty, trace clay, well graded, maximum particle size = 250 mm. Clasts: phyllite, platy, extremely to very weak (R0 to R1), yellowish brown/grey/orange, moist, foliated. [HIGHLY WEATHERED METASEDIMENTARY BEDROCK] At 4.4 m - 250 mm thick quartz vein.				
6					End of test pit at 5.6 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) Light seepage from 0.1-0.3 m (directly beneath rootmat). 4) Not frozen. 5) Sloughing near bottom of test pit.				
7									
8									

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-105

LOCATION: WEST FLANK TIN DOME

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458939E, 7101190N
 GROUND ELEVATION (m): 878
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 09 Aug 11
 FINISH DATE: 09 Aug 11
 FINAL DEPTH OF PIT (m): 3.0
 LOGGED BY: AKU
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	
						×	○	○	×	
0					ORGANICS Root mat and organic silt. [TOPSOIL]					
0.5					SAND (SM) Silty, some gravel, trace to some cobbles, moist, well graded, loose to compact, sub-angular, quartz pieces, light brown becoming grey with depth, metasedimentary clasts. [COLLUVIUM]					
3.0			4		METASEDIMENTARY ROCK Grey, very weak to weak (R1.5), highly fractured, iron stained in places. [HIGHLY WEATHERED BEDROCK]					
3.0					End of test pit at 3.0 m. Notes: 1) Completed at target depth. 2) Test pit used for plate load testing. 3) No seepage. 4) Not frozen.					

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-106

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460375E, 7100012N
 GROUND ELEVATION (m): 1155
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 06 Aug 11
 FINISH DATE: 06 Aug 11
 FINAL DEPTH OF PIT (m): 3.0
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0					ORGANICS Rootmat. [TOPSOIL]				
0.45					ORGANIC SILT (OL) Trace sand, non-plastic, wet, dark brown. [PEAT]				
0.8	G	1			SAND (SW) Trace to some fine gravel, trace cobbles, maximum particle size = 80 mm, brown, wet. Clasts: elongate to equidimensional, angular to subangular, granodiorite, strong to very strong (R4 to R5). Frozen: Nbn. [COLLUVIUM]		○		
1.4					GRAVEL (GW) Cobbley, sandy, some boulders, maximum particle size = 2.5 m, wet, yellowish brown, homogeneous. Clasts: elongated to equidimensional, angular to subangular, granodiorite, strong to very strong (R4 to R5), fresh to faintly weathered (W1 to W1.5). Frozen: Vx (20%), ice is colourless, cloudy to clear, up to 10% organic sediment inclusion. [COLLUVIUM] 1.4 - 1.9 m - Frozen: Vx (5-10%).				
3.0					End of test pit at 3.0 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.45 m to bottom of excavation. 5) Logged from cut made for drill pad for BH-BGC11-61 and BH-BGC11-66.				

EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-107

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460182E, 7100261N
 GROUND ELEVATION (m): 1044
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 16 Aug 11
 FINISH DATE: 16 Aug 11
 FINAL DEPTH OF PIT (m): 3.2
 LOGGED BY: JD/KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			Pocket Pen /2
						Moisture Content			
						W _p %	W _l %	W _u %	W _l %
						×	○	○	×
0					ORGANICS Rootmat, organic silt. [TOPSOIL]				
1					SILT (ML) Gravelly, sandy, trace cobbles, low plastic, brown, no odour, wet when thawed, no cementation, homogeneous, rapid dilatancy, maximum particle size = 200 mm. Clasts: some flat clasts, angular, metasedimentary, slightly weathered (W2), weak to medium strong (R2 to R3). Frozen: Vx/Vr (5-15%), in lenses 0.5-5 mm thick and gravel-size crystals. [COLLUVIUM]				
2.6					Below 2.6 m - Grades to sand (SM), some silt, some sand.				
2.8					2.8 - 3.0 m - Sample 1, grain size analysis, 25% gravel, 41% sand, 30% silt, 4% clay.				
2.9					Below 2.9 m - Excavation with ripper shank required.				
3.2					End of test pit at 3.2 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.25 m to bottom of excavation. 4) Test pit excavated at drill pad for BH-BGC11-64.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-108

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460285E, 7099975N
 GROUND ELEVATION (m): 1145
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 09 Aug 11
 FINISH DATE: 09 Aug 11
 FINAL DEPTH OF PIT (m): 1.5
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, organic soil. [TOPSOIL]				
1	G	1			SILT (ML) Trace sand, trace boulder, (>10% of silt is organic), non plastic, homogeneous, rapid dilatancy, dark brown, organic odour, wet to moist, seams of yellowish brown sand (SW), maximum particle size = 600 mm, trace rootlets. Frozen: Nbn. [COLLUVIUM]	○	★		
2					GRAVEL (GM) Silty, cobbley, sandy, well graded, maximum particle size = 350 mm, particles are elongated to equidimensional, angular, light brown, no odour, wet when thawed. Frozen: Vx (20-30%), ice is clear, colourless, granular. Clasts: granodiorite, strong to very strong (R4 to R5), fresh to slightly weathered (W1 to W2). [COLLUVIUM] 0.9 - 1.15 m - Sample 1, grain size analysis, 37% gravel, 37% sand, 26% fines.				
3					End of test pit at 1.5 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.15 m to bottom of excavation. 5) Wet from thawing of excess ice.				
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

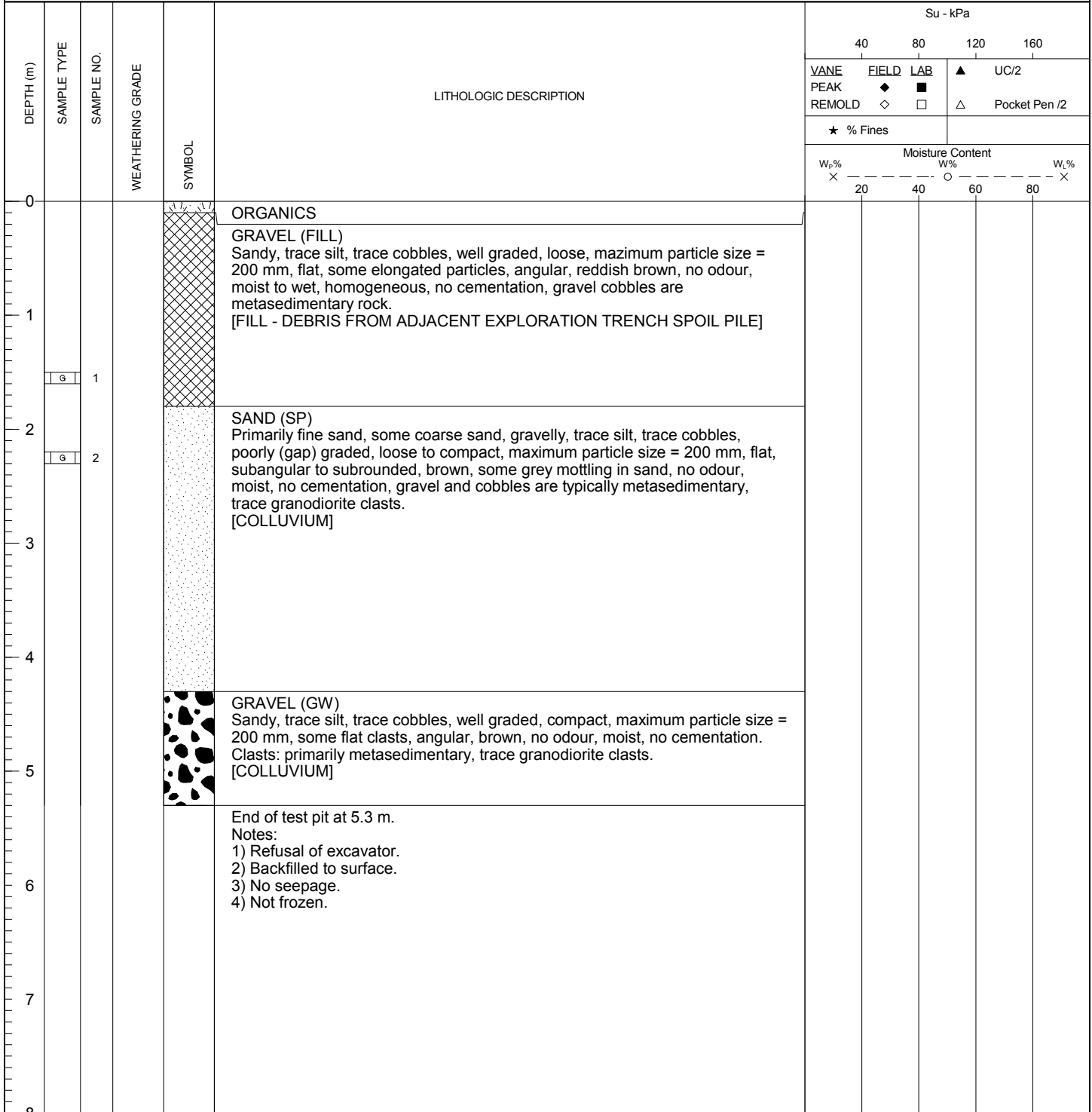
TEST PIT # TP-BGC11-109

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460137E, 7100520N
 GROUND ELEVATION (m): 977
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 16 Aug 11
 FINISH DATE: 16 Aug 11
 FINAL DEPTH OF PIT (m): 5.3
 LOGGED BY: KH
 REVIEWED BY: PQ/DW



EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-110

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459851E, 7101062N
 GROUND ELEVATION (m): 942
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 16 Aug 11
 FINISH DATE: 16 Aug 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	
						×	○	○	×	
0					ORGANICS Lichen, rootmat, organic soil. [TOPSOIL]					
1		1			SAND (SM) Gravelly, some silt, trace cobbles, well graded, loose to compact, maximum particle size = 100 mm, some flat gravel/cobbles, subangular to subrounded, light brown, no odour, moist, no cementation, gravel/cobbles are metasedimentary rock. [COLLUVIUM]					
2					2.0 - 2.3 m - Reddish brown.					
3										
4										
5					End of test pit at 5.0 m. Notes: 1) Refusal on moderately weathered metasedimentary bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.					
6										
7										
8										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-111

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460653E, 7100373N
 GROUND ELEVATION (m): 1185
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 17 Aug 11
 FINISH DATE: 17 Aug 11
 FINAL DEPTH OF PIT (m): 2.0
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Rootmat. [TOPSOIL]				
1	G	1			BOULDERS Angular to subangular, typically flat, maximum particle size = 800 mm, oriented approximately parallel to slope, granodiorite boulders. [COLLUVIUM]		○		
2					BOULDERS Some cobbles, some coarse sand, some silt, trace gravel, poorly (gap graded), compact, maximum particle size = 800 mm, oversize typically flat, sand and gravel equidimensional, subangular to angular, brown sand/silt, grey/white oversize, no odour, moist, no cementation, sand/gravel and oversize are granodiorite. [COLLUVIUM]				
2.0					End of test pit at 2.0 m. Notes: 1) Refusal on boulders/cobbles. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				
3									
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-112

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460350E, 7100504N
 GROUND ELEVATION (m): 1033
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 17 Aug 11
 FINISH DATE: 17 Aug 11
 FINAL DEPTH OF PIT (m): 2.5
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	×	×
0					ORGANICS Rootmat. [TOPSOIL]				
0.6		1			GRAVEL (GP) Cobbly, poorly (uniformly) graded, loose, flat, angular, brown, moist, metasedimentary clasts. [COLLUVIUM]				
1.3					GRAVEL (GM) Silty, trace cobbles, trace sand, well graded, maximum particle size = 100 mm, angular to subangular, some flat gravels, brown, no cementation, clasts are metasedimentary, low plastic fines. Frozen: Vx (10-20%). [COLLUVIUM] 1.3 - 1.4 m - Sample 1, grain size analysis, 48% gravel, 33% sand, 19% fines. Below 1.5 m - Colour changes to light brown.				○★
2.5					End of test pit at 2.5 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.6 m to bottom of excavation. 5) Frozen material also noted on lower half of access track cut by excavator from existing road down to test pit.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-113

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459071E, 7099490N
 GROUND ELEVATION (m): 878
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 17 Aug 11
 FINISH DATE: 17 Aug 11
 FINAL DEPTH OF PIT (m): 2.5
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _p %	W _p %	W _p %	W _p %
						×	○	○	○	×
0					SILT/FINE SAND [WASHOUT FROM DRILLPAD ABOVE]					
1					SILT (ML) Some gravel, trace sand, trace cobbles, low plastic, soft to firm, light brown, grey mottling, no odour, moist, homogeneous, no cementation, maximum particle size = 200 mm, gravel and cobbles are primarily metasedimentary, trace granodiorite, angular to subangular. [COLLUVIUM] At 1.5 m - Seepage entering pit from downhill side.					
2										
3					End of test pit at 2.5 m. Notes: 1) Terminated due to slouging of walls and seepage. 2) Backfilled to surface. 3) Seepage at 1.5 m. 4) Not frozen. 5) Pit located approximately 30 cm below natural ground (organics removed) and approximately 3 m downslope of existing drill pad.					
4										
5										
6										
7										
8										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-114

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459190E, 7099369N
 GROUND ELEVATION (m): 914
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 17 Aug 11
 FINISH DATE: 17 Aug 11
 FINAL DEPTH OF PIT (m): 3.2
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _p %	W _p %	W _p %	W _p %
						×	○	○	○	×
						20	40	60	80	
0					ORGANICS					
0.7					SAND (SW) Sand, some silt, some gravel, trace clay, trace cobbles, loose to compact, maximum particle size = 200 mm, subangular, dark brown, no odour, moist, no cementation, gravel & cobbles are metasedimentary and granodiorite. [COLLUVIUM] Below 0.7 m - Colour change to light brown.					
2.3					GRAVEL (GW) Sandy (coarse), some clay, trace silt, trace cobbles, compact to dense, maximum particle size = 100 mm, moist, equidimensional, angular to subangular, light brown, no odour, moist, no cementation, granodiorite clasts. [COLLUVIUM?] Below 2.3 m - Frozen: Vx/Vc (<5%). Cobble content increases to some.					
3.2					End of test pit at 3.2 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 2.3 m to bottom of excavation. 4) No samples taken.					

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-115

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459187E, 7099453N
 GROUND ELEVATION (m): 899
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 17 Aug 11
 FINISH DATE: 17 Aug 11
 FINAL DEPTH OF PIT (m): 5.7
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _p %	W _p %	W _p %	W _p %
						×	○	○	○	×
0		1			<p>SILT (ML) Some coarse sand, some gravel, trace cobbles, low plastic, firm, light brown, no odour, moist, no cementation. Clasts: mostly metasedimentary, trace granodiorite, angular to subangular, some flat, maximum particle size = 150 mm. [COLLUVIUM]</p> <p>Below 2.5 m - Cobble content increases to some.</p> <p>Below 3.7 m - Maximum particle size = 300 mm, some gravel to gravelly, trace boulders.</p>					
6					<p>End of test pit at 5.7 m. Notes: 1) Completed at the maximum reach of excavator. 2) Backfilled to surface. 3) No seepage 4) Not frozen. 5) Test pit located on abandoned access road and located approximately 20 cm below natural ground surface.</p>					

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-116

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459251E, 7099423N
 GROUND ELEVATION (m): 905
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 18 Aug 11
 FINISH DATE: 18 Aug 11
 FINAL DEPTH OF PIT (m): 2.3
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _p %	W _p %	W _p %	
						×	○	×	×	
0					ORGANICS At 0.2 m - Light seepage.					
0.2	1				SILT (ML) Gravelly, some sand, trace cobbles, low plasticity, soft to firm, light brown, no odour, moist, no cementation, maximum particle size = 200 mm. Clasts: metasedimentary, angular to subangular, some flat. [COLLUVIUM]					
1.5					Below 1.5 m - Frozen: Vx/Vs/Vc (approximately 5-10%), coatings and lenses 1-3 mm thick and sand-sized crystals.					
2.2	2				2.2 - 2.3 m - Sample 2, grain size analysis, 49% gravel, 36% sand, 15% fines.	○	★	×		
2.3					End of test pit at 2.3 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) Light seepage at 0.2 m. 4) Frozen ground encountered below 1.5 m to bottom of excavation. 3) Unfrozen walls sloughing throughout excavation.					

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-117

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459184E, 7099578N
 GROUND ELEVATION (m): 896
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 18 Aug 11
 FINISH DATE: 18 Aug 11
 FINAL DEPTH OF PIT (m): 8.0
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	W ₄ %
						×	○	○	○	×
0		1			ORGANICS Rootmat. [TOPSOIL]					
0.5					SAND (SP) Coarse sand, some gravel, trace cobbles, trace silt, poorly (gap) graded, loose to compact, maximum particle size = 150 mm, angular to subangular, some flat, colour varies throughout unit: brown, reddish, orange (colour variation occurs in horizontal lines), no odour, moist, metasedimentary clasts. [COLLUVIUM]					
7					At 7.0 m - Cobble content increases to some.					

(Continued on next page)

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-117

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459184E, 7099578N
 GROUND ELEVATION (m): 896
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 18 Aug 11
 FINISH DATE: 18 Aug 11
 FINAL DEPTH OF PIT (m): 8.0
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _p %	W _p %	W _p %	W _p %
						×	○	○	○	×
						20	40	60	80	
8					End of test pit at 8.0 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Pit excavated at base of existing 2 m high cut slope, adjacent to historic hole: MW96-25.					
9										
10										
11										
12										
13										
14										
15										
16										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-118

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459335E, 7100268N
 GROUND ELEVATION (m): 951
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 18 Aug 11
 FINISH DATE: 18 Aug 11
 FINAL DEPTH OF PIT (m): 5.3
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
						20	40	60	80
0					ORGANICS Rootmat. [TOPSOIL]				
0.5					SAND (SP) Fine sand, trace coarse sand, gravelly, trace silt, trace cobbles, poorly (gap) graded, loose to compact, maximum particle size = 100 mm, some gravel/cobbles are flat, subangular, no odour, moist, no cementation. [COLLUVIUM]				
1.5		5			SAND (SP) Trace gravel, trace silt, poorly graded, compact, maximum particle size = 50 mm, some equidimensional, some flat, angular, light reddish brown, no odour, moist to wet, homogeneous, no cementation, metasedimentary clasts. [COMPLETELY WEATHERED METASEDIMENTARY ROCK]				
2.5					SAND AND GRAVEL (SW/SW) Trace cobbles, dense, maximum particle size = 100 mm, flat, some elongated, angular, grey/brown/light orange, no odour, moist, foliated/bedded structure. Clasts: metasediments and very weak (R1). Cobble content increases to some with depth. [HIGHLY WEATHERED METASEDIMENTS]				
5.3					End of test pit at 5.3 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-119

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458587E, 7100548N
 GROUND ELEVATION (m): 824
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 18 Aug 11
 FINISH DATE: 18 Aug 11
 FINAL DEPTH OF PIT (m): 2.1
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Moss, organics. [TOPSOIL]				
1	G	1			SILT (OL) Low plastic, dark brown to black, moist, trace rootlets. [ORGANICS]		×		
2					SILT (ML) Some fine sand, trace gravel, flat, angular, non-plastic, soft, rapid dilatancy, orange, wet when thawed, no cementation. [ALLUVIUM?] Below 0.45 m - Frozen: Vs (<2%), ice lenses 0.5-3 mm thick.				
3					SILT (ML) Trace fine sand, low plasticity, orangey brown. Frozen: Vs (<2%), ice lenses 0.5 - 2 mm thick. [ALLUVIUM?] At 0.9 m - Ice lens: 30 mm thick. 0.9 - 1.1 m - Sample 1, modified proctor test, optimized dry density 1780 kg/m ³ at 13.5% moisture content. Permeability analysis, 4.2x10 ⁻¹⁰ m/s. Below 1.1 m - Trace clay. At 2.0 m - Ice lens: 25 mm thick.				
4					End of test pit at 2.05 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.45 m to bottom of excavation.				
5									
6									
7									
8									

EGR (TESTPIT)_EGR_TESTPIT.GDL_BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-120

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458511E, 7100484N
 GROUND ELEVATION (m): 816
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 18 Aug 11
 FINISH DATE: 18 Aug 11
 FINAL DEPTH OF PIT (m): 4.4
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0					ORGANICS Rootmat, moss, organics. [TOPSOIL]				
0.35 - 1.0					SILT (ML) Some fine sand, some gravel, trace clay, trace cobbles, low plastic, soft, brown, moist to wet, stratified, no cementation. Clasts: metasedimentary and granodiorite, slightly weathered (W1.5 to W2), medium strong (R3), subrounded to subangular, maximum particle size = 200 mm. [ALLUVIUM?] 0.35 - 1.0 m - Frozen: Nbn. At 0.9 m - Sand and gravel layer, 30 mm thick. 1.0 - 3.0 m - Frozen: Vs (5%), ice lenses 2-25 mm thick.				
3.8 - 4.0	G	1			SILT (ML) Some clay, trace gravel, trace cobbles, subrounded, low plastic, grey, firm. Frozen: Vs (10-20%), ice lenses 1-40 mm thick. [ALLUVIUM?] 3.8 - 4.0 m - Sample 1, modified proctor test, optimized dry density 1840 kg/m ³ at 13% moisture content. Permeability analysis, 6.5x10 ⁻¹⁰ m/s.				××
4.0 - 4.4					ICE 10-20% sediment inclusions. End of test pit at 4.4 m. Notes: 1) Refusal on massive ice. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.35 m to bottom of excavation.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-121

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458502E, 7100588N
 GROUND ELEVATION (m): 815
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 3.3
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						VANE	FIELD	LAB	UC/2	
						40	80	120	160	
						PEAK	◆	■	▲	Pocket Pen /2
						REMOLD	◇	□	△	
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	
						×	○	○	×	
0					<p>ORGANICS Rootmat, moss, silt. [TOPSOIL]</p>					
0.7					<p>SILT (ML) Trace fine sand, trace gravel, trace cobbles, trace boulders, low plastic, soft to firm, orangey brown, moist. Clasts: metasedimentary and granodiorite, medium strong to strong (R3 to R4), slightly weathered (W2), maximum particle size > 300 mm, elongated and flat, subangular. [ALLUVIUM?] Vs0.7 - 0.8 m - Frozen: 10 cm thick ice lens, granular ice. 0.8 - 1.2 m - Frozen: Vs (50%) in lenses 2-20 mm thick. 1.2 - 2.2 m - Vs (20%).</p>					
2.2					<p>At 2.2 m - A 10 cm thick, darker brown, organic layer.</p>					
2.9	G	1			<p>SILT (ML) Trace medium to coarse gravel, trace cobbles, trace boulders, low plastic, grey-brown, homogeneous. Clasts: metasedimentary and granodiorite, rounded to subangular, slightly weathered (W2), medium strong (R3). Frozen: Vs (1-5%). [ALLUVIUM?] 2.9 - 3.1 m - Sample 1, modified proctor test, optimized dry density 1940 kg/m³ at 13.5% moisture content. Permeability analysis, 4x10⁻¹⁰ m/s.</p>					×
3.3					<p>End of test pit at 3.3m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.7 m to bottom of excavation.</p>					

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-122

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458684E, 7100707N
 GROUND ELEVATION (m): 825
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 5.8
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Moss. [TOPSOIL]				
1					SILT (ML) Trace sand, trace to some gravel, some cobbles, trace boulders, non-plastic, soft to firm, orangey brown, moist, rapid dilatancy. Clasts: sub rounded to subangular, flat to equidimensional, slightly weathered (W1.5 to W2), metasedimentary, medium strong to strong (R3 to R 4.5). [PLACER TAILINGS]				
2					0.7 - 1.55 m - Zones of trace to some clay, medium plastic, grey-brown, no sand, some seams contain orangey brown, less plastic silt. 1.55 - 5.8 m - Made up of 50% grey-brown silt and approximately 50% orangey-brown less plastic silt. Not segregated into obvious layers. [PLACER TAILINGS]				
3									
4									
5									
6	G	1			At 5.4 m - An 800 mm boulder. 5.4 - 5.8 m - Sample 1, modified proctor test, optimized dry density 2110 kg/m ³ at 9% moisture content. Permeability analysis, 3.15x10 ⁻¹⁰ m/s.		⊗	⊗	
7					End of test pit at 5.8 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Some sloughing.				
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-123

LOCATION: WEST FLANK TIN DOME

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458498E, 7101322N
 GROUND ELEVATION (m): 808
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 2.4
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, moss, organics. [TOPSOIL]				
0.4 - 0.5					SILT (ML) Some medium to coarse sand, gravelly, trace cobbles, non to low plastic, rapid dilatancy, firm, wet, orangey brown. Clasts: metasedimentary, angular, equidimensional, strong (R4), to slightly weathered (W1.5 to W2). [COLLUVIUM]				
0.5 - 1.3					0.4 - 0.5 m - Frozen: massive ice, granular, 10% sediment inclusions. 0.5 - 1.3 m - Frozen: Vs, ice in lenses 1-15 mm thick, grades from 60% at 0.5 m to 15% at 1.3 m.				
1.3 - 2.4	G	1			SILT (ML) Gravelly, sandy, some cobbles. Frozen: Vs (10-40%), ice lenses, 2-20 mm thick with trace organics. [COLLUVIUM]				
2.4					End of test pit at 2.4 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.4 m to bottom of excavation.				
3									
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458559E, 7101502N
 GROUND ELEVATION (m): 825
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 6.5
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	▲
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0					ORGANICS Rootmat, moss. [TOPSOIL]				
0.5					SILT (ML) Gravelly, some sand, trace cobbles, trace boulders, low plastic, firm, orange brown, moist to wet, no cementation, rapid dilatancy, layered. Contains several organic layers and layers with iron staining 1-5 cm thick. Clasts: metasedimentary, subangular to angular, platy, faintly to slightly weathered (W1.5 to W2), medium strong to strong (R3 to R4), maximum particle size = 300 mm. [COLLUVIUM]				
1.5					SILT (ML) Some gravel, some sand, trace cobbles non-plastic, soft, brown to dark brown, dry to moist, no cementation. Clasts: subangular to angular, platy to equidimensional, metasedimentary, slightly weathered (W1.5 to W2), medium strong (R3), trace organics. [COLLUVIUM]				
5.5					At 5.5 m - Seepage.				
6.2 - 6.4	G	1			6.2 - 6.4 m - Sample 1, grain size analysis, 21% gravel, 29% sand, 50% fines.		○ ××	★	
6.5					End of test pit at 6.5 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) Seepage at approximately 5.5 m. 4) Not frozen. 5) Significant sloughing.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-125

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459492E, 7100047N
 GROUND ELEVATION (m): 995
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 2.5
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, organic silt. [TOPSOIL]				
1	G	1			ORGANIC SILT (OL) Trace sand, trace fine gravel, trace cobbles, low plastic, firm, light to dark brown, organic odour, moist to wet, no cementation, rapid dilatancy.				
2					SILT (ML) Some sand, some gravel, trace cobbles, low plastic, dark brownish grey, slight organic odour, wet when thawed, rapid dilatancy. Frozen: Vx/Vs (15-25%). [COLLUVIUM] At 1.5m - Sandy, trace boulders, yellowish brown. Frozen: Vx(5-10%). Clasts: metasedimentary, medium strong to strong (R3 to R4), faintly to slight weathered (W1.5 to W2), flat to elongated, angular.				
3					End of test pit at 2.5 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.55 m to bottom of excavation. 5) Used ripper shank to penetrate frozen ground.				
4									
5									
6									
7									
8									

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

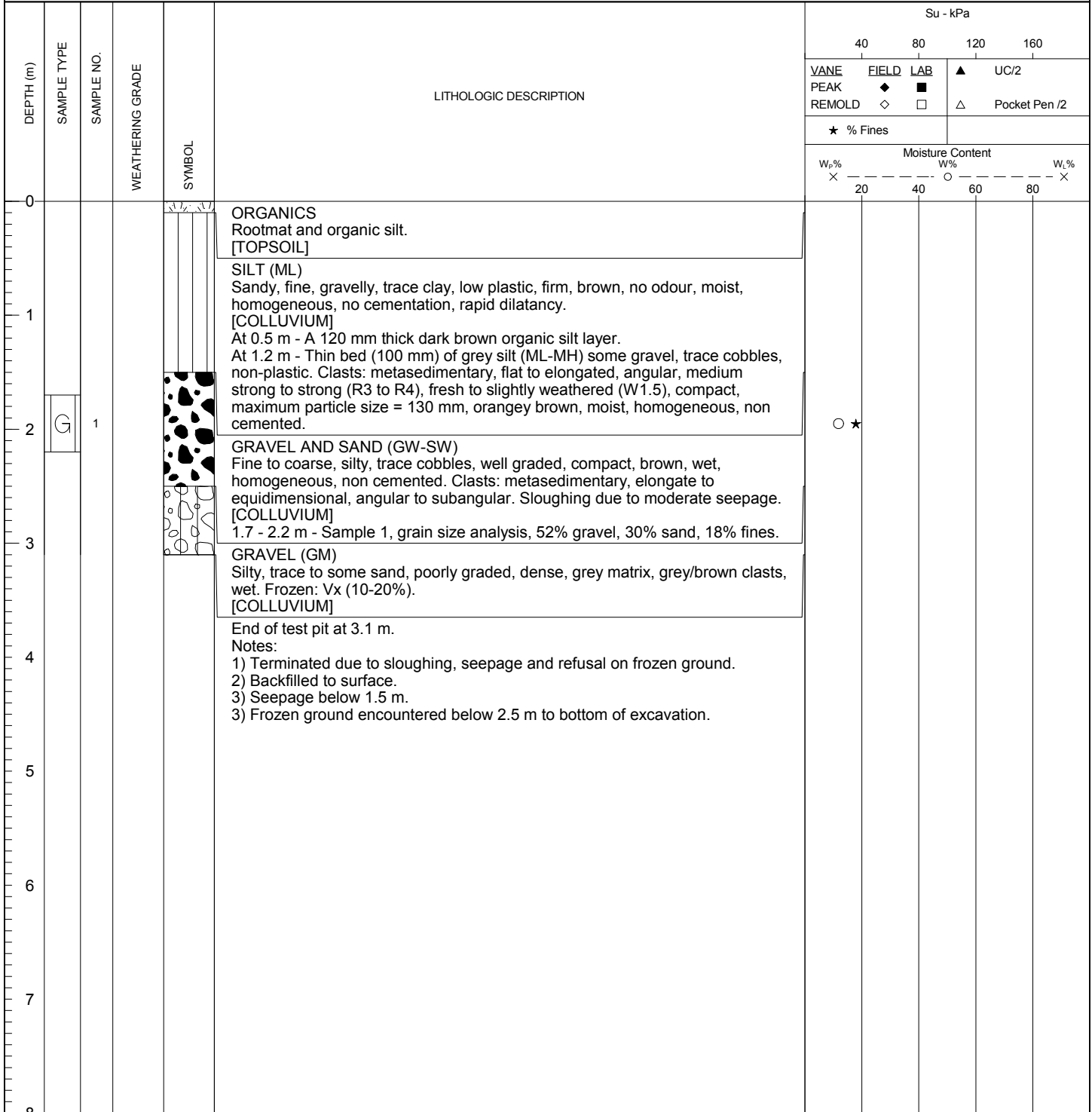
TEST PIT # TP-BGC11-126

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459464E, 7100300N
 GROUND ELEVATION (m): 946
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 3.1
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW



EGR (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-127

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459889E, 7100117N
 GROUND ELEVATION (m): 1096
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 2.5
 LOGGED BY: LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Rootmat. [TOPSOIL]				
1					SILT (ML) Gravelly, some cobbles, some sand, non plastic, firm, orange brown, no odour, moist, non cemented, no dry strength, rapid dilatancy. Clasts: metasedimentary, equidimensional to flat, angular, slightly weathered (W2), weak to medium strong (R2 to R3). [COLLUVIUM]				
2			2		METASEDIMENTARY ROCK Orange brown, fine grained, foliated, very closely fractured, slightly weathered (W2), weak to medium strong (R2.5 to R3), 3 joint sets, iron stained, crushed rock infill, 0.1-0.5 mm aperture, 20-60 mm spacing, joint roughness (JRC) = 2-6. Foliation: 033°/326° (dip varies from 15°-35°) Set: 034°/175° Set: 062°/282° [SLIGHTLY WEATHERED BEDROCK]				
3					End of test pit at 2.5 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) No samples taken. 6) Structural measurements: Dip/Dip direction.				
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-128

LOCATION: HAGGART CREEK

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458423E, 7101526N
 GROUND ELEVATION (m): 803
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 19 Aug 11
 FINISH DATE: 19 Aug 11
 FINAL DEPTH OF PIT (m): 3.0
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	▲ UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△ Pocket Pen /2
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
0					ORGANICS Rootmat, moss. [TOPSOIL]				
0.6					SILT (ML) Gravelly, some sand, trace cobbles, non-plastic, rapid dilatancy, soft, wet, brown. Clasts: metasedimentary, subangular to angular, flat, medium strong to strong (R3 to R4), slightly weathered (W1.5 to W2). [COLLUVIUM] Below 0.6 m - Frozen: Vs/Vx (10-20%), ice lenses 2-20 mm thick.				
1.8					At 1.8 m - Recovered one piece of ice and soil that is clear, 20 cm thick, has trace organics, and consists of approximately 60% ice. Below 1.8 m - Vs (1-10%).				
2.5 - 3.0		1			2.5 - 3.0 m - Sample 1, grain size analysis, 7% gravel, 32% sand, 61% silt.		○	★	
3.0	G				End of test pit at 3.0 m Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 0.6 m to bottom of excavation. 5) Sloughing of non-frozen upper layer.				
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458563E, 7101340N
 GROUND ELEVATION (m): 822
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 20 Aug 11
 FINISH DATE: 20 Aug 11
 FINAL DEPTH OF PIT (m): 6.8
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, moss, organics. [TOPSOIL]				
1					SILT (SM) Some medium to coarse sand, gravelly, trace cobbles, non plastic, firm, wet, orangey brown, maximum particle size = 140 mm, trace organics. Clasts: metasedimentary, subangular to angular, flat to equidimensional, slightly weathered (W2), medium strong to strong (R3 to R4). [COLLUVIUM] At 1.2 m - An orange layer, 10 cm thick, with greater iron staining. At 1.3 m - A brown layer, 5 cm thick, of mostly decomposed organics.				
2					At 2.0 m - A brown layer, 5 cm thick, of mostly decomposed organics.				
3	G	2							
4					SILT (ML) Trace medium to coarse sand, some gravel, trace cobbles, trace organics, low plastic, firm to stiff, moist, greyish brown. Clasts: metasedimentary, medium strong to strong (R3 to R4), subangular to angular, flat to equidimensional, fresh to slightly weathered (W1.5), metasedimentary, maximum particle size = 140 mm. [COLLUVIUM]				
5									
6									
7	G	1			6.6 - 6.8 m - Sample 1, grain size analysis, 16% gravel, 31% sand, 53% fines. 6.7 - 6.8 m - Frozen: Vs (1-5%), ice lenses 2-5 mm thick, clear.	⊗		★	
7					End of test pit at 6.8 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Frozen ground encountered below 6.7 m to bottom of excavation.				
8									

(Continued on next page)

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-129

LOCATION: WEST FLANK TIN DOME

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458563E, 7101340N
 GROUND ELEVATION (m): 822
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 20 Aug 11
 FINISH DATE: 20 Aug 11
 FINAL DEPTH OF PIT (m): 6.8
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	UC/2
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W ₁ %	W ₂ %	W ₃ %	W ₄ %
						×	○	○	○	×
8					5) Some sloughing at 1.2 m depth, extensive sloughing at 6.8 m depth.					
9										
10										
11										
12										
13										
14										
15										
16										

EGR (TESTPIT) EGP TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458709E, 7101183N
 GROUND ELEVATION (m): 865
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 20 Aug 11
 FINISH DATE: 20 Aug 11
 FINAL DEPTH OF PIT (m): 6.0
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	×	W _o %	○	W _L %
						20	40	60	80	
0					ORGANICS Rootmat, organics, moss. [TOPSOIL]					
1					SILT (ML) Sandy, some gravel, trace cobbles, non plastic, brown, dry, firm, no to weak cementation. Clasts: subangular to angular, flat to equidimensional, metasedimentary, medium strong to very strong (R3 to R5), slightly weathered (W2), maximum particle size = 200 mm. [COLLUVIUM] Below 1.2 m - Trace boulders, maximum particle size = 600 mm.					
2										
3										
4					4.0 - 4.5 m - Increased cobble content.					
5										
6	G	1			5.8 - 6.0 m - Sample 1, grain size analysis, 35% gravel, 43% sand, 22% fines. End of test pit at 6.0 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.	○	★			
7										
8										

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

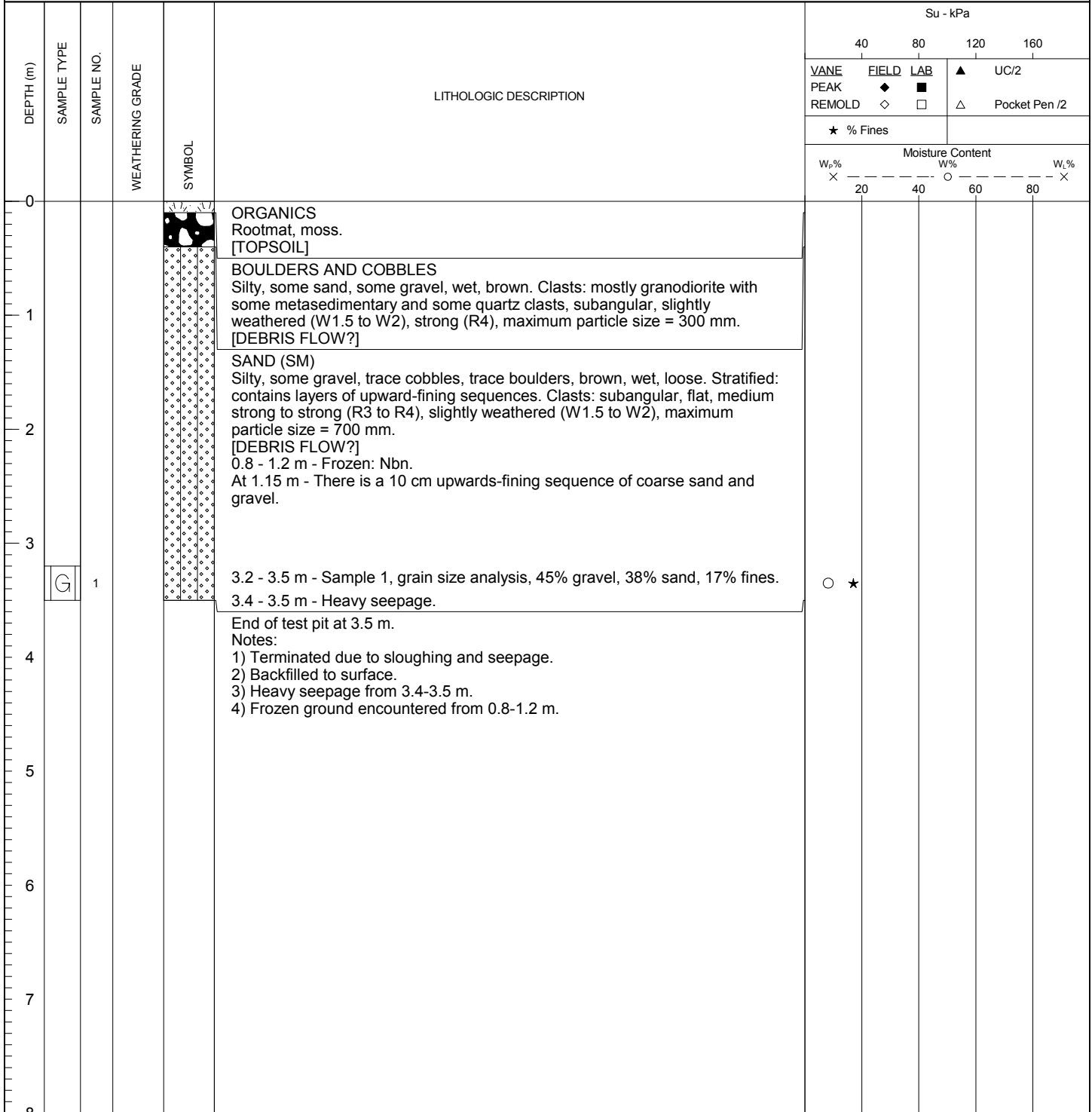
TEST PIT # TP-BGC11-131

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459934E, 7101430N
 GROUND ELEVATION (m): 921
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 20 Aug 11
 FINISH DATE: 20 Aug 11
 FINAL DEPTH OF PIT (m): 3.5
 LOGGED BY: JND
 REVIEWED BY: PQ/DW



EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-132

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459840E, 7101515N
 GROUND ELEVATION (m): 922
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 20 Aug 11
 FINISH DATE: 20 Aug 11
 FINAL DEPTH OF PIT (m): 5.3
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, moss. [TOPSOIL]				
0.5					SILT (ML) Some gravel, some cobbles, trace coarse sand, trace boulders, non plastic, rapid dilatancy, firm, orangish brown, dry. Clasts: metasedimentary, weak to strong (R2 to R4), slightly weathered (W2), angular, maximum particle size = 300 mm. [COLLUVIUM]				
1.5		4	4		SILT (ML) Bouldery, some sand, some gravel, some cobbles, orangish brown, dry. Clasts: very weak to medium strong (R1 to R3), highly weathered (W4), metasedimentary, maximum particle size = 800 mm. [COLLUVIUM]				
2.5			2.5		METASEDIMENTARY ROCK Very weak to medium strong (R1 to R3), highly weathered (W4), iron stained, metasedimentary, approximately 60% disintegrated to soil, orangish brown. [HIGHLY WEATHERED BEDROCK]				
3.5			2.5		METASEDIMENTARY ROCK Orangish brown, metasedimentary, weak (R2), slightly to moderately weathered (W2 to W3), heavily jointed (spacing of 2-5 cm). [MODERATELY TO SLIGHTLY WEATHERED BEDROCK]				
4.5			2		METASEDIMENTARY ROCK Orange-green brown, metasedimentary, weak to medium strong (R2 to R3), slightly weathered (W2), chlorite altered rock, blocky (approximately 50 cm). [SLIGHTLY WEATHERED BEDROCK]				
5.3					End of test pit at 5.3 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				

EGR (TESTPIT) EGR TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-133

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460048E, 7101608N
 GROUND ELEVATION (m): 984
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 21 Aug 11
 FINISH DATE: 21 Aug 11
 FINAL DEPTH OF PIT (m): 4.4
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, moss. [TOPSOIL]				
0.5					GRAVEL (GM) Silty, some sand, trace cobbles, loose, orangish brown, wet. Clasts: angular, flat, metasedimentary, maximum particle size = 220 mm, weak (R2), slightly weathered (W2). [COLLUVIUM]				
1.5					SILT (ML) Some sand, some gravel, trace cobbles, low plastic, grey. Clasts: angular, flat, metasedimentary, slightly weathered (W2), weak (R2), maximum particle size = 240 mm. Frozen: Vs (10-20%), ice lenses 1-40 mm thick, clear. [COLLUVIUM]				
2.5					GRAVEL (GM) Sandy, silty, trace cobbles, orangish brown. Clasts: metasedimentary, phyllitic, angular, flat, reddish brown, very weak to medium strong (R1 to R3), slightly to moderately weathered (W2 to W3). Frozen: Vx/Vs (10-20%), ice lenses 1-5 mm thick. [MODERATELY TO HIGHLY WEATHERED METASEDIMENTARY BEDROCK]				
4.3 - 4.4	1				4.3 - 4.4 m - Sample 1, grain size analysis, 51% gravel, 34% sand, 15% fines.				
4.4					End of test pit at 4.4 m. Notes: 1) Refusal on frozen ground. 2) Backfilled to surface. 3) Some seepage and sloughing from top unfrozen layer 0.2-0.6 m. 4) Frozen ground encountered below 0.6 m to bottom of excavation.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460101E, 7101419N
 GROUND ELEVATION (m): 945
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 21 Aug 11
 FINISH DATE: 21 Aug 11
 FINAL DEPTH OF PIT (m): 4.2
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	×	×
0					SILT (ML) Sandy, some gravel, trace cobbles, trace boulders, non plastic, rapid dilatancy, brown, moist. Clasts: metasedimentary and granodiorite, slightly weathered (W2), medium strong to strong (R3 to R4), subrounded to angular, stratified: contains upward fining sequences, maximum particle size = 400 mm. [DEBRIS FLOW?]				
1					COBBLES Gravelly, some sand, some silt. Clasts: metasedimentary and granodiorite, flat, subrounded to angular, faintly to slightly weathered (W2), medium strong to strong (R3 to R4). Matrix non-plastic, brown, moist. [DEBRIS FLOW?]				
2					BOULDERS Sandy, some gravel, some cobbles, trace silt, moist, brown. Clasts: granodiorite and metasedimentary, subrounded to subangular, medium strong to very strong (R3 to R5), slightly weathered (W2), maximum particle size = 1000 mm. [DEBRIS FLOW?] At 1.3 m - A 1700 by 1000 mm boulder.				
3									
4	G	1			4.1 - 4.2 m - Moderate seepage. End of test pit at 4.2 m. Notes: 1) Terminated due to sloughing and seepage at base of pit. 2) Backfilled to surface. 3) Moderate seepage from 4.1-4.2 m. 4) Not frozen.				
5									
6									
7									
8									

EGR (TESTPIT)_EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-135

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459998E, 7101383N
 GROUND ELEVATION (m): 931
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 21 Aug 11
 FINISH DATE: 21 Aug 11
 FINAL DEPTH OF PIT (m): 3.5
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, moss. [TOPSOIL]				
1	G	1			SILT (ML) Trace sand, some gravel, trace organics, trace cobbles, low plastic, rapid dilatancy, greyish brown. Clasts: metasedimentary, reddish brown, medium strong (R3), slightly weathered (W2), subangular, flat, maximum particle size = 120 mm. [COLLUVIUM] At 0.5 m - Some seepage. Below 1.2 m - Some sand, trace boulders, non-plastic, orangish brown. Clasts: granodiorite and metasedimentary, medium strong to strong (R3 to R4), maximum particle size = 500 mm.				
3.5					End of test pit at 3.5 m. Notes: 1) Refusal on boulders. 2) Backfilled to surface. 3) Some seepage at 0.5 m. 4) Not frozen. 5) Some sloughing.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-136

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459896E, 7101306N
 GROUND ELEVATION (m): 910
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 21 Aug 11
 FINISH DATE: 21 Aug 11
 FINAL DEPTH OF PIT (m): 4.8
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0					ORGANICS Rootmat, moss. [TOPSOIL]				
1	G	1			SILT (ML) Trace to some sand, trace gravel to gravelly, trace cobbles, low plastic, moist, firm. Comprises alternating layers of 70% orange-brown material (gravelly, trace organics), and 30% grey-brown material 3-10 cm thick (few clasts, no organics). Clasts: metasedimentary, medium strong (R3), slightly weathered (W1.5 to W2), subangular to angular, platy to equidimensional. [PLACER TAILINGS]				
2.1 - 2.5 m					Becomes trace sand, trace gravel, grey-brown.				
3					SAND (SM) Silty, some gravel, trace cobbles, trace boulders, orangish brown, moist. Clasts: metasedimentary and granodiorite, medium strong to strong (R3 to R4), slightly weathered (W2), rounded to angular, maximum particle size = 500 mm. [DEBRIS FLOW?]				
4.8					End of test pit at 4.8 m. Notes: 1) Refusal on boulders. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Sloughing of pit walls due to rainy weather conditions. 6) Test pit excavated at drill pad for BH-BGC11-52				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-137

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459920E, 7101171N
 GROUND ELEVATION (m): 943
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 21 Aug 11
 FINISH DATE: 21 Aug 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: JND
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat, moss. [TOPSOIL]				
0.5	G	1			SILT (ML) Gravelly, some sand, trace cobbles, trace boulders, low plastic, rapid dilatancy, brown. Clasts: metasedimentary, weak to medium strong (R2 to R3), reddish brown and reddish grey, subangular to angular, flat to equidimensional, maximum particle size = 300 mm. [COLLUVIUM]				
1.5			4.5		METASEDIMENTARY ROCK Brownish grey, extremely weak to weak (R0 to R2), highly to completely weathered (W4 to W5), phyllitic, crumbles into micaceous flakes under finger pressure. [HIGHLY TO COMPLETELY WEATHERED BEDROCK]				
2.5			2.5		METASEDIMENTARY ROCK Reddish brown, 4 visible joint sets, weak to medium strong (R2 to R3), slightly to moderately weathered (W2 to W3). [SLIGHTLY TO MODERATELY WEATHERED BEDROCK]				
3.5			3.5		METASEDIMENTARY ROCK Brownish grey, silver along foliation, highly fractured, phyllitic, very weak to weak (R1 to R2), moderately to highly weathered (W3 to W4). [MODERATELY TO HIGHLY WEATHERED BEDROCK]				
5.0					End of test pit at 5.0 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				

EGP (TESTPIT) EGP_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-138

LOCATION: STUTTLE GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459771E, 7100041N
 GROUND ELEVATION (m): 1050
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 24 Aug 11
 FINISH DATE: 24 Aug 11
 FINAL DEPTH OF PIT (m): 6.3
 LOGGED BY: DW/LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa				
						40	80	120	160	
						VANE	FIELD	LAB	▲	UC/2
						PEAK	◆	■		
						REMOLD	◇	□	△	Pocket Pen /2
						★ % Fines				
						Moisture Content				
						W _p %	W _p %	W _p %	W _p %	W _p %
						×	○	○	○	×
0					ORGANICS Rootmat. [TOPSOIL]					
1	G	1			GRAVEL AND SAND (GM) Silty, well-graded, compact, flat and elongated, metasedimentary clasts, brown, moist, homogeneous, maximum particle size = 120 mm. [COLLUVIUM]					
2			4		METASEDIMENTARY ROCK Brown, orangish, fine grained, foliated, weak to medium strong (R2 to R3), 3 main joint sets, hematite staining, crushed rock and silty sand infill, 0.5-5 mm aperture, JRC = 2-5. [HIGHLY WEATHERED BEROCK]					
3					METASEDIMENTARY ROCK Brown, orangish/reddish, fine grained, foliated, medium strong (R3), slightly weathered (W2), hematite infill, 0.5 - 5 mm aperture, JRC = 2 - 5. Joint set 1: 17/74 Joint set 2: 84/244 Joint set 3: 78/345 [SLIGHTLY WEATHERED BEDROCK]					
4			2							
5										
6										
7					End of test pit at 6.3 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.					
8										

EGR (TESTPIT) EGR TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-139

LOCATION: SOUTH OF PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459001E, 7099033N
 GROUND ELEVATION (m): 935
 DATUM: UTM NAD 83

EXCAVATOR: Hand dug
 OPERATOR: N/A

START DATE: 24 Aug 11
 FINISH DATE: 24 Aug 11
 FINAL DEPTH OF PIT (m): 3.0
 LOGGED BY: DW/LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	×	W _p %	○
						20	40	60	80
0					ORGANICS Rootmat. [TOPSOIL]				
0.5	G	1	5		GRAVEL AND SAND (GW/SW) Trace to some silt, trace cobbles, well graded, maximum particle size = 85 mm, loose to compact, flat and elongated, angular, yellowish/orangish brown, moist, homogeneous. [COLLUVIUM]				
1.5			3		SAND (SW) Some fine to medium gravel, trace silt, well graded, loose to compact, maximum particle size = 30 mm, flat and elongated, angular, moist, orange to yellow, non cemented, relict bedding structures of the bedrock are visible as alternating orange/brown layers. [COMPLETELY WEATHERED BEDROCK]				
2.5					METASEDIMENTARY ROCK Grey to orangish, fine grained, foliated, medium strong (R3), moderately weathered (W3), 3 joint sets, tabular blocks, hematite infill, aperture 0.5-3 mm, spacing of sets: Foliation: extremely to very closely spaced joints (225°/220°) Joint set 1: no exposure (70°/80°) Joint set 2: very closely to closely spaced joints (50°/344°) [MODERATELY WEATHERED BEDROCK]				
3.0					End of test pit at 3.0 m. Notes: 1) Logged by hand digging the slope of an existing exploration trench. 2) No seepage. 3) Not frozen.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459228E, 7097912N
 GROUND ELEVATION (m): 954
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 24 Aug 11
 FINISH DATE: 24 Aug 11
 FINAL DEPTH OF PIT (m): 4.5
 LOGGED BY: DW/LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						VANE	FIELD	LAB	UC/2
						Moisture Content			
						W _p %	W _l %	W _u %	W _u %
						×	○	○	×
0					ORGANICS Rootmat and organic silt. [TOPSOIL]				
1	G	1			SILT (ML) Gravelly (fine to medium), some sand, trace cobbles, trace clay, trace rootlets, low plasticity, firm, no odour, moist, no cementation, rapid dilatancy. Clasts: metasedimentary, elongated, angular, medium strong (R3), faintly to slightly weathered (W1.5). [COLLUVIUM] PP/2 = 50 kPa. 1.0 - 1.2 m - Sample 1, grain size analysis, 21% gravel, 34% sand, 45% fines. At 1.2 m - Thin layer of sandy silt, brown, vesicular, rapid dilatancy, wet.				
2			2		METASEDIMENTARY ROCK Phyllite, grey, bluish-grey, brownish-grey, fine grained, extremely closely spaced foliation planes, foliation is sub-horizontal, extremely weak (R1), slightly weathered (W2), mica rich, hematite staining on foliation plane, trace clay on foliation joint walls. [SLIGHTLY WEATHERED BEDROCK]				
3									
4					At 3.7 m - Very difficult to excavate through quartzite beds (1-3 cm thick).				
5					End of test pit at 4.5 m. Notes: 1) Refusal on bedrock. 2) Backfilled to surface. 3) No seepage. 4) Not frozen.				
6									
7									
8									

EGR (TESTPIT)_EGR_TESTPIT.GDL_BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-141

LOCATION: PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460019E, 7098871N
 GROUND ELEVATION (m): 1154
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 24 Aug 11
 FINISH DATE: 24 Aug 11
 FINAL DEPTH OF PIT (m): 5.0
 LOGGED BY: DW/LGT
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			Pocket Pen /2
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0					ORGANICS Rootmat, organic silt. [TOPSOIL]				
1					GRAVEL (GM) Silty, trace to some sand, trace rootlets, trace boulders, loose, maximum particle size = 1000 mm, dark brown, no odour, wet, no cementation. Clasts: metasedimentary, elongated, angular to subangular, medium strong (R3), slightly weathered (W2). [COLLUVIUM] At 1.0 m - Seepage.				
2					1.9 - 2.5 m - Lenses of sand, 10-15 cm thick, fine, silty, trace gravel, fine to coarse, grey, moist.				
3									
4					3.75 - 4.1 m - Lenses of sand, 10-15 cm thick, fine, silty, trace gravel, grey, moist.				
5	G	1			End of test pit at 5.0 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) Seepage at 1.0 m. 4) Not frozen. 5) Some sloughing.				
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-142

LOCATION: SOUTH OF PLATINUM GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459037E, 7098318N
 GROUND ELEVATION (m): 913
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 26 Aug 11
 FINISH DATE: 26 Aug 11
 FINAL DEPTH OF PIT (m): 4.5
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W ₃ %
						×	○	○	×
0					ORGANICS Rootmat, moss. [TOPSOIL]				
1					SAND (SM) Some silt, some gravel, trace cobbles, well graded, loose to compact, maximum particle size = 100 mm, some clasts are flat and elongated, angular to subangular, brown, no odour, moist, no cementation, gravel and cobbles are metasedimentary. [COLLUVIUM]				
3					GRAVEL (GW) Fine to coarse, some sand to sandy, trace cobbles, well graded, compact, maximum particle size = 200 mm, most clasts are flat, some elongated, angular, orange and greyish black, no odour, moist, bedded/foliated structure, talc on some clasts surfaces. Clasts: typically very weak (R1) and break along foliation/bedding. [HIGHLY WEATHERED METASEDIMENTARY ROCK]				
4				4					
5					End of test pit at 4.5 m. Notes: 1) Completed at maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) No samples taken.				
6									
7									
8									

EGR (TESTPIT)_EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-143

LOCATION: STEINER AREA

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 458644E, 7100441N
 GROUND ELEVATION (m): 838
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 26 Aug 11
 FINISH DATE: 26 Aug 11
 FINAL DEPTH OF PIT (m): 4.0
 LOGGED BY: KH/DW
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
0					ORGANICS Rootmat. [TOPSOIL]				
0.3					SILT (ML) Sandy (fine), poorly (uniformly) graded, low plastic, firm, greyish brown, moist, slight organic odour, no cementation. [ALLUVIUM?] At 0.3 m - Light seepage.				
0.6					SAND (SP) Medium to coarse sand, gravelly, trace cobbles, trace silt, poorly (gap) graded, loose to compact, maximum particle size = 100 mm, sand subrounded, gravel and cobbles rounded, grey, wet, primarily metasedimentary clasts, trace granodiorite clasts. [ALLUVIUM?] At 0.6 m - Light seepage.				
1.7					ORGANICS Brownish black.				
1.7					SILT (ML) Some fine to coarse sand, some gravel, trace cobbles, low plastic, soft to firm, brown, no odour, moist, no cementation. Clasts: metasedimentary, subrounded to angular, typically flat. [ALLUVIUM?] At 1.7 m - Moderate seepage.				△
4.0					SILT (ML) Some sand, trace clay, low plastic, firm to stiff, light grey, no odour, dry to moist, no cementation. Clasts: metasedimentary, subrounded, maximum particle size = gravel sized. PP/2 = 60-150 kPa. [ALLUVIUM?]				
4.0					End of test pit at 4.0 m. Notes: 1) Terminated due to sloughing. 2) Backfilled to surface. 3) Light seepage found at 0.3 and 0.6 m. Moderate seepage found at 1.7 m. 3) Not frozen. 4) Walls sloughing between 1.0 and 1.7 m.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-144

LOCATION: EAGLE PUP

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 460540E, 7100144N
 GROUND ELEVATION (m): 1158
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 27 Aug 11
 FINISH DATE: 27 Aug 11
 FINAL DEPTH OF PIT (m): 1.0
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W _p %	W _p %	W _p %
						×	○	○	×
						20	40	60	80
0					ORGANICS Rootmat. [TOPSOIL]				
0.4					SAND (SW) Sand, some gravel, some silt, trace cobbles, well graded, loose, maximum particle size = 100 mm, sand is subrounded, gravels and cobbles are subangular to angular, grey, no odour, wet, homogeneous, no cementation, granodiorite clasts. Heavy seepage at organics/colluvium contact (0.4 m). [COLLUVIUM]				
1.0					End of test pit at 1.0 m. Notes: 1) Terminated due to sloughing and seepage. 2) Backfilled to surface. 3) Heavy seepage at 0.4 m. 4) Not frozen.				
2									
3									
4									
5									
6									
7									
8									

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

TEST PIT # TP-BGC11-145

LOCATION: MIDDLE REACH DUBLIN GULCH

SURVEY METHOD: Handheld GPS
 CO-ORDINATES (m): 459853E, 7101471N
 GROUND ELEVATION (m): 959
 DATUM: UTM NAD 83

EXCAVATOR: 325D Cat
 OPERATOR: EWING

START DATE: 27 Aug 11
 FINISH DATE: 27 Aug 11
 FINAL DEPTH OF PIT (m): 5.1
 LOGGED BY: KH
 REVIEWED BY: PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	WEATHERING GRADE	SYMBOL	LITHOLOGIC DESCRIPTION	Su - kPa			
						40	80	120	160
						VANE	FIELD	LAB	UC/2
						PEAK	◆	■	
						REMOLD	◇	□	△
						★ % Fines			
						Moisture Content			
						W _p %	W ₁ %	W ₂ %	W _L %
						×	○	○	×
0					DRILL PAD FILL				
0					ORGANICS				
0					SAND (SM) Gravelly, some silt, trace cobbles, well graded, compact, maximum particle size = 100 mm, sand is equidimensional, subrounded to subangular, gravel and cobbles are typically flat, some elongated, subangular to angular, light brown, no odour, moist, no cementation, metasedimentary clasts. [COLLUVIUM]				
2.5					Below 2.5 m - Cobble content increases to some.				
4.5					Below 4.5 m - Trace boulders.				
5.1					End of test pit at 5.1 m. Notes: 1) Completed to maximum reach of excavator. 2) Backfilled to surface. 3) No seepage. 4) Not frozen. 5) Slight sloughing. 6) Test pit excavated at drill pad for BH-BGC11-31.				

EGR (TESTPIT) EGR_TESTPIT.GDL BGC.GDT 1/20/12

APPENDIX D TEST PIT PHOTOS



TP-BGC11-50 – Site photo; drill pad of BH-BGC11-36.



TP-BGC11-50 – Cut slope from construction of drill pad.



TP-BGC11-50 – Pit back wall; pit completed at a depth of 5.8 m.



TP-BGC11-50 – Detail of an excavated bucket.



TP-BGC11-51 – Site photo.



TP-BGC11-51 – Cut slope from access construction; frozen ground.



TP-BGC11-51 – Pit back wall; pit completed at a depth of 4.7 m.



TP-BGC11-51 – Detail of spoil pile.



TP-BGC11-52 – Site photo; drill pad of BH-BGC11-29.



TP-BGC11-52 – Cut slope and test pit; pit completed at depth of 5.0 m.



TP-BGC11-52 – Detail of spoil pile.



TP-BGC11-52 – Detail of a bucket excavated from 3.30 m.



TP-BGC11-53 – Site photo; drill pad of BH-BGC11-27.



TP-BGC11-53 – Cut slope and test pit; pit completed at depth of 4.0 m.



TP-BGC11-53 – Detail of spoil pile.



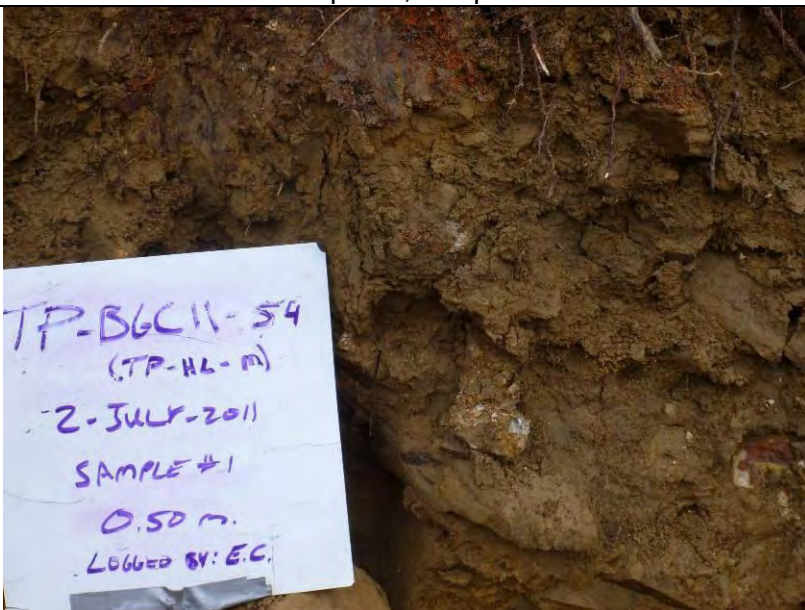
TP-BGC11-53 – Detail of a bucket excavated from 2.0 m.



TP-BGC11-54 – Site photo; drill pad of BH-BGC11-25.



TP-BGC11-54 – Back wall of test pit; pit completed at depth of 2.0 m.



TP-BGC11-54 – Detail of pit wall at 0.5 m.



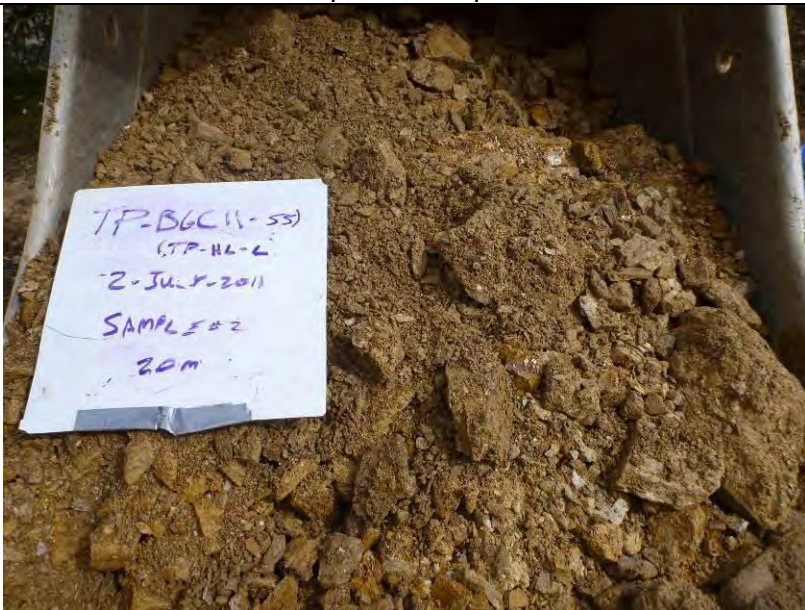
TP-BGC11-54 – Detail of pit wall at 2.0 m; bedrock at base.



TP-BGC11-55 – Site photo; drill pad of BH-BGC11-24.



TP-BGC11-55 – Back wall of test pit; pit completed at depth of 2.0 m.



TP-BGC11-55 – Detail of a bucket excavated from 2.0 m.



TP-BGC11-55 – Detail of pit wall at 0.5 m.



TP-BGC11-56 – Site photo.



TP-BGC11-56 – Back wall of test pit; pit completed at depth of 2.0 m.



TP-BGC11-56 – Detail of spoil pile.



TP-BGC11-56 – Detail of pit wall at 0.5 m.



TP-BGC11-57 – Site photo; drill pad of BH-BGC11-26.



TP-BGC11-57 – Back wall of test pit; pit completed at depth of 5.0 m.



TP-BGC11-57 – Detail of spoil pile; bedrock.



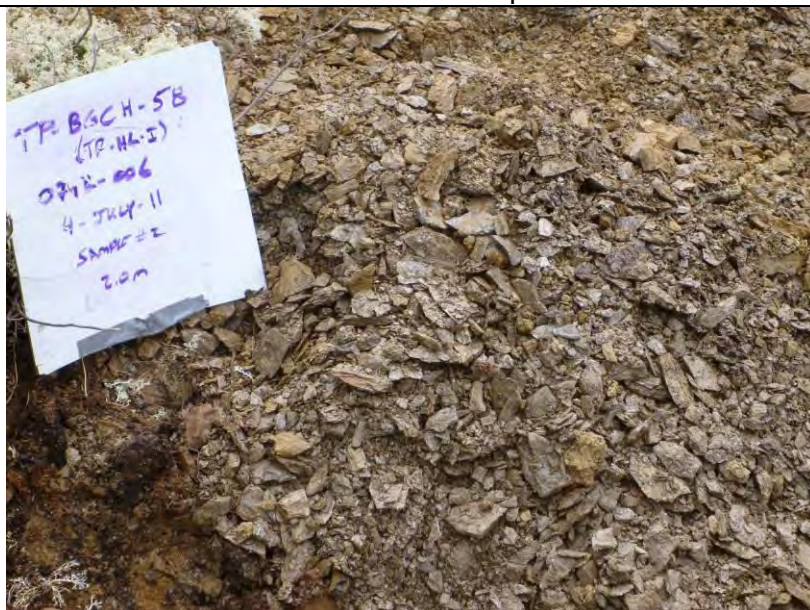
TP-BGC11-57 – Detail of pit wall at 1.0 m.



TP-BGC11-58 – Site photo.



TP-BGC11-58 – Back wall of test pit; pit completed at depth of 5.0 m.



TP-BGC11-58 – Detail of spoil pile; excavated from 2.0 m.



TP-BGC11-58 – Detail of pit wall.



TP-BGC11-59 – Site photo; drill pad of BH-BGC11-50.



TP-BGC11-59 – Cut slope and test pit; pit completed at depth of 2.6 m.



TP-BGC11-59 – Detail of pit wall at 0.5 m.



TP-BGC11-60 – Site photo.



TP-BGC11-60 – Cut slope and test pit; pit completed at depth of 2.7 m.



TP-BGC11-60 – Detail of spoil pile; frozen ground 0.7-0.9 m.



TP-BGC11-60 – Detail of pit wall.



TP-BGC11-61 – Site photo.



TP-BGC11-61 – Cut slope and test pit; pit completed at depth of 2.7 m.



TP-BGC11-61 – Detail of spoil pile.



TP-BGC11-61 – Detail of pit wall 0.7-0.8 m.



TP-BGC11-62 – Back wall of test pit.



TP-BGC11-62 – Bottom of test pit; pit completed at depth of 6.2 m.



TP-BGC11-62 – Detail of a bucket excavated from 3.5-3.7 m.



TP-BGC11-62 – Detail of pit wall from 0.9-1.0 m.



TP-BGC11-63 – Site photo.



TP-BGC11-63 – Back wall of test pit; pit completed at depth of 4.6 m.



TP-BGC11-63 – Detail of spoil pile.



TP-BGC11-63 – Detail of pit wall from 0.5-0.7 m.



TP-BGC11-64 – Site photo.



TP-BGC11-64 – Back wall of test pit; pit completed at depth of 4.7 m.



TP-BGC11-64 – Detail of spoil pile.



TP-BGC11-64 – Detail of pit wall from 1.5-1.6 m.



TP-BGC11-65 – Site photo.



TP-BGC11-65 – Back wall of test pit; pit completed at depth of 8.0 m.



TP-BGC11-65 – Detail of spoil pile.



TP-BGC11-65 – Detail of pit wall from 0.4-0.5 m.



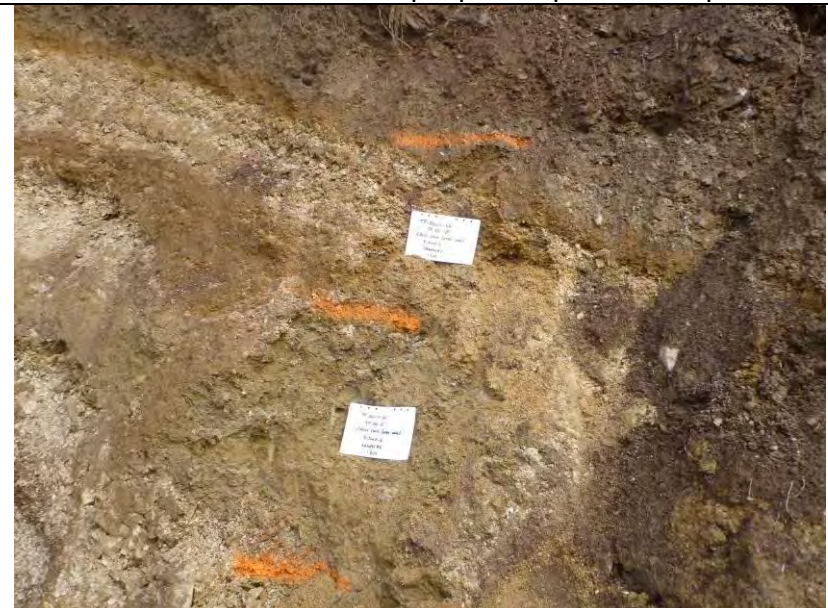
TP-BGC11-66 – Site photo.



TP-BGC11-66 – Back wall of test pit; pit completed at depth of 7.5 m.



TP-BGC11-66 – Detail of spoil pile.

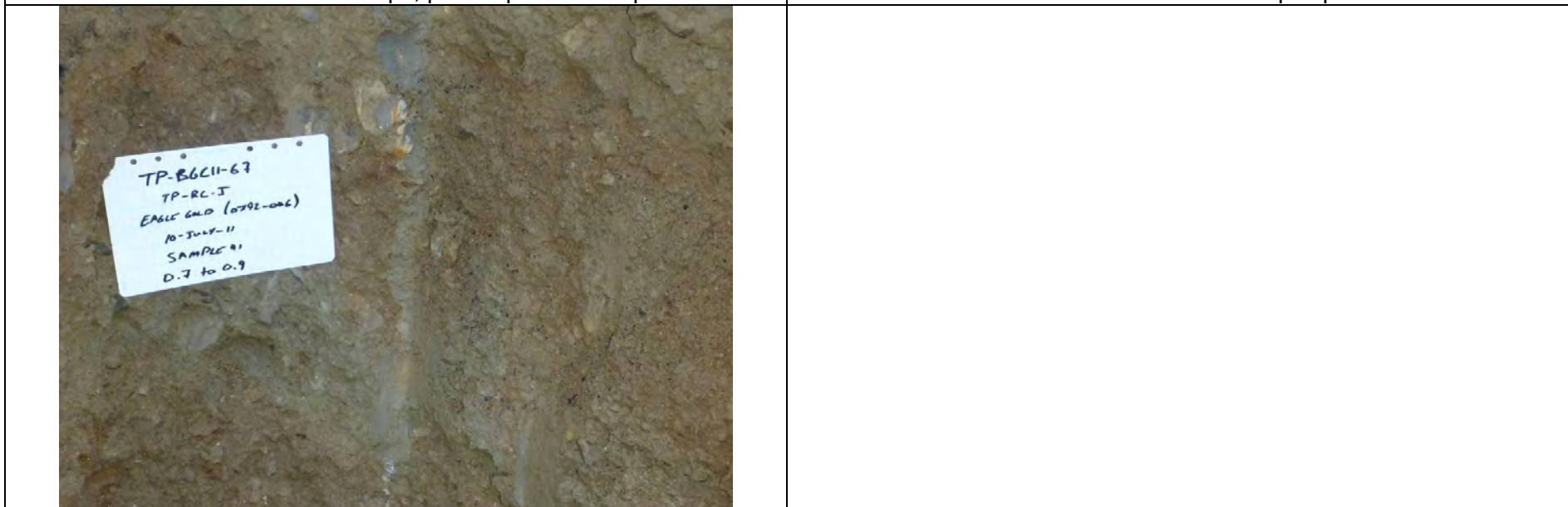


TP-BGC11-66 – Detail of pit wall from 0.0-2.0 m.



TP-BGC11-67 – Back wall of test pit; pit completed at depth of 6.5 m.

TP-BGC11-67 – Detail of spoil pile.



TP-BGC11-67 – Detail of pit wall from 0.7-0.9 m.

 A vertical photograph showing the back wall of a test pit. The wall is composed of dark, silty soil. The pit is deep, and the top edge shows some vegetation and a white object, possibly a measuring tape or marker.	 A close-up photograph of a spoil pile. The soil is light brown and contains numerous small, rounded rocks and pebbles. A small, light-colored rectangular tag is placed on the surface of the soil.
<p>TP-BGC11-68 – Back wall of test pit; pit completed at depth of 6.5 m.</p>	<p>TP-BGC11-68 – Detail of spoil pile.</p>
 A close-up photograph of the pit wall. The soil is dark brown and appears to be silty clay. A white tag is attached to the wall with handwritten text. The tag reads: TP-BGC11-68, TP-RC-A, 6742.600 (1277-006), 10-30-11, S.M.W., 1.6-1.8.	
<p>TP-BGC11-68 – Detail of pit wall from 1.6-1.8 m.</p>	



TP-BGC11-69 – Site photo.



TP-BGC11-69 – Back wall of test pit; pit completed at depth of 6.5 m.



TP-BGC11-69 – Detail of spoil pile.



TP-BGC11-69 – Detail of pit wall from 2.8-2.9 m.



TP-BGC11-70 – Site photo.



TP-BGC11-70 – Cut slope wall; pit completed at depth of 3.0 m.



TP-BGC11-71 – Site photo; drill pad of BH-BGC11-59.



TP-BGC11-71 – Cut slope from access construction.



TP-BGC11-71 – Detail of pit wall 1.3-1.6 m.



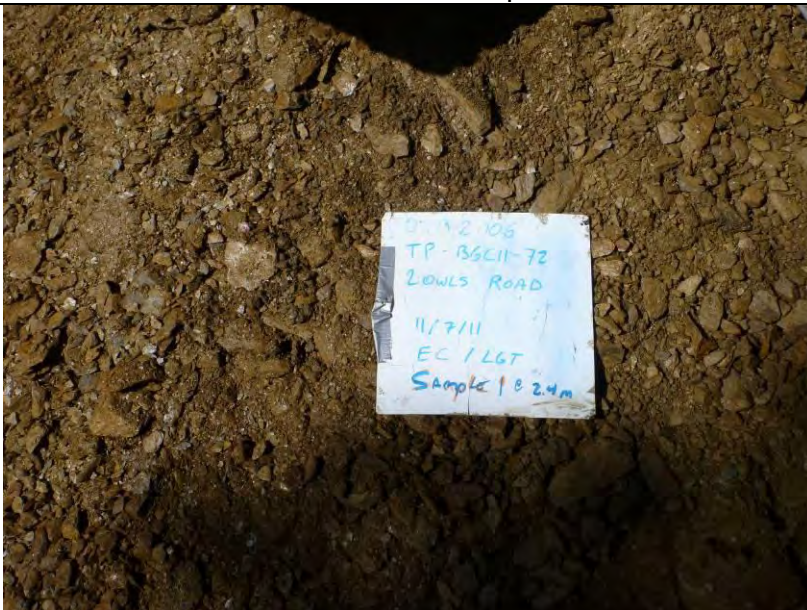
TP-BGC11-71 – Detail of Vx (30%) in pit wall.



TP-BGC11-72 – Site photo.



TP-BGC11-72 – Back wall of test pit; pit completed at depth of 4.3 m.



TP-BGC11-72 – Detail of spoil pile.



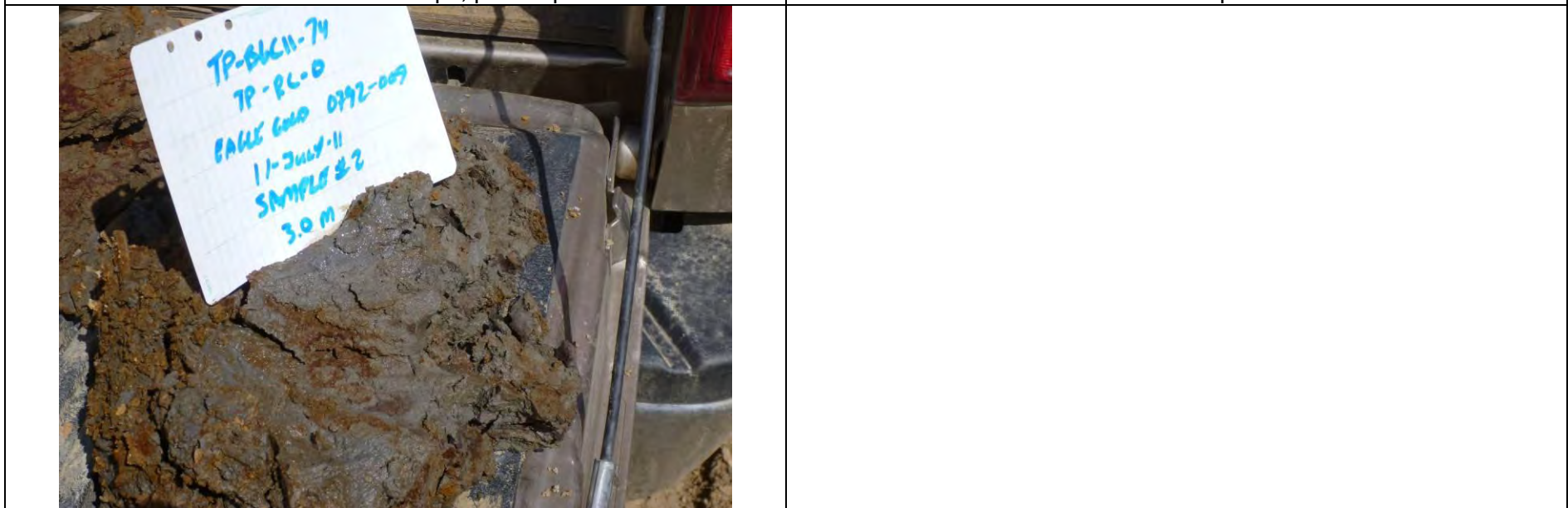
TP-BGC11-73 – Back wall of test pit; pit completed at depth of 3.0 m.

TP-BGC11-73 – Detail of pit wall from 0.8-1.0 m.



TP-BGC11-74 – Back wall of test pit; pit completed at 5.5 m.

TP-BGC11-74 – Detail of pit wall 1.2-1.4 m.



TP-BGC11-74 – Detail of sample taken from 3.0 m.



TP-BGC11-75 – Site photo.



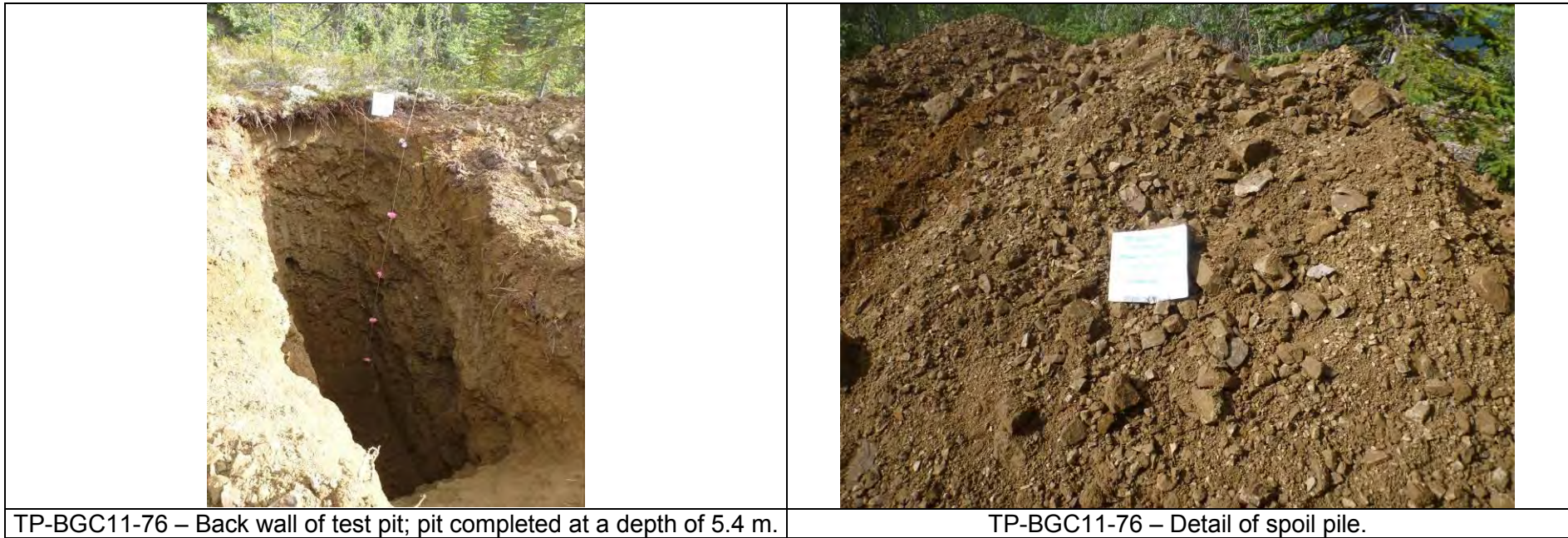
TP-BGC11-75 – Back wall of test pit; pit completed at a depth of 4.2 m.



TP-BGC11-75 – Detail of spoil pile.



TP-BGC11-75 – Detail of pit wall 0.4-0.6 m.





TP-BGC11-77 – Site photo.



TP-BGC11-77 – Back wall of test pit; pit completed at a depth of 5.5 m.



TP-BGC11-77 – Detail of spoil pile.



TP-BGC11-77 – Detail of pit wall 0.4-0.5 m.



TP-BGC11-78 – Site photo.



TP-BGC11-78 – Back wall of test pit; pit completed at a depth of 5.5 m.



TP-BGC11-78 – Detail of spoil pile.



TP-BGC11-78 – Water collected in pit from seepage through the walls.



TP-BGC11-79 – Site photo.



TP-BGC11-79 – Back wall of test pit; pit completed at a depth of 3.8 m.



TP-BGC11-79 – Detail of spoil pile.



TP-BGC11-79 – Detailed view of ice lenses beneath organic mat.



TP-BGC11-80 – Site photo.



TP-BGC11-80 – Back wall of test pit; pit completed at a depth of 6.7 m.



TP-BGC11-80 – Detail of spoil pile.



TP-BGC11-80 – Detail of pit wall.



TP-BGC11-81 – Site photo.



TP-BGC11-81 – Back wall of test pit; pit completed at a depth of 3.9 m.



TP-BGC11-81 – Detail of spoil pile.



TP-BGC11-81 – Detail of pit wall.



TP-BGC11-82 – Site photo; North wall.



TP-BGC11-82 – Site photo; East wall.



TP-BGC11-82 – Back wall of test pit; pit completed at a depth of 7.0 m.



TP-BGC11-82 – Exposed massive ice (ca.0.6m x 0.3m) at 6.5m depth.



TP-BGC11-83 – Site photo.



TP-BGC11-83 – Back wall of test pit; pit completed at a depth of 1.3 m.



TP-BGC11-83 – Detail of spoil pile.



TP-BGC11-83 – Detail of frozen soil sample taken from 0.6-0.8 m.



TP-BGC11-84 – Site photo.



TP-BGC11-84 – Back wall of test pit; pit completed at a depth of 2.6 m.



TP-BGC11-84 – Detail of pit wall 0.4-0.6 m.



TP-BGC11-84 – Detail of frozen soil sample taken from 2.5 m.



TP-BGC11-85 – Back wall of test pit; pit completed at a depth of 2.5 m.

TP-BGC11-85 – Detail of spoil pile.



TP-BGC11-85 – Detail of frozen soil sample taken from 1.0 m.



TP-BGC11-86 – Site photo.



TP-BGC11-86 – Back wall of test pit; pit completed at a depth of 7.5 m.



TP-BGC11-86 – Detail of spoil pile.



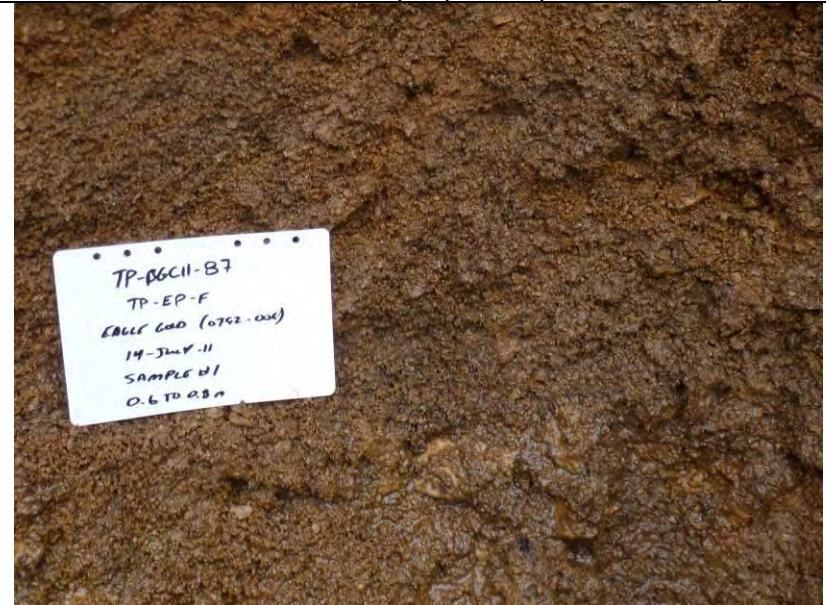
TP-BGC11-87 – Site photo.



TP-BGC11-87 – Back wall of test pit; pit completed at a depth of 3.5 m.



TP-BGC11-87 – Detail of spoil pile.



TP-BGC11-87 – Detail of pit wall at 0.6-0.8 m.

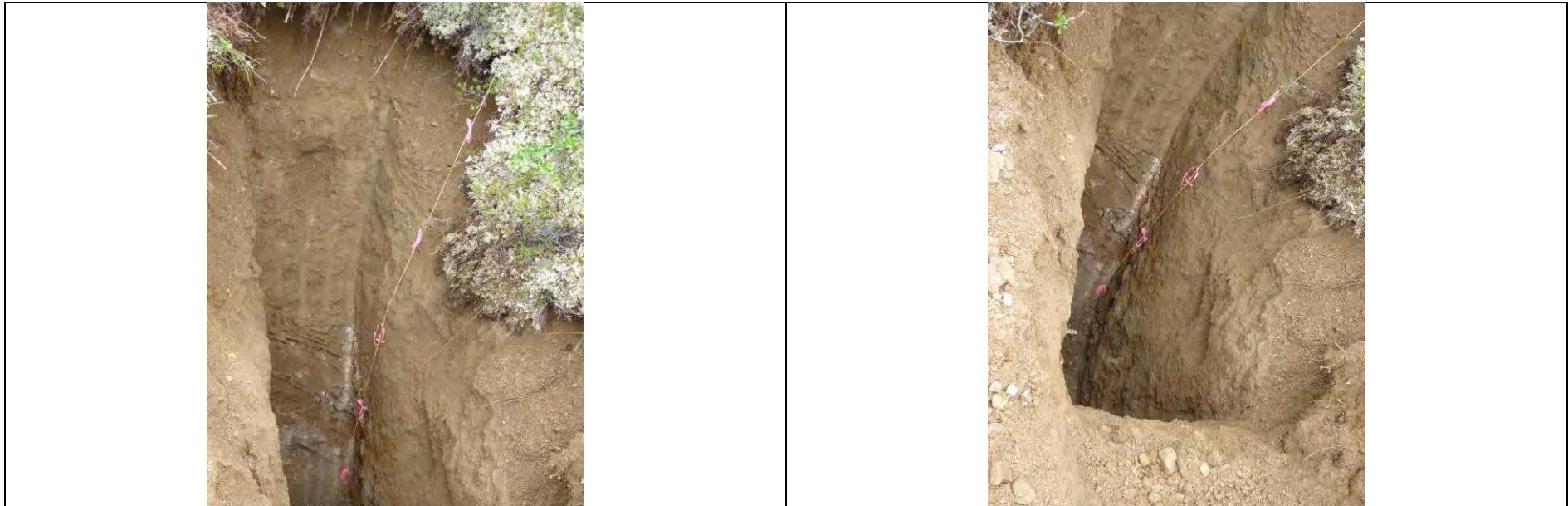


TP-BGC11-88 – Back wall of test pit; pit completed at a depth of 6.0 m.

TP-BGC11-88 – Detail of spoil pile.



TP-BGC11-88 – Detail of pit wall at 1.6-1.8 m.



TP-BGC11-89 – Back wall of test pit; pit completed at a depth of 6.5 m.

TP-BGC11-89 – Bottom of test pit; pit completed at a depth of 6.5 m.



TP-BGC11-89 – Detail of spoil pile.



TP-BGC11-89 – Detail of pit wall at 1.3-1.5 m.

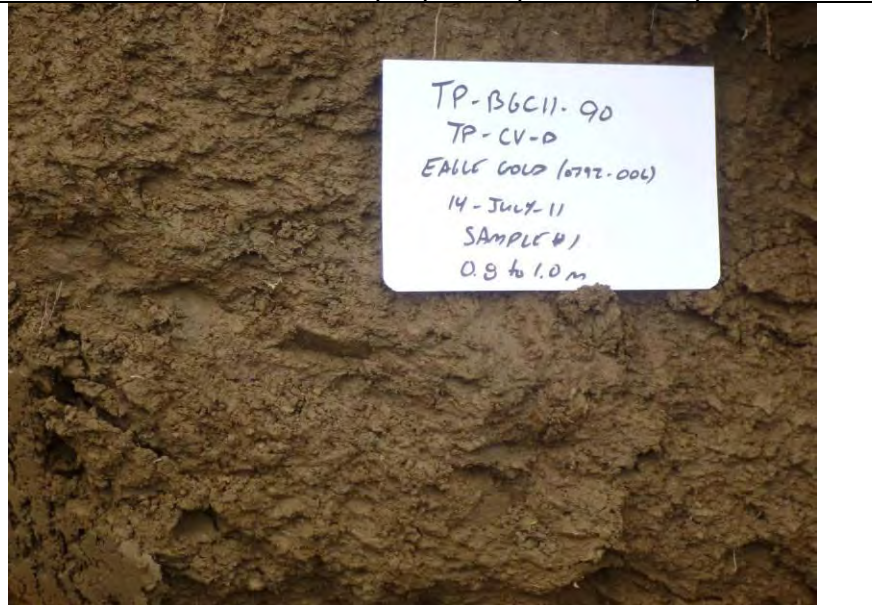


TP-BGC11-90 – Back wall of test pit; pit completed at a depth of 6.5 m.

TP-BGC11-90 – Bottom of test pit; pit completed at a depth of 6.5 m.



TP-BGC11-90 – Detail of spoil pile.



TP-BGC11-90 – Detail of pit wall at 0.8-1.0 m.



TP-BGC11-90 – Detail of pit wall at 3.5-3.6 m.



TP-BGC11-91 – Back wall of test pit; pit completed at a depth of 2.05 m.

TP-BGC11-91 – Detail of spoil pile.



TP-BGC11-91 – Detail of sample taken from 0.8-1.0 m.

TP-BGC11-91 – Detail of sample taken from 1.6-1.8 m.



TP-BGC11-92 – Site photo.



TP-BGC11-92 – Back wall of test pit; pit completed at a depth of 1.65 m.



TP-BGC11-92 – Detail of spoil pile.



TP-BGC11-92 – Detail of frozen soil sample taken from 0.6-0.8 m.



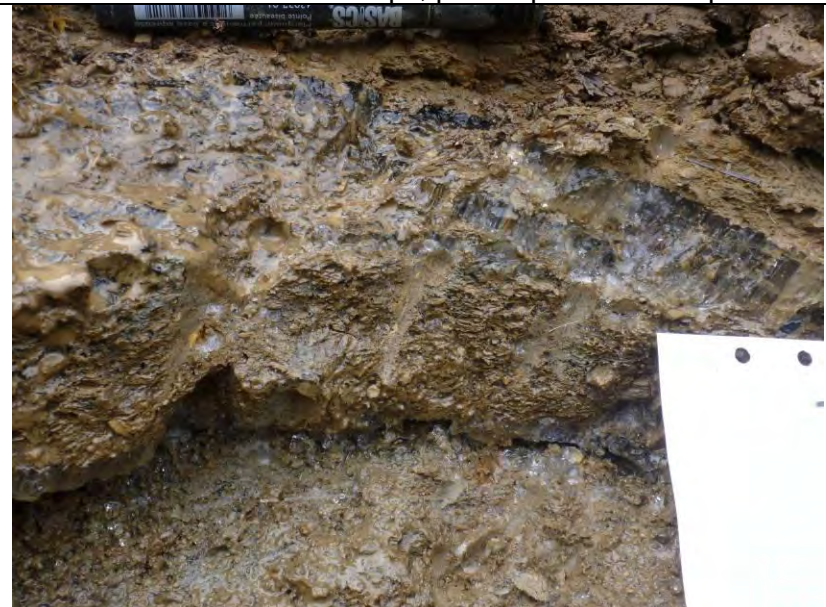
TP-BGC11-93 – Site photo.



TP-BGC11-93 – Back wall of test pit; pit completed at a depth of 1.85 m.



TP-BGC11-93 – Detail of spoil pile.



TP-BGC11-93 – Detail of ice in pit wall at 0.4-0.7 m.



TP-BGC11-94 – Back wall of test pit; pit completed at a depth of 5.0 m.

TP-BGC11-94 – Detail of spoil pile.



TP-BGC11-94 – Detail of frozen soil sample taken from 1.2-1.3 m.



TP-BGC11-94 – Detail of soil sample taken from 2.3-2.5 m.



TP-BGC11-95 – Back wall of test pit; pit completed at a depth of 3.2 m.

TP-BGC11-95 – Detail of spoil pile.



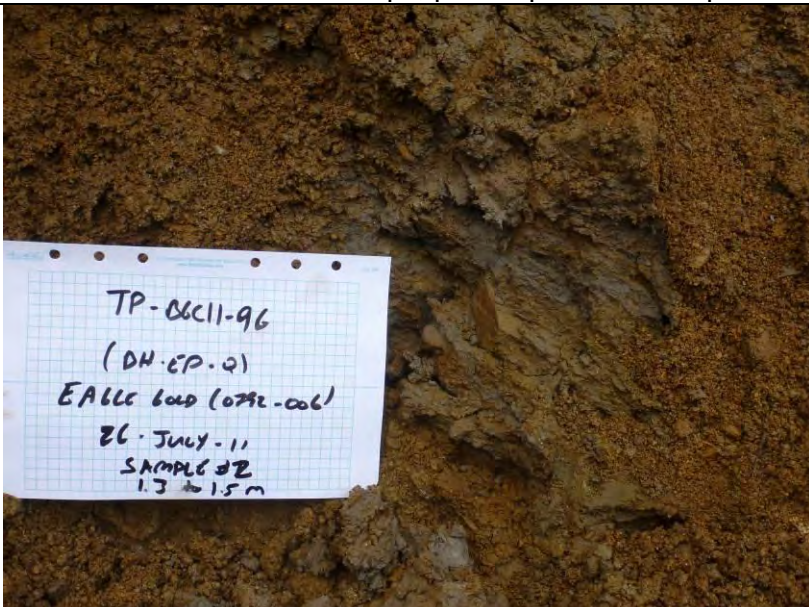
TP-BGC11-95 – Detail of pit wall at 0.9-1.1 m.

TP-BGC11-95 – Detail of ice lens.



TP-BGC11-96 – Back wall of test pit; pit completed at a depth of 2.0 m.

TP-BGC11-96 – Detail of spoil pile.



TP-BGC11-96 – Detail of pit wall at 1.3-1.5 m.

TP-BGC11-96 – Detail of frozen soil sample taken from 1.8-2.0 m.



TP-BGC11-97 – Site photo.



TP-BGC11-97 – Trench.



TP-BGC11-97 – Back wall of test pit; pit completed at a depth of 4.0 m.



TP-BGC11-97 – Detail of spoil pile.



TP-BGC11-98 – Back wall of test pit; pit completed at a depth of 2.2 m.

TP-BGC11-98 – Detail of spoil pile.



TP-BGC11-99 – Back wall of test pit; pit completed at a depth of 2.5 m.



TP-BGC11-99 – Detail of pit wall.





TP-BGC11-101 – Site photo.



TP-BGC11-101 – Back wall of test pit; pit completed at a depth of 2.4 m.



TP-BGC11-101 – Detail of visible ice in pit wall.



TP-BGC11-101 – Detail of visible ice in pit wall.



TP-BGC11-102 – Site photo; drill pad of BH-BGC11-45.



TP-BGC11-102 – Back wall of test pit; pit completed at a depth of 4.6 m.



TP-BGC11-102 – Detail of spoil pile.



TP-BGC11-102 – Detail of pit wall.



TP-BGC11-103 – Site photo; drill pad of BH-BGC11-67 and 69.



TP-BGC11-103 – Back wall of test pit; pit completed at a depth of 6.0 m.



TP-BGC11-103 – Detail of an excavated bucket.



TP-BGC11-104 – Back wall of test pit; pit completed at a depth of 5.55 m.

TP-BGC11-104 – Detail of spoil pile.



TP-BGC11-104 – Detail of an excavated bucket from below 3.45 m.

TP-BGC11-104 – Detail of pit wall.



TP-BGC11-105 – Site photo.



TP-BGC11-105 – Back wall of test pit; pit completed at a depth of 3.0 m.



TP-BGC11-105 – Detail of pit wall.



TP-BGC11-105 – Detail of bottom of pit.



TP-BGC11-106 – Site photo; drill pad of BH-BGC11-61 and 66.



TP-BGC11-106 – Back wall of test pit; pit completed at a depth of 3.0 m.



TP-BGC11-106 – Detail of visible ice in pit wall.



TP-BGC11-107 – Back wall of test pit; pit completed at a depth of 3.2 m.

TP-BGC11-107 – Detail of spoil pile; excavated from 1.1 m.



TP-BGC11-107 – Detail of melting ice in pit wall.



TP-BGC11-108 – Back wall of test pit; pit completed at a depth of 1.5 m.

TP-BGC11-108 – Detail of spoil pile.



TP-BGC11-108 – Detail of scraped frozen ground.

TP-BGC11-108 – Detail of visible ice.



TP-BGC11-109 – Site photo.



TP-BGC11-109 – Back wall of test pit; pit completed at a depth of 5.3 m.



TP-BGC11-109 – Detail of a bucket excavated from 2.3 m.



TP-BGC11-109 – Detail of bottom of pit wall.



TP-BGC11-110 – Site photo.



TP-BGC11-110 – Back wall of test pit; pit completed at a depth of 5.0 m.



TP-BGC11-110 – Detail of a bucket excavated from 5.0 m.



TP-BGC11-110 – Detail of pit wall at 1.2 m.



TP-BGC11-111 – Site photo.



TP-BGC11-111 – Back wall of test pit; pit completed at a depth of 2.0 m.



TP-BGC11-111 – Detail of spoil pile from 2.0 m.



TP-BGC11-111 – Detail of bucket excavated from 2.0 m.



TP-BGC11-112 – Site photo.



TP-BGC11-112 – Back wall of test pit; pit completed at a depth of 2.5 m.



TP-BGC11-112 – Detail of a bucket excavated from 2.5 m.



TP-BGC11-112 – Detail of visible ice in pit wall near surface.



TP-BGC11-113 – Site photo.



TP-BGC11-113 – Back wall of test pit; pit completed at a depth of 2.5 m.



TP-BGC11-113 – Detail of a bucket excavated from 1.5 m.



TP-BGC11-113 – Detail of seepage collecting in pit below 1.5 m.



TP-BGC11-114 – Site photo.



TP-BGC11-114 – Back wall of test pit; pit completed at a depth of 3.2 m.



TP-BGC11-114 – Detail of a bucket excavated from 2.3 m.



TP-BGC11-114 – Detail of frozen material from 2.3 m.



TP-BGC11-115 – Site photo.



TP-BGC11-115 – Back wall of test pit; pit completed at a depth of 5.7 m.



TP-BGC11-115 – Detail of a bucket excavated from 2.5 m.



TP-BGC11-115 – Detail of material from 5.7 m.



TP-BGC11-116 – Back wall of test pit; pit completed at a depth of 2.3 m.



TP-BGC11-116 – Detail of pit wall at 1.3 m.



TP-BGC11-116 – Detail of seepage in pit wall.



TP-BGC11-116 – Detail of frozen material.



TP-BGC11-117 – Pit completed at depth of 8.0 m.



TP-BGC11-117 – Cut slope wall above pit.



TP-BGC11-117 – Detail of pit wall at 2.3 m.



TP-BGC11-117 – Detail of cut slope wall.



TP-BGC11-118 – Site photo.



TP-BGC11-118 – Back wall of test pit; pit completed at a depth of 5.3 m.



TP-BGC11-118 – Detail of highly weathered bedrock in bucket from 2.3 m.



TP-BGC11-118 – Detail of pit wall at 1.2 m.



TP-BGC11-119 – Site photo.



TP-BGC11-119 – Back wall of test pit; pit completed at a depth of 2.05 m.



TP-BGC11-119 – Detail of spoil pile.



TP-BGC11-119 – Detail of pit wall.



TP-BGC11-120 – Site photo.



TP-BGC11-120 – Spoil pile.



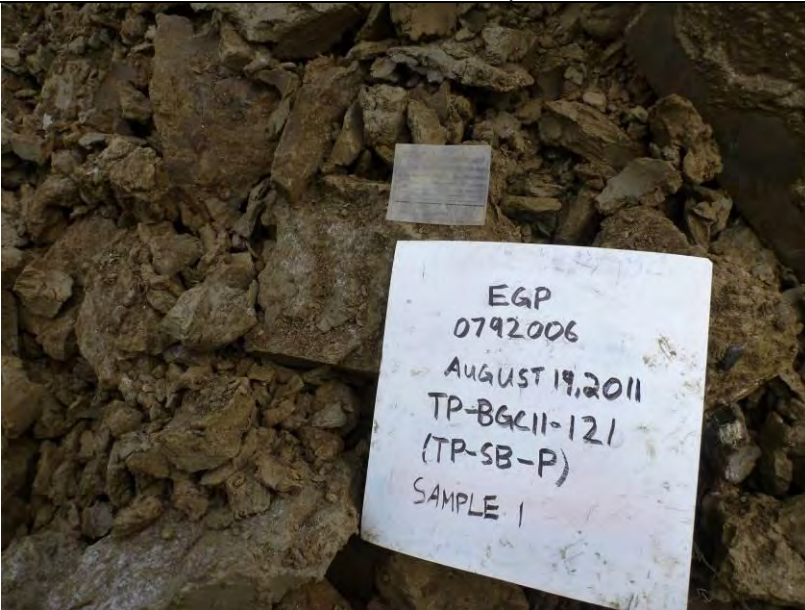
TP-BGC11-120 – Detail of spoil pile.



TP-BGC11-121 – Site photo.



TP-BGC11-121 – Back wall of test pit; pit completed at a depth of 3.3 m.



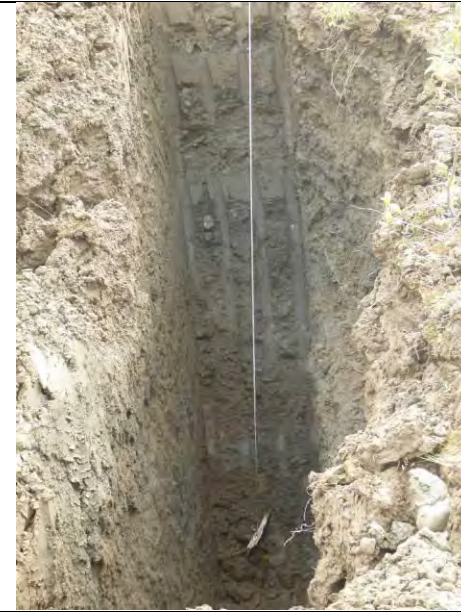
TP-BGC11-121 – Detail of spoil pile.



TP-BGC11-121 – Detail of lensed ice in pit wall.



TP-BGC11-122 – Site photo.



TP-BGC11-122 – Back wall of test pit; pit completed at a depth of 5.8 m.



TP-BGC11-122 – Detail of a bucket excavated from 5.4-5.8 m.



TP-BGC11-122 – Detail of upper pit wall.



TP-BGC11-123 – Site photo.



TP-BGC11-123 – Back wall of test pit; pit completed at a depth of 2.4 m.



TP-BGC11-123 – Detail of a bucket excavated from 2.4 m.



TP-BGC11-123 – Detail of massive ice in pit wall.

				
<p>TP-BGC11-124 – Site photo.</p>			<p>TP-BGC11-124 – Back wall of test pit; pit completed at a depth of 6.5 m.</p>	
				
<p>TP-BGC11-124 – Detail of a bucket excavated from 6.2-6.4 m.</p>			<p>TP-BGC11-124 – Detail of pit wall.</p>	



TP-BGC11-125 – Site photo.



TP-BGC11-125 – Pit completed at a depth of 2.5 m.



TP-BGC11-125 – Detail of frozen soil chunks in spoil pile.



TP-BGC11-125 – Using ripper shank to penetrate frozen ground.



TP-BGC11-126 – Site photo.



TP-BGC11-126 – Back wall of test pit; pit completed at a depth of 3.1 m.



TP-BGC11-126 – Detail of spoil pile.



TP-BGC11-126 – Detail of topsoil.



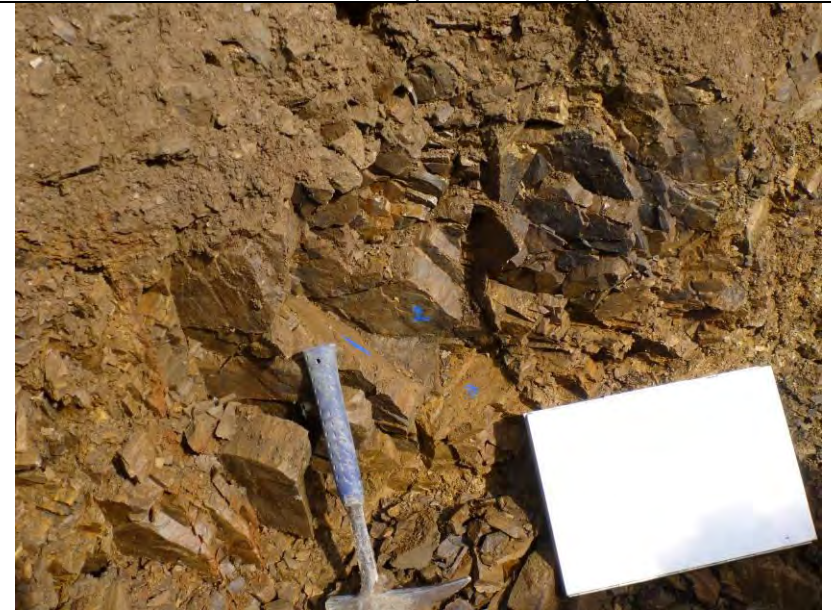
TP-BGC11-127 – Site photo.



TP-BGC11-127 – Pit completed at a depth of 2.5 m.



TP-BGC11-127 – Detail of spoil pile.



TP-BGC11-127 – Detail of structure of bedrock unit at base of pit.



TP-BGC11-128 – Back wall of test pit; pit completed at a depth of 3.0 m.



TP-BGC11-128 – Detail of spoil pile.



TP-BGC11-128 – Detail of pit wall.



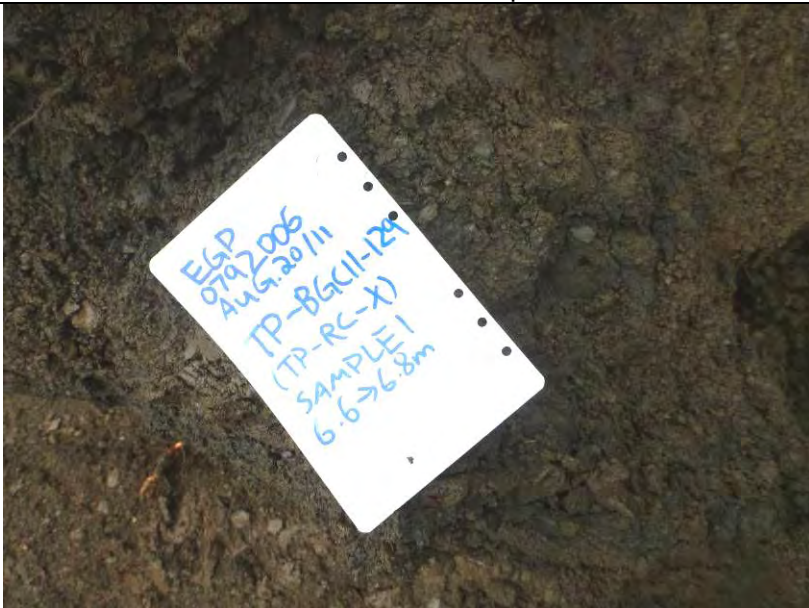
TP-BGC11-128 – Chunk of intact ice recovered at 1.8 m.



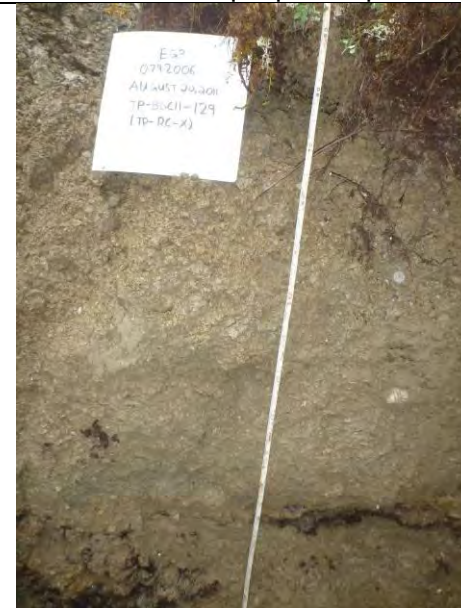
TP-BGC11-129 – Site photo.



TP-BGC11-129 – Back wall of test pit; pit completed at a depth of 6.8 m.



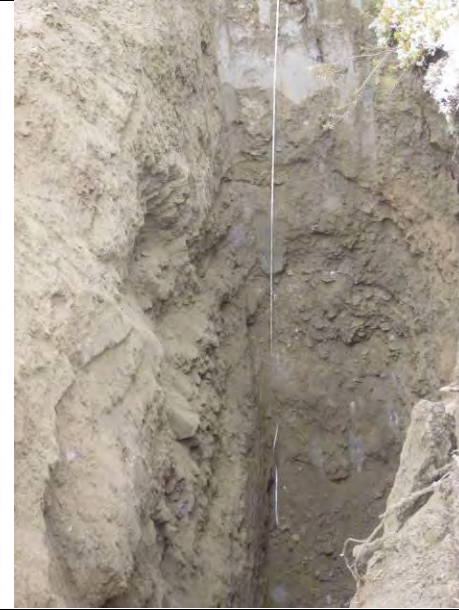
TP-BGC11-129 – Detail of a bucket excavated from 6.6-6.8 m.



TP-BGC11-129 – Detail of upper pit wall.



TP-BGC11-130 – Site photo.



TP-BGC11-130 – Back wall of test pit; pit completed at a depth of 6.0 m.



TP-BGC11-130 – Detail of a bucket excavated from 5.8-6.0 m.



TP-BGC11-130 – Detail of upper pit wall.



TP-BGC11-131 – Site photo.



TP-BGC11-131 – Back wall of test pit; pit completed at a depth of 3.5 m.



TP-BGC11-131 – Detail of spoil pile.



TP-BGC11-131 – Detail of fluvial material in pit wall.



TP-BGC11-132 – Site photo after completion of test pit.



TP-BGC11-132 – Back wall of test pit; pit completed at a depth of 5.3 m.



TP-BGC11-132 – Detail of spoil pile.



TP-BGC11-132 – Detail of pit wall.



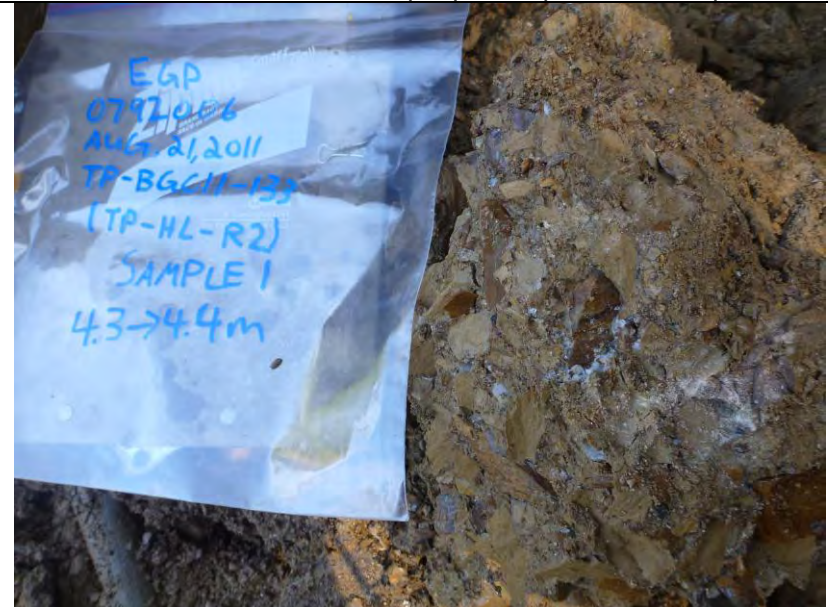
TP-BGC11-133 – Site photo.



TP-BGC11-133 – Back wall of test pit; pit completed at a depth of 4.4 m.



TP-BGC11-133 – Detail of gravelly unit in upper pit wall.



TP-BGC11-133 – Detail of frozen gravel/silt material in bucket.



TP-BGC11-134 – Site photo.




TP-BGC11-134 – Back wall of test pit; pit completed at a depth of 4.2 m.



TP-BGC11-134 – Spoil pile (hammer for scale).



TP-BGC11-134 – Detail of pit wall.

	 A vertical photograph showing the back wall of a test pit. The soil is light brown and appears to be a silty clay or sand. A thin white string is suspended vertically from the top edge of the pit to provide a scale reference.		 A close-up photograph of a spoil pile. The soil is dark brown and appears to be a silty clay or sand. A yellow measuring tape is placed horizontally across the pile to provide a scale reference.
<p>TP-BGC11-135 – Back wall of test pit; pit completed at a depth of 3.5 m.</p>		<p>TP-BGC11-135 – Detail of spoil pile.</p>	
	 A vertical photograph showing the upper part of the pit wall. The soil is dark brown and appears to be a silty clay or sand. A thin white string is suspended vertically from the top edge of the pit to provide a scale reference.		 A vertical photograph showing the base of the pit. The soil is dark brown and appears to be a silty clay or sand. A thin white string is suspended vertically from the top edge of the pit to provide a scale reference.
<p>TP-BGC11-135 – Detail of upper pit wall to 1.5 m.</p>		<p>TP-BGC11-135 – Detail of base of pit at 3.5 m.</p>	



TP-BGC11-136 – Site photo; drill pad of BH-BGC11-52.



TP-BGC11-136 – Back wall of test pit; pit completed at a depth of 4.8 m.



TP-BGC11-136 – Detail of upper pit wall to 1.4 m.



TP-BGC11-136 – Detail of sample taken from 1.0-1.2 m.



TP-BGC11-137 – Site photo.



TP-BGC11-137 – Back wall completed to depth of 5.0 m.



TP-BGC11-137 – Gradational bedrock weathering in pit wall.



TP-BGC11-137 – Detail of weathered bedrock in spoil pile.



TP-BGC11-138 – Back wall of test pit; pit completed at a depth of 6.3 m.



TP-BGC11-138 – Detail of spoil pile at 5.9 m.



TP-BGC11-138 – Detail of pit wall at 5.5 m.



TP-BGC11-138 – Detail of bedrock structure at base of pit (2.0 m).



TP-BGC11-139 – Site photo; along wall of exploration trench.



TP-BGC11-139 – Dug areas along trench wall (3.0 m).



TP-BGC11-139 – Detail of highly weathered bedrock at 1.2 m.



TP-BGC11-139 – Detail of lightly weathered bedrock at base of trench.



TP-BGC11-140 – Site photo.



TP-BGC11-140 – Back wall of test pit; pit completed at a depth of 4.5 m.



TP-BGC11-140 – Detail of spoil pile.



TP-BGC11-140 – Detail of pit wall at 2.8 m.



TP-BGC11-141 – Site photo.



TP-BGC11-141 – Back wall of test pit; pit completed at a depth of 4.95 m.



TP-BGC11-141 – Detail of spoil pile at 4.0 m.



TP-BGC11-142 – Site photo.

TP-BGC11-142 – Back wall of test pit; pit completed at a depth of 4.1 m.



TP-BGC11-142 – Detail of a bucket from 2.6 m.

TP-BGC11-142 – Detail of pit wall at 0.5 m.



TP-BGC11-143 – Site photo at beginning of excavation (0.9 m).



TP-BGC11-143 – Back wall of test pit; pit completed at a depth of 2.2 m.



TP-BGC11-143 – Detail of a bucket excavated from 2.2 m.



TP-BGC11-143 – Detail of pit wall 0.0-0.5 m.



TP-BGC11-144 – Site photo; angular boulders on surface.



TP-BGC11-144 – Back wall of test pit; pit completed at a depth of 1.0 m.



TP-BGC11-144 – Detail of back wall; pit abandoned due to seepage.



TP-BGC11-144 – Seepage upslope from test pit.



TP-BGC11-145 – Site photo; drill pad of BH-BGC11-31.



TP-BGC11-145 – Back wall of test pit; pit completed at a depth of 1.5 m.



TP-BGC11-145 – Detail of wall at 0.5 m.



TP-BGC11-145 – Detail of wall at 1.2 m.

APPENDIX E BOREHOLE LOGS

BOREHOLE LOGS

Table E- 1: Summary of Boreholes

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Drilling Method	Hole Depth (m)	Installation	Comments
BH-BGC11-24	7102237.7	460624.8	1208.4	Diamond	20.9	--	
BH-BGC11-25	7102179.3	460414.7	1182.8	Diamond	20.4	2" solid PVC	
BH-BGC11-26	7102088.1	460205.8	1150.0	Diamond	30.2	Standpipe piezometer	
BH-BGC11-27	7102232.5	460043.5	1100.2	Diamond	26.5	--	
BH-BGC11-28	7101702.7	459926.5	1010.6	Diamond	40.8	2" solid PVC	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-29	7101802.1	459977.8	1045.3	Diamond	41.2	Standpipe piezometer	
BH-BGC11-30	7101567.8	460122.0	952.0	Diamond	35.1	Standpipe piezometer	
BH-BGC11-31	7101483.0	459850.9	917.8	Diamond	35.1	2" solid PVC	
BH-BGC11-32	7100934.4	458745.2	818.7	Diamond	24.4	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-33	7100983.6	459115.7	833.0	Diamond	41.4	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-34	7101053.5	459308.5	848.3	Diamond	38.1	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-35	7100440.3	459695.7	986.3	Diamond	50.3	Standpipe piezometer	
BH-BGC11-36	7100274.7	459699.9	1002.5	Diamond	50.3	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-37	7100180.5	459762.8	1034.4	Diamond	43.6	Standpipe piezometer	Standpipe damaged, Jul 2011
BH-BGC11-38	7100416.0	459820.0	1013.0	Diamond	50.5	Standpipe piezometer	
BH-BGC11-39	7101056	458462	804	Auger	28.2	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-40A	7100038	459763	1050	Diamond	33.2	--	
BH-BGC11-40B	7100038.8	459767.0	1049.8	Diamond	45.7	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-41	7100999	459777	914	Auger	4.3	--	

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Drilling Method	Hole Depth (m)	Installation	Comments
BH-BGC11-42	7100150.1	460272.3	1098.7	Auger	28.2	25 m thermistor	
BH-BGC11-43	7099106.6	459658.1	1034.0	Diamond	23.8	--	
BH-BGC11-44	7100547.3	458690.4	830.0	Auger	14.7	10 m thermistor	
BH-BGC11-45	7099109.5	460556.2	1354.4	Diamond	20.7	Standpipe piezometer	
BH-BGC11-46	7099004.0	460262.7	1246.1	Diamond	20.1	Standpipe piezometer	
BH-BGC11-47	7100698.9	458811.9	841.8	Auger	16.0	10 m thermistor	
BH-BGC11-48	7099118.1	459936.0	1140.0	Diamond	32.9	Standpipe piezometer	
BH-BGC11-49	7100635.9	458746.5	835.1	Auger	16.0	10 m thermistor	Thermistor removed, Aug 2011
BH-BGC11-50	7099997.6	459791.3	1058.4	Diamond	41.2	--	
BH-BGC11-51	7100743.6	458643.5	818.4	Auger	25.2	25 m thermistor	
BH-BGC11-52	7101316.4	459891.2	909.0	Diamond	22.6	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-53	7100993.0	459483.6	875.7	Auger	14.5	--	
BH-BGC11-54	7101245.6	458886.2	883.8	Diamond	41.2	Vibrating wire piezometer	
BH-BGC11-55	7100918.4	459440.6	881.0	Auger	14.5	Standpipe piezometer	
BH-BGC11-56	7099822.7	458774.8	845.6	Auger	13.7	2" solid PVC	
BH-BGC11-57	7099978.2	458798.6	859.2	Auger	12.1	Standpipe piezometer and 10 m thermistor	
BH-BGC11-58	7100119.4	458799.8	859.0	Auger	10.8	10 m thermistor	
BH-BGC11-59	7101239.4	459113.1	883.7	Diamond	30.2	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-60	7100041.6	458795.6	859.2	Auger	9.2	Standpipe piezometer	
BH-BGC11-61	7100013	460367	1157	Auger	2.7	--	
BH-BGC11-62	7100334.1	459785.9	1017.9	Diamond	35.1	Standpipe piezometer	Conducted downhole seismic investigation, Aug 2011
BH-BGC11-63	7100114.1	460302.9	1100.7	Auger	26.7	Vibrating wire piezometer and 25 m thermistor	
BH-BGC11-64	7100262.9	460182.4	1044.1	Diamond	48.8	Standpipe piezometer	
BH-BGC11-65	7100939	458771	825	Auger	6.9	--	

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Drilling Method	Hole Depth (m)	Installation	Comments
BH-BGC11-66	7100011.1	460360.2	1151.9	Diamond	33.5	2" solid PVC	
BH-BGC11-67	7101148	458823	867	Auger	9.9	--	
BH-BGC11-69	7101144.3	458829.3	861.6	Diamond	21.3	2" solid PVC	Conducted downhole seismic investigation, Aug 2011

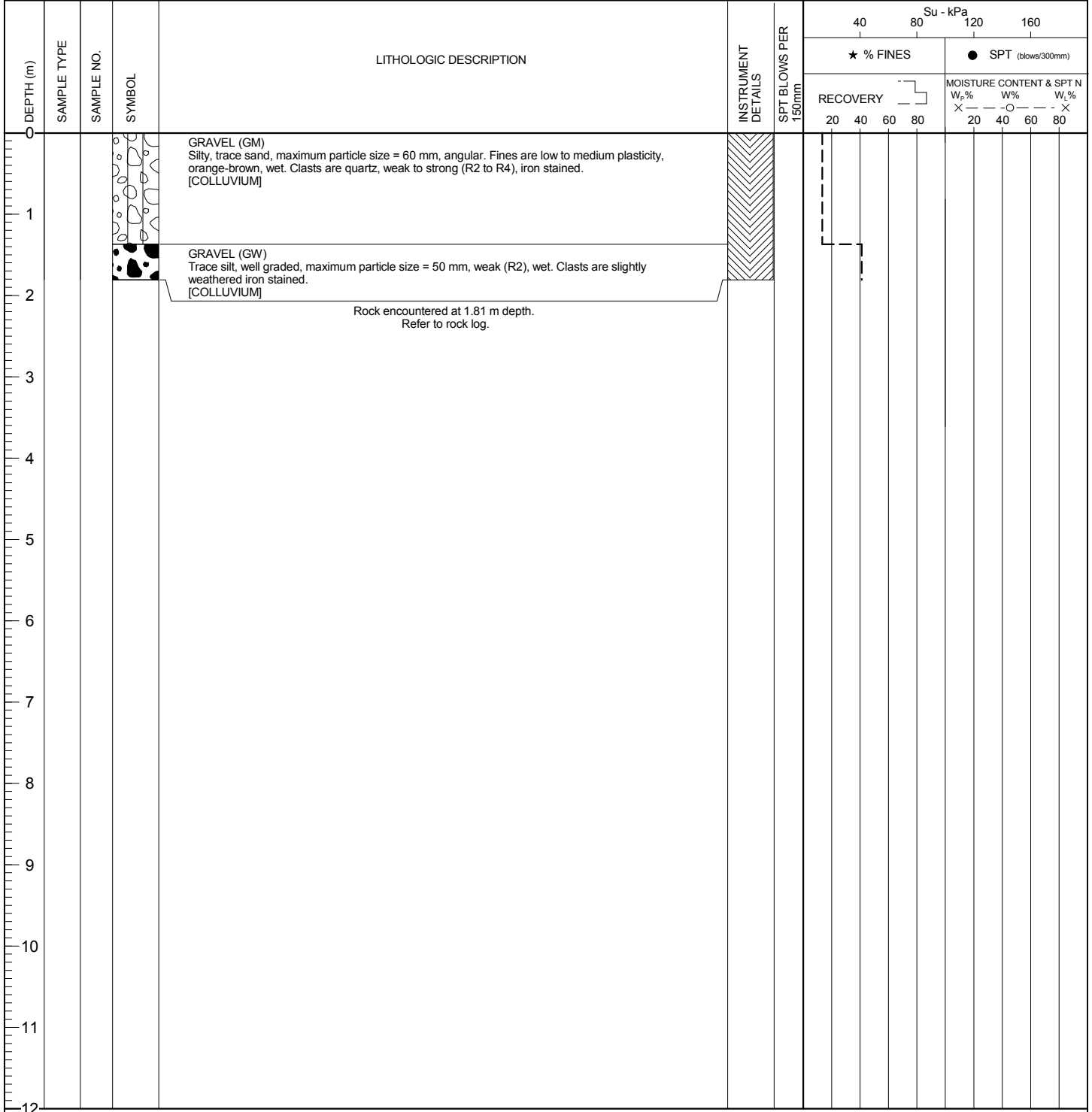
NOTES:

1. Holes surveyed in August and September 2011 using a differential GPS, with the exception of BH-BGC11-39,40A,41,61,65 and 67, which were surveyed by BGC staff using a handheld GPS.
2. Coordinates in UTM NAD 83, Zone 8N.

CO-ORDINATES (m): 460,624.8E - 7,102,237.7N
 GROUND ELEVATION (m) : 1,208.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 0.00

START DATE : 28 Jun 11
 FINISH DATE : 29 Jun 11
 FINAL DEPTH (m) : 20.9
 DEPTH TO TOP OF ROCK (m) : 1.8
 LOGGED BY : JND/SP/EB
 REVIEWED BY : PQ/DW

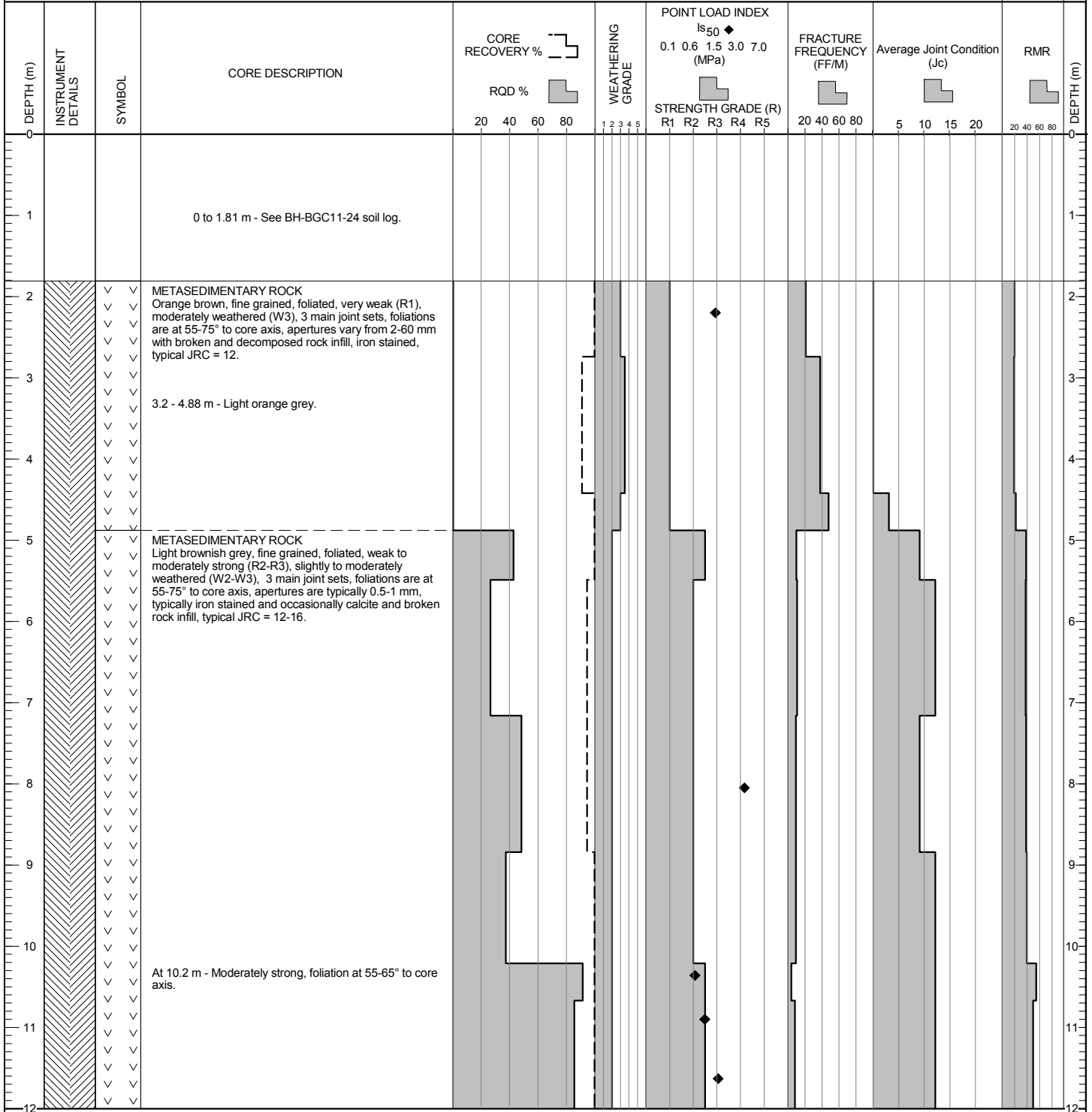


EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/2/12

CO-ORDINATES (m) 460,624.8E - 7,102,237.7N
 GROUND ELEVATION (m) : 1,208.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 0

START DATE : 28 Jun 11
 FINISH DATE : 29 Jun 11
 FINAL DEPTH (m) : 20.9
 DEPTH TO TOP OF ROCK (m) : 1.8
 LOGGED BY : JND/SP/EB
 REVIEWED BY : PQ/DW

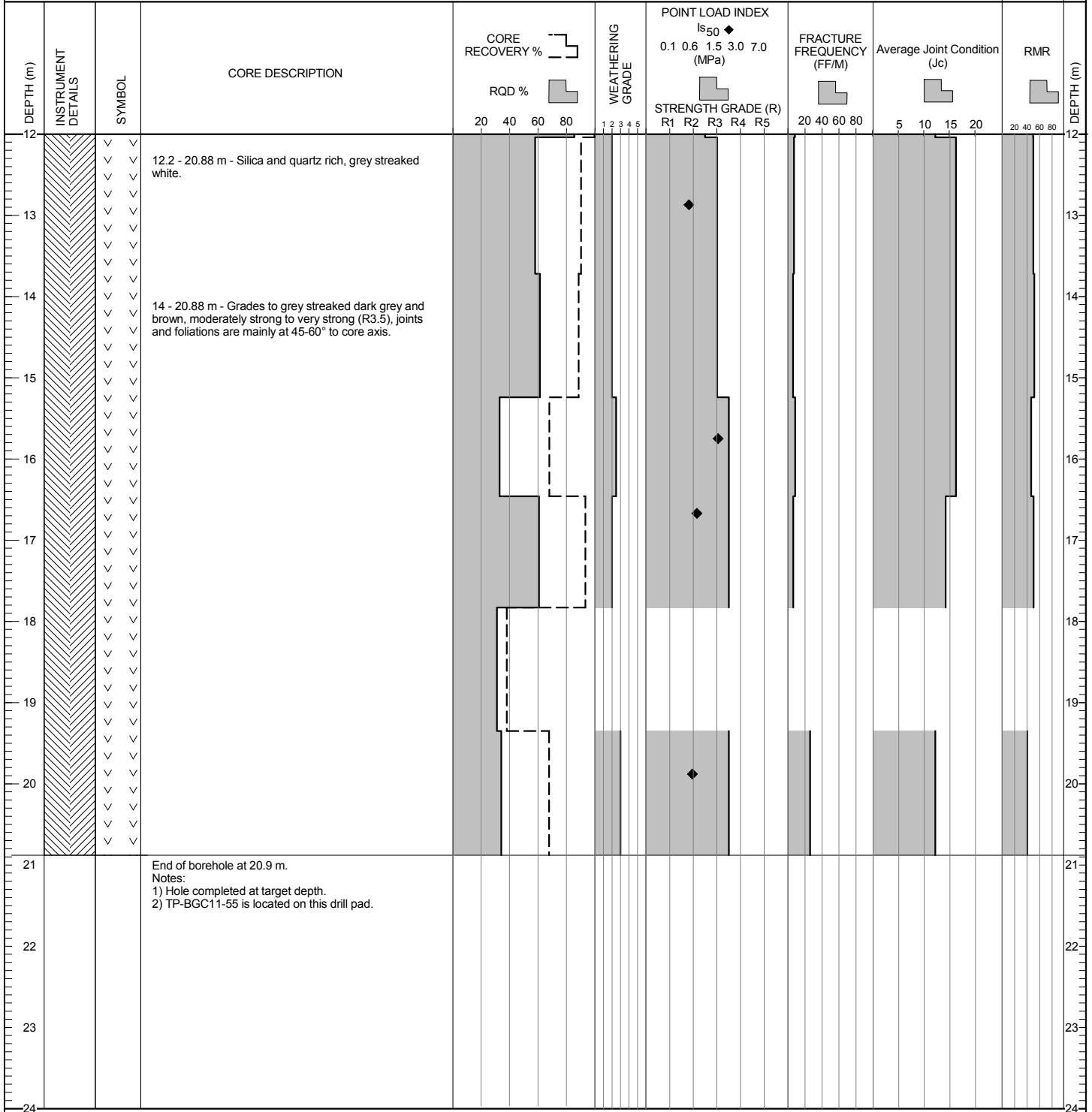


(CONTINUED ON NEXT PAGE)

CO-ORDINATES (m) 460,624.8E - 7,102,237.7N
 GROUND ELEVATION (m) : 1,208.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 0

START DATE : 28 Jun 11
 FINISH DATE : 29 Jun 11
 FINAL DEPTH (m) : 20.9
 DEPTH TO TOP OF ROCK (m) : 1.8
 LOGGED BY : JND/SP/EB
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-25

LOCATION : ANN GULCH

CO-ORDINATES (m): 460,414.7E - 7,102,179.3N
 GROUND ELEVATION (m) : 1,182.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 3.05

START DATE : 29 Jun 11
 FINISH DATE : 30 Jun 11
 FINAL DEPTH (m) : 20.4
 DEPTH TO TOP OF ROCK (m) : 0.9
 LOGGED BY : SD/SP/EB
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa						
							★ % FINES		● SPT (blows/300mm)				
							RECOVERY		MOISTURE CONTENT & SPT N				
							20	40	60	80	W _p %	W ₅ %	W ₁ %
0				GRAVEL (GM) Silty, some sand, trace cobbles, trace boulders, well graded, loose, maximum particle size = 250 mm, angular, iron staining. Fines: low plastic, firm to stiff, brown, moist, heterogeneous. [COLLUVIUM]									
0.9				Rock encountered at 0.90 m depth. Refer to rock log.									
1													
2													
3													
4													
5													
6													
7													
8													
9													
10													
11													
12													

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/0/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

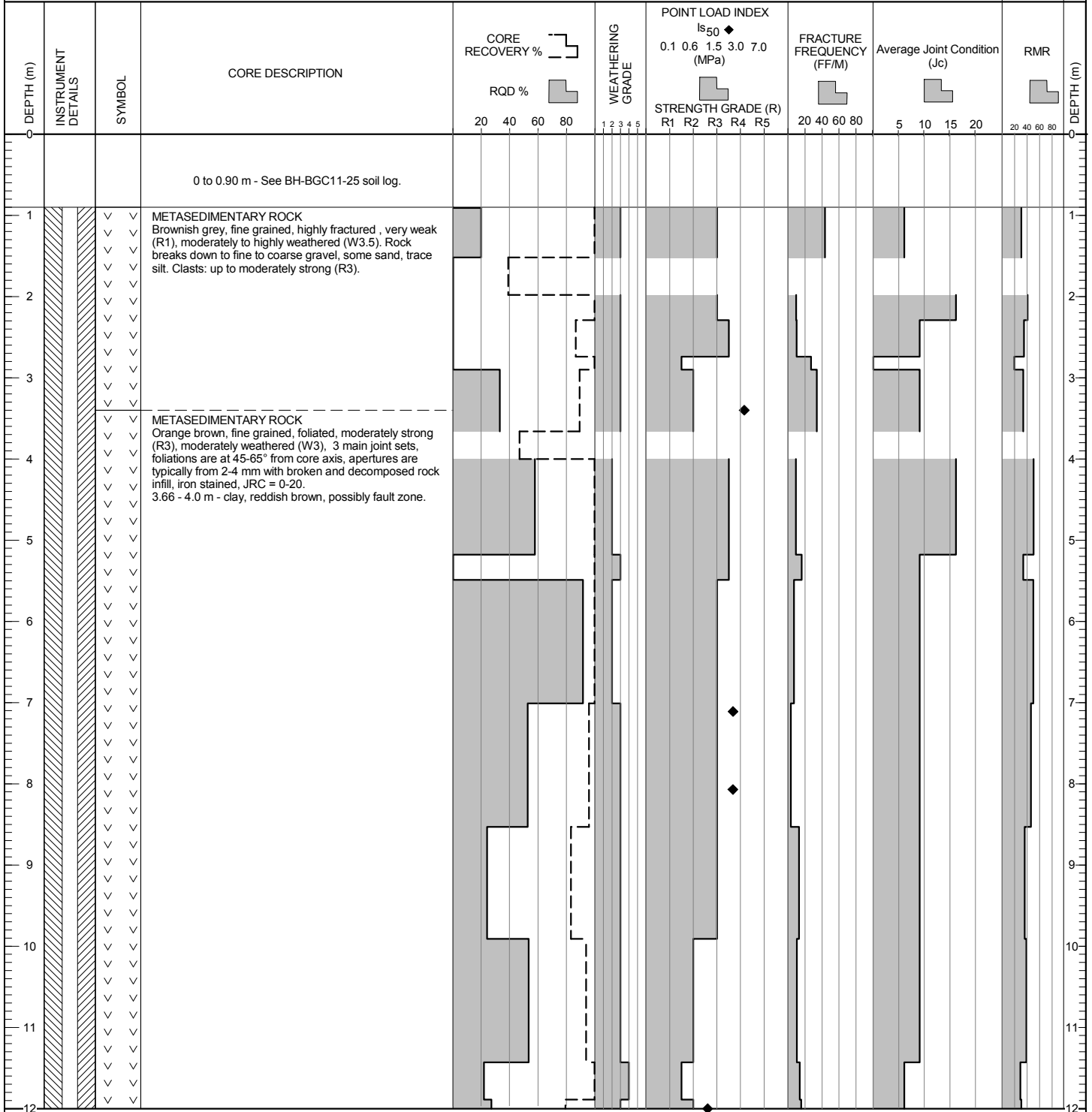
DRILL HOLE # BH-BGC11-25

LOCATION : ANN GULCH

CO-ORDINATES (m) 460,414.7E - 7,102,179.3N
 GROUND ELEVATION (m) : 1,182.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 29 Jun 11
 FINISH DATE : 30 Jun 11
 FINAL DEPTH (m) : 20.4
 DEPTH TO TOP OF ROCK (m) : 0.9
 LOGGED BY : SD/SP/EB
 REVIEWED BY : PQ/DW



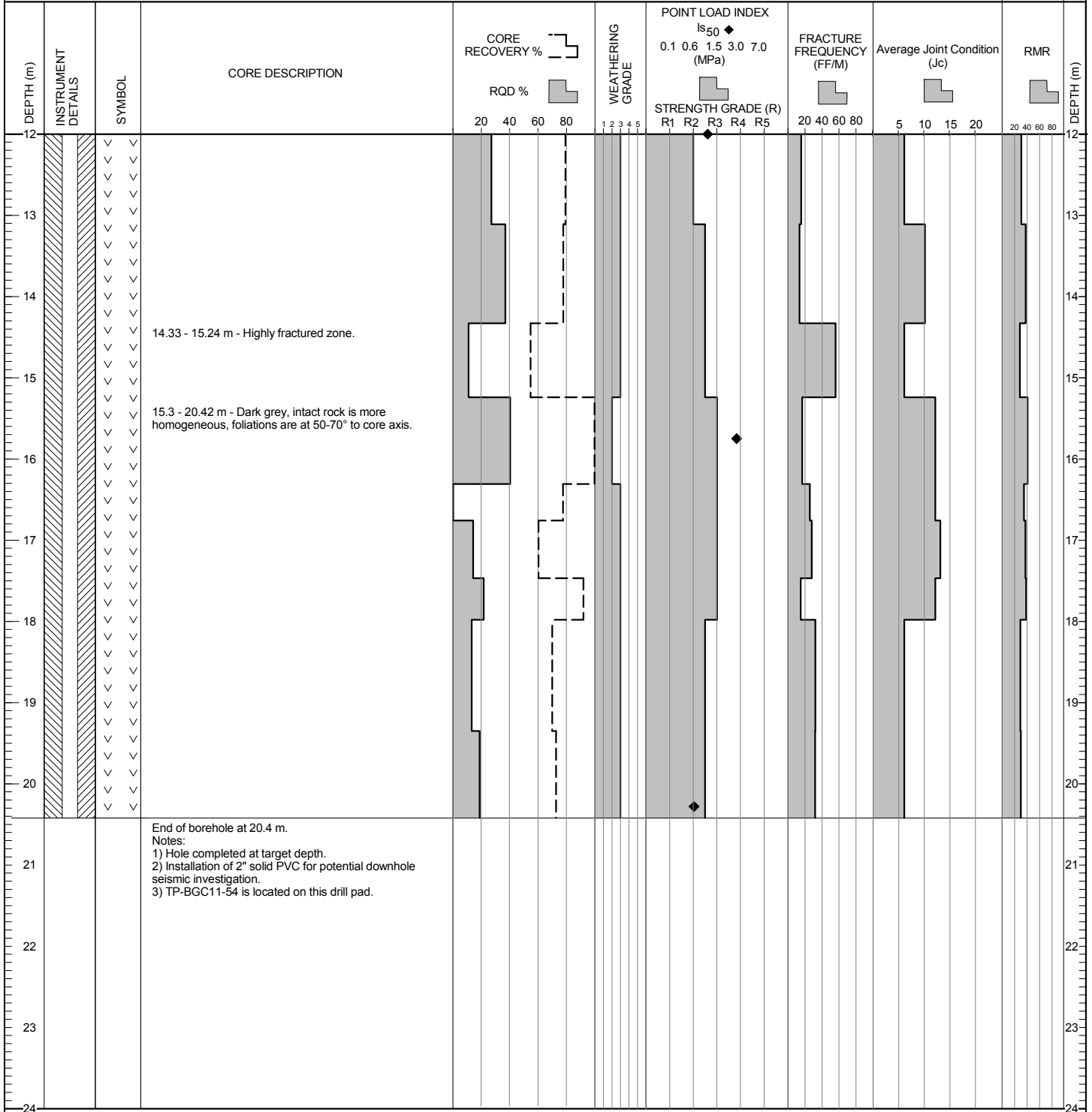
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ESP (ROCK) ESP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,414.7E - 7,102,179.3N
 GROUND ELEVATION (m) : 1,182.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 29 Jun 11
 FINISH DATE : 30 Jun 11
 FINAL DEPTH (m) : 20.4
 DEPTH TO TOP OF ROCK (m) : 0.9
 LOGGED BY : SD/SP/EB
 REVIEWED BY : PQ/DW

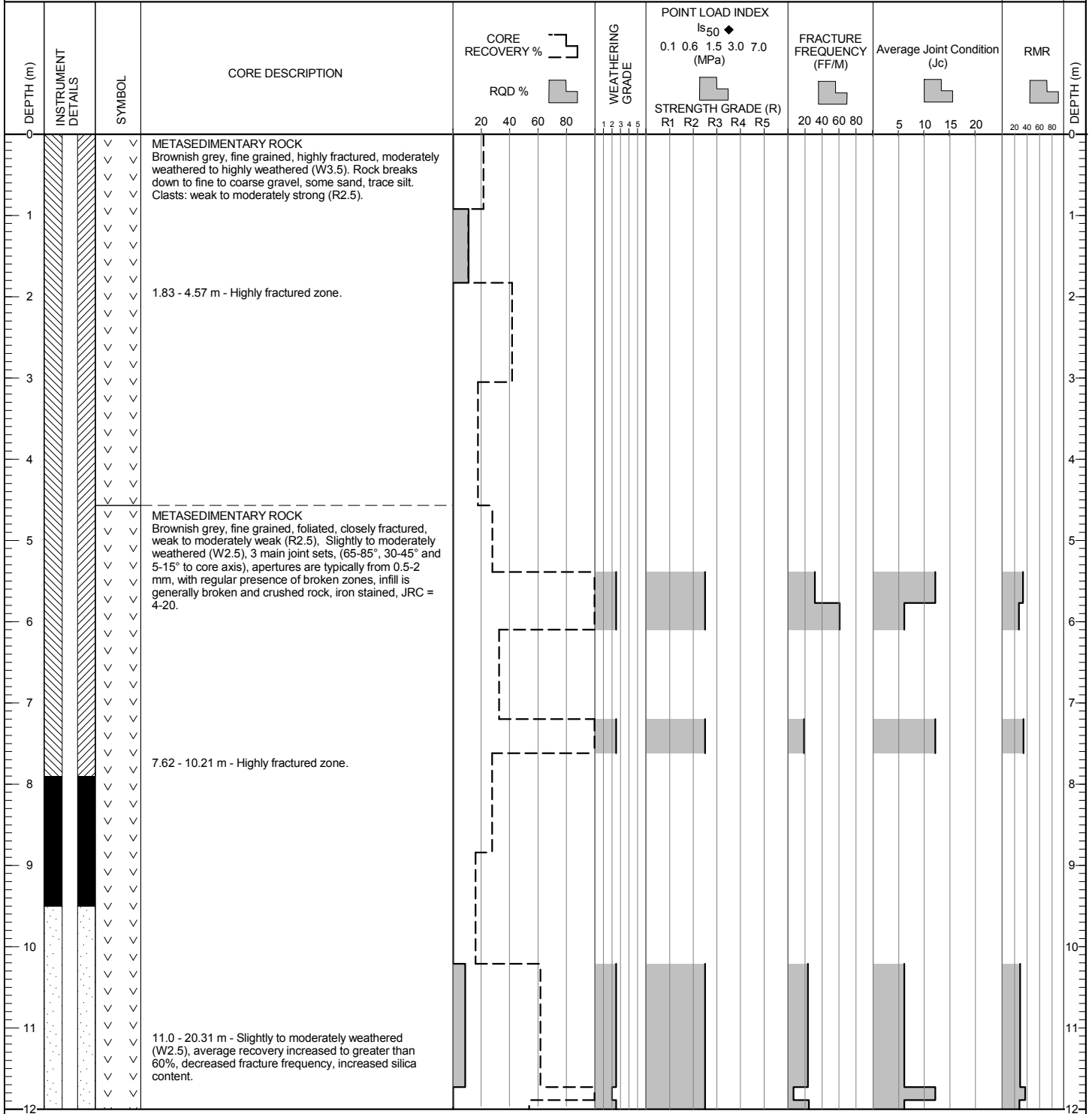


EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,205.8E - 7,102,088.1N
 GROUND ELEVATION (m) : 1,139.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 6.1

START DATE : 01 Jul 11
 FINISH DATE : 01 Jul 11
 FINAL DEPTH (m) : 30.2
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



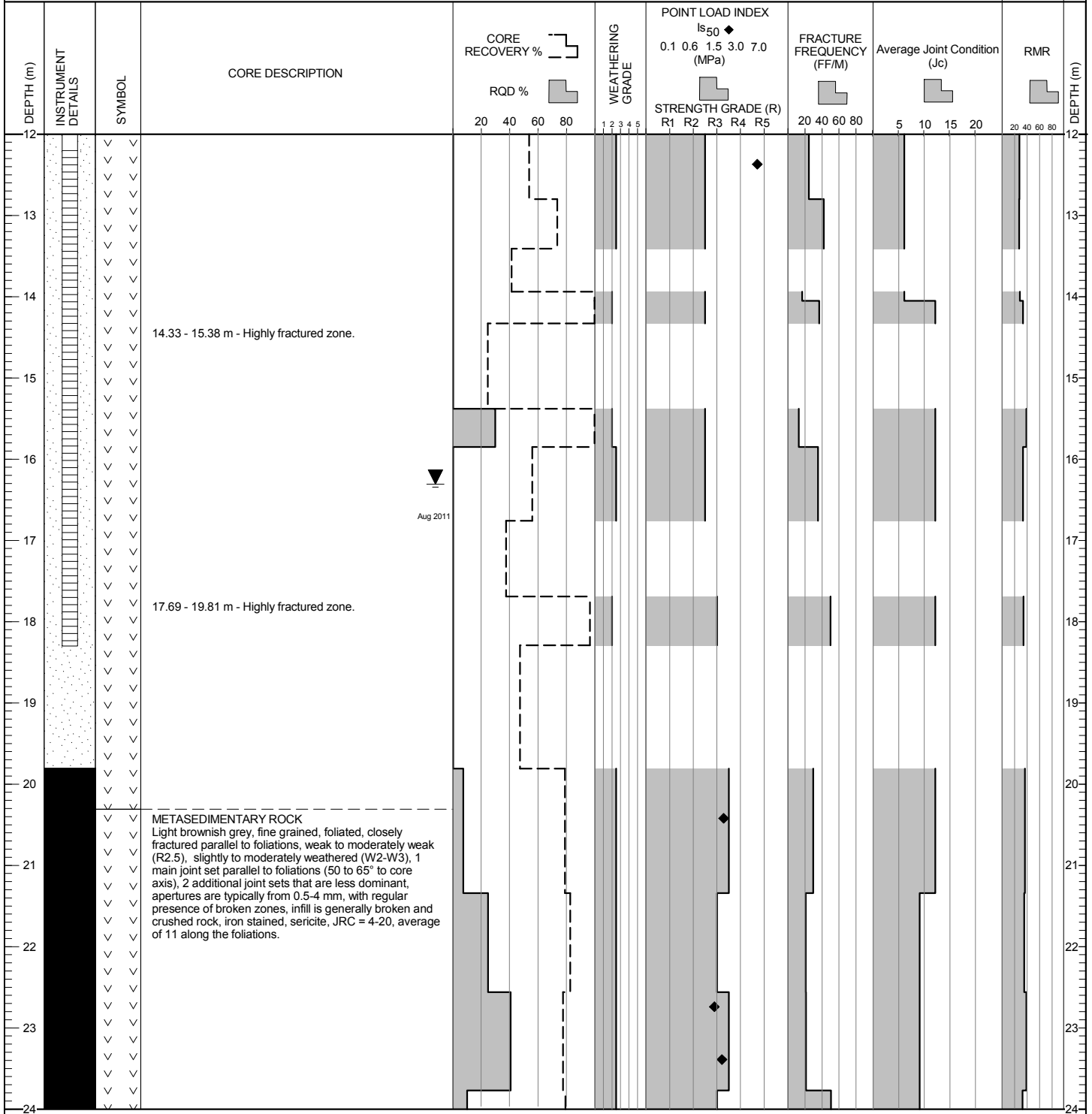
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,205.8E - 7,102,088.1N
 GROUND ELEVATION (m) : 1,139.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 6.1

START DATE : 01 Jul 11
 FINISH DATE : 01 Jul 11
 FINAL DEPTH (m) : 30.2
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



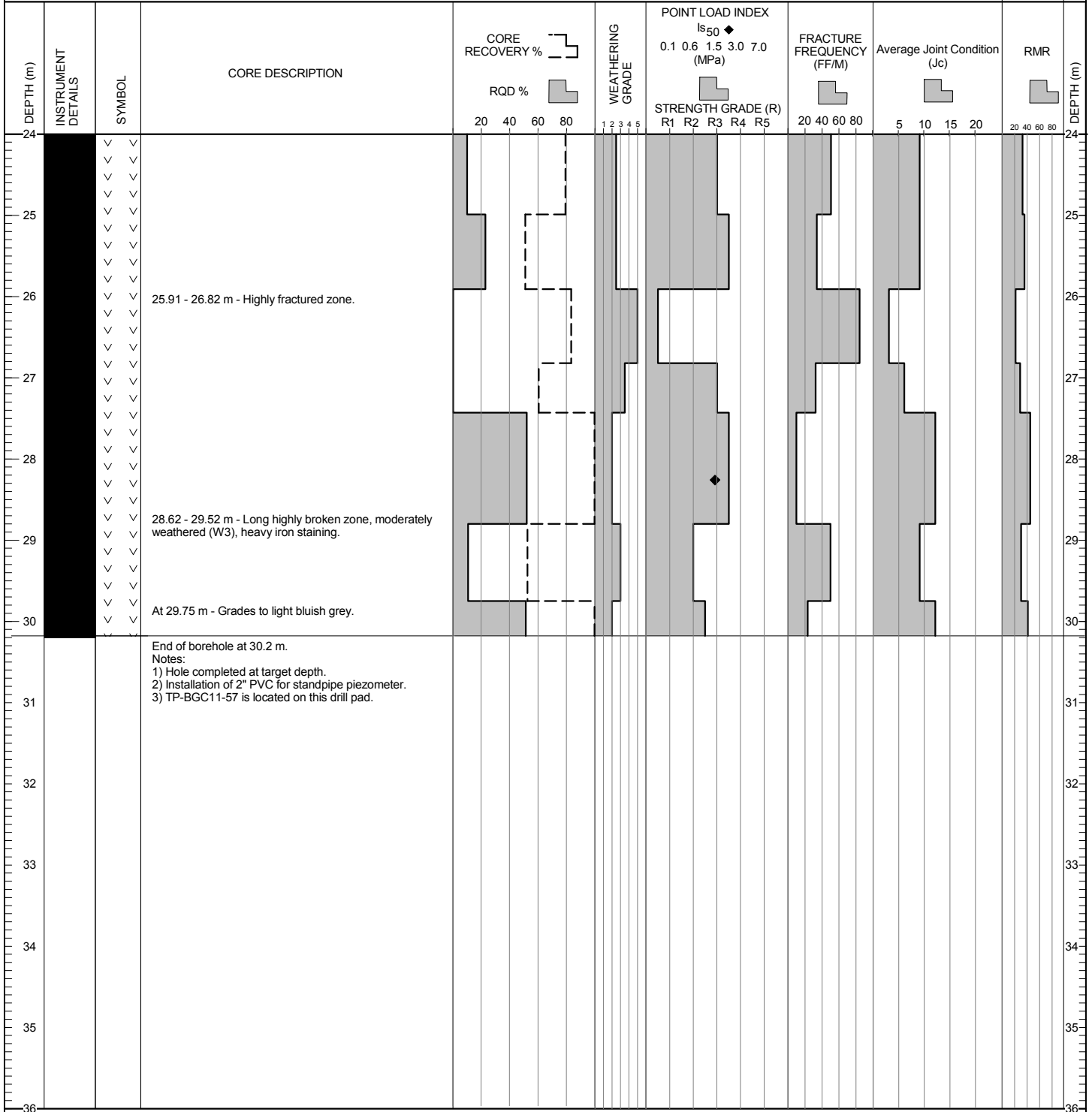
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,205.8E - 7,102,088.1N
 GROUND ELEVATION (m) : 1,139.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 6.1

START DATE : 01 Jul 11
 FINISH DATE : 01 Jul 11
 FINAL DEPTH (m) : 30.2
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-27

LOCATION : ANN GULCH

CO-ORDINATES (m): 460,043.5E - 7,102,232.5N
 GROUND ELEVATION (m) : 1,100.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 0.00

START DATE : 04 Jul 11
 FINISH DATE : 05 Jul 11
 FINAL DEPTH (m) : 26.5
 DEPTH TO TOP OF ROCK (m) : 2.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa							
							★ % FINES	● SPT (blows/300mm)	MOISTURE CONTENT & SPT N					
							40	80	120	160				
							RECOVERY				W _p %			
							20	40	60	80	20	40	60	80
0				GRAVEL (GM) Some silt, trace sand, trace cobbles, well graded, maximum particle size = 200 mm, flat and elongated, angular, iron staining. [HIGHLY WEATHERED METASEDIMENTARY ROCK]										
2.1				Rock encountered at 2.10 m depth. Refer to rock log.										
1														
2														
3														
4														
5														
6														
7														
8														
9														
10														
11														
12														

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/20/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 460,043.5E - 7,102,232.5N
 GROUND ELEVATION (m) : 1,100.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 0

START DATE : 04 Jul 11
 FINISH DATE : 05 Jul 11
 FINAL DEPTH (m) : 26.5
 DEPTH TO TOP OF ROCK (m) : 2.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

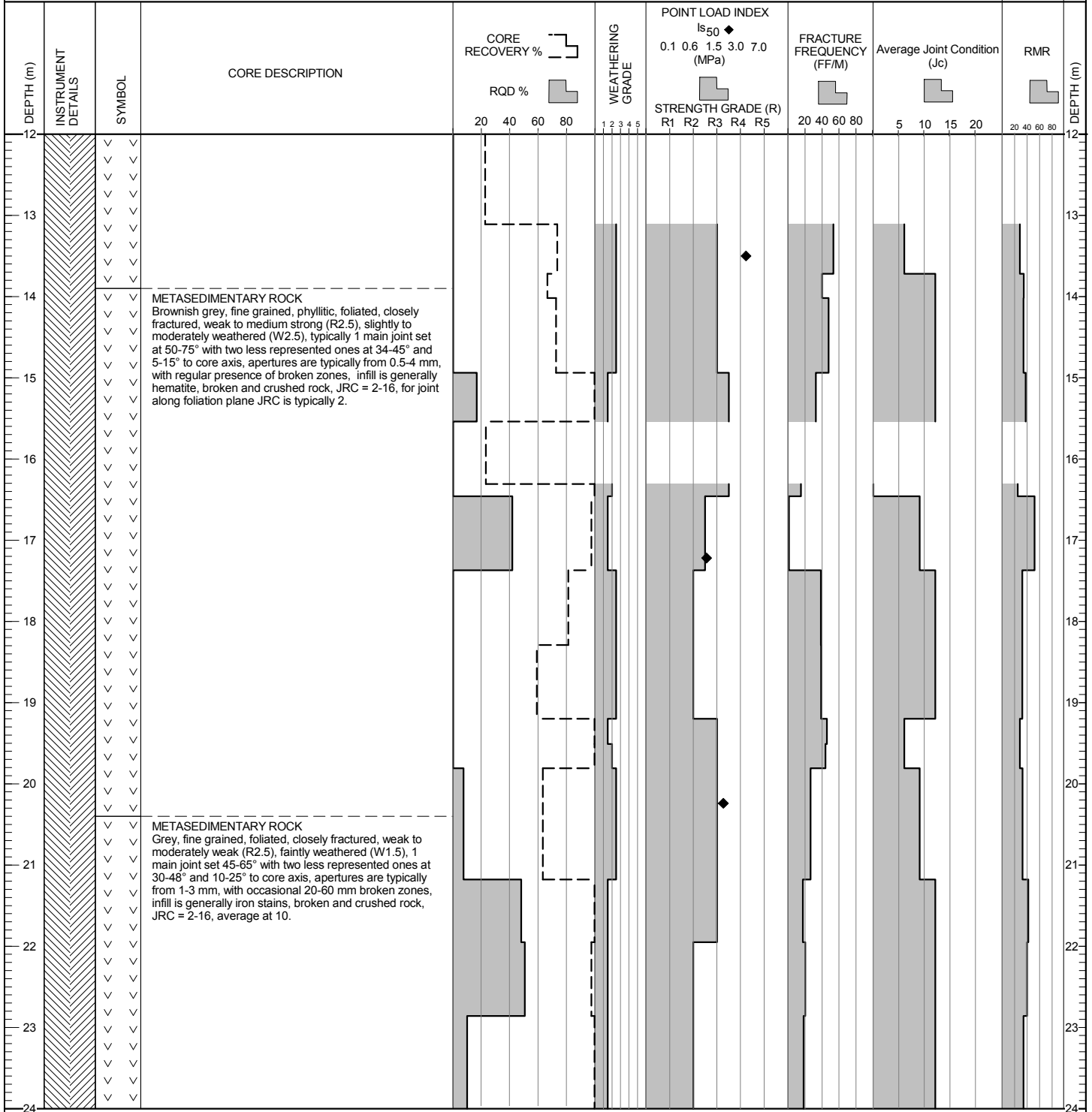
DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
0															0
0 to 2.10 m			0 to 2.10 m - See BH-BGC11-27 soil log.												
2.13 - 7.62 m		Metasedimentary Rock	METASEDIMENTARY ROCK Brownish grey, fine grained, highly fractured, very weak to strong (R1 to R3), moderately weathered to highly weathered (W3.5). Rock breaks down to fine to coarse gravel, some sand, trace silt. 2.13 - 7.62 m - Highly fractured zone, very low recovery.												
9.14 - 13.11 m			9.14 - 13.11 m - Highly fractured zone.												

(CONTINUED ON NEXT PAGE)

CO-ORDINATES (m) 460,043.5E - 7,102,232.5N
 GROUND ELEVATION (m) : 1,100.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 0

START DATE : 04 Jul 11
 FINISH DATE : 05 Jul 11
 FINAL DEPTH (m) : 26.5
 DEPTH TO TOP OF ROCK (m) : 2.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



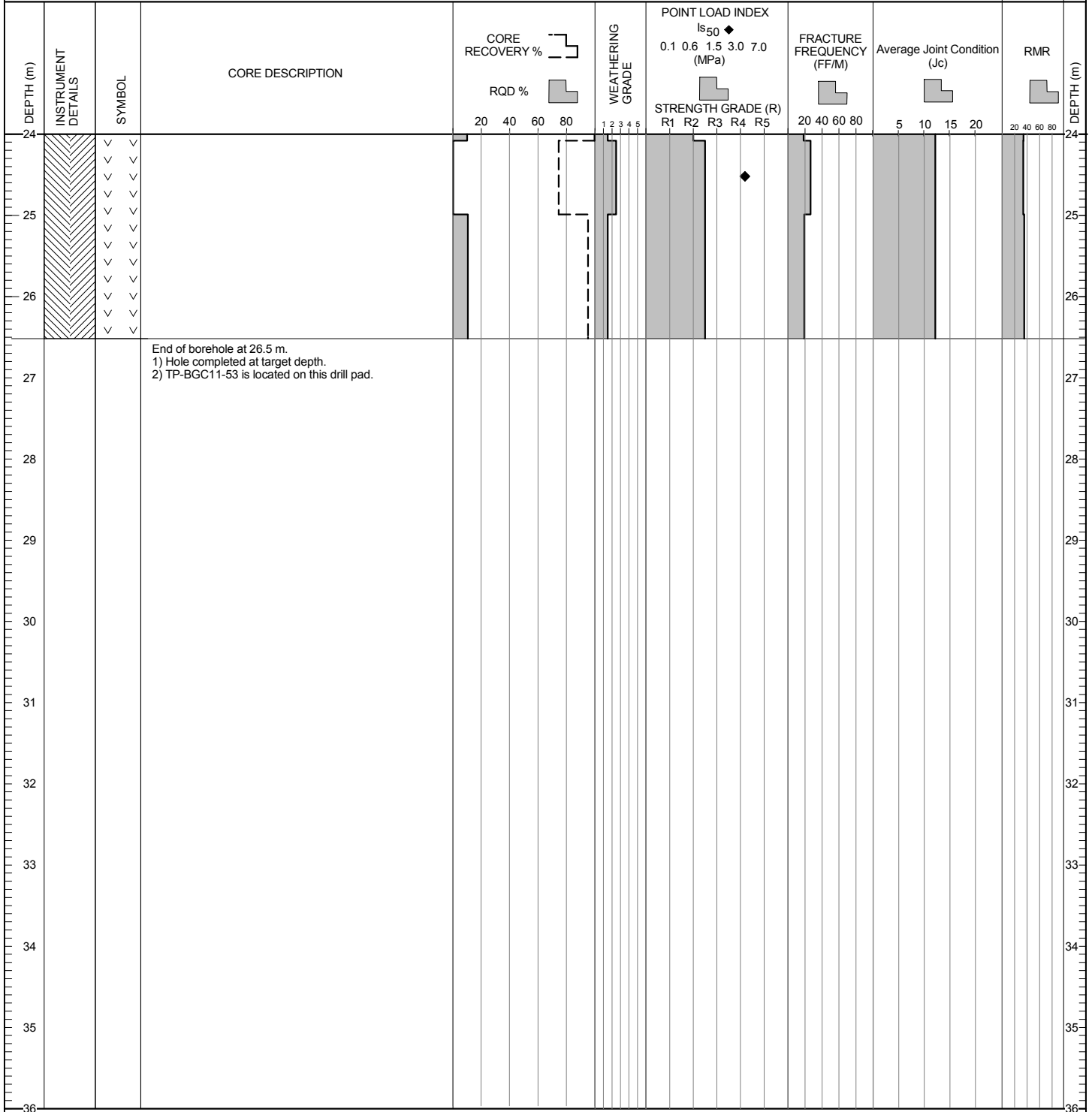
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,043.5E - 7,102,232.5N
 GROUND ELEVATION (m) : 1,100.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 0

START DATE : 04 Jul 11
 FINISH DATE : 05 Jul 11
 FINAL DEPTH (m) : 26.5
 DEPTH TO TOP OF ROCK (m) : 2.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

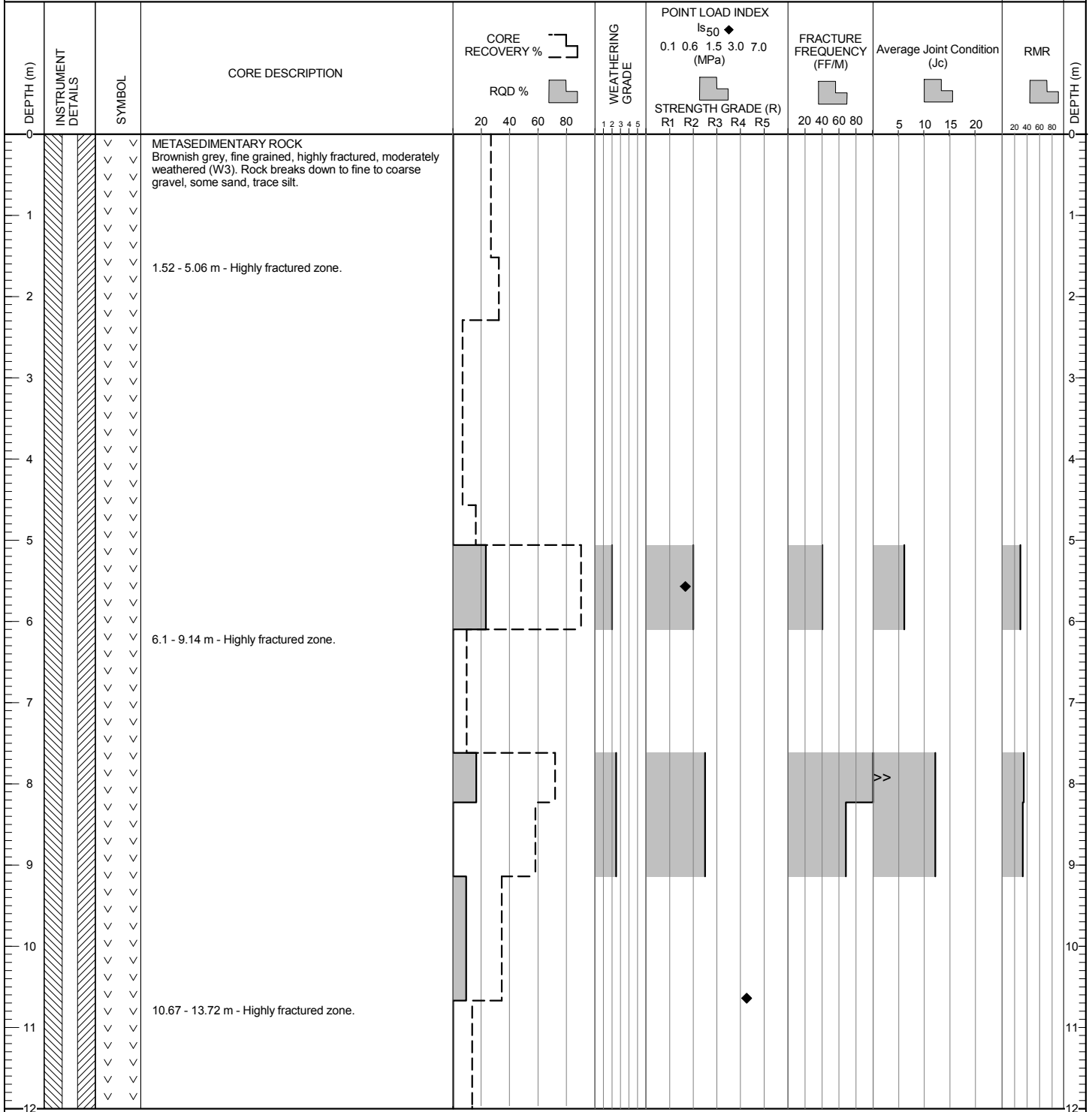
DRILL HOLE # BH-BGC11-28

LOCATION : ANN GULCH

CO-ORDINATES (m) 459,926.5E - 7,101,702.7N
 GROUND ELEVATION (m) : 1,010.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 7.62

START DATE : 04 Jul 11
 FINISH DATE : 06 Jul 11
 FINAL DEPTH (m) : 40.8
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



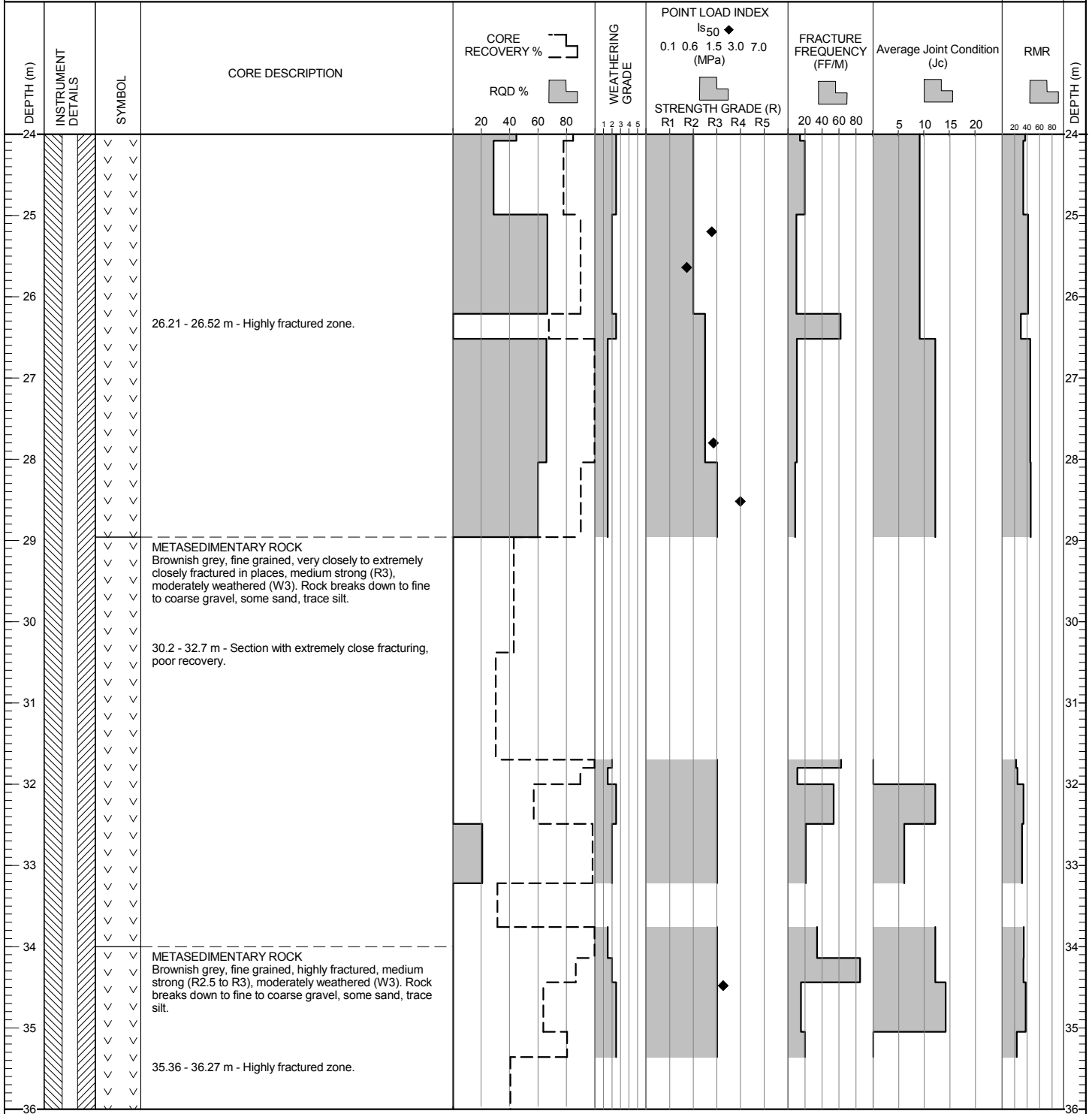
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,926.5E - 7,101,702.7N
 GROUND ELEVATION (m) : 1,010.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 7.62

START DATE : 04 Jul 11
 FINISH DATE : 06 Jul 11
 FINAL DEPTH (m) : 40.8
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



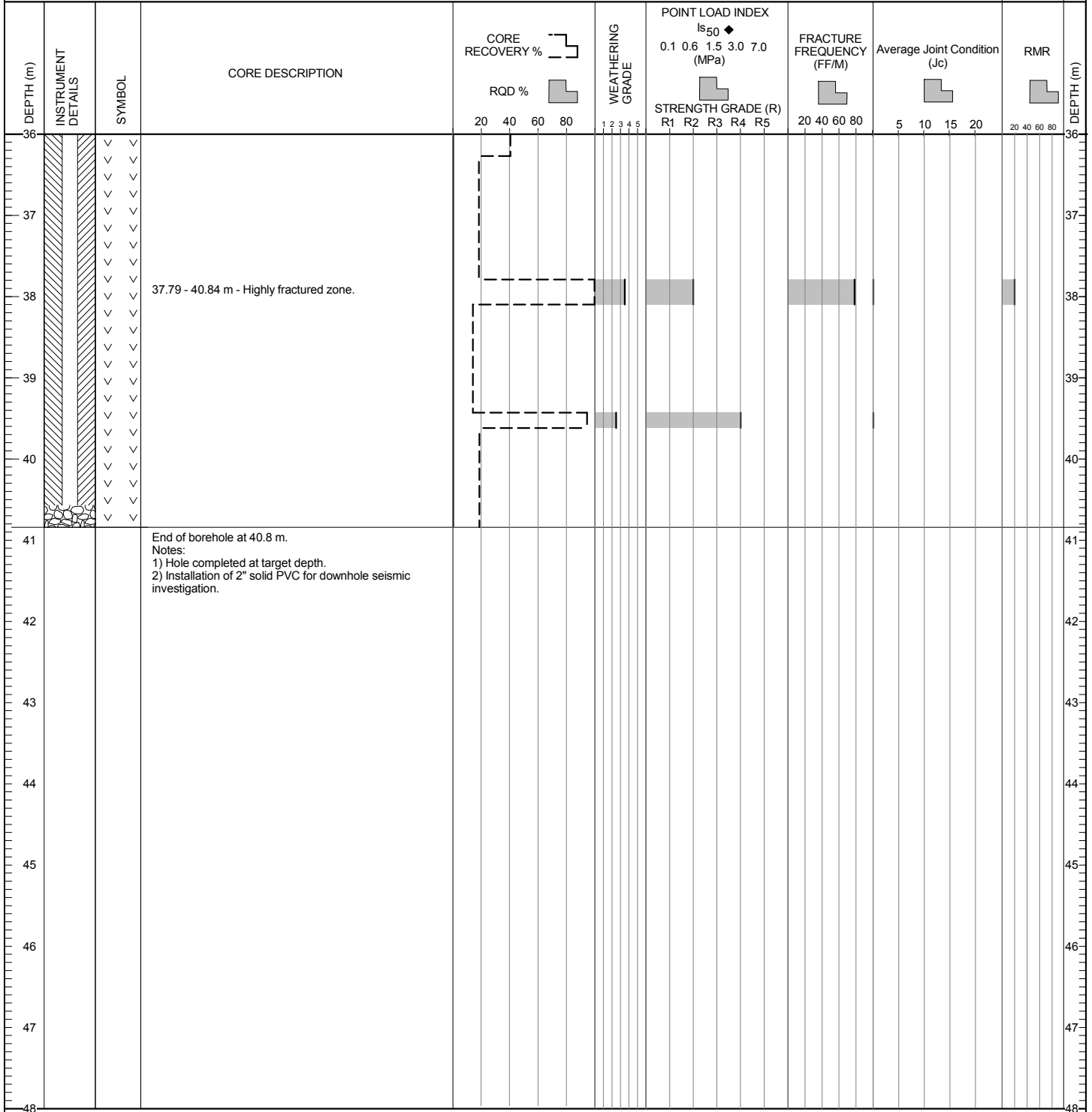
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,926.5E - 7,101,702.7N
 GROUND ELEVATION (m) : 1,010.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 7.62

START DATE : 04 Jul 11
 FINISH DATE : 06 Jul 11
 FINAL DEPTH (m) : 40.8
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

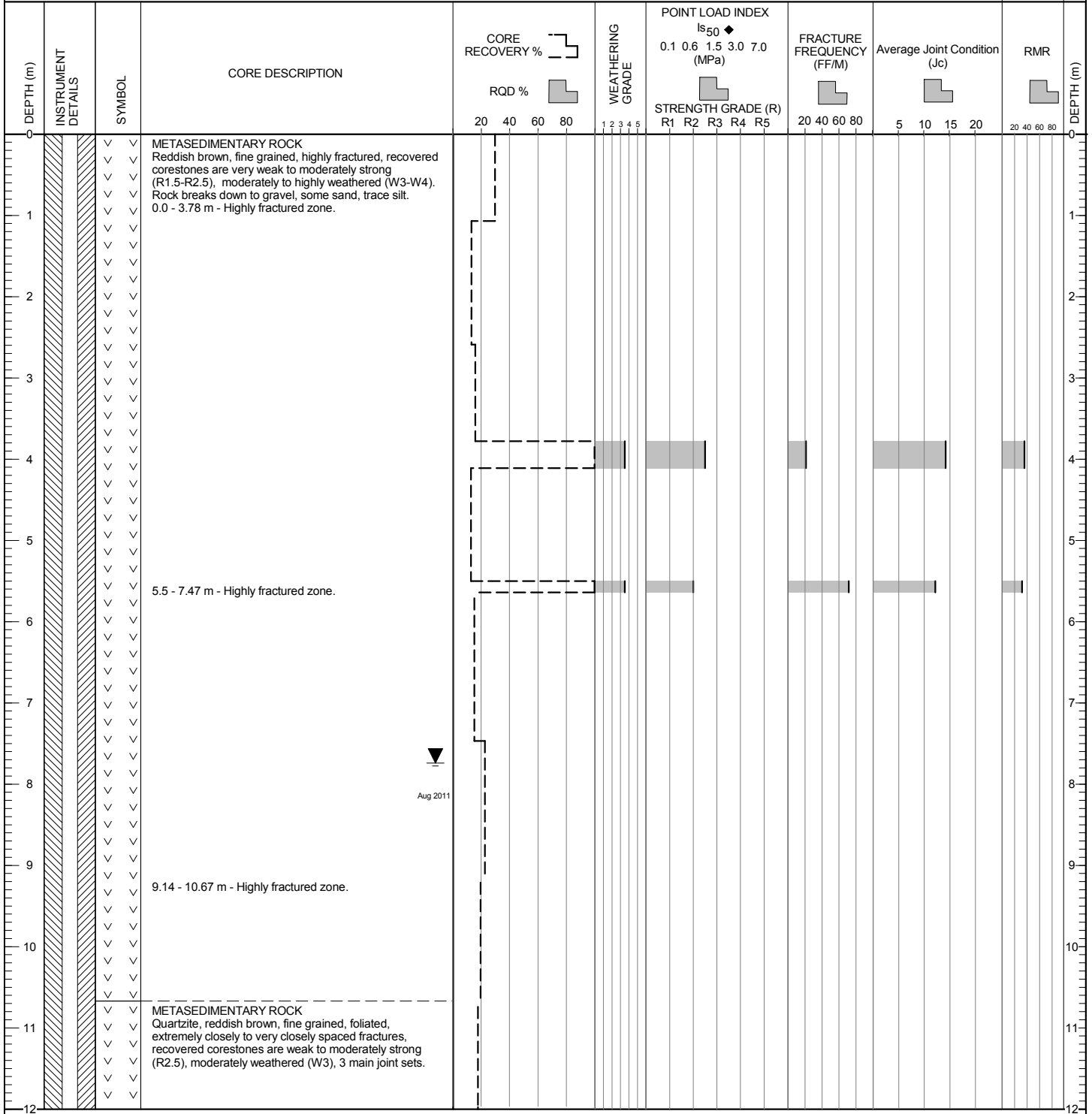
DRILL HOLE # BH-BGC11-29

LOCATION : ANN GULCH

CO-ORDINATES (m) 459,977.8E - 7,101,802.1N
 GROUND ELEVATION (m) : 1,045.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) :-90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m):

START DATE : 08 Jul 11
 FINISH DATE : 10 Jul 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



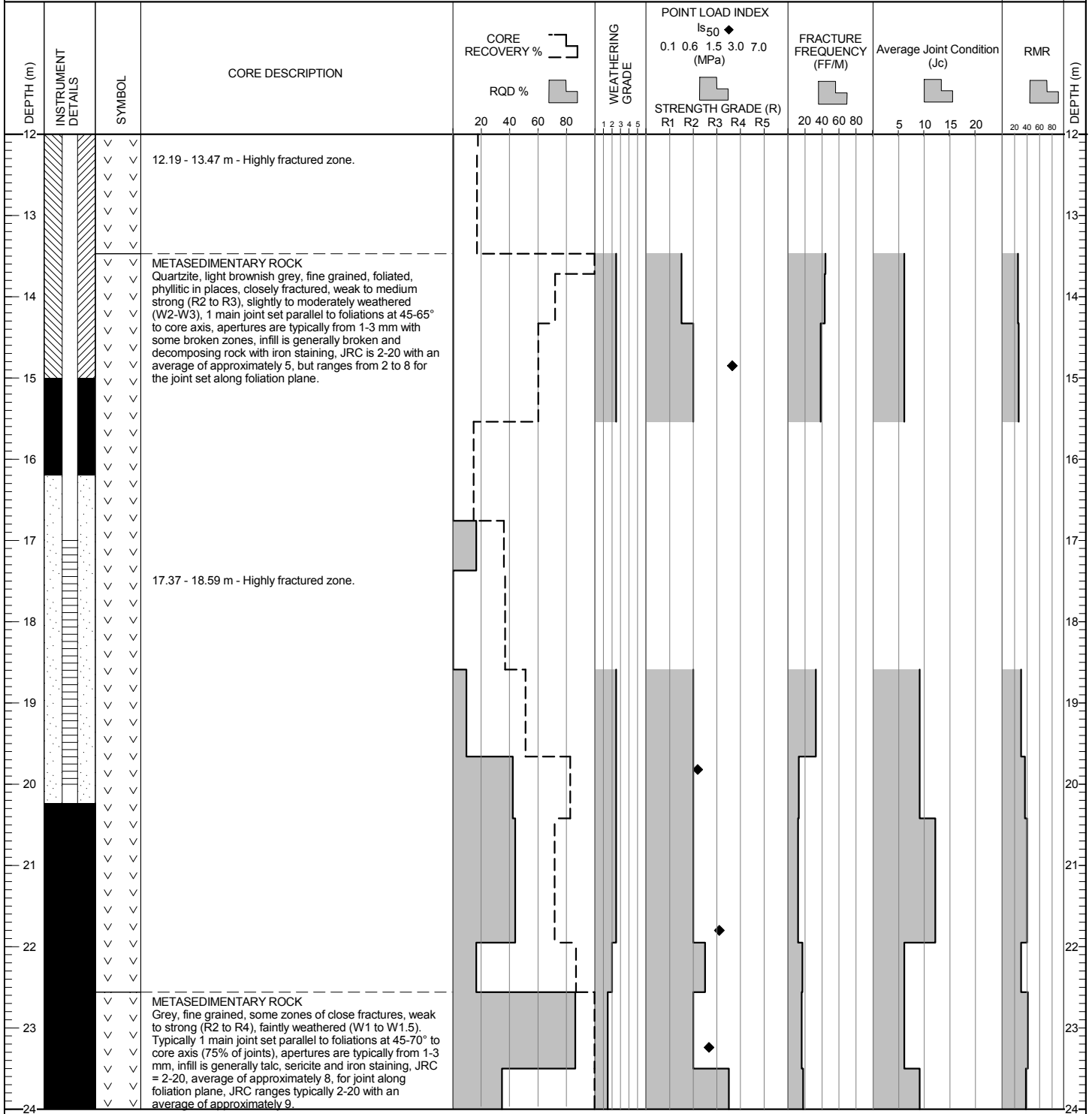
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,977.8E - 7,101,802.1N
 GROUND ELEVATION (m) : 1,045.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m):

START DATE : 08 Jul 11
 FINISH DATE : 10 Jul 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



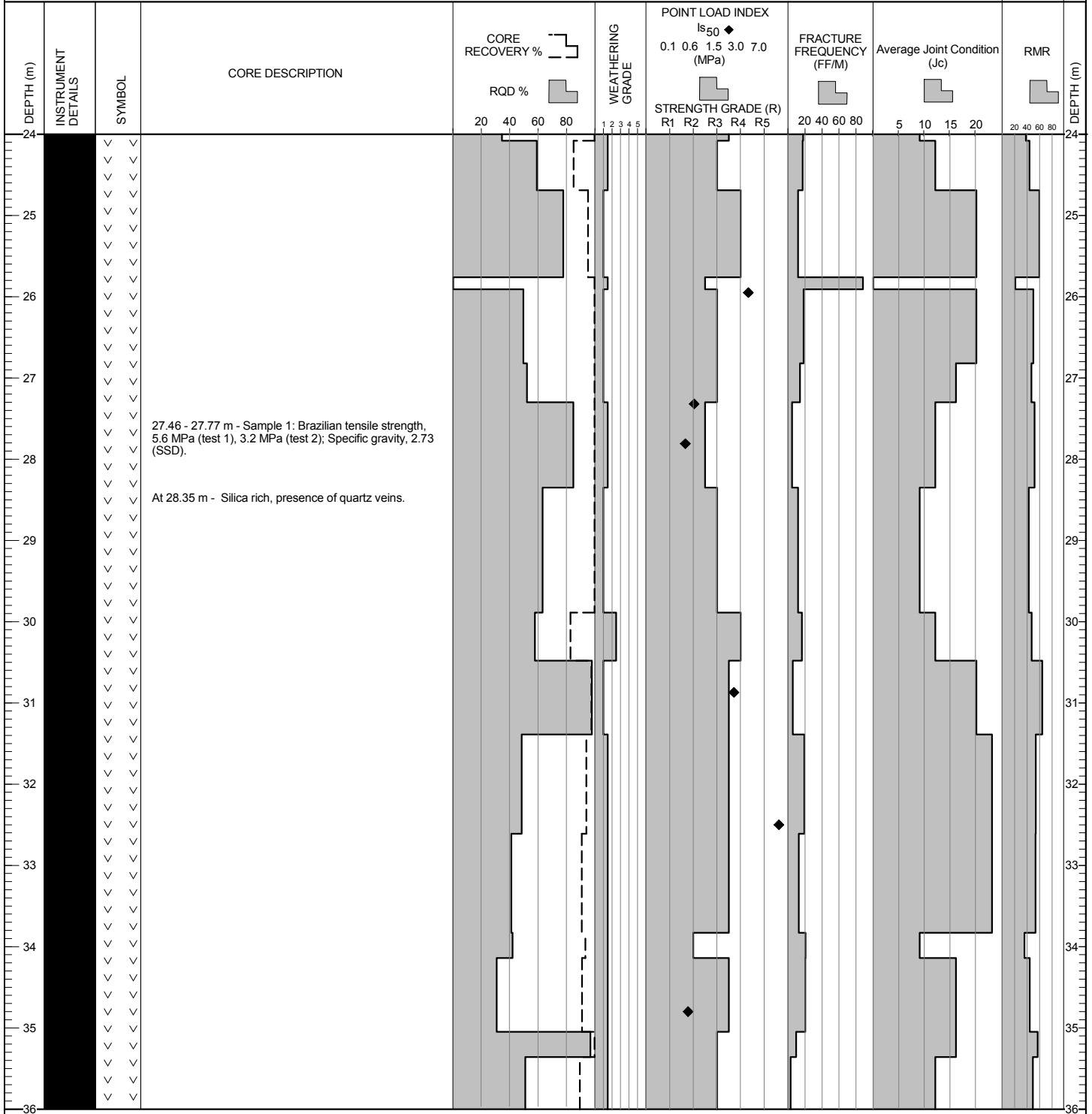
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,977.8E - 7,101,802.1N
 GROUND ELEVATION (m) : 1,045.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 08 Jul 11
 FINISH DATE : 10 Jul 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

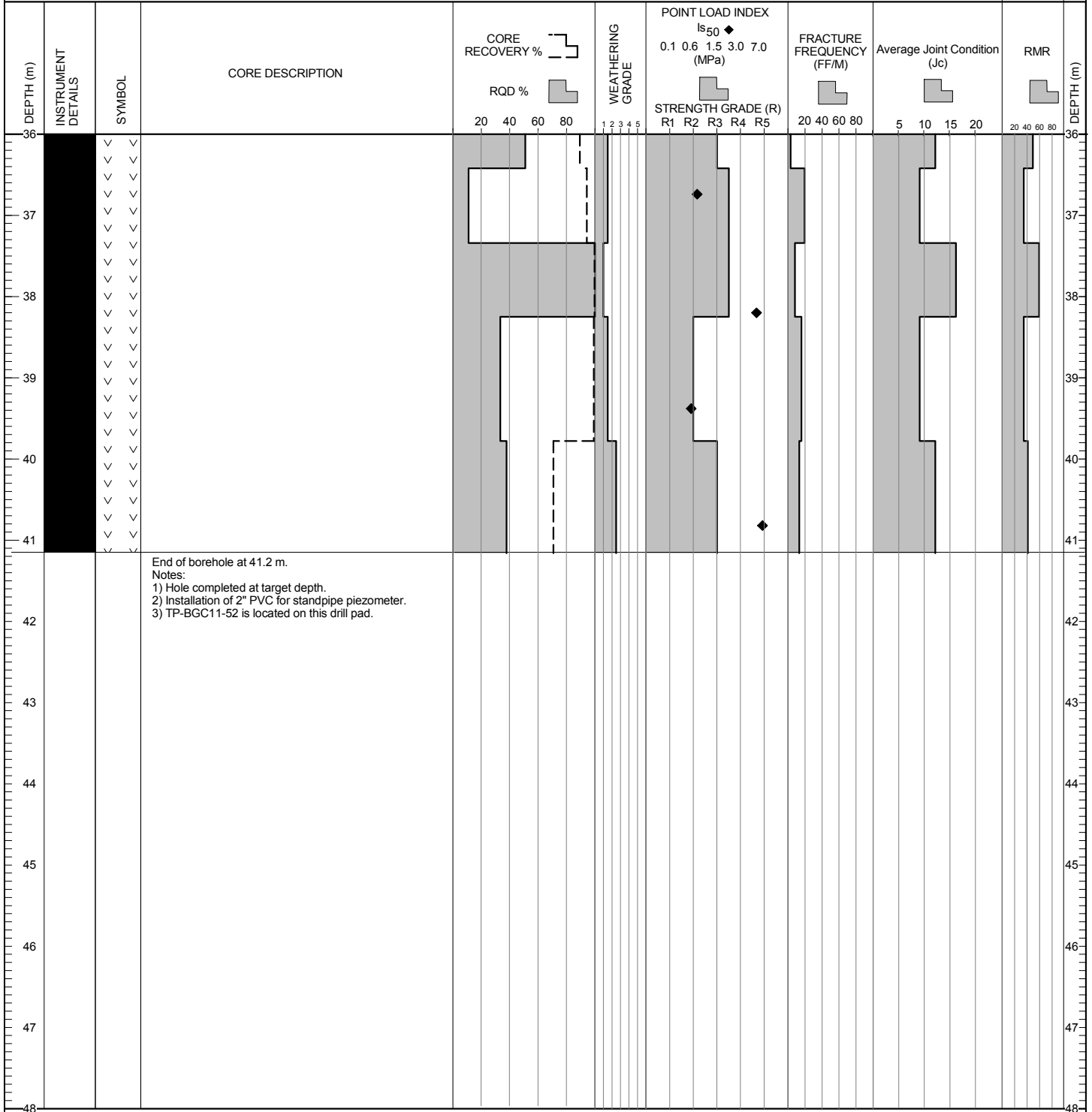
DRILL HOLE # BH-BGC11-29

LOCATION : ANN GULCH

CO-ORDINATES (m) 459,977.8E - 7,101,802.1N
 GROUND ELEVATION (m) : 1,045.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 08 Jul 11
 FINISH DATE : 10 Jul 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 0.0
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-30

LOCATION : MIDDLE REACH DUBLIN GULCH

CO-ORDINATES (m): 460,122.0E - 7,101,567.8N
 GROUND ELEVATION (m) : 952.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 9.14

START DATE : 07 Jul 11
 FINISH DATE : 08 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa							
							★ % FINES		● SPT (blows/300mm)					
							RECOVERY		MOISTURE CONTENT & SPT N					
							20	40	60	80	W _p %	W ₅₀ %	W ₁₀ %	
0				GRAVEL (GM) Some cobbles, some sand, some silt, trace boulders, maximum particle size = 300 mm, subangular to subrounded, elongated to equidimensional, weak to medium strong (R2 to R3) clasts, light to medium brown, moist, non-plastic fines. Sampled at surface, not from core barrel. [COLLUVIUM]										
1														
1.5				Rock encountered at 1.52 m depth. Refer to rock log.										
2														
3														
4														
5														
6														
7														
8														
9														
10														
11														
12														

EGR(SOIL) EGP_SOIL_GDL BGC.GDT 12/01/12

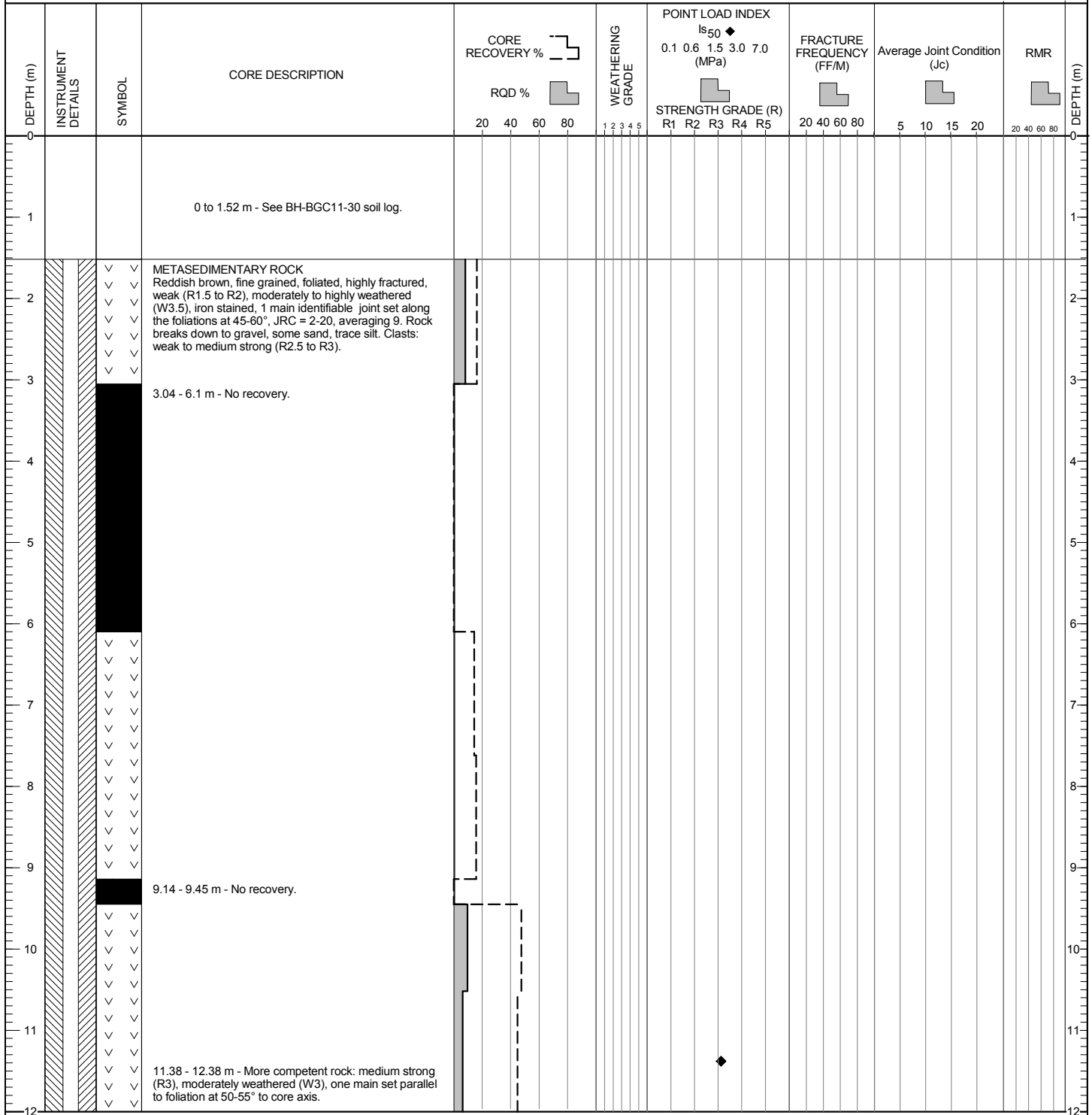


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 460,122.0E - 7,101,567.8N
 GROUND ELEVATION (m) : 952.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 07 Jul 11
 FINISH DATE : 08 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

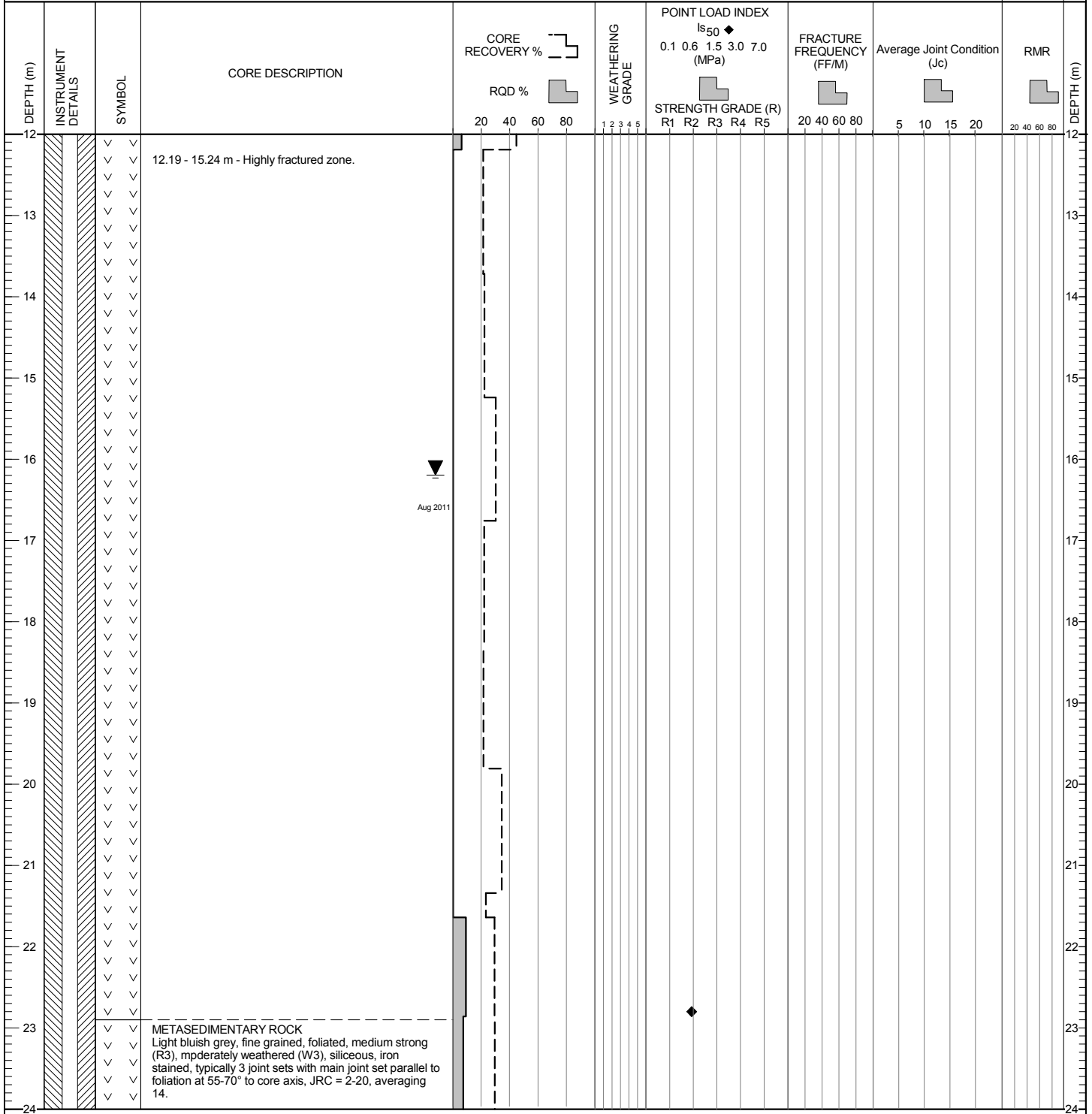


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CO-ORDINATES (m) 460,122.0E - 7,101,567.8N
 GROUND ELEVATION (m) : 952.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 07 Jul 11
 FINISH DATE : 08 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



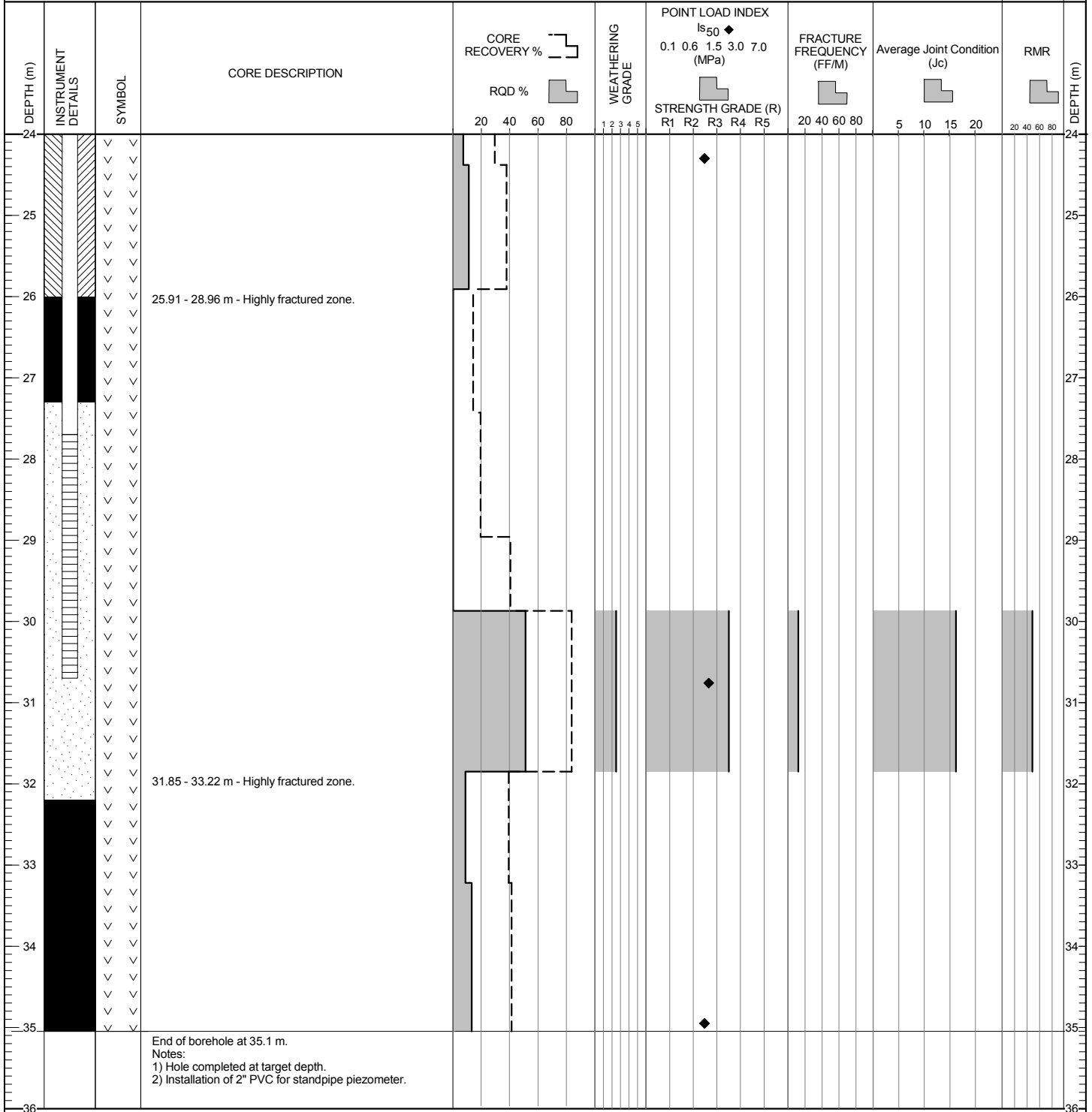
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,122.0E - 7,101,567.8N
 GROUND ELEVATION (m) : 952.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 07 Jul 11
 FINISH DATE : 08 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

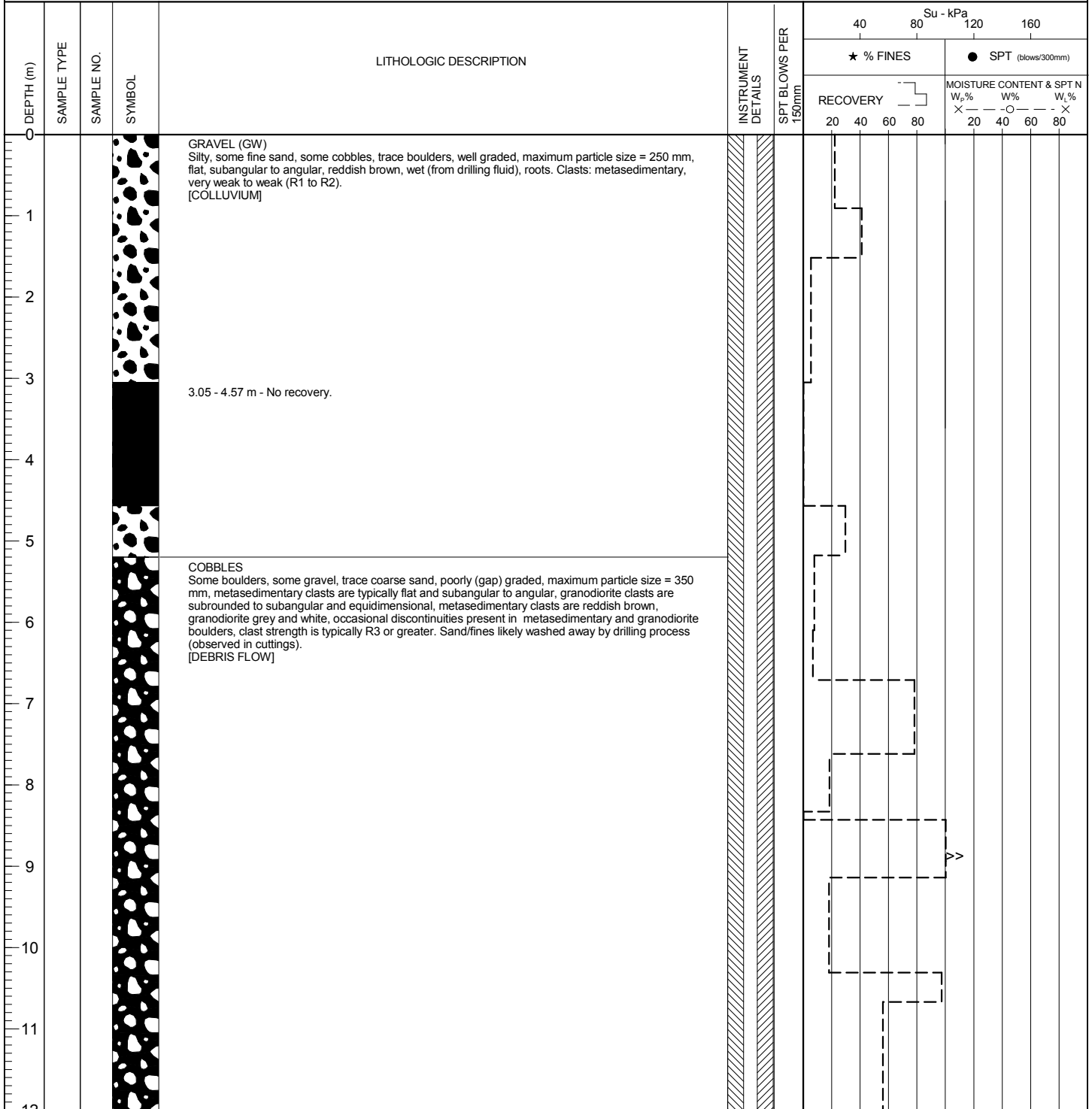
DRILL HOLE # BH-BGC11-31

LOCATION : MIDDLE REACH DUBLIN GULCH

CO-ORDINATES (m): 459,850.9E - 7,101,483.0N
 GROUND ELEVATION (m) : 917.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m) : 6.10

START DATE : 10 Jul 11
 FINISH DATE : 11 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 16.8
 LOGGED BY : EB/SD/KH
 REVIEWED BY : PQ/DW



(Continued on next page)

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/9/12

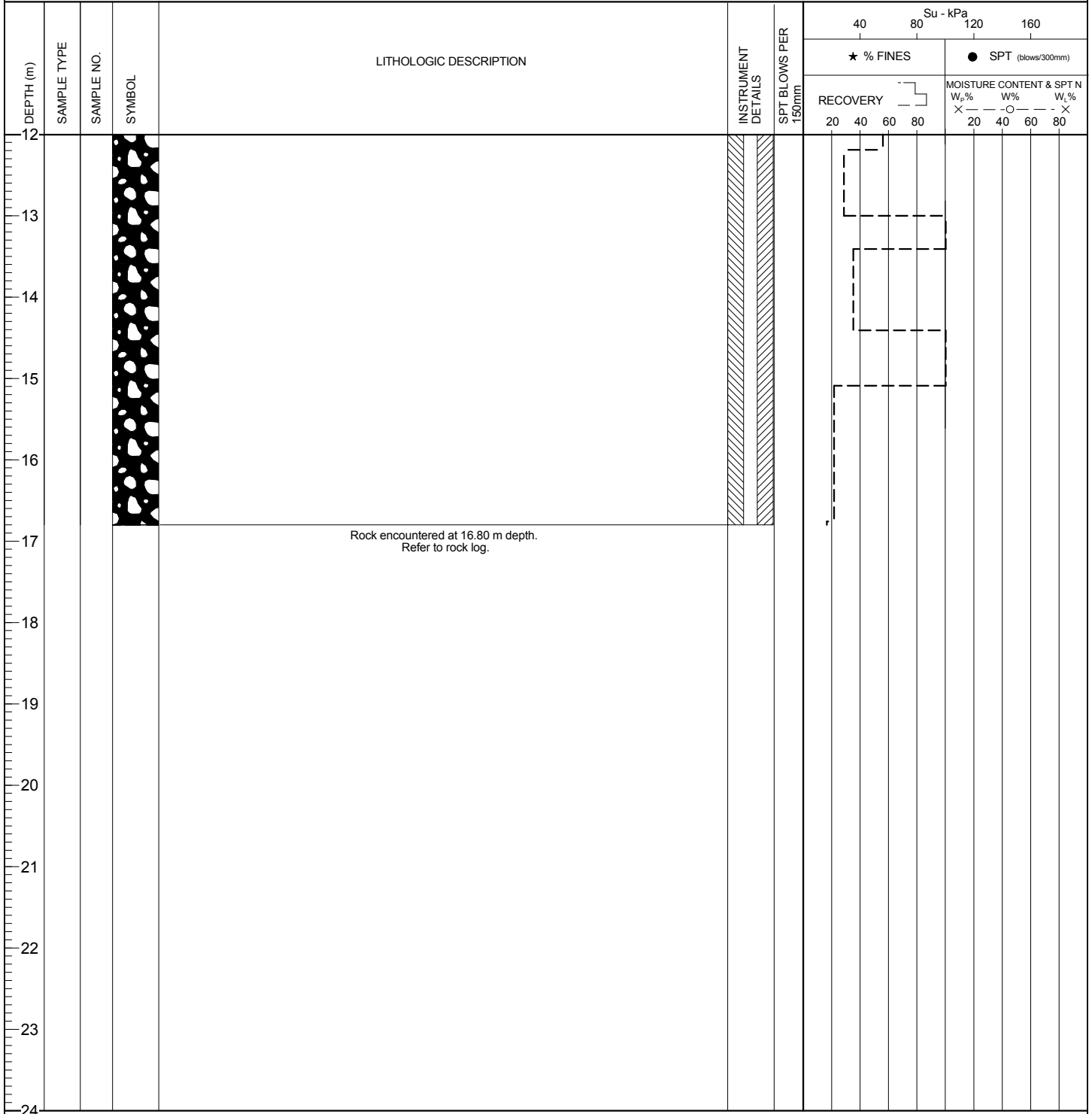


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m): 459,850.9E - 7,101,483.0N
 GROUND ELEVATION (m) : 917.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 6.10

START DATE : 10 Jul 11
 FINISH DATE : 11 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 16.8
 LOGGED BY : EB/SD/KH
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/9/12

CO-ORDINATES (m) 459,850.9E - 7,101,483.0N
 GROUND ELEVATION (m) : 917.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 6.1

START DATE : 10 Jul 11
 FINISH DATE : 11 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 16.8
 LOGGED BY : EB/SD/KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)				RMR	DEPTH (m)													
				CORE	RQD %		Is ₅₀ (MPa)																								
				20	40	60	80	1	2	3	4	5	R1	R2	R3	R4	R5	20	40	60	80	5	10	15	20	20	40	60	80		
12			0 to 16.80 m - See BH-BGC11-31 soil log.																												
17		∨ ∨	METASEDIMENTARY ROCK Phyllite, light bluish grey with iron oxide staining, very weak to weak (R1-R2), highly weathered (W4), foliated/bedded texture, highly fractured core recovery, occasional random joints, generally parallel to bedding/foliation, infill is typically iron oxide staining, clay, broken and disintegrated rock, joints typically smooth and planar, typical fracture spacing <20 mm, much of recovery is coarse gravel-sized pieces.																												
18		∨ ∨																													
19		∨ ∨																													
20		∨ ∨	19.81 - 21.34 m - Highly fractured zone.																												
21		∨ ∨																													
22		∨ ∨																													
23		∨ ∨	22.86 - 30.48 m - Highly fractured zone.																												
24		∨ ∨																													

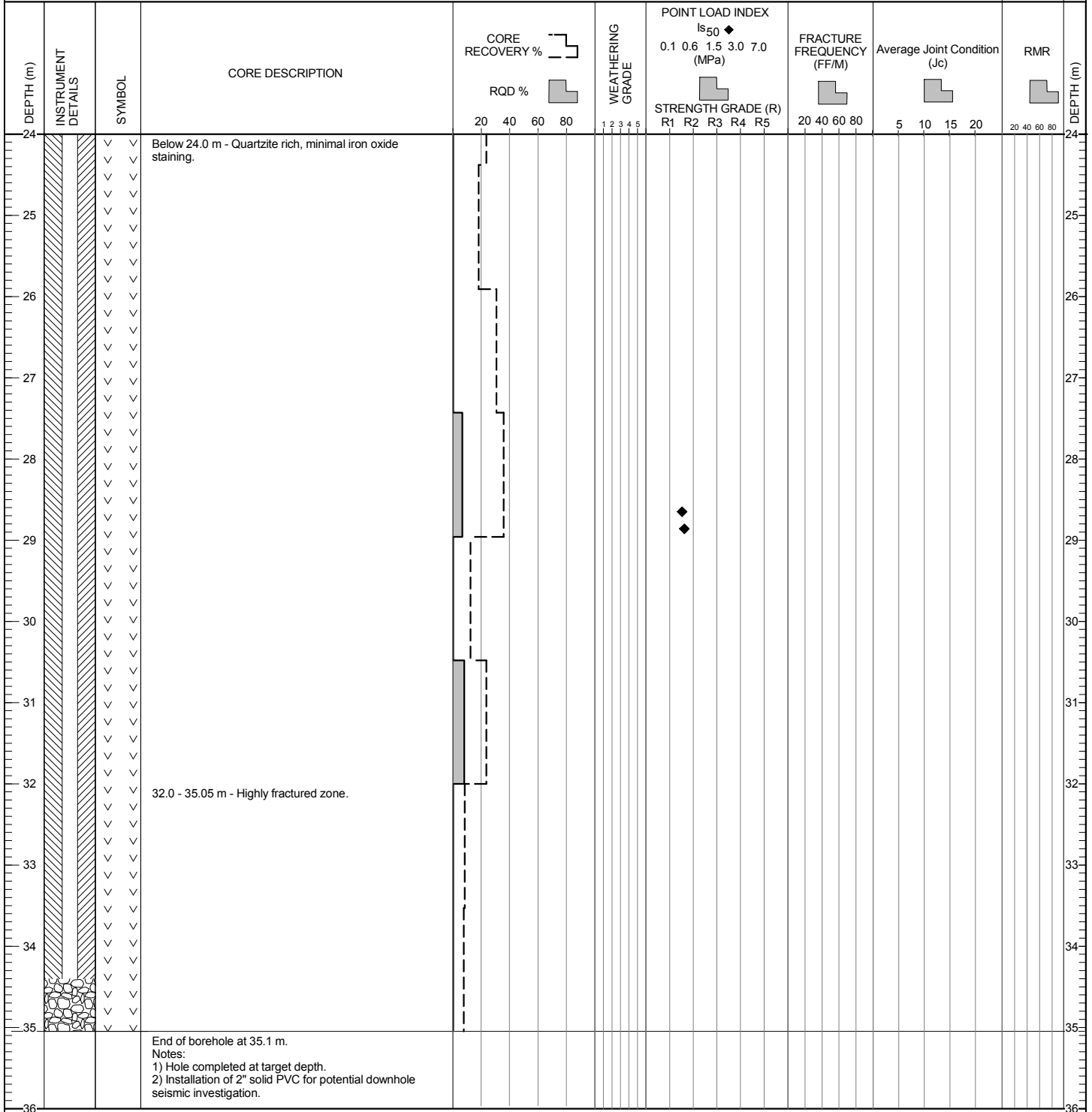
(CONTINUED ON NEXT PAGE)

ESP (ROCK) ESP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,850.9E - 7,101,483.0N
 GROUND ELEVATION (m) : 917.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 6.1

START DATE : 10 Jul 11
 FINISH DATE : 11 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 16.8
 LOGGED BY : EB/SD/KH
 REVIEWED BY : PQ/DW



(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,850.9E - 7,101,483.0N
 GROUND ELEVATION (m) : 917.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 6.1

START DATE : 10 Jul 11
 FINISH DATE : 11 Jul 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 16.8
 LOGGED BY : EB/SD/KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
36			3) TP-BGC11-145 is located on this drill pad.												36
37															37
38															38
39															39
40															40
41															41
42															42
43															43
44															44
45															45
46															46
47															47
48															48

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

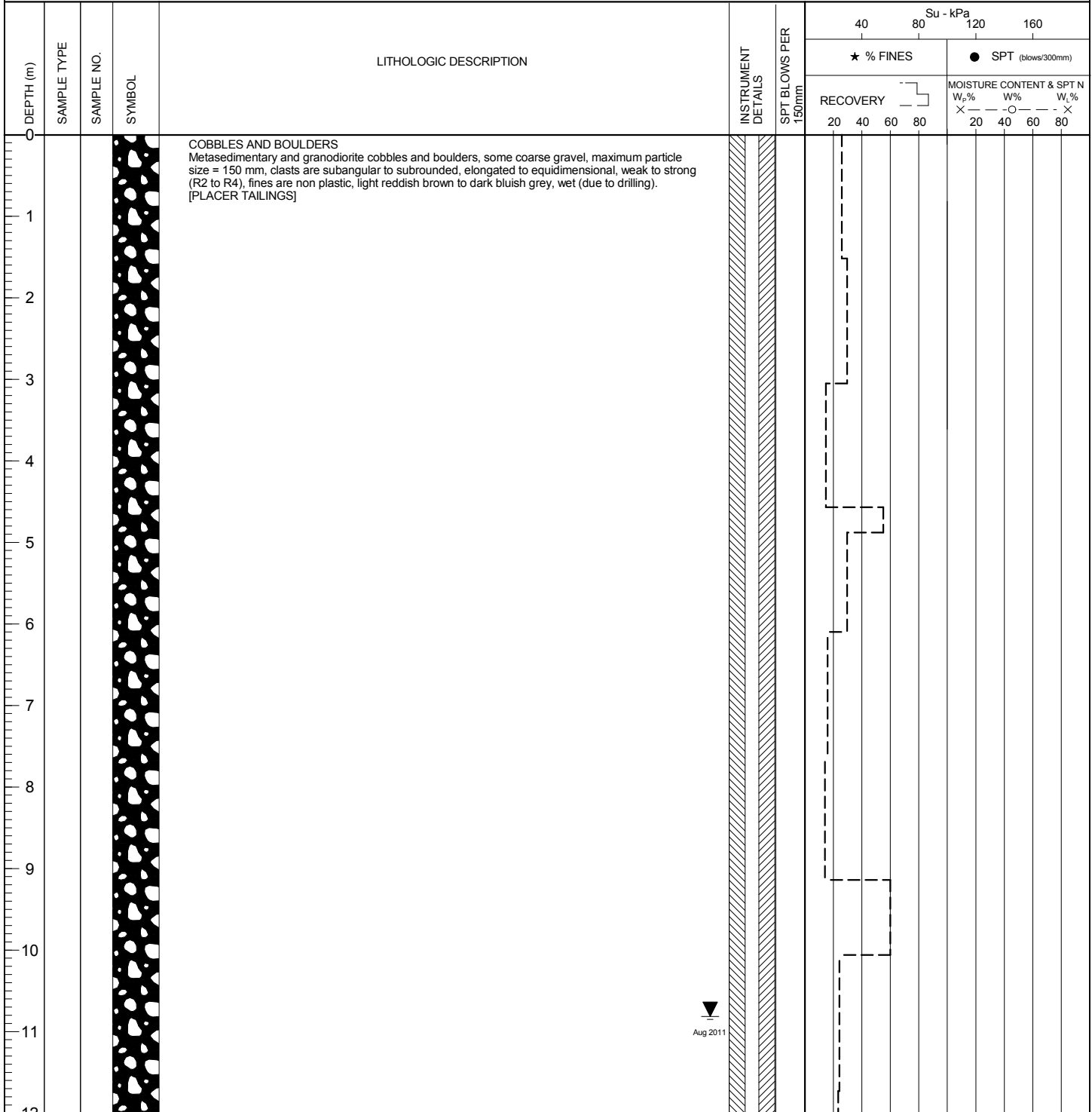
DRILL HOLE # BH-BGC11-32

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m): 458,745.2E - 7,100,934.4N
 GROUND ELEVATION (m) : 818.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 14.63

START DATE : 12 Jul 11
 FINISH DATE : 13 Jul 11
 FINAL DEPTH (m) : 24.4
 DEPTH TO TOP OF ROCK (m) : 19.8
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



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Aug 2011

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EGR\SOIL\ EGP_SOIL_GDL BGC.GDT 12/20/12

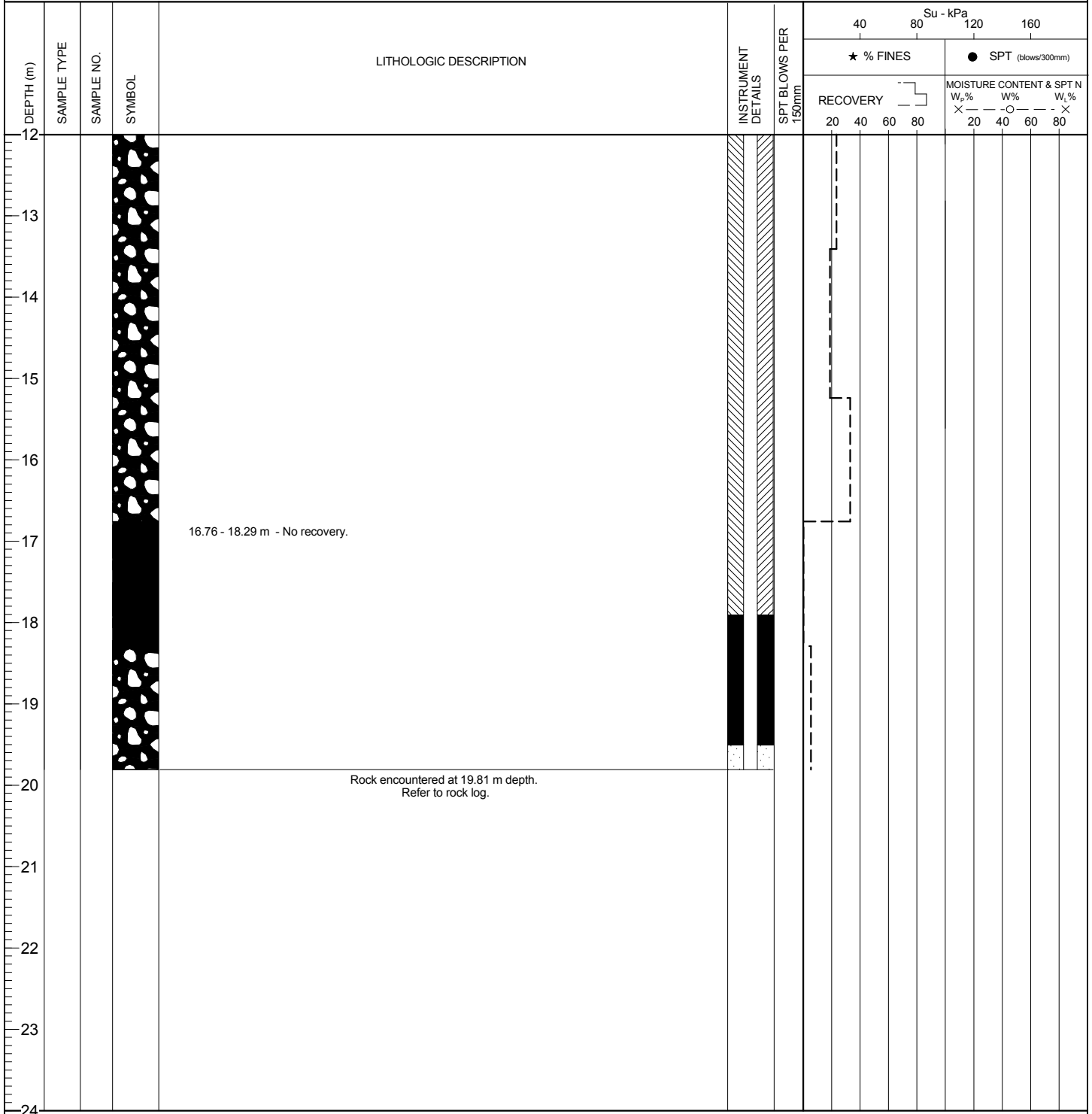


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m): 458,745.2E - 7,100,934.4N
 GROUND ELEVATION (m) : 818.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 14.63

START DATE : 12 Jul 11
 FINISH DATE : 13 Jul 11
 FINAL DEPTH (m) : 24.4
 DEPTH TO TOP OF ROCK (m) : 19.8
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

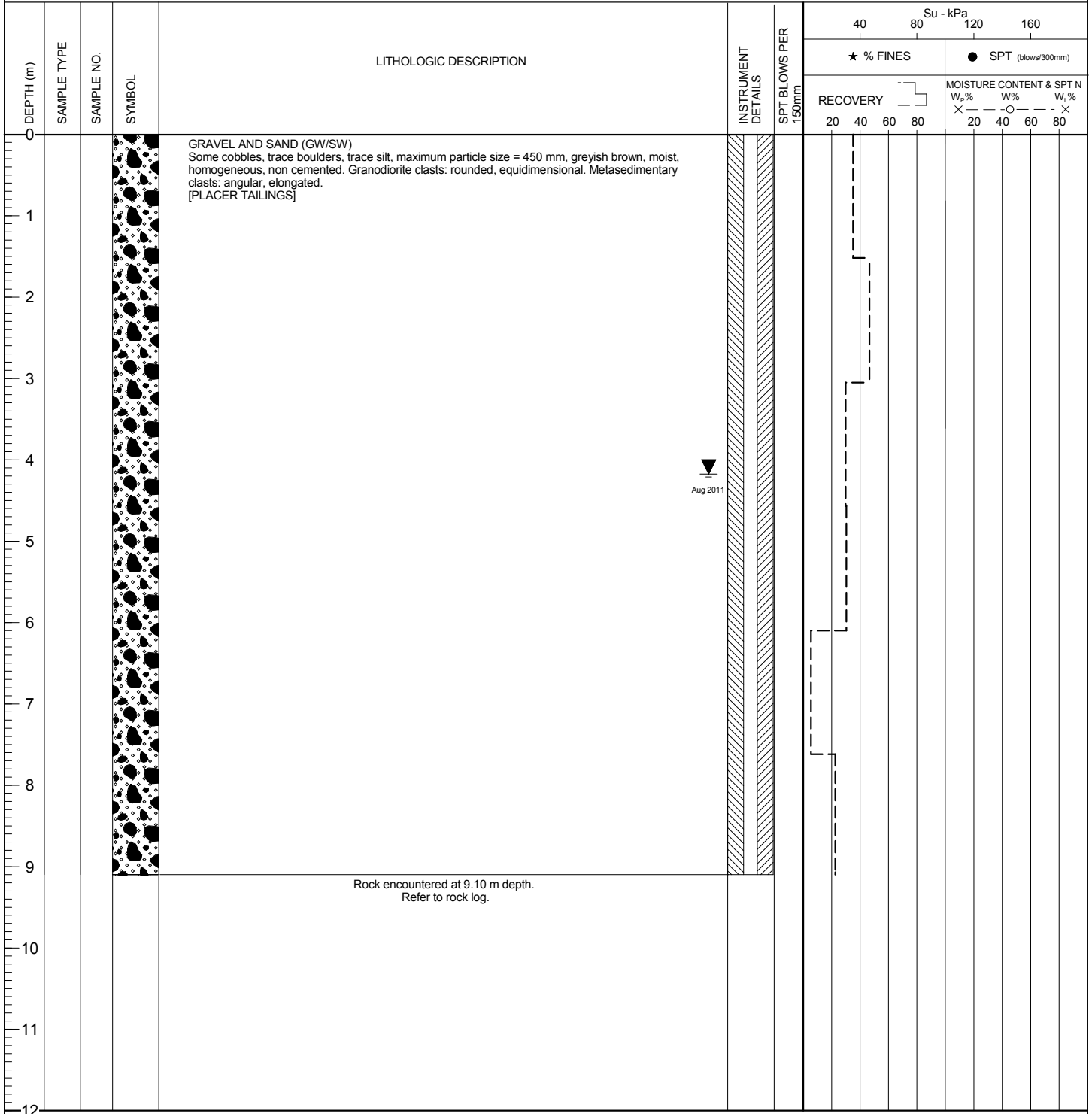
DRILL HOLE # BH-BGC11-33

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m): 459,115.7E - 7,100,983.6N
 GROUND ELEVATION (m) : 833.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 11.58

START DATE : 13 Jul 11
 FINISH DATE : 14 Jul 11
 FINAL DEPTH (m) : 41.4
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



Aug 2011

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/01/12

CO-ORDINATES (m) 459,115.7E - 7,100,983.6N
 GROUND ELEVATION (m) : 833.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 11.58

START DATE : 13 Jul 11
 FINISH DATE : 14 Jul 11
 FINAL DEPTH (m) : 41.4
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)		
				CORE RECOVERY %	RQD %		MPa									20	40
0				20	40	60	80	1	2	3	4	5	20	40	60	80	0
1																	1
2																	2
3																	3
4																	4
5			0 to 9.10 m - See BH-BGC11-33 soil log.														5
6																	6
7																	7
8																	8
9																	9
10		✓	METASEDIMENTARY ROCK Light grey, fine grained, foliated, pyrophyllite alteration, very weak to weak, moderately to highly weathered/alterd, one main joint set parallel to foliations, running at 65-85° to core axis, apertures vary from 1 to 90 mm, infill is typically crushed and decomposed rock, sericite, clay. 9.14 - 18.29 m - Highly fractured zone.														10
11		✓	10.67 - 12.19 m - Very low recovery, unable to determine representative joint condition and fracture frequency.														11
12		✓															12

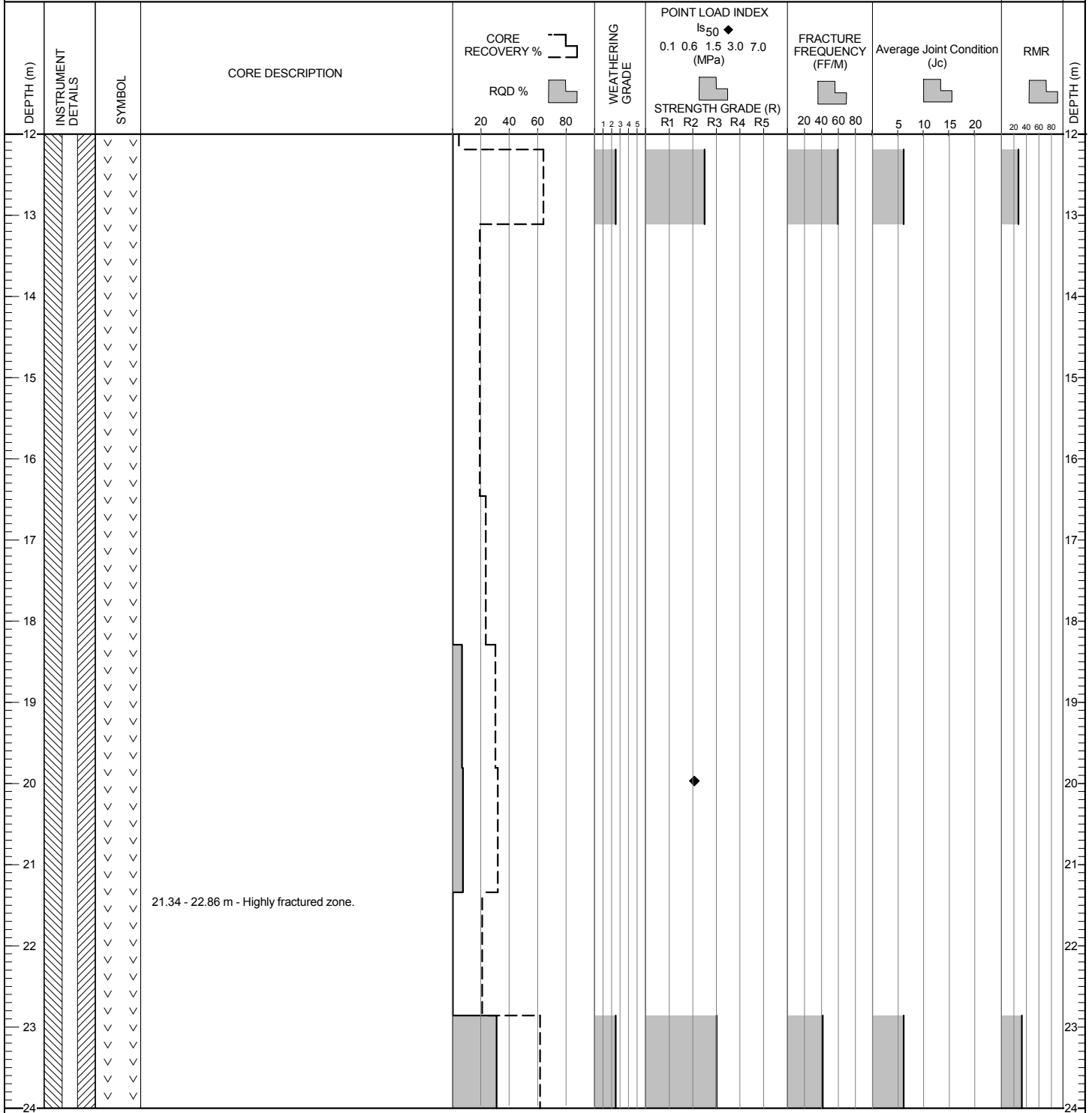
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,115.7E - 7,100,983.6N
 GROUND ELEVATION (m) : 833.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 11.58

START DATE : 13 Jul 11
 FINISH DATE : 14 Jul 11
 FINAL DEPTH (m) : 41.4
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



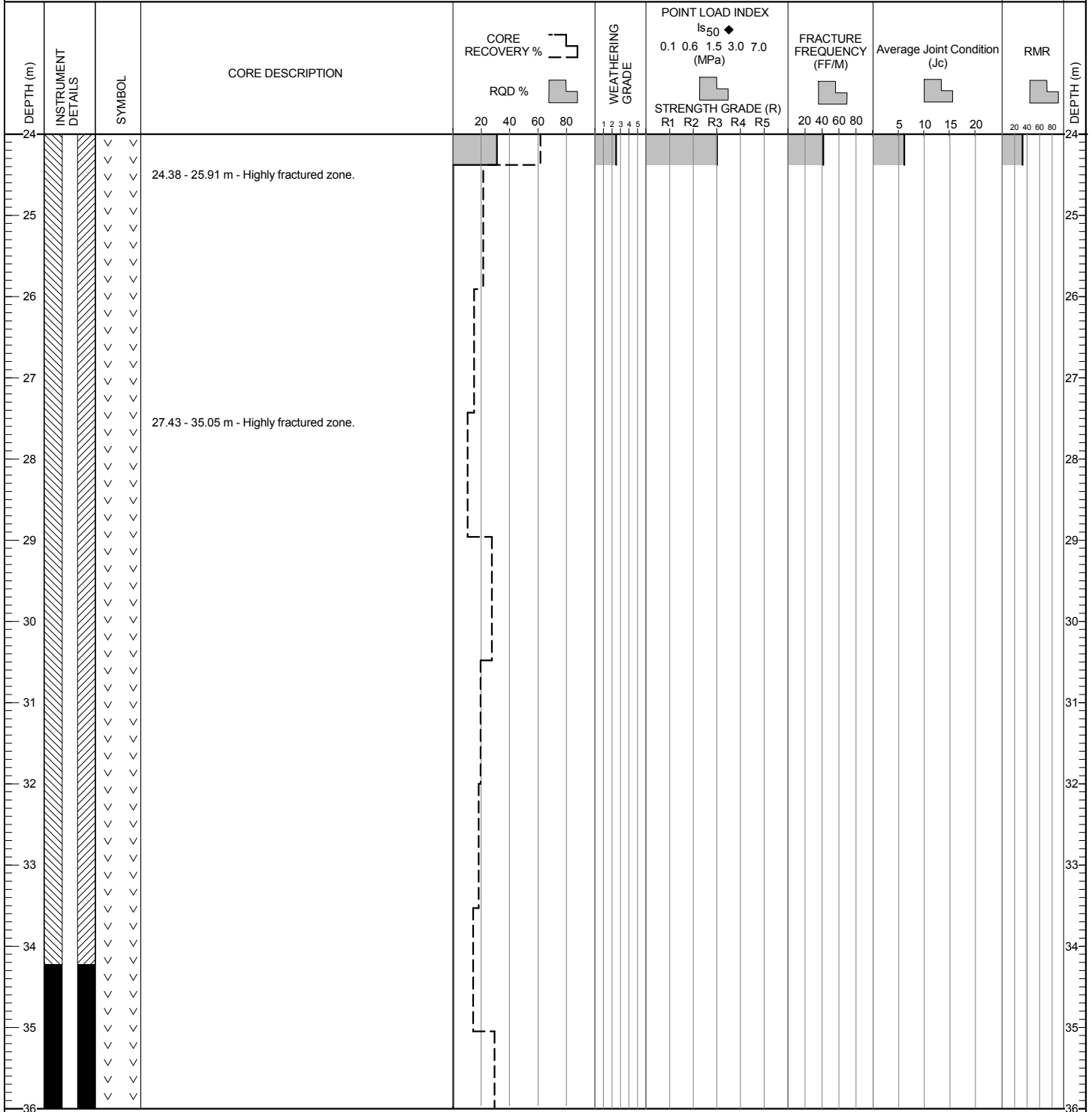
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,115.7E - 7,100,983.6N
 GROUND ELEVATION (m) : 833.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 11.58

START DATE : 13 Jul 11
 FINISH DATE : 14 Jul 11
 FINAL DEPTH (m) : 41.4
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/2/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

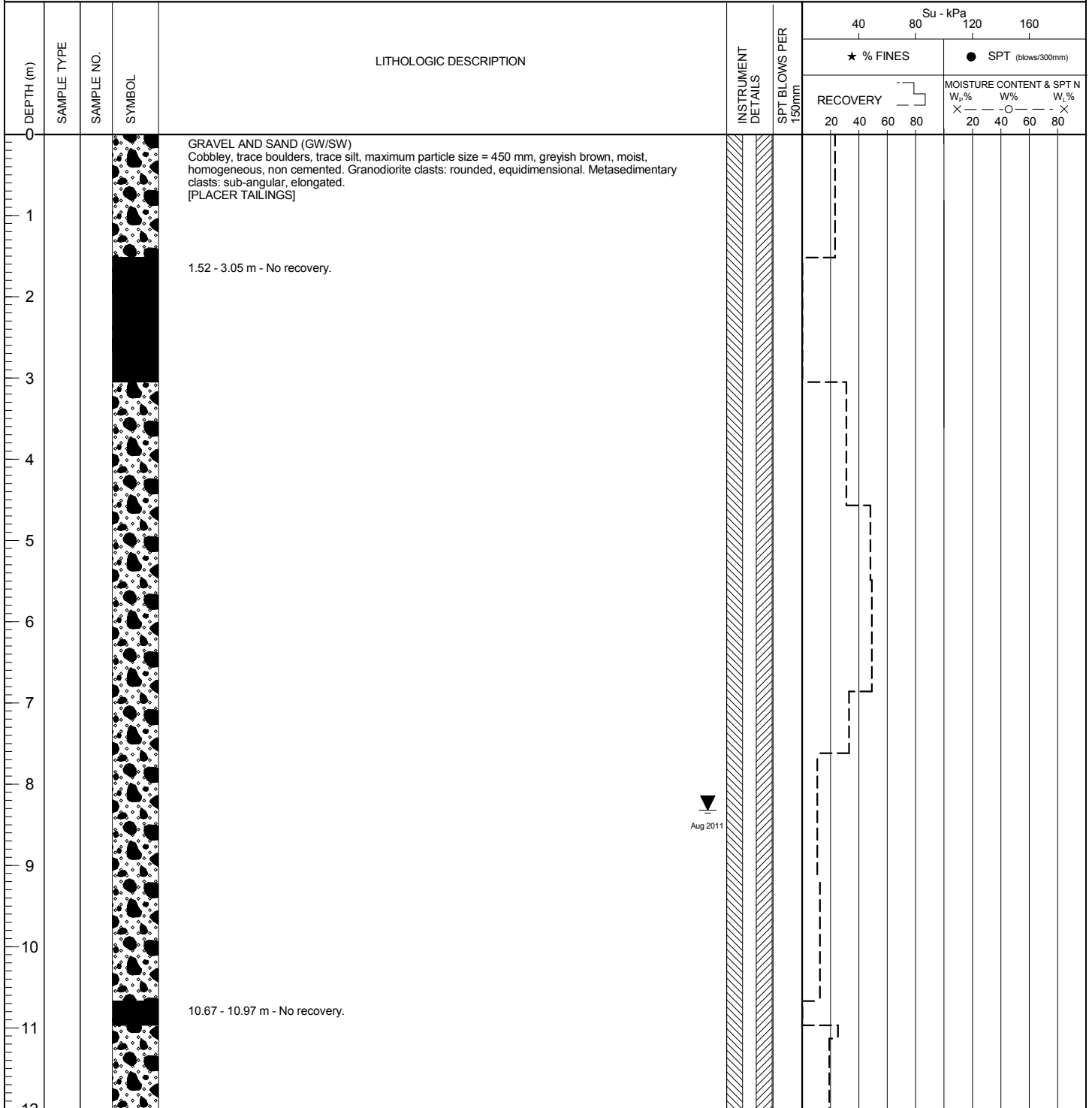
DRILL HOLE # BH-BGC11-34

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m): 459,308.5E - 7,101,053.5N
 GROUND ELEVATION (m) : 848.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 9.14

START DATE : 15 Jul 11
 FINISH DATE : 16 Jul 11
 FINAL DEPTH (m) : 38.1
 DEPTH TO TOP OF ROCK (m) : 16.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



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Aug 2011

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EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/20/12

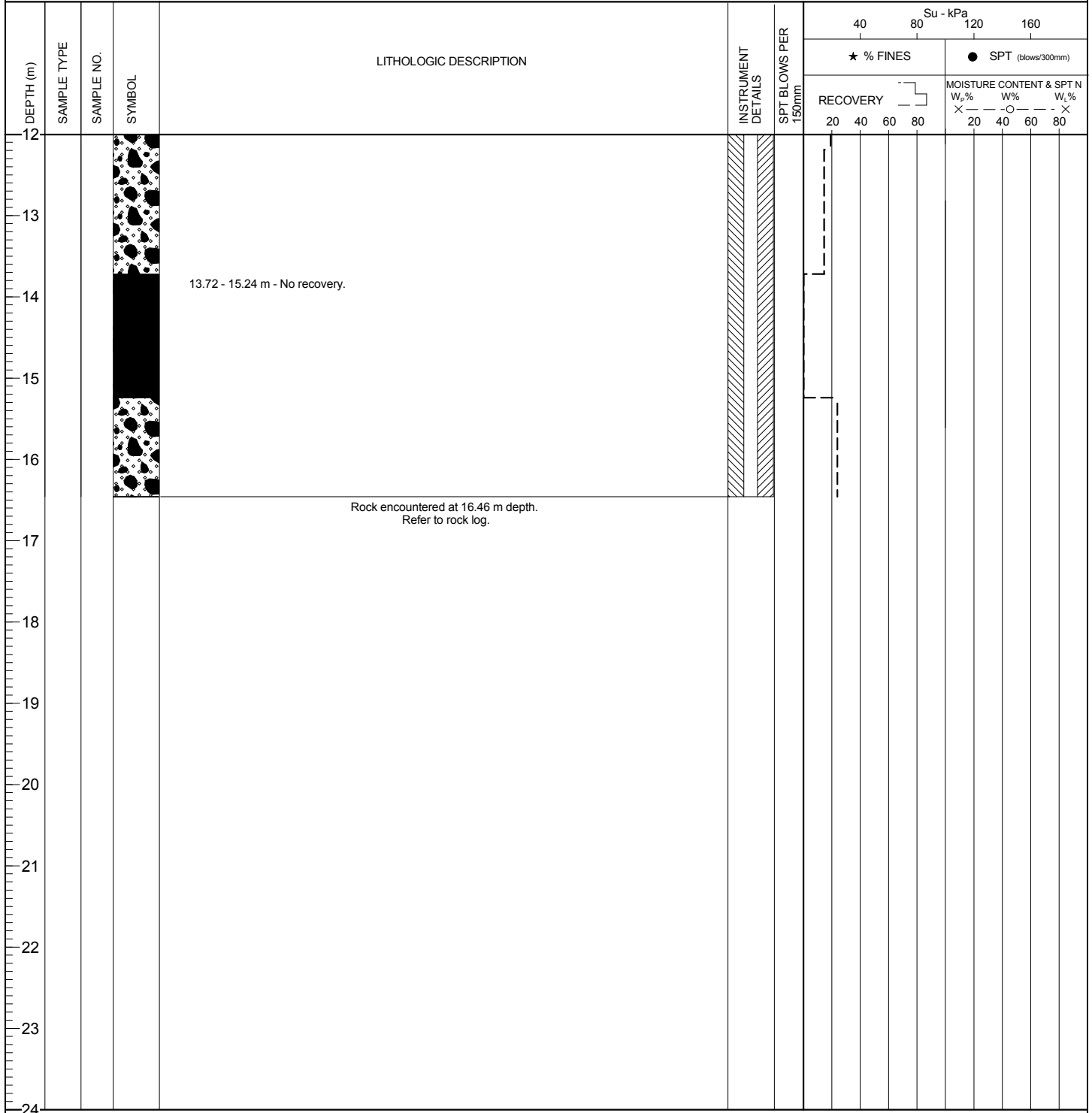
DRILL HOLE # BH-BGC11-34

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m): 459,308.5E - 7,101,053.5N
 GROUND ELEVATION (m) : 848.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 9.14

START DATE : 15 Jul 11
 FINISH DATE : 16 Jul 11
 FINAL DEPTH (m) : 38.1
 DEPTH TO TOP OF ROCK (m) : 16.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/9/12

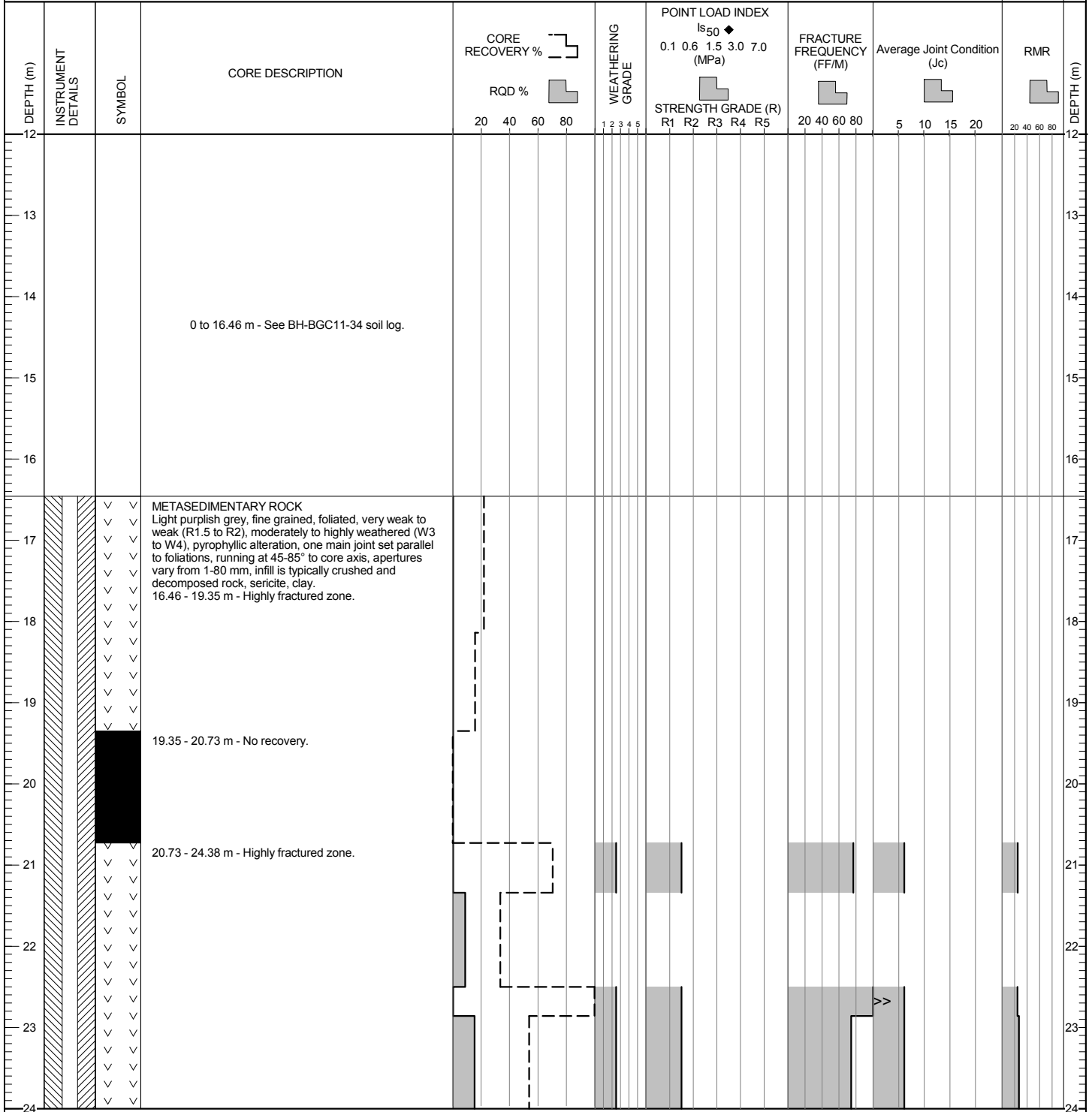
DRILL HOLE # BH-BGC11-34

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m) 459,308.5E - 7,101,053.5N
 GROUND ELEVATION (m) : 848.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 9.14

START DATE : 15 Jul 11
 FINISH DATE : 16 Jul 11
 FINAL DEPTH (m) : 38.1
 DEPTH TO TOP OF ROCK (m) : 16.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

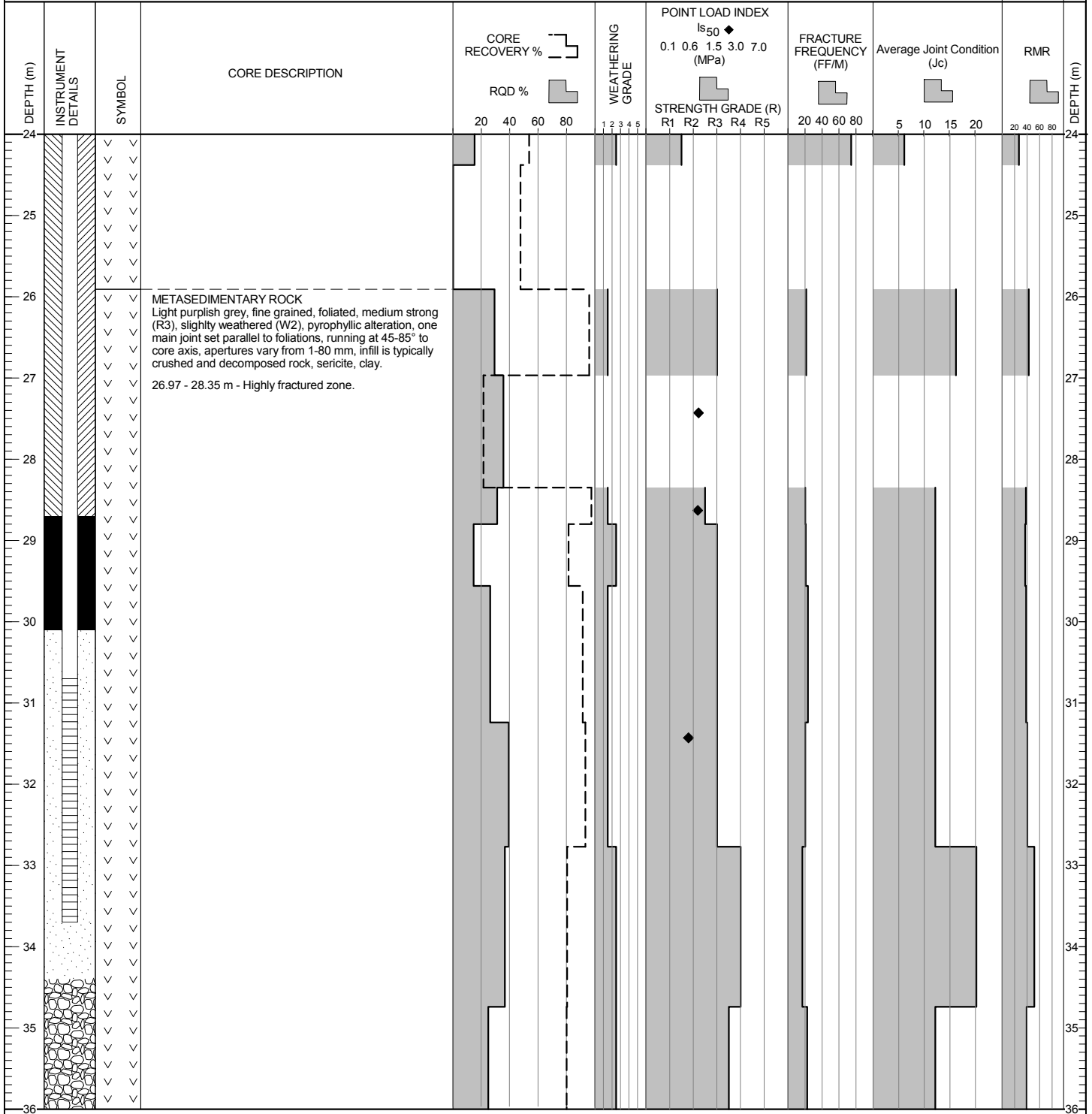
DRILL HOLE # BH-BGC11-34

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m) 459,308.5E - 7,101,053.5N
 GROUND ELEVATION (m) : 848.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 15 Jul 11
 FINISH DATE : 16 Jul 11
 FINAL DEPTH (m) : 38.1
 DEPTH TO TOP OF ROCK (m) : 16.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

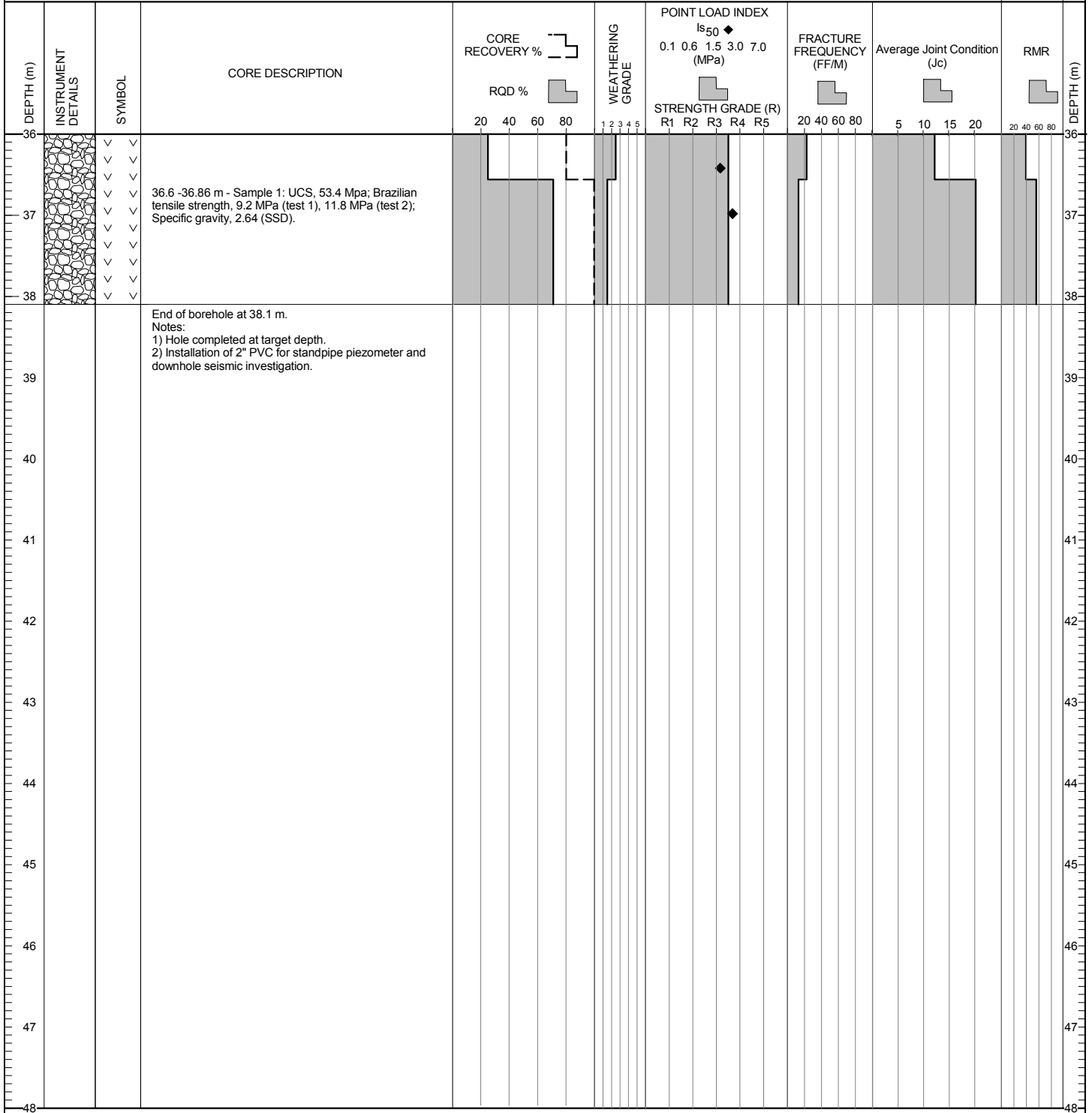
DRILL HOLE # BH-BGC11-34

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m) 459,308.5E - 7,101,053.5N
 GROUND ELEVATION (m) : 848.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 9.14

START DATE : 15 Jul 11
 FINISH DATE : 16 Jul 11
 FINAL DEPTH (m) : 38.1
 DEPTH TO TOP OF ROCK (m) : 16.5
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

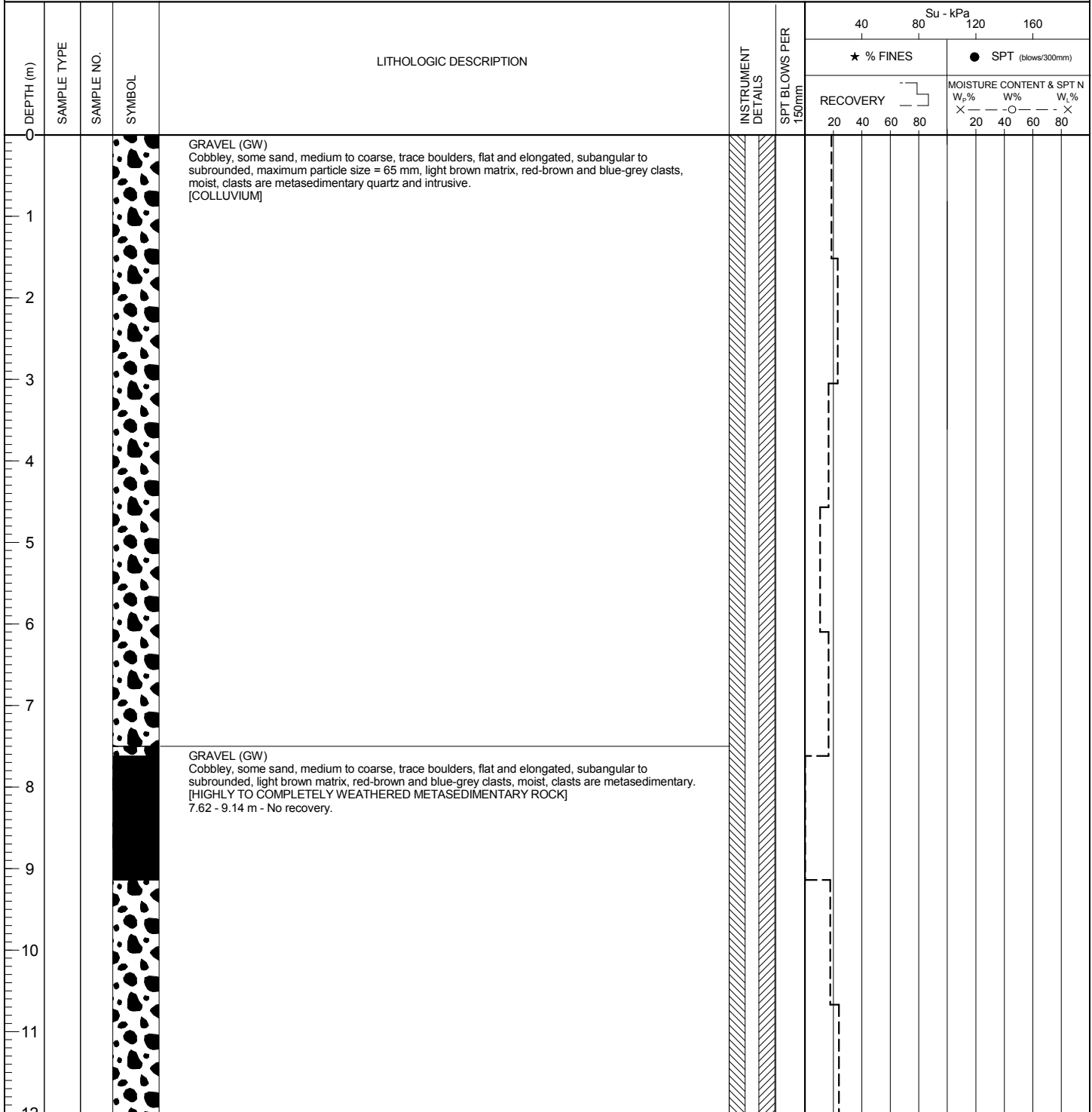
DRILL HOLE # BH-BGC11-35

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,695.7E - 7,100,440.3N
 GROUND ELEVATION (m) : 986.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 9.14

START DATE : 17 Jul 11
 FINISH DATE : 19 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 15.2
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



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EGR/SOIL/ EGR_SOIL_GDL BGC.GDT 12/20/12



CLIENT: Victoria Gold Corporation

PRINT DATE: 1/20/2012

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

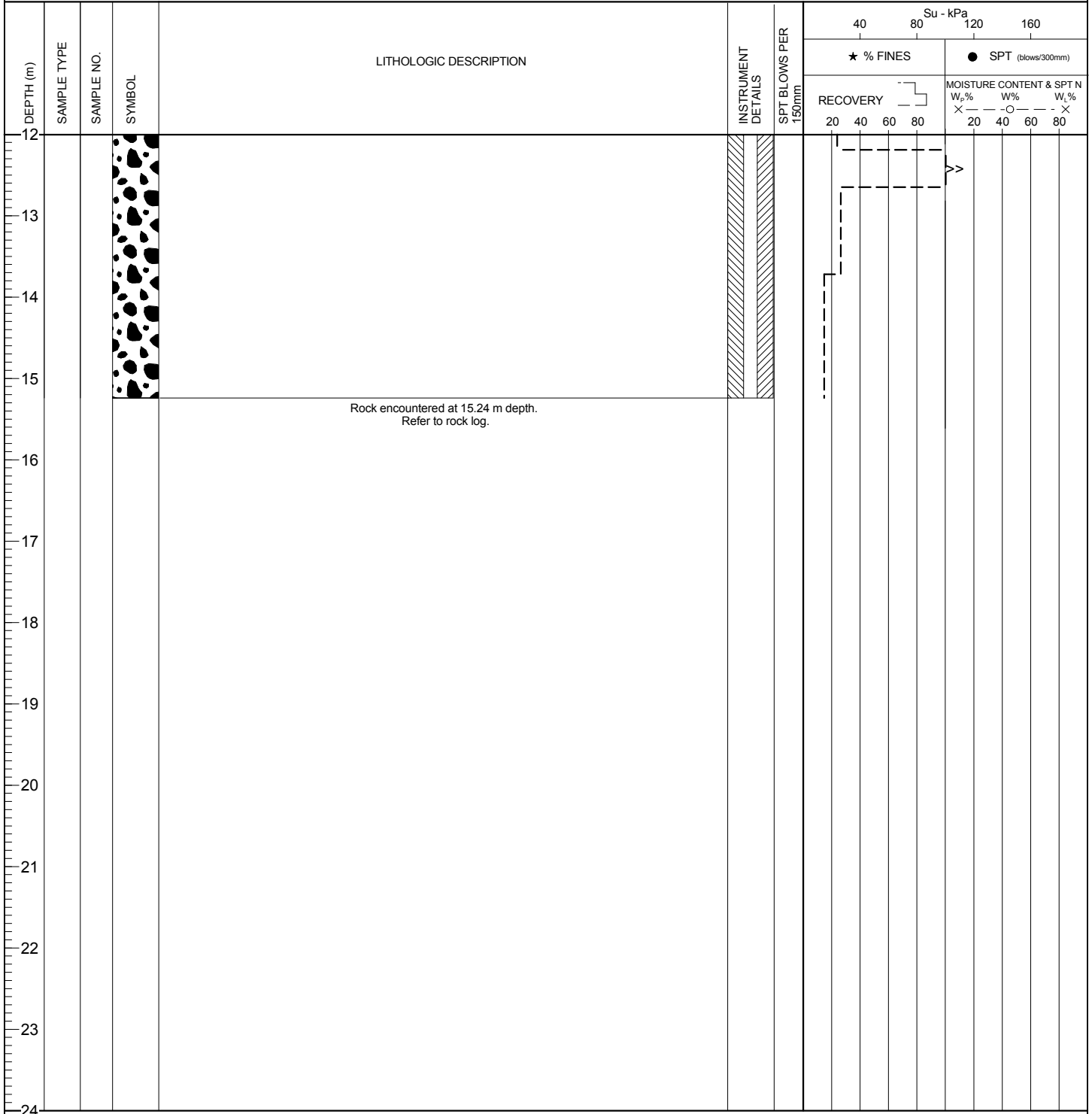
DRILL HOLE # BH-BGC11-35

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,695.7E - 7,100,440.3N
 GROUND ELEVATION (m) : 986.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 9.14

START DATE : 17 Jul 11
 FINISH DATE : 19 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 15.2
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/9/12

CO-ORDINATES (m) 459,695.7E - 7,100,440.3N
 GROUND ELEVATION (m) : 986.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 17 Jul 11
 FINISH DATE : 19 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 15.2
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

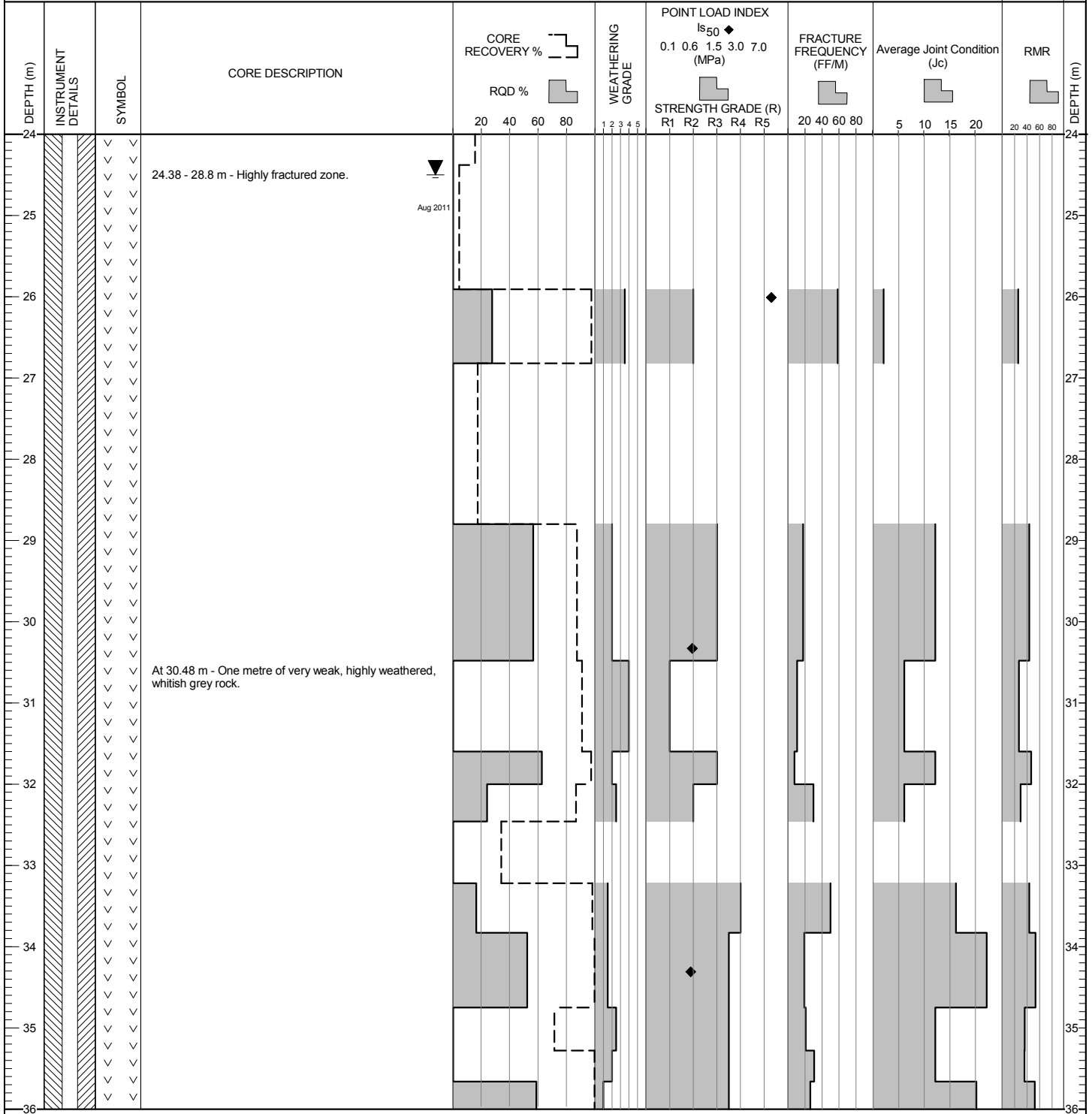
DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)				RMR	DEPTH (m)
				CORE	RQD %		R1	R2	R3	R4	R5		20	40	60	80		
12																	12	
13																	13	
14			0 to 15.24 m - See BH-BGC11-35 soil log.														14	
15																	15	
16	✓	✓	METASEDIMENTARY ROCK Light brownish grey, fine grained, foliated, very weak (R1), highly weathered (W4), phyllitic, siliceous, iron stained. 15.24 - 22.86 m - Highly fractured zone.														16	
17	✓	✓															17	
18	✓	✓	METASEDIMENTARY ROCK Light grey, fine grained, foliated, weak to moderately strong (R2.5), moderately to highly weathered (W2.5 to W3.5), phyllitic, siliceous, iron stained, typically 2 joint sets: one sub-parallel to foliations at 20-35° from core axis and the other at 50-70° from core axis, JRC = 2-20, averaging 11.														18	
19	✓	✓															19	
20	✓	✓															20	
21	✓	✓															21	
22	✓	✓															22	
23	✓	✓															23	
24	✓	✓															24	

(CONTINUED ON NEXT PAGE)

CO-ORDINATES (m) 459,695.7E - 7,100,440.3N
 GROUND ELEVATION (m) : 986.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 17 Jul 11
 FINISH DATE : 19 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 15.2
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

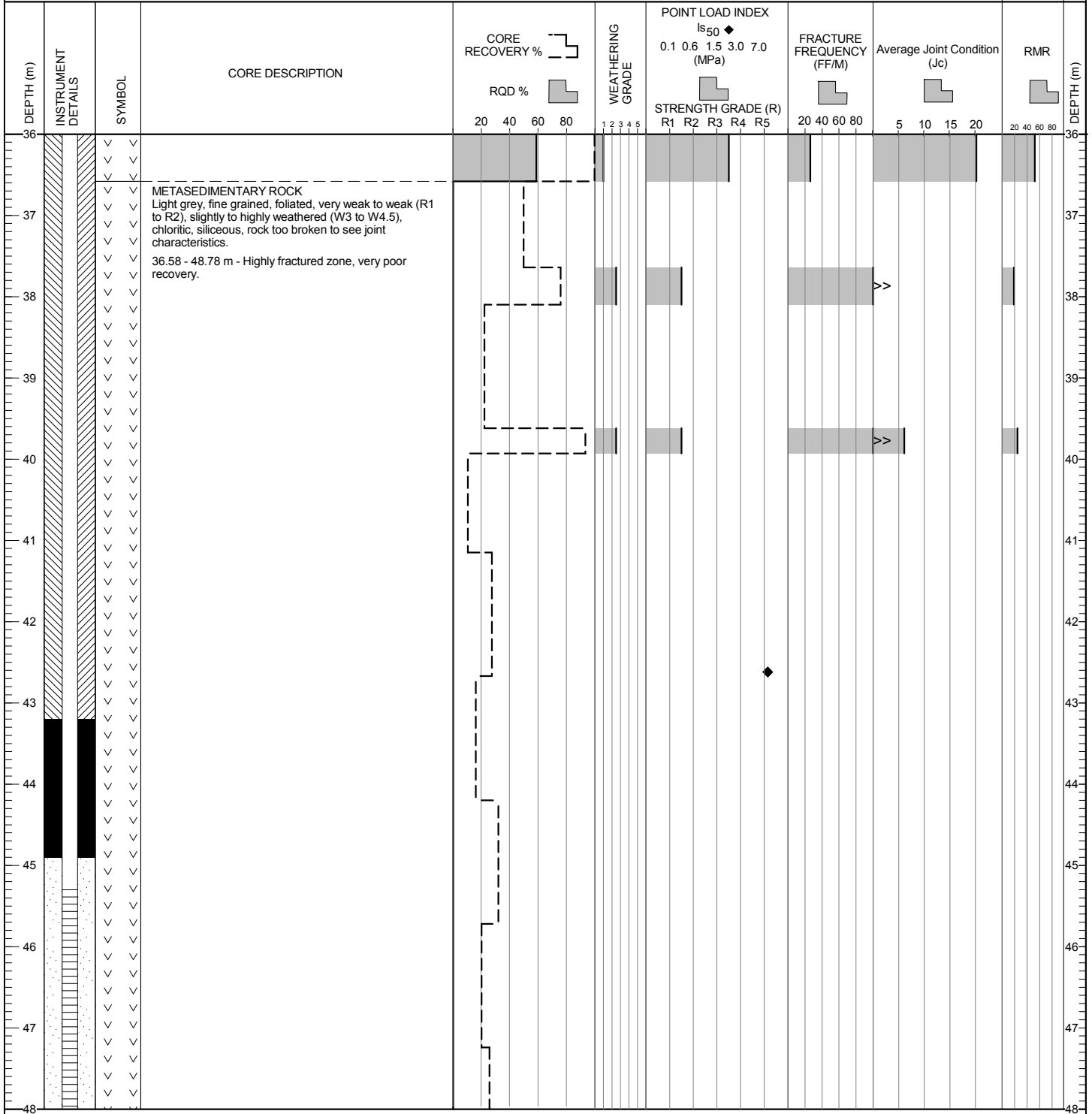


EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,695.7E - 7,100,440.3N
 GROUND ELEVATION (m) : 986.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 17 Jul 11
 FINISH DATE : 19 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 15.2
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,695.7E - 7,100,440.3N
 GROUND ELEVATION (m) : 986.3m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 9.14

START DATE : 17 Jul 11
 FINISH DATE : 19 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 15.2
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
48															48
49															49
50															50
51			End of borehole at 50.3 m. Notes: 1) Hole completed at target depth. 2) Installation of 2" PVC for standpipe piezometer. 3) TP-BGC11-51 is located on this drill pad.												51
52															52
53															53
54															54
55															55
56															56
57															57
58															58
59															59
60															60

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

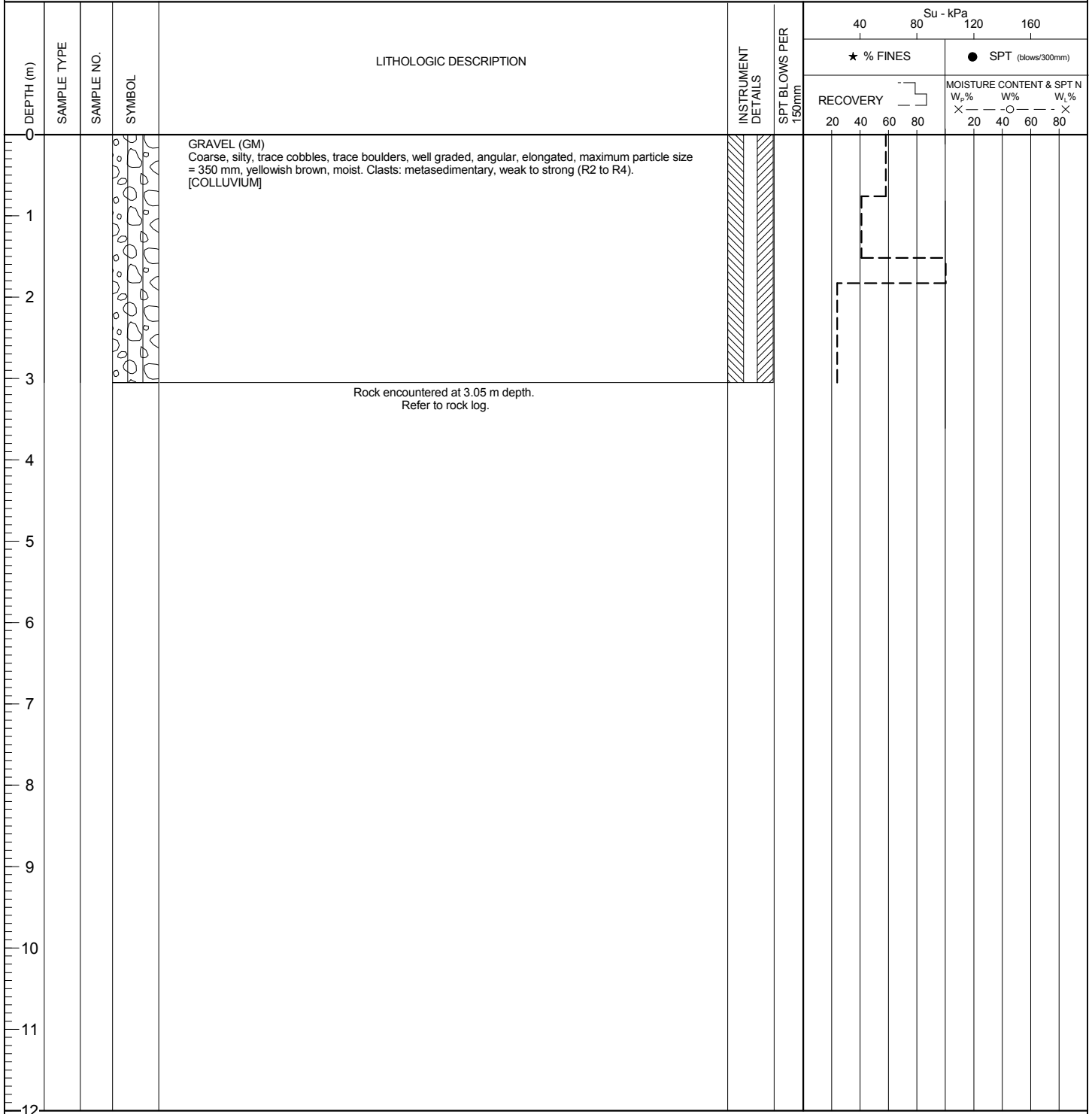
DRILL HOLE # BH-BGC11-36

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,699.9E - 7,100,274.7N
 GROUND ELEVATION (m) : 1,002.5m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 3.05

START DATE : 19 Jul 11
 FINISH DATE : 23 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/20/12

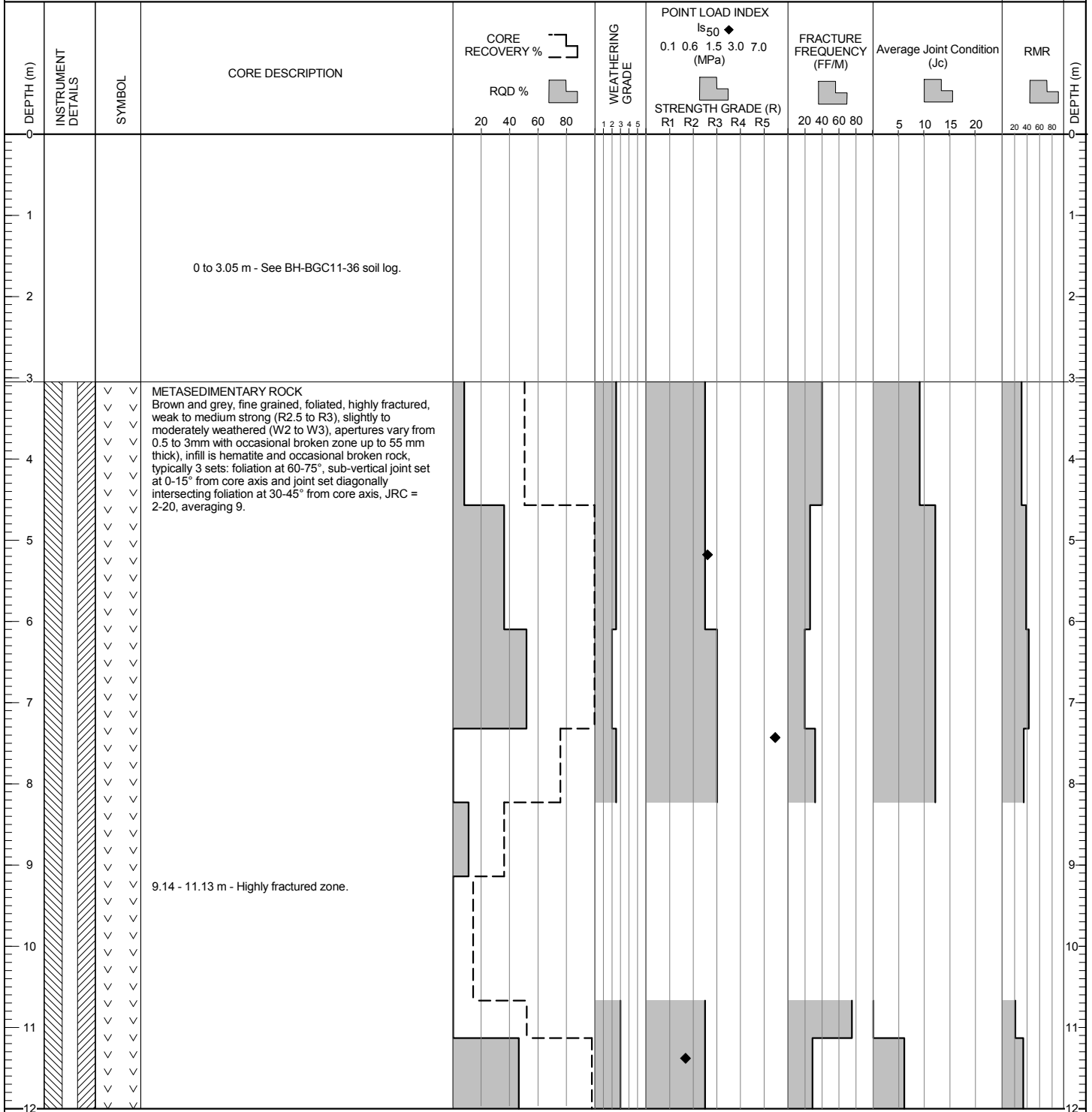


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 459,699.9E - 7,100,274.7N
 GROUND ELEVATION (m) : 1,002.5m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 19 Jul 11
 FINISH DATE : 23 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

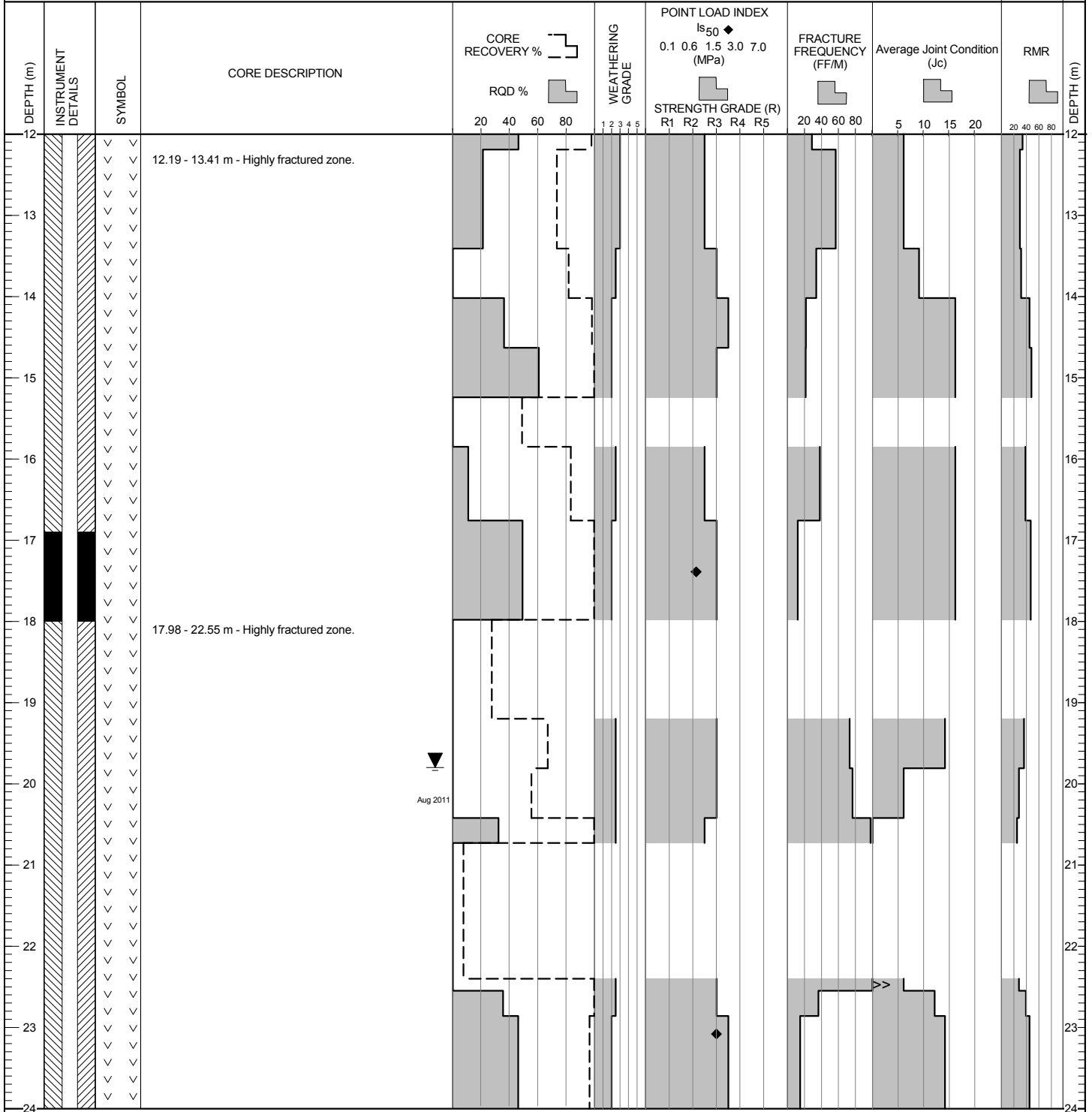


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CO-ORDINATES (m) 459,699.9E - 7,100,274.7N
 GROUND ELEVATION (m) : 1,002.5m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 19 Jul 11
 FINISH DATE : 23 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

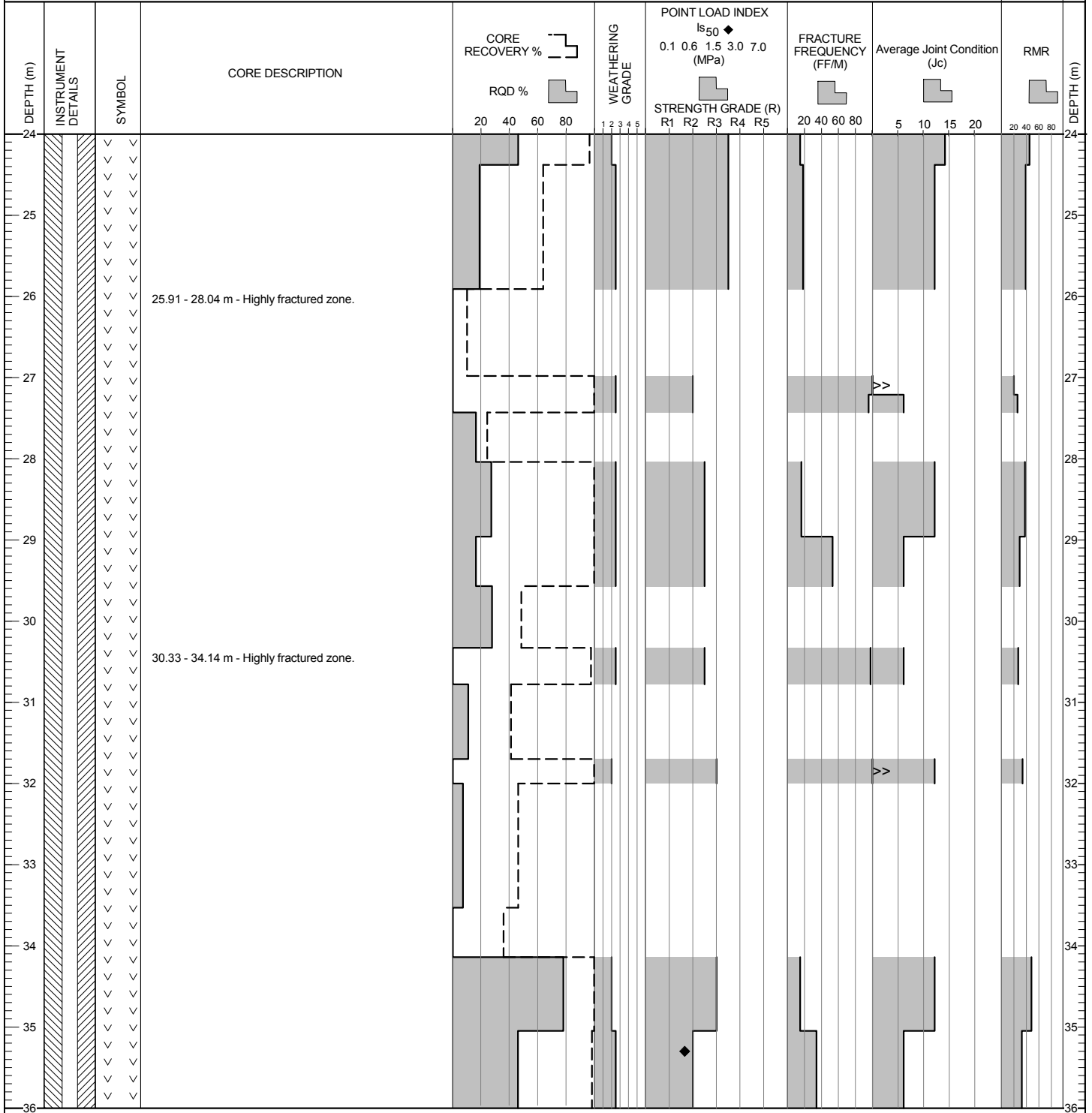


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CO-ORDINATES (m) 459,699.9E - 7,100,274.7N
 GROUND ELEVATION (m) : 1,002.5m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 19 Jul 11
 FINISH DATE : 23 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



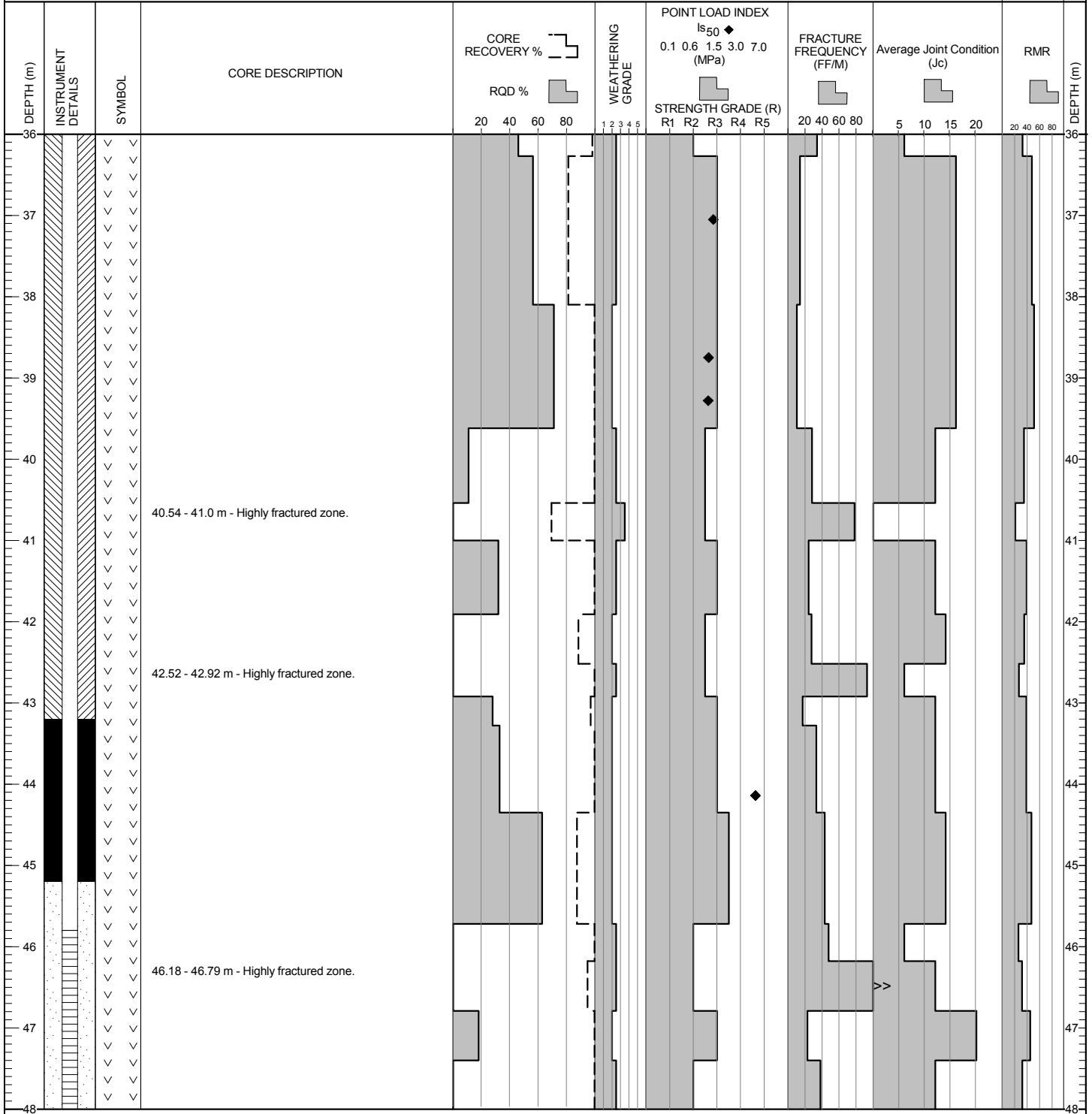
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,699.9E - 7,100,274.7N
 GROUND ELEVATION (m) : 1,002.5m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 19 Jul 11
 FINISH DATE : 23 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



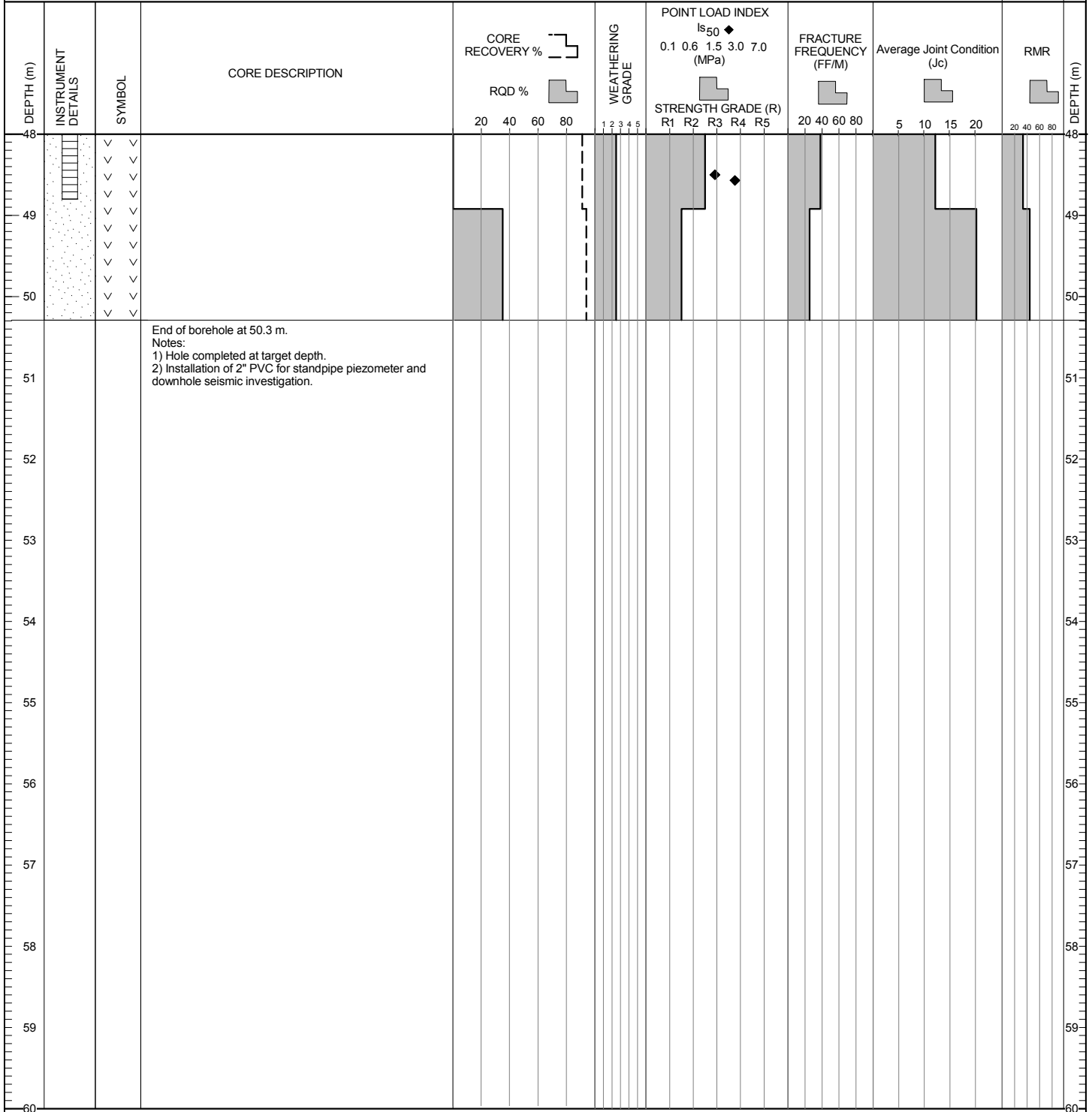
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,699.9E - 7,100,274.7N
 GROUND ELEVATION (m) : 1,002.5m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 3.05

START DATE : 19 Jul 11
 FINISH DATE : 23 Jul 11
 FINAL DEPTH (m) : 50.3
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-37

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,762.8E - 7,100,180.5N
 GROUND ELEVATION (m) : 1,034.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 3.75

START DATE : 24 Jul 11
 FINISH DATE : 26 Jul 11
 FINAL DEPTH (m) : 43.6
 DEPTH TO TOP OF ROCK (m) : 3.8
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa							
							★ % FINES		● SPT (blows/300mm)					
							RECOVERY		MOISTURE CONTENT & SPT N					
							20	40	60	80	W _p %	W ₉₀ %	W _L %	
0				GRAVEL (GW) Some cobbles, some boulders, metasedimentary, angular, iron stained. Fractured clasts from fill for drill pad. [FILL]										
1														
2														
3														
4				Rock encountered at 3.75 m depth. Refer to rock log.										
5														
6														
7														
8														
9														
10														
11														
12														

EGR/SOIL/ EGR_SOIL_GDL BGC.GDT 12/9/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

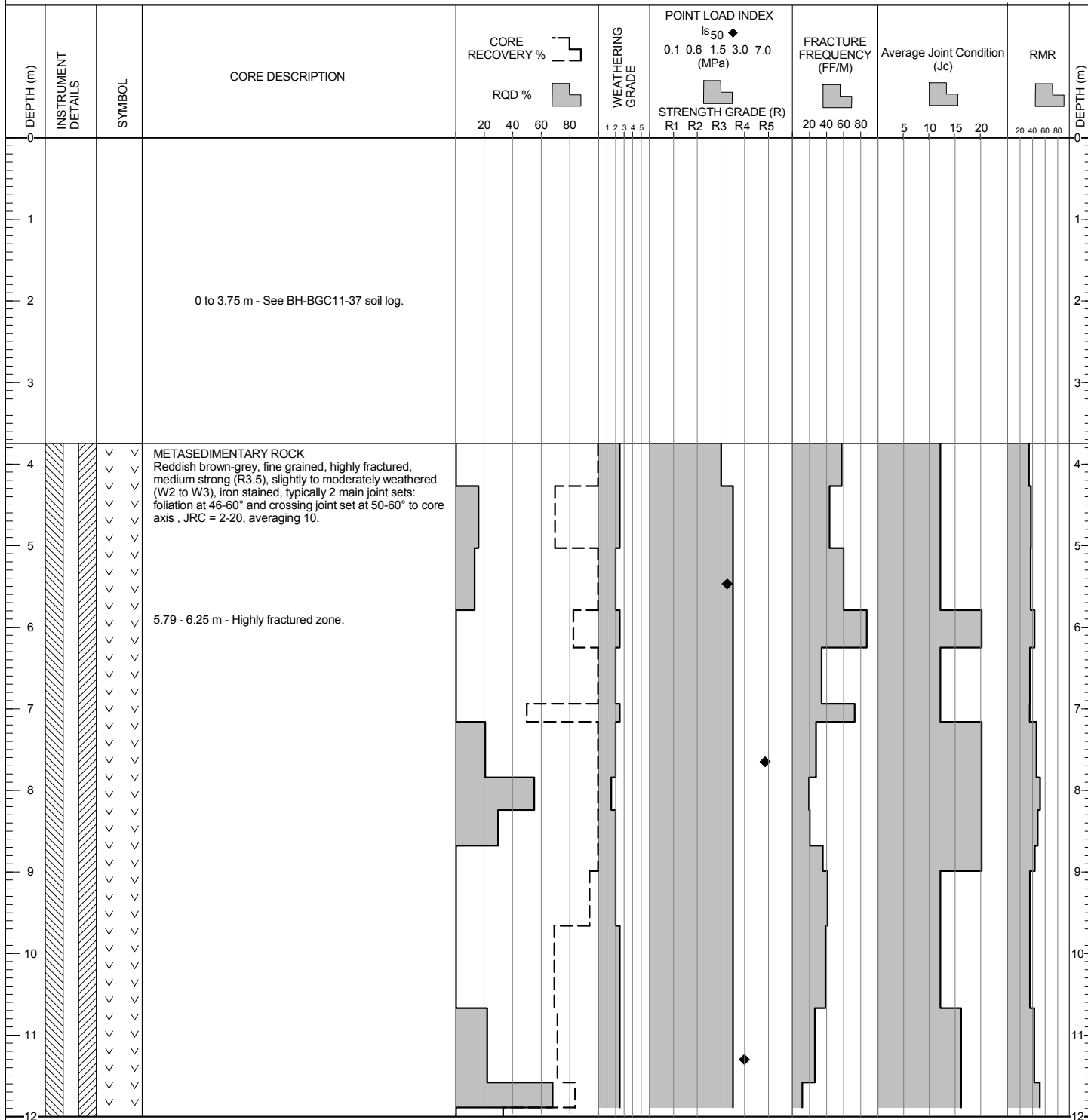
DRILL HOLE # BH-BGC11-37

LOCATION : STUTTLE GULCH

CO-ORDINATES (m) 459,762.8E - 7,100,180.5N
 GROUND ELEVATION (m) : 1,034.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.75

START DATE : 24 Jul 11
 FINISH DATE : 26 Jul 11
 FINAL DEPTH (m) : 43.6
 DEPTH TO TOP OF ROCK (m) : 3.8
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



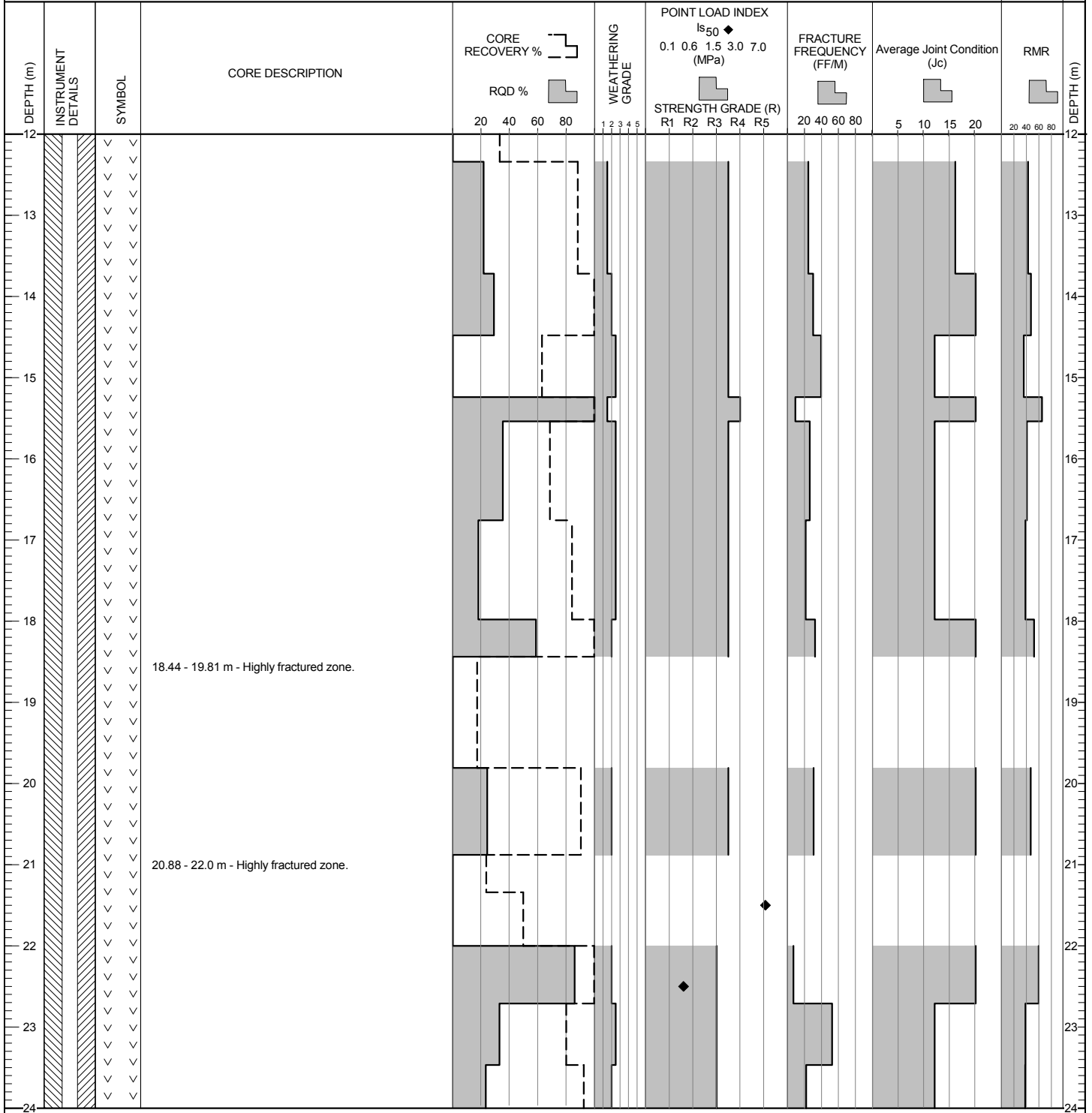
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,762.8E - 7,100,180.5N
 GROUND ELEVATION (m) : 1,034.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 3.75

START DATE : 24 Jul 11
 FINISH DATE : 26 Jul 11
 FINAL DEPTH (m) : 43.6
 DEPTH TO TOP OF ROCK (m) : 3.8
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW

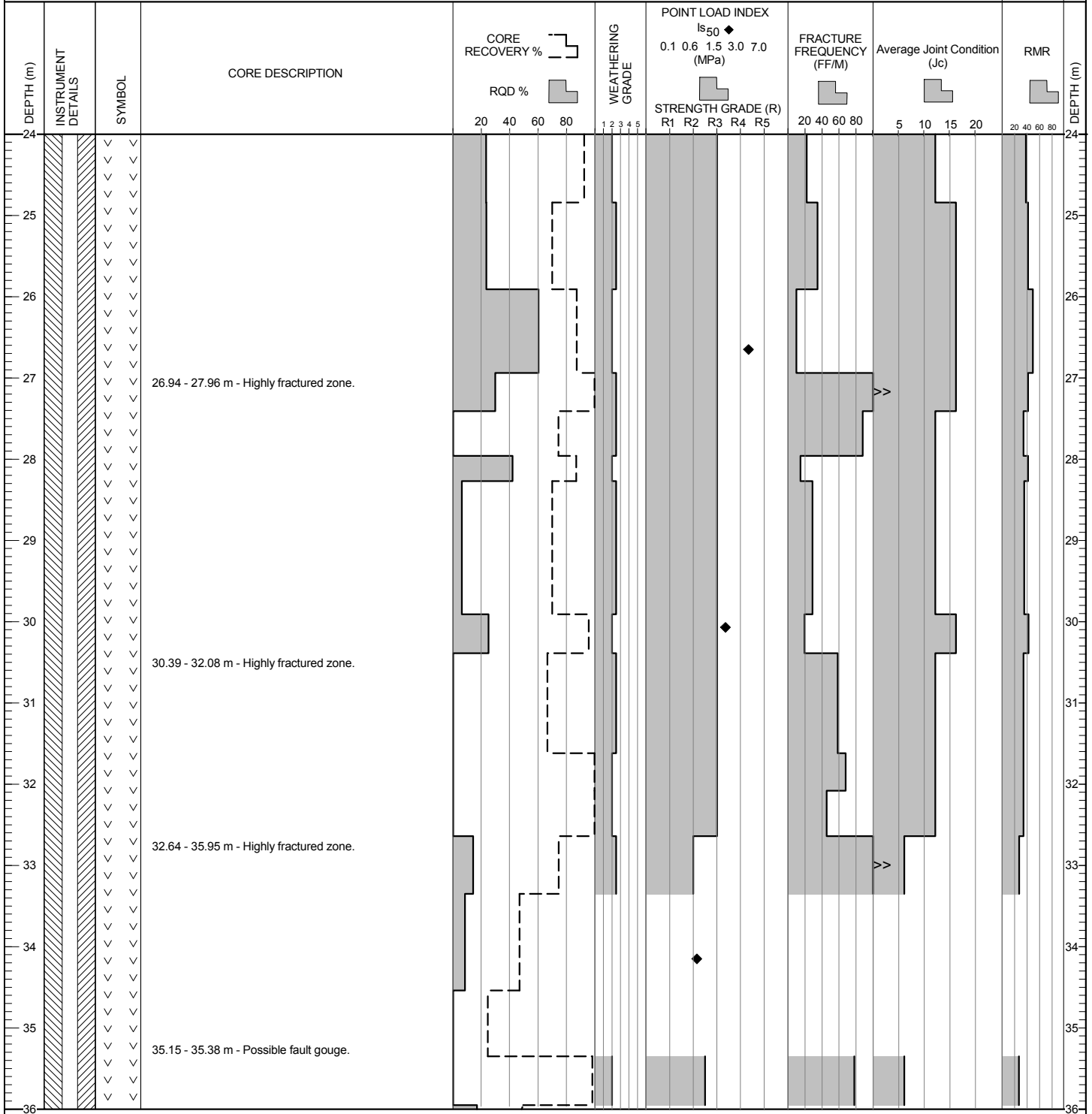


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CO-ORDINATES (m) 459,762.8E - 7,100,180.5N
 GROUND ELEVATION (m) : 1,034.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.75

START DATE : 24 Jul 11
 FINISH DATE : 26 Jul 11
 FINAL DEPTH (m) : 43.6
 DEPTH TO TOP OF ROCK (m) : 3.8
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



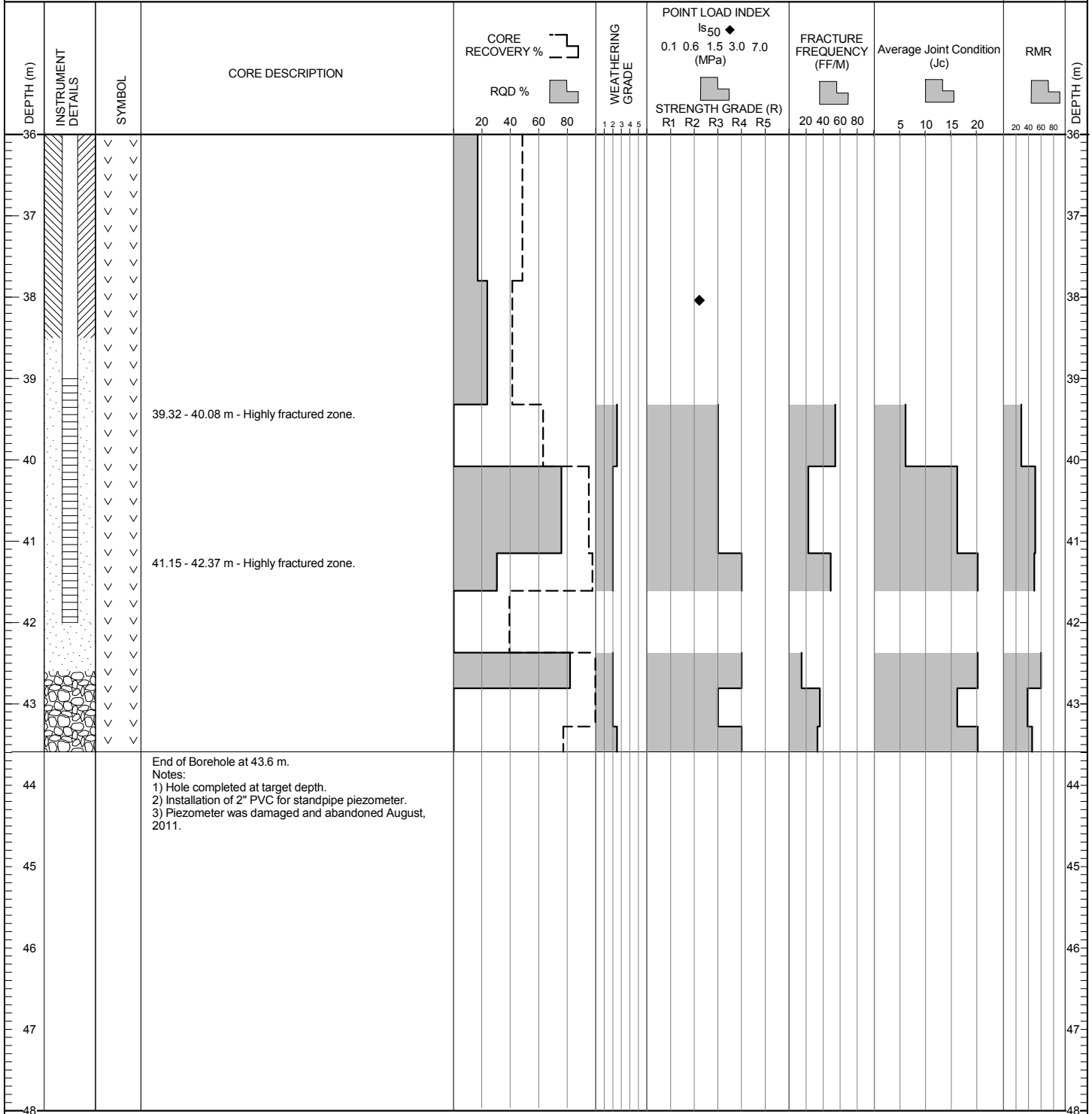
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,762.8E - 7,100,180.5N
 GROUND ELEVATION (m) : 1,034.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.75

START DATE : 24 Jul 11
 FINISH DATE : 26 Jul 11
 FINAL DEPTH (m) : 43.6
 DEPTH TO TOP OF ROCK (m) : 3.8
 LOGGED BY : EB/SD
 REVIEWED BY : PQ/DW



ESP (ROCK) ESP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

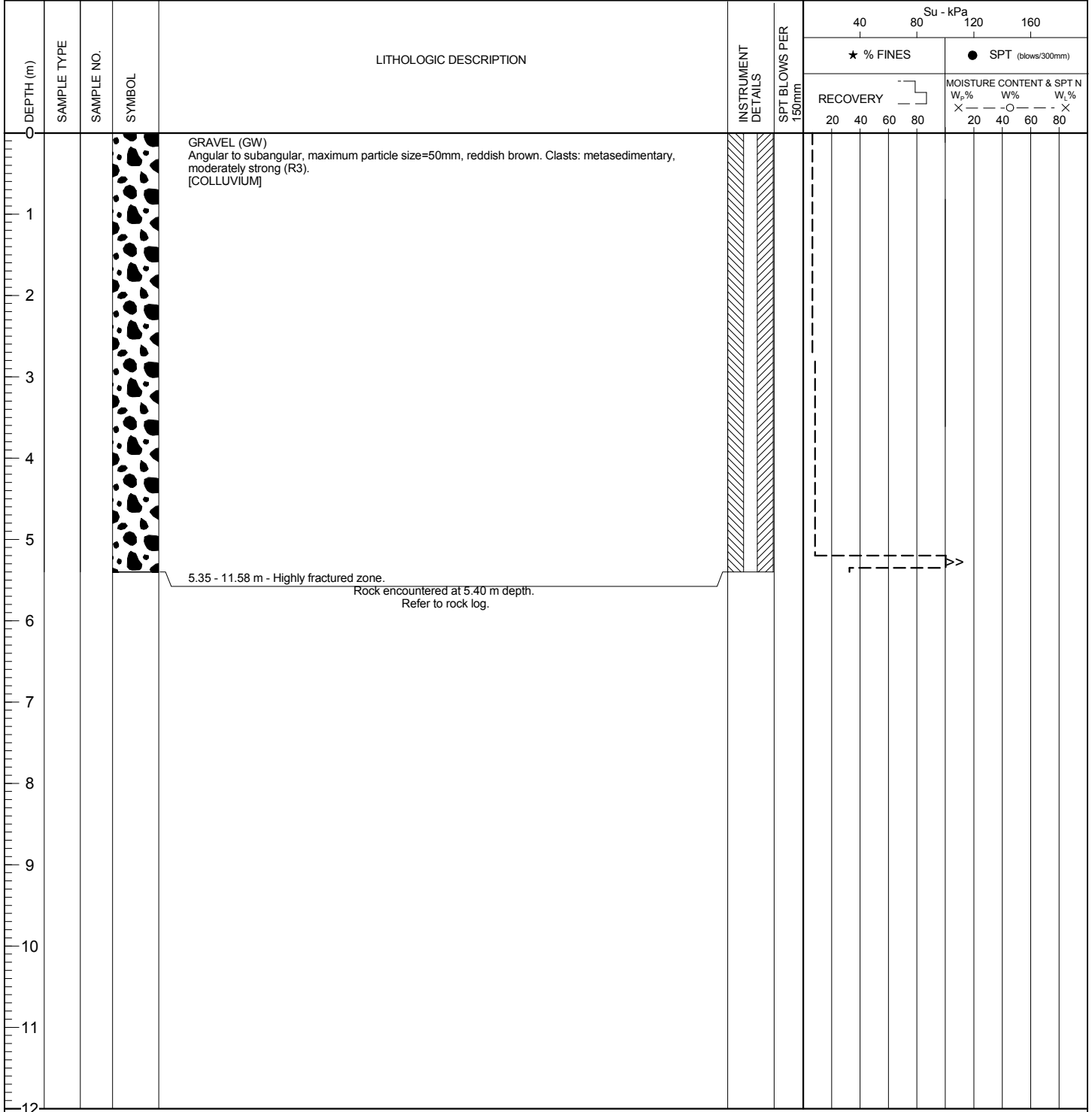
DRILL HOLE # BH-BGC11-38

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,820.0E - 7,100,416.0N
 GROUND ELEVATION (m) : 1,013.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 8.53

START DATE : 26 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 50.5
 DEPTH TO TOP OF ROCK (m) : 5.4
 LOGGED BY : EB/SD/SP
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/20/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

DRILL HOLE # BH-BGC11-38

LOCATION : STUTTLE GULCH

CO-ORDINATES (m) 459,820.0E - 7,100,416.0N
 GROUND ELEVATION (m) : 1,013.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 8.53

START DATE : 26 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 50.5
 DEPTH TO TOP OF ROCK (m) : 5.4
 LOGGED BY : EB/SD/SP
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		R1	R2	R3	R4	R5				
0															0
1															1
2															2
3			0 to 5.40 m - See BH-BGC11-38 soil log.												3
4															4
5															5
6		✓	METASEDIMENTARY ROCK Reddish brown-grey, fine grained, highly fractured, foliated, weak to medium strong (R2 to R3), slightly to moderately weathered (W2 to W3), iron stained, siliceous, typically 2 joints sets: foliation at 40-65° from core axis, subhorizontal joint and sets diagonal to foliation at 25-65° from core axis, JRC = 2-20, averaging 12.												6
7		✓													7
8		✓													8
9		✓													9
10		✓													10
11		✓													11
12		✓													12

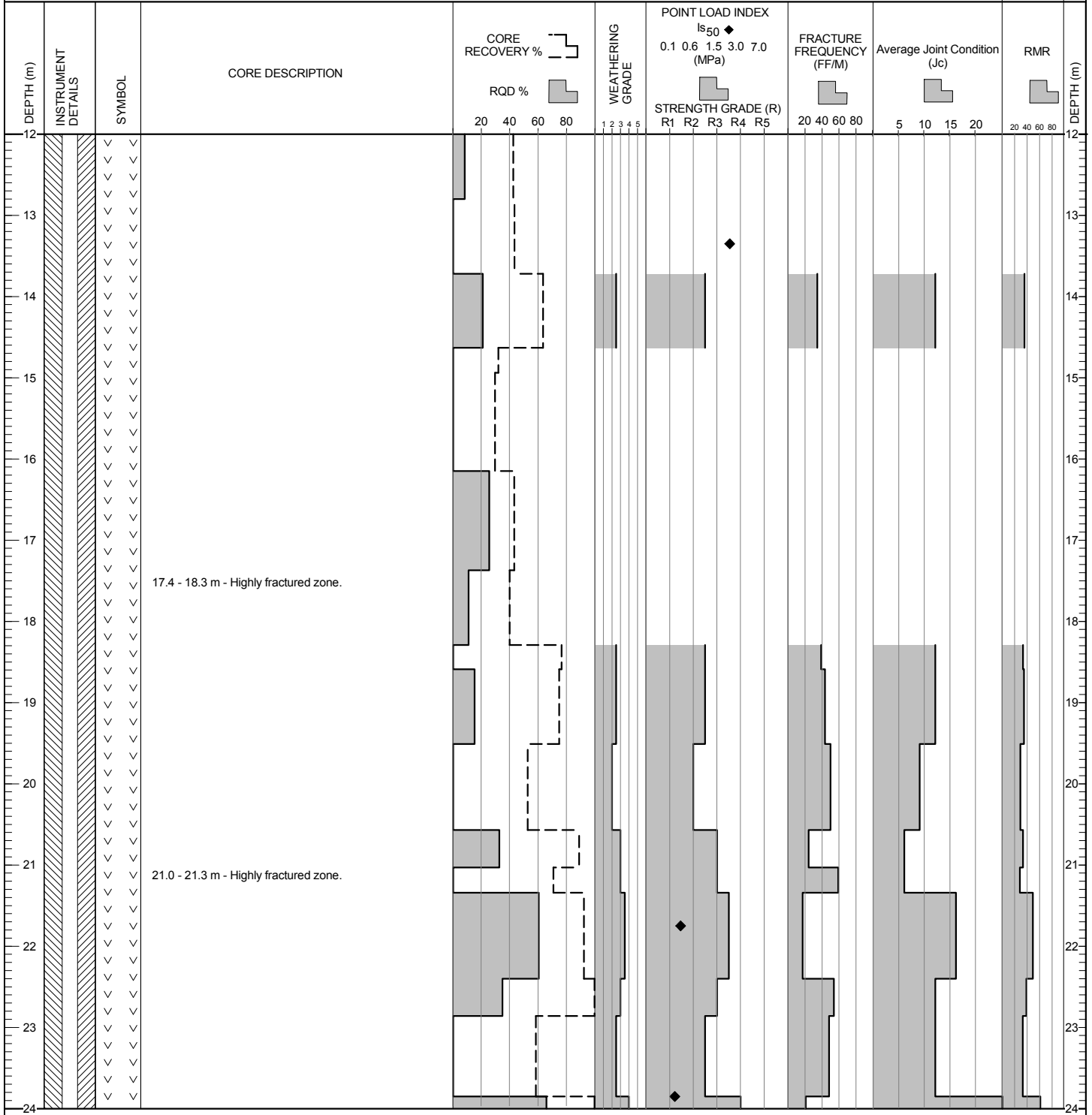
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,820.0E - 7,100,416.0N
 GROUND ELEVATION (m) : 1,013.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 8.53

START DATE : 26 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 50.5
 DEPTH TO TOP OF ROCK (m) : 5.4
 LOGGED BY : EB/SD/SP
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

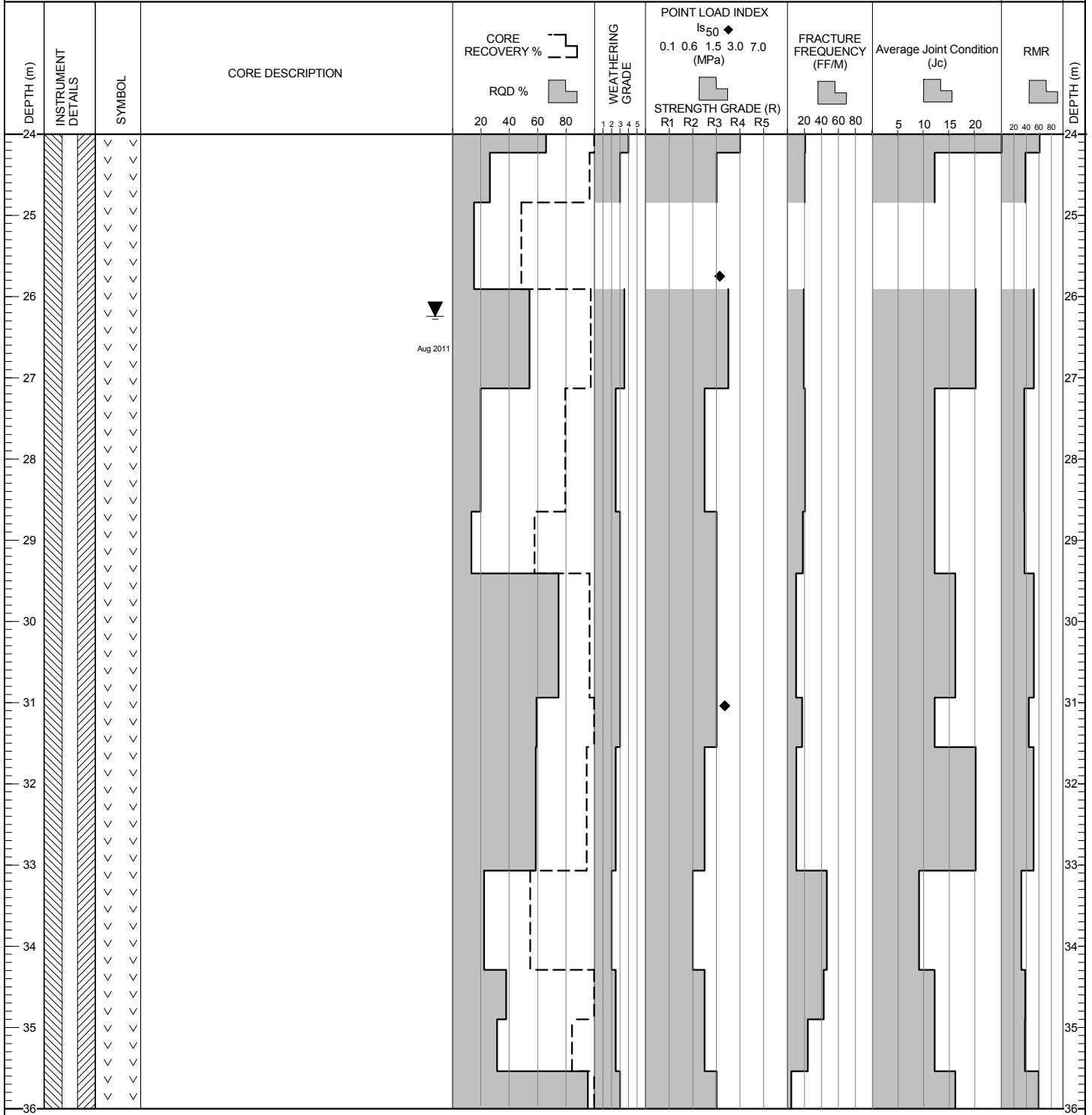
DRILL HOLE # BH-BGC11-38

LOCATION : STUTTLE GULCH

CO-ORDINATES (m) 459,820.0E - 7,100,416.0N
 GROUND ELEVATION (m) : 1,013.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 8.53

START DATE : 26 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 50.5
 DEPTH TO TOP OF ROCK (m) : 5.4
 LOGGED BY : EB/SD/SP
 REVIEWED BY : PQ/DW



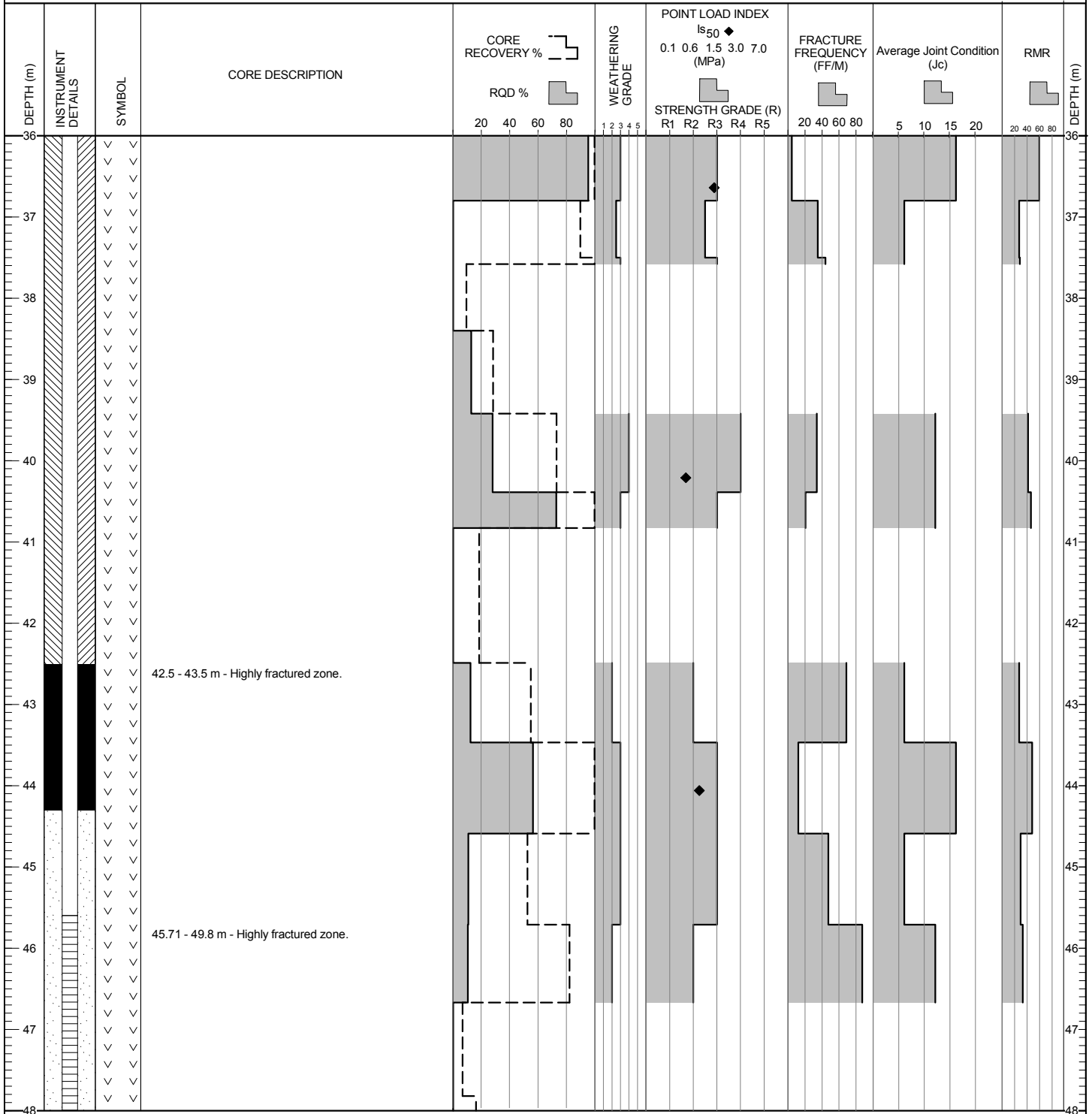
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,820.0E - 7,100,416.0N
 GROUND ELEVATION (m) : 1,013.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 8.53

START DATE : 26 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 50.5
 DEPTH TO TOP OF ROCK (m) : 5.4
 LOGGED BY : EB/SD/SP
 REVIEWED BY : PQ/DW



(CONTINUED ON NEXT PAGE)

CO-ORDINATES (m) 459,820.0E - 7,100,416.0N
 GROUND ELEVATION (m) : 1,013.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 8.53

START DATE : 26 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 50.5
 DEPTH TO TOP OF ROCK (m) : 5.4
 LOGGED BY : EB/SD/SP
 REVIEWED BY : PQ/DW

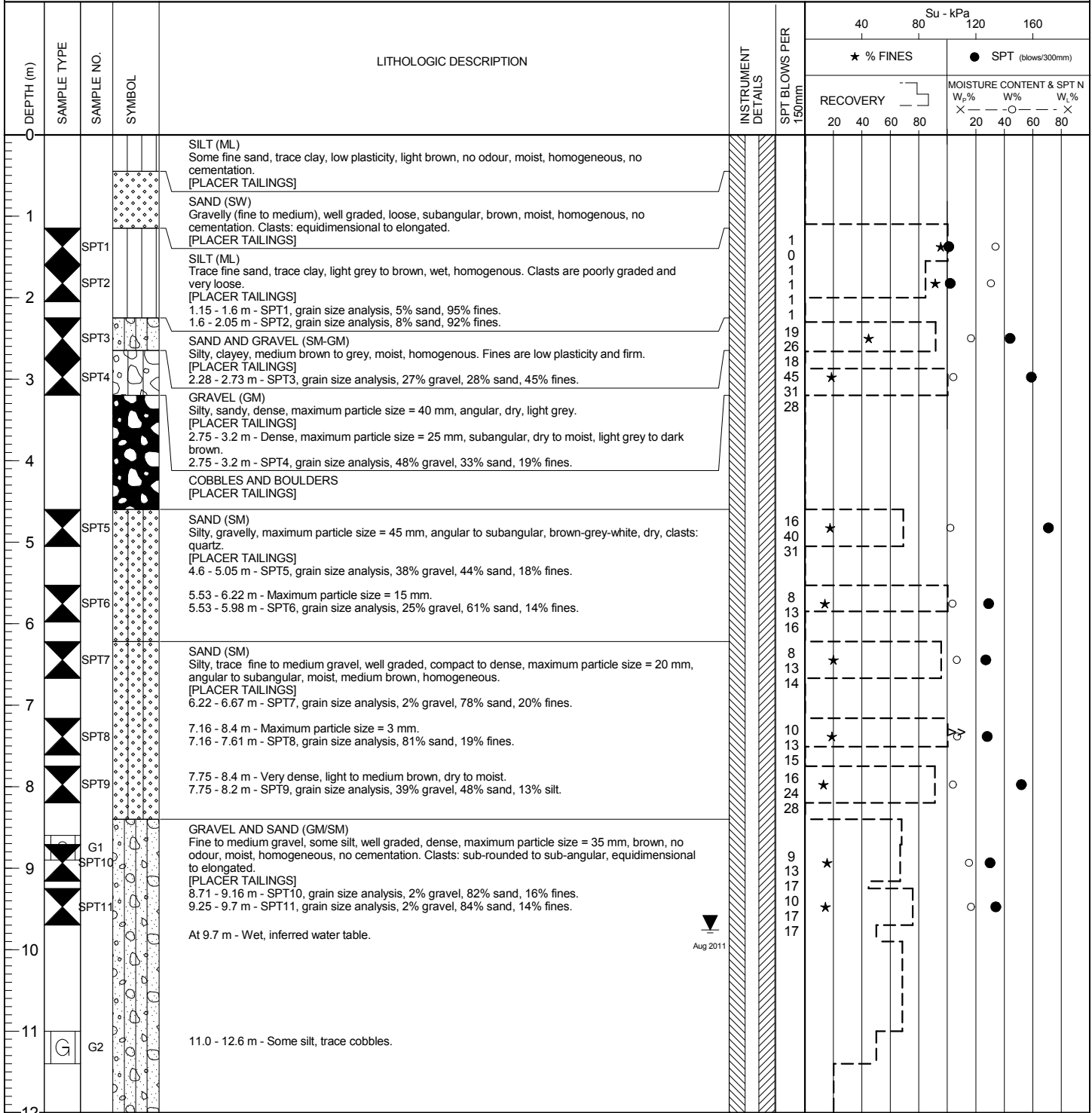
DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
48															48
49															49
50															50
51			End of borehole at 50.5 m. Notes: 1) Hole completed at target depth. 2) Installation of 2" PVC for standpipe piezometer.												51
52															52
53															53
54															54
55															55
56															56
57															57
58															58
59															59
60															60

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 458,462.0E - 7,101,056.0N
 GROUND ELEVATION (m) : 797.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASSED TO (m) :

START DATE : 27 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 28.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT/EC/SD
 REVIEWED BY : PQ/DW



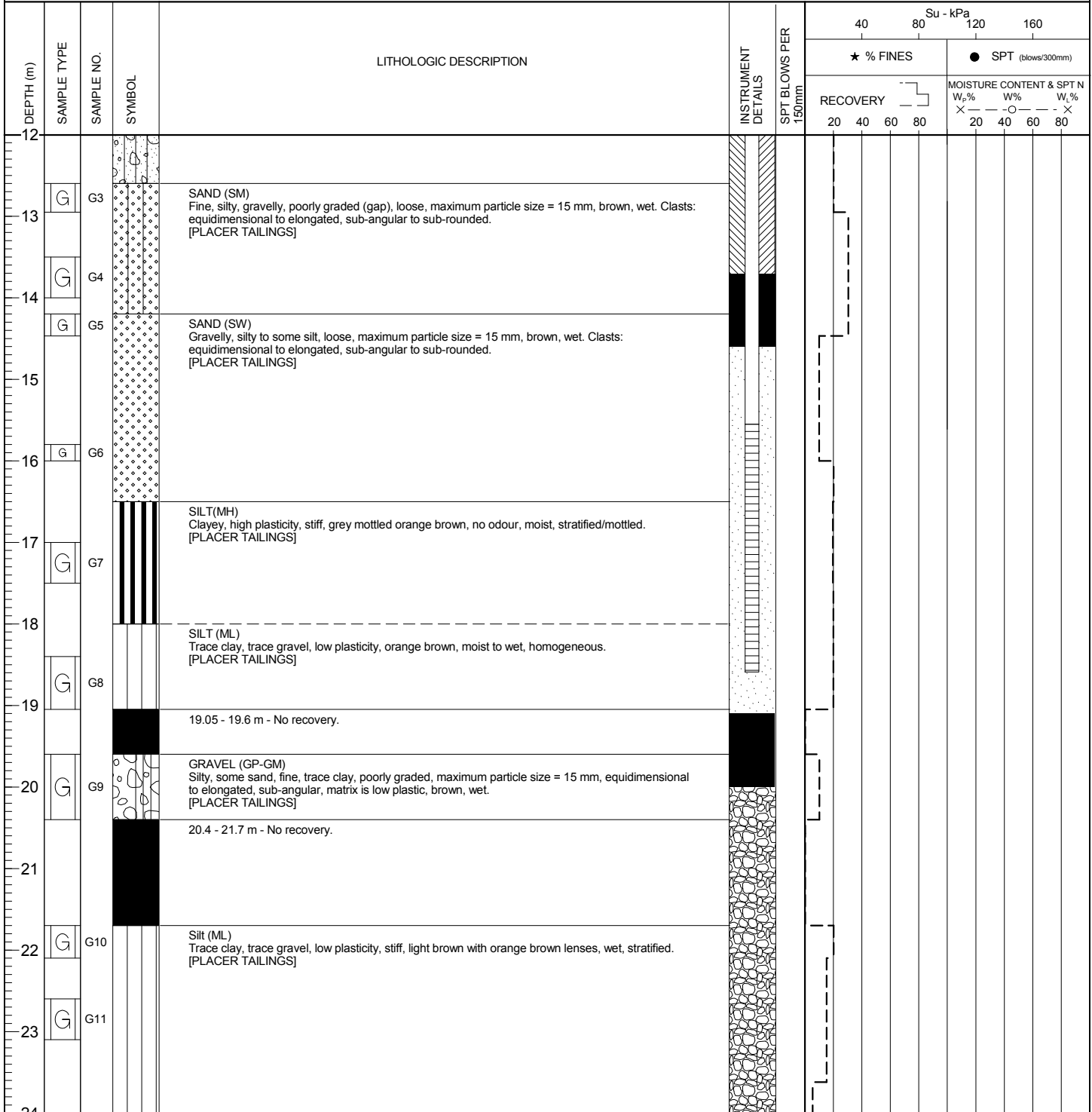
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EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/01/12

CO-ORDINATES (m): 458,462.0E - 7,101,056.0N
 GROUND ELEVATION (m) : 797.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 27 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 28.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT/EC/SD
 REVIEWED BY : PQ/DW



(Continued on next page)

EGR (SOIL) EGR_SOIL_GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-39

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m): 458,462.0E - 7,101,056.0N
 GROUND ELEVATION (m) : 797.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 27 Jul 11
 FINISH DATE : 27 Jul 11
 FINAL DEPTH (m) : 28.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT/EC/SD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa									
							★ % FINES		● SPT (blows/300mm)							
							RECOVERY		MOISTURE CONTENT & SPT N							
							20	40	60	80	W _p %	W ₉₀ %	W _L %	SPT N		
24				24.6 - 25.25 m - Gravelly, some sand, wet. Poor recovery.												
25	G	G12		25.25 - 28.22 m - No recovery. Material would wash from the flights during travel back up through the water and mud infilled hole. Drilling indicates the bit was in gravel.												
26																
27																
28				End of borehole at 28.2 m. Notes: 1) Hole completed due to refusal on cobbles, gravel or weathered rock. 2) Installation of 2" PVC for standpipe piezometer. 3) A separate hole was drilled in direct vicinity of this hole in attempt to auger deeper despite the heaving. This twin hole was used for the piezometer installation.												
29																
30																
31																
32																
33																
34																
35																
36																

EGR/SOIL/ EGR_SOIL_GDL BGC.GDT 12/2/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-40A

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,763.0E - 7,100,038.0N
 GROUND ELEVATION (m) : 1,050.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 12.19

START DATE : 29 Jul 11
 FINISH DATE : 31 Jul 11
 FINAL DEPTH (m) : 33.2
 DEPTH TO TOP OF ROCK (m) : 7.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa						
							★ % FINES		● SPT (blows/300mm)				
							RECOVERY		MOISTURE CONTENT & SPT N				
							20	40	60	80	W _p %	W ₉₀ %	W _L %
0				SAND AND GRAVEL (SW/GW) Well graded, maximum particle size = 250 mm, particles are elongated, angular, brown, moist, homogeneous, non cemented. [FILL]									
1													
2				GRAVEL (GP) Some sand, trace silt, maximum particle size = 60 mm, elongated, subangular to subrounded, blue grey, iron staining, some cementation, clasts are metasedimentary. [HIGHLY TO COMPLETELY WEATHERED METASEDIMENTARY ROCK]									
3													
4													
5													
6													
7													
8				Rock encountered at 7.80 m depth. Refer to rock log.									
9													
10													
11													
12													

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/01/12

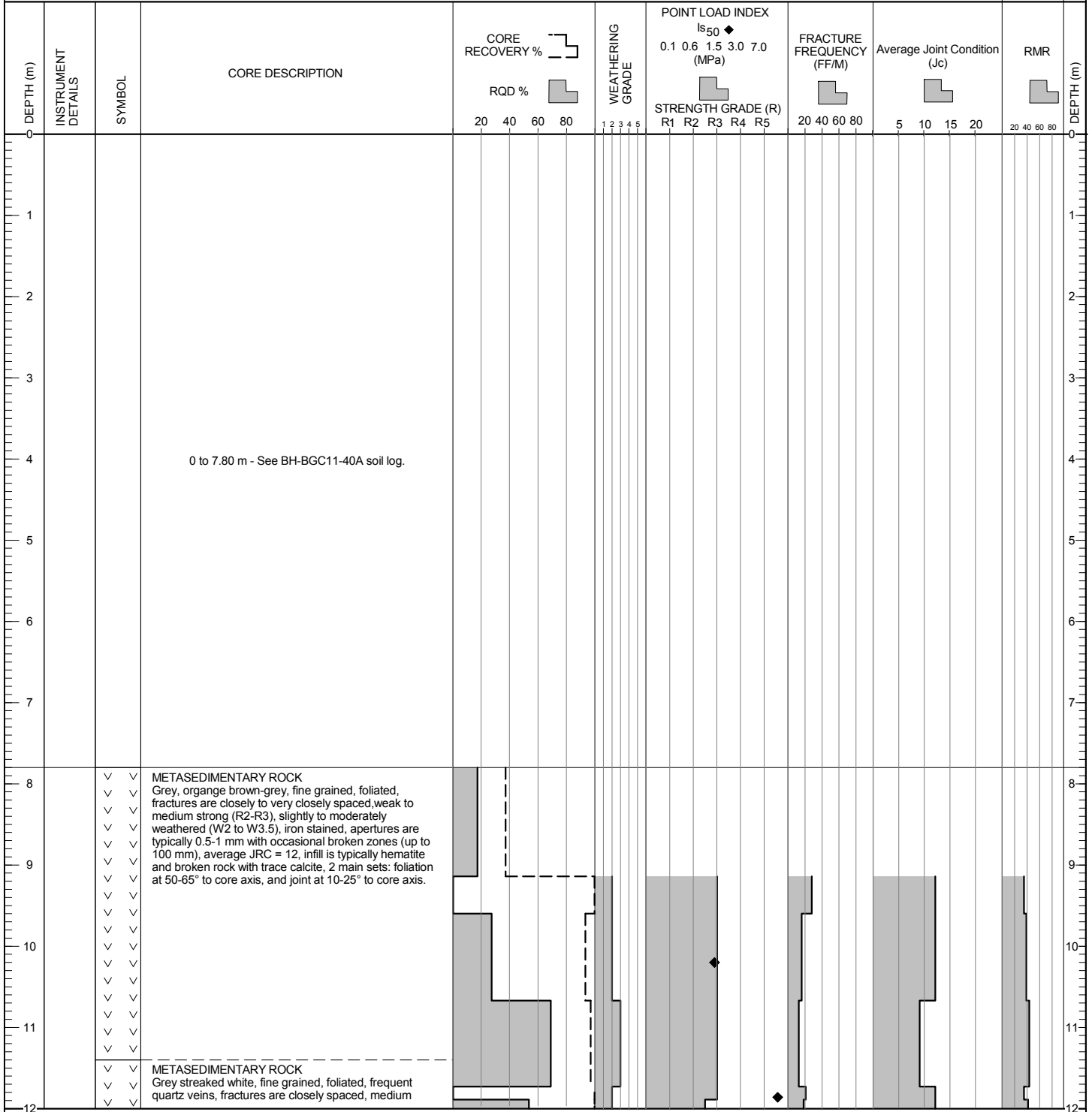


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 459,763.0E - 7,100,038.0N
 GROUND ELEVATION (m) : 1,050.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 12.19

START DATE : 29 Jul 11
 FINISH DATE : 31 Jul 11
 FINAL DEPTH (m) : 33.2
 DEPTH TO TOP OF ROCK (m) : 7.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



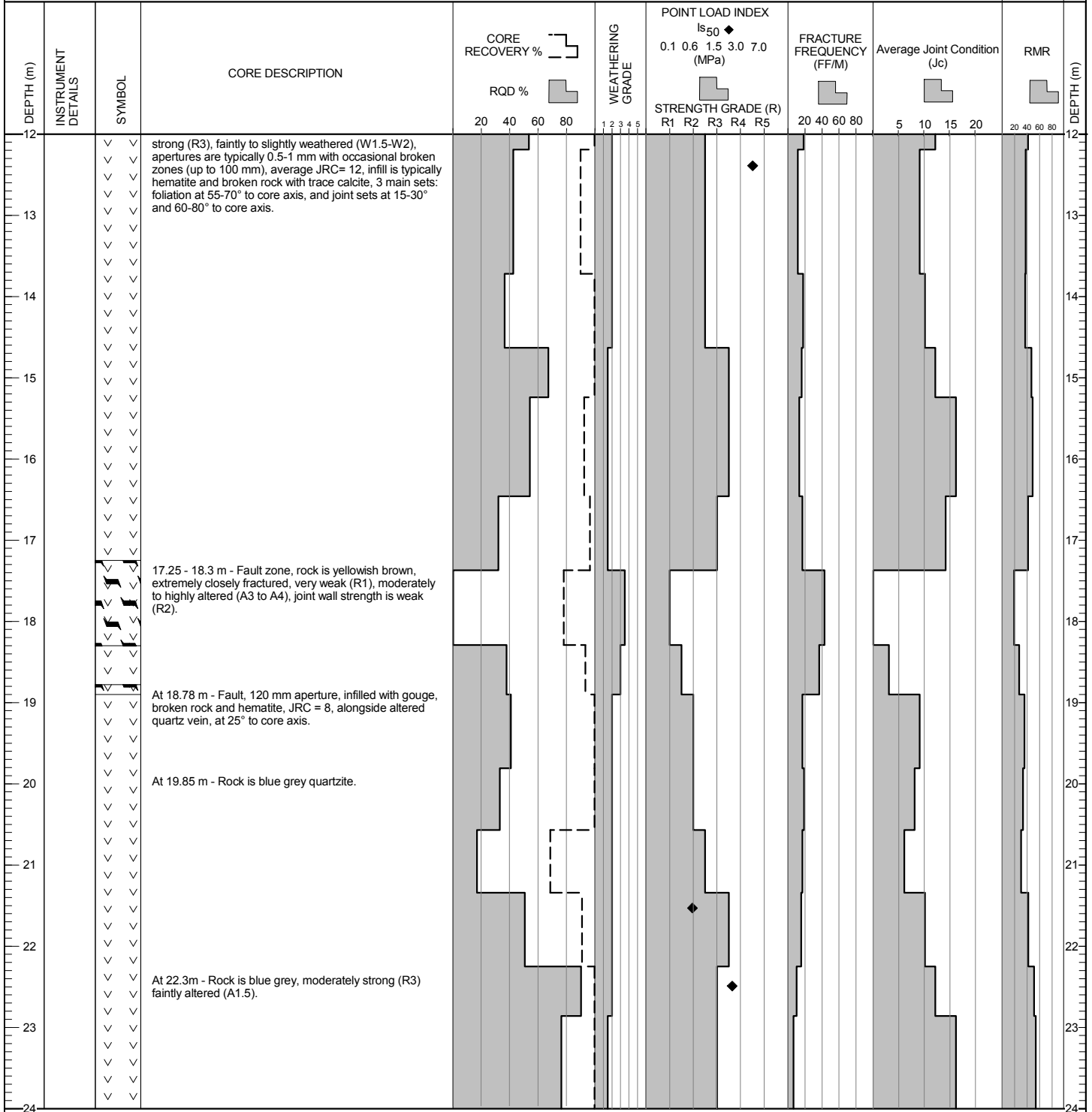
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ESP (ROCK) ESP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,763.0E - 7,100,038.0N
 GROUND ELEVATION (m) : 1,050.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 12.19

START DATE : 29 Jul 11
 FINISH DATE : 31 Jul 11
 FINAL DEPTH (m) : 33.2
 DEPTH TO TOP OF ROCK (m) : 7.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



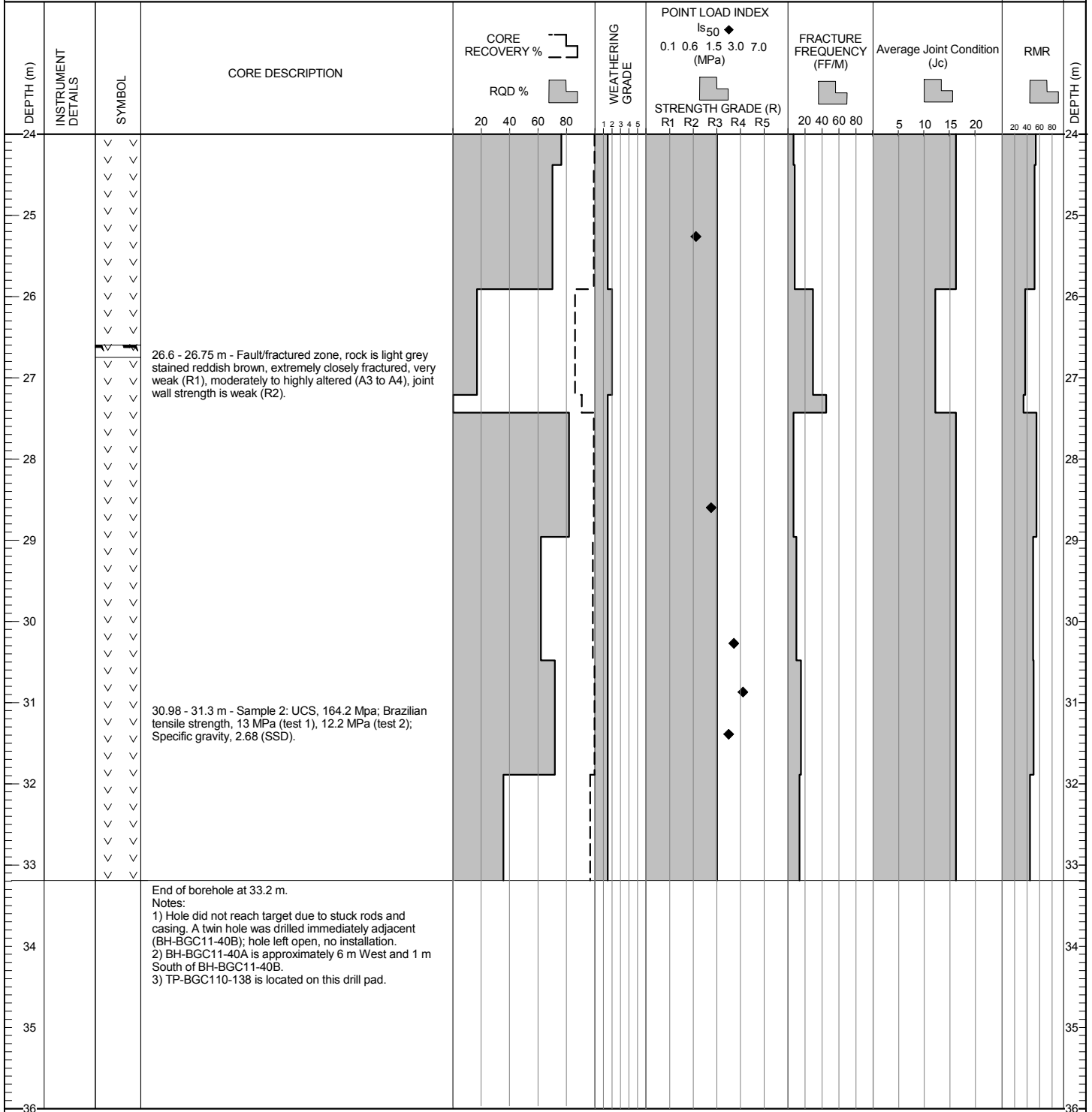
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,763.0E - 7,100,038.0N
 GROUND ELEVATION (m) : 1,050.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 12.19

START DATE : 29 Jul 11
 FINISH DATE : 31 Jul 11
 FINAL DEPTH (m) : 33.2
 DEPTH TO TOP OF ROCK (m) : 7.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-40B

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,767.0E - 7,100,038.8N
 GROUND ELEVATION (m) : 1,049.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) :

START DATE : 31 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 45.7
 DEPTH TO TOP OF ROCK (m) : 8.7
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa									
							★ % FINES		● SPT (blows/300mm)							
							RECOVERY		MOISTURE CONTENT & SPT N							
							20	40	60	80	W _p %	W ₅ %	W ₁ %	W _{0.75} %		
0				0.0 - 8.69 m - No recovery. For soil description please refer to TP-BGC11-138.												
1																
2																
3																
4																
5																
6																
7																
8																
9				Rock encountered at 8.69 m depth. Refer to rock log.												
10																
11																
12																

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/01/12

CO-ORDINATES (m) 459,767.0E - 7,100,038.8N
 GROUND ELEVATION (m) : 1,049.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m):

START DATE : 31 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 45.7
 DEPTH TO TOP OF ROCK (m) : 8.7
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)	
				CORE RECOVERY %	RQD %		R1	R2	R3	R4	R5					
0															0	
1															1	
2															2	
3															3	
4															4	
5			0 to 8.69 m - See BH-BGC11-40B soil log.												5	
6															6	
7															7	
8															8	
9		✓	METASEDIMENTARY ROCK Dark grey and orangish, weak to medium strong (R2.5 to R3), slightly to moderately weathered (W2-W3), foliated, fine grained, 2 to 3 main sets: foliation at 35-45° and diagonal crossing joints at 20-40 and 50-60° to core axis, hematite infill, JRC 4-8, tight to very tight, joint set is 20-25° to core axis, hematite clay and crushed rock infill, JRC approximately 8, tight to partly open, fractures are closely spaced.												9	
10		✓														10
11		✓														11
12		✓														12

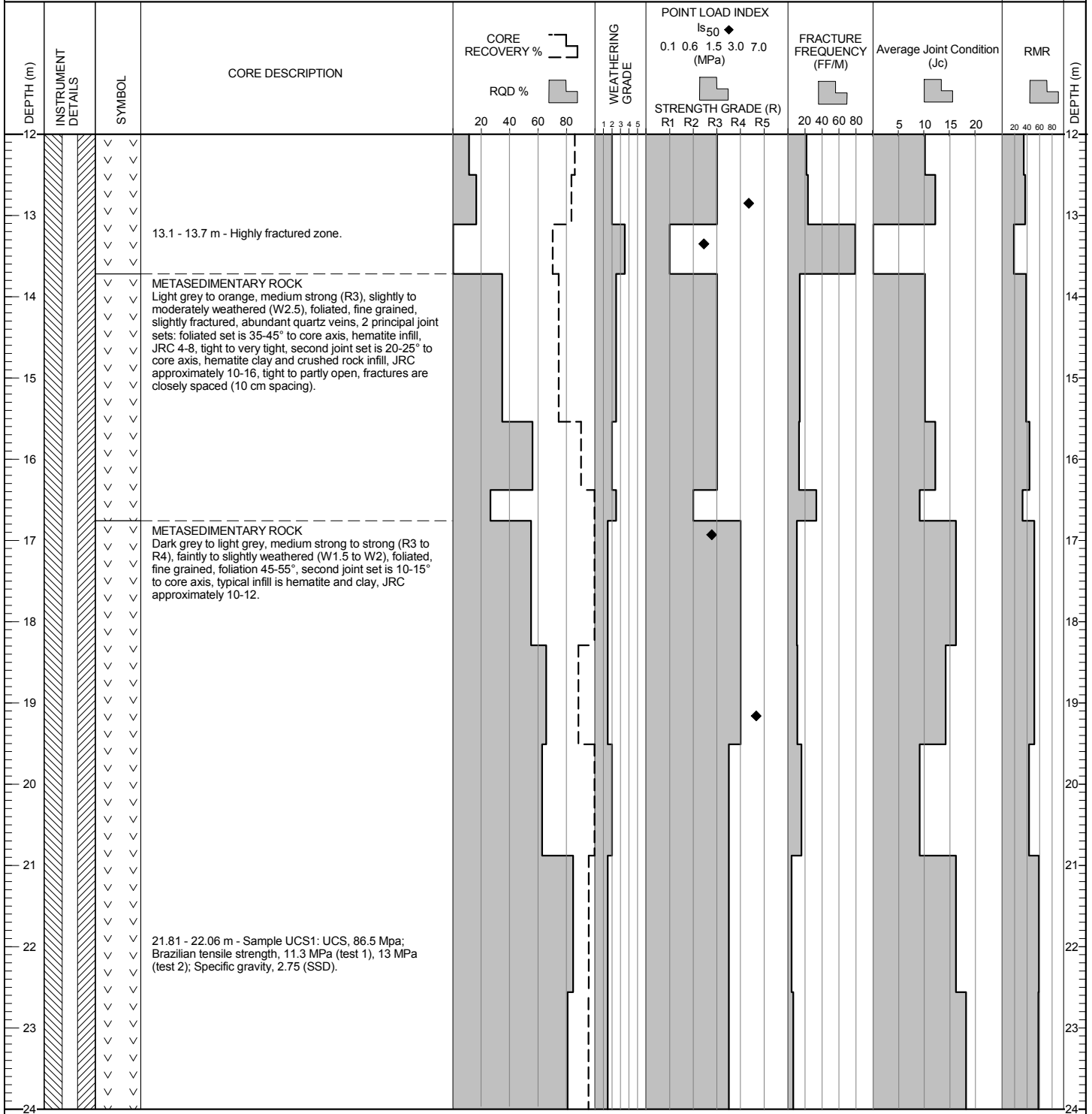
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,767.0E - 7,100,038.8N
 GROUND ELEVATION (m) : 1,049.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 31 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 45.7
 DEPTH TO TOP OF ROCK (m) : 8.7
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



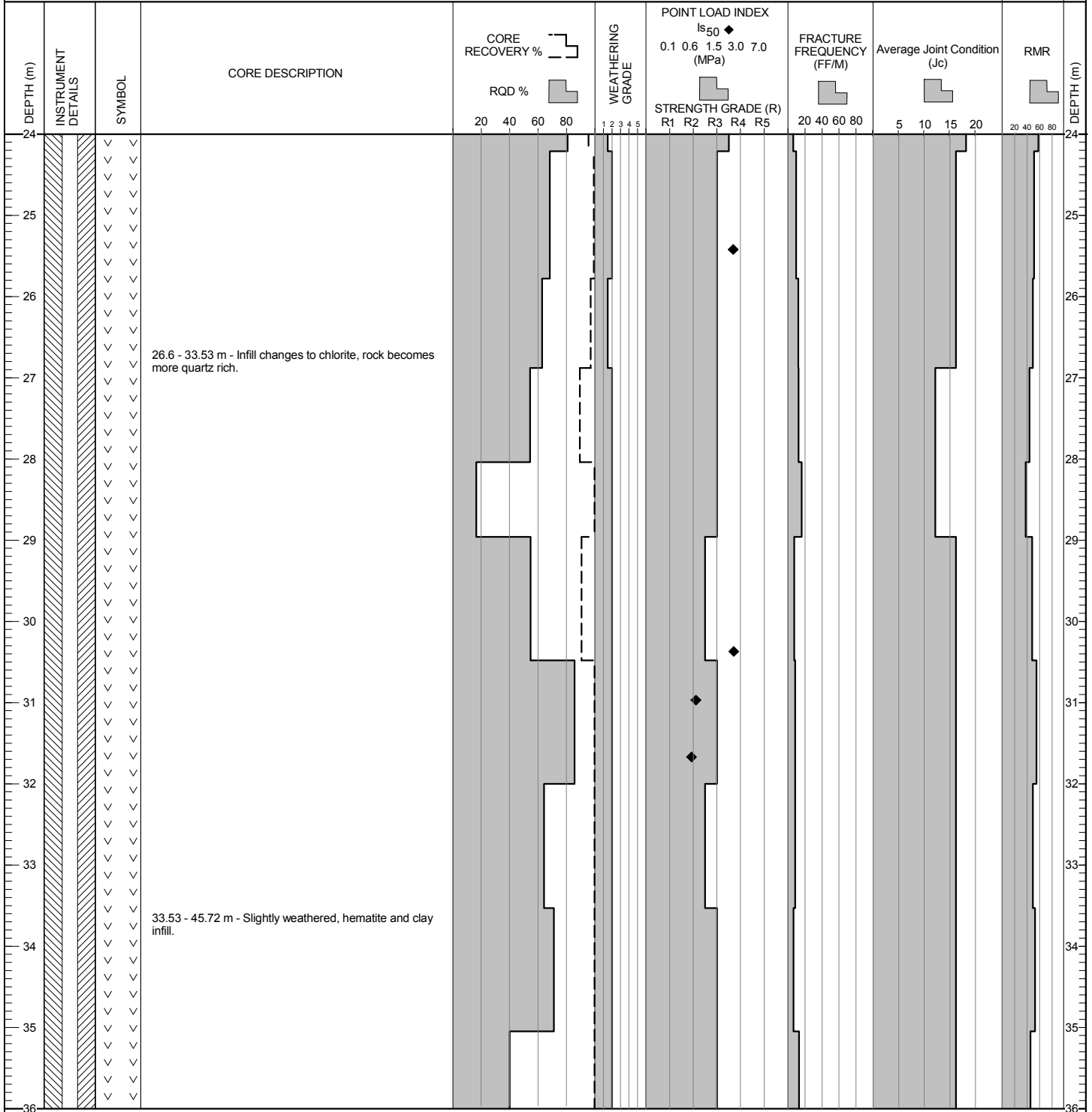
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,767.0E - 7,100,038.8N
 GROUND ELEVATION (m) : 1,049.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 31 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 45.7
 DEPTH TO TOP OF ROCK (m) : 8.7
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



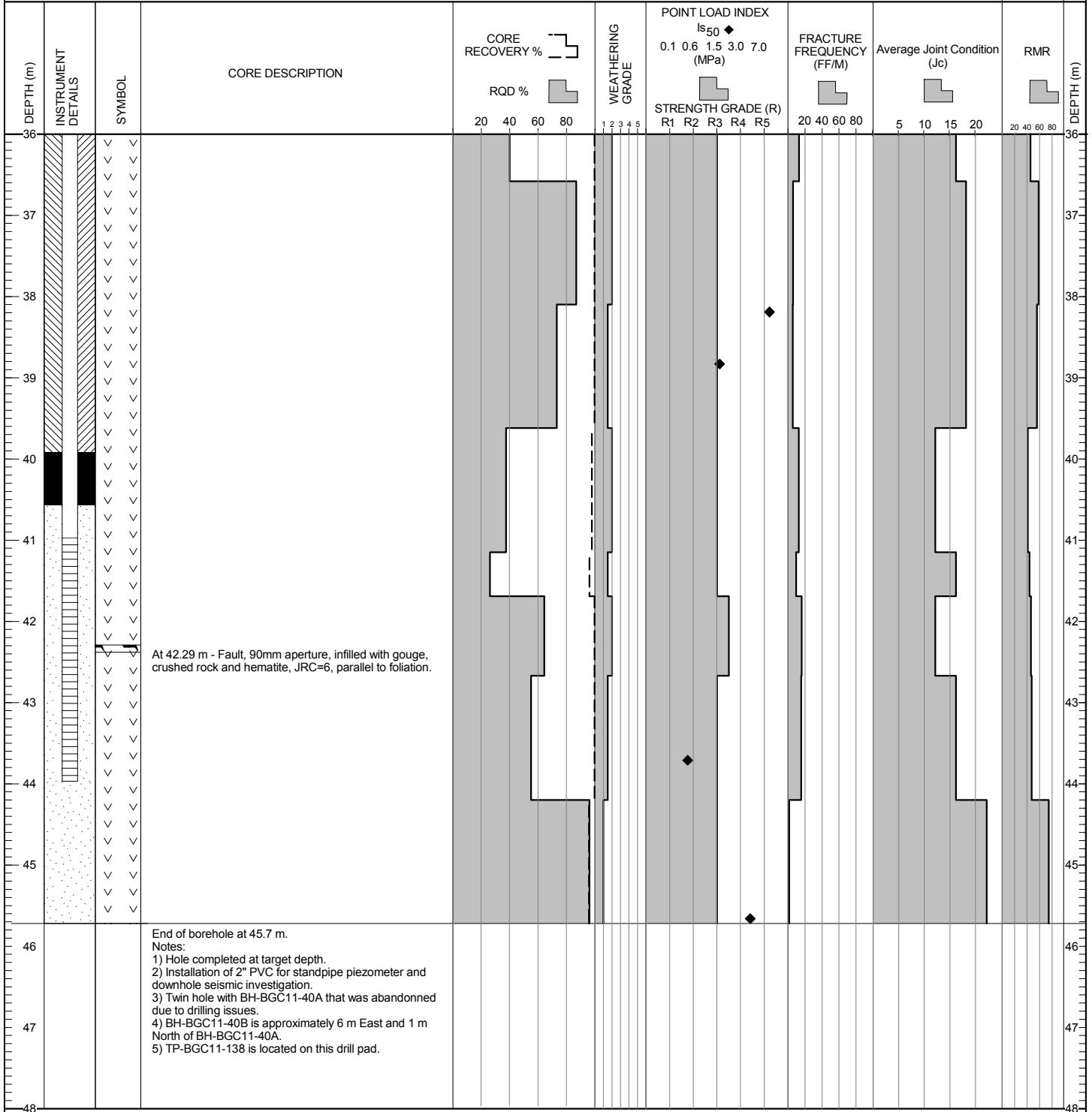
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,767.0E - 7,100,038.8N
 GROUND ELEVATION (m) : 1,049.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-10
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 31 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 45.7
 DEPTH TO TOP OF ROCK (m) : 8.7
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



At 42.29 m - Fault, 90mm aperture, infilled with gouge, crushed rock and hematite, JRC=6, parallel to foliation.

EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-41

LOCATION : EAGLE PUP

CO-ORDINATES (m) : 459,777.0E - 7,100,999.0N
 GROUND ELEVATION (m) : 914.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 29 Jul 11
 FINISH DATE : 29 Jul 11
 FINAL DEPTH (m) : 4.3
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	Su - kPa						
						★ % FINES		● SPT (blows/300mm)				
						RECOVERY		MOISTURE CONTENT & SPT N				
						20	40	60	80	W _p %	W _l %	W _u %
0				ORGANICS Rootmat, silt and sand, some organics, moist, brown. [TOPSOIL]								
0.5	G	1		SAND AND GRAVEL (SW/GW) Trace silt, trace cobbles, trace boulders, well graded, loose, maximum particle size = 350 mm, greyish brown, dry to moist, homogeneous. Clasts: elongated to equidimensional, sub-rounded to angular, metasediment and granodiorite (the boulders are granodiorite). Metasedimentary clasts: strong (R4). Granodiorite clasts: very strong (R5) and faintly weathered (W1.5). [COLLUVIUM]								
3.0				SAND (SW) Gravelly, well graded, compact, maximum particle size= 50 mm, equidimensional to elongated, subangular to angular, greyish brown, dry to moist, homogeneous. Clasts: medium strong to strong (R3 to R4). [MODERATELY TO HIGHLY WEATHERED ROCK]								
4.3				End of borehole at 4.3 m. Notes: 1) Refusal on bedrock. 2) Backfilled with cuttings. 3) No water encountered. 4) Attempted to drill deeper in four different spots, but couldn't exceed a depth of 4.3 m. 5) 0.0-2.8 m logged from existing cut slope above pad, 2.8-4.3 m logged from auger hole (2.8m BGL was the pad level).								

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

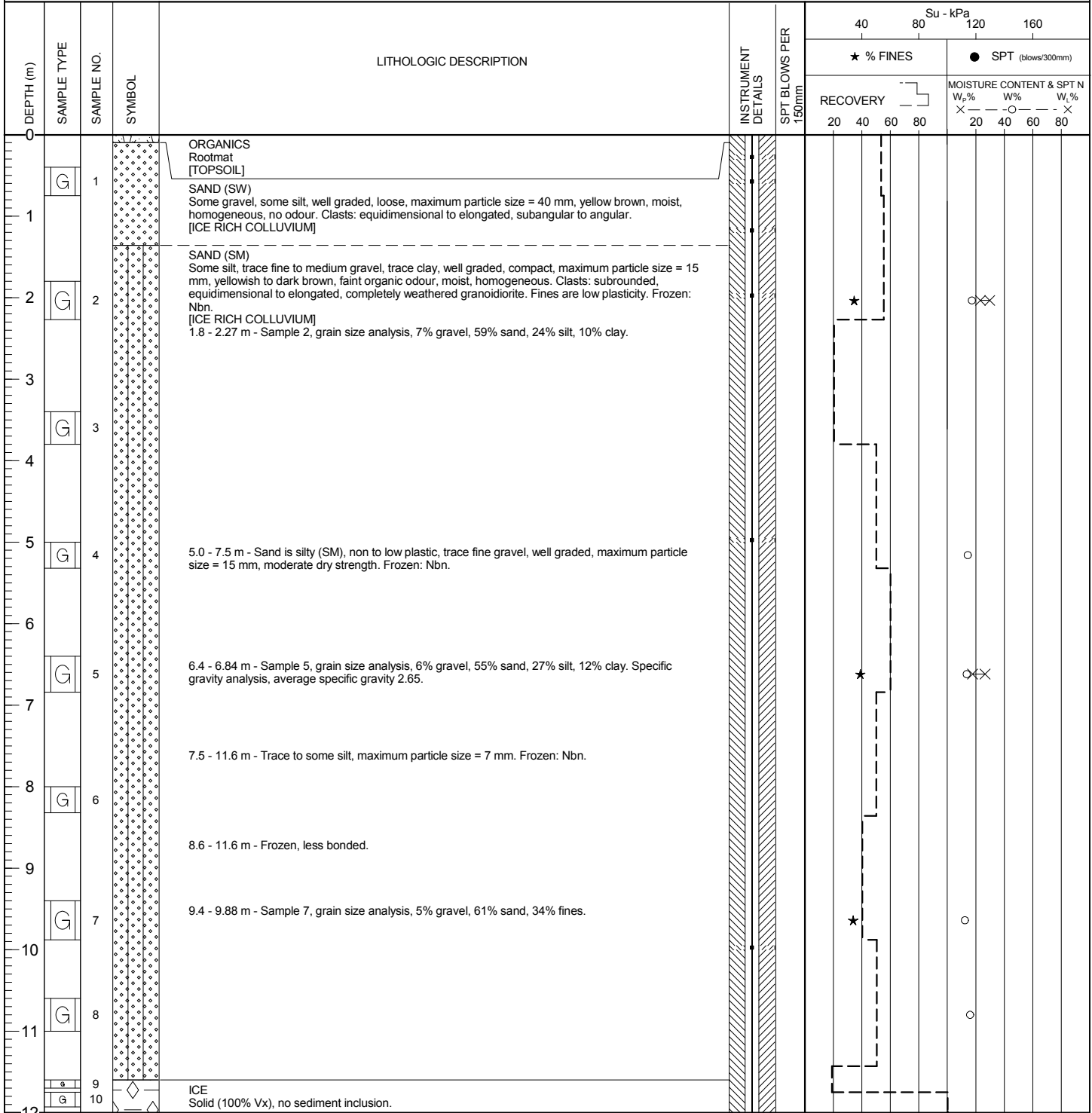
DRILL HOLE # BH-BGC11-42

LOCATION : EAGLE PUP

CO-ORDINATES (m): 460,272.3E - 7,100,150.1N
 GROUND ELEVATION (m) : 1,098.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 29 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 28.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT
 REVIEWED BY : PQ/DW



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EGR (SOIL) EGR_SOIL_GDL BGC.GDT 1/20/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m): 460,272.3E - 7,100,150.1N
 GROUND ELEVATION (m) : 1,098.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 29 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 28.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa							
							★ % FINES		● SPT (blows/300mm)					
						RECOVERY		MOISTURE CONTENT & SPT N						
						20	40	60	80	W ₅ %	W ₁₀ %	W _L %	W _U %	
12				11.7 - 15.85 m - Drilled with CRREL barrel. 11.73 - 11.93 m - Bed of SAND (SW) Some silt, wet, brown. Frozen: Vx (20-30%), ice crystals, no sediment inclusions. 12.12 - 12.17 m - Ice with sediment inclusions (30-40%), presence of 2 subhorizontal fractures in the ice. 12.35 - 12.5 m - 0-2% sediment inclusions along subhorizontal beds. Below 12.9 m - Ice grades to ice with sediment inclusions (10-20%).										
13	G	12		SAND AND GRAVEL (SM/GM) Some silt, well graded, maximum particle size = 60 mm, orange, wet, homogeneous. Clasts: equidimensional, subrounded, highly weathered granodiorite, very weak to weak (R1 to R2). Frozen: Vx (5-10%), occasional blocks of ice 30-50 mm thick, with 50% sediment inclusions. [ICE RICH COLLUVIUM]		★								
14	G	13		13.3 - 13.65 m - Sample 12, grain size analysis, 31% gravel, 60% sand, 9% fines. At 13.86 m - A layer of Vx, 45 mm thick, <10% sediment inclusion. At 14.26 m - Vx (5-10%), large gravel size blocks of very weak granodiorite present.										
15	G	15		At 14.62 m - There is 170 mm of ICE, colourless, clear, no bubbles, organized in 3-5 mm subhorizontal lenses, <5% sediment inclusion. At 14.9 m - Frozen: Vx (5%).										
16	G	16		SAND AND GRAVEL (SM/GM) Some silt, trace cobbles, well graded, maximum particle size = 80 mm, equidimensional to elongated, subrounded to angular, very weak to weak (R1 to R2) granodiorite, clasts are slightly to moderately weathered (W2.5 to W3), brownish grey, wet, homogeneous. Frozen: Nbn. [ICE RICH COLLUVIUM]										
17	G	17		At 15.4 m - Frozen: Vx (5-10%), maximum particle size = 95 mm (granodiorite cobble, medium strong to strong (R3 to R4), slightly weathered (W2)). At 15.85 m - CRREL barrel refused on cobbles - switched to solid stem auger.										
18				SAND (SM) Trace fine gravel, well graded, maximum particle size = 10 mm yellowish brown, wet, homogeneous. Clasts: granodiorite, subrounded, equidimensional, weak (R2), slightly to moderately weathered (W2.5). Frozen: Vx (5-10%). [ICE RICH COLLUVIUM]										
19				17.53 - 18.3 m - Drilled with CRREL barrel. At 17.9 m - Moderately weathered granodiorite boulder R0 to R1.										
20	G	20		SAND AND GRAVEL (SM/GM) Fine to coarse, well graded, maximum particle size = 65 mm, pinkish brown, wet, homogeneous. Clasts: granodiorite, angular to subangular, weak (R2), slightly to moderately weathered (W2 to W2.5). Frozen: Vx (0-10%). [ICE RICH COLLUVIUM]		★								
21	G	21		19.7 - 20.0 m - Sample 20, grain size analysis, 10% gravel, 64% sand, 16% silt, 10% clay.										
22	G	22		21.55 - 21.9 m - Sample 22, grain size analysis, 4% gravel, 66% sand, 18% silt, 12% clay. At 22.0 m - Not visible ice, penetration is more difficult for auger, disturbed.		★								
23	G	23												
24														

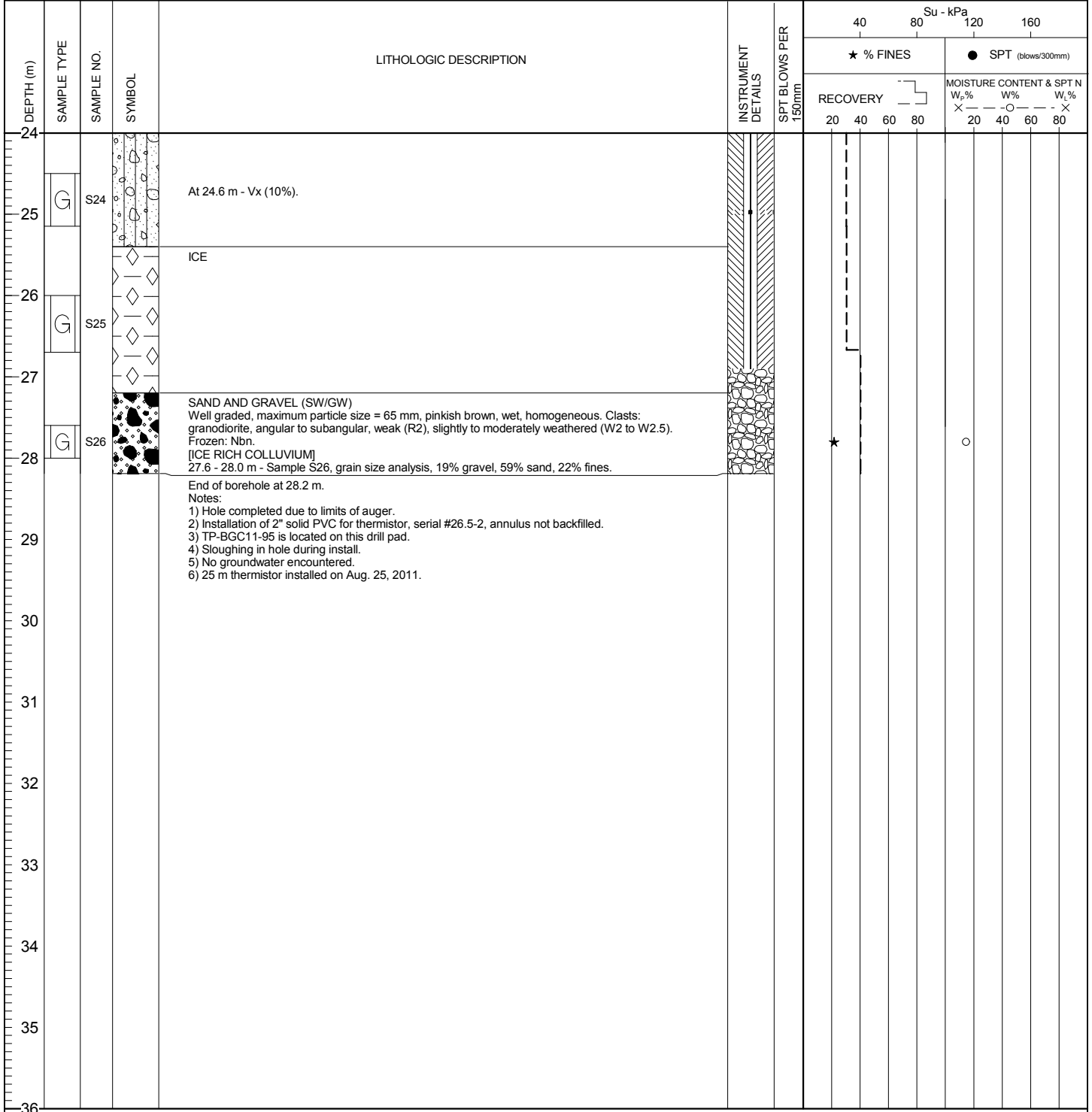
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EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 460,272.3E - 7,100,150.1N
 GROUND ELEVATION (m) : 1,098.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 29 Jul 11
 FINISH DATE : 01 Aug 11
 FINAL DEPTH (m) : 28.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT
 REVIEWED BY : PQ/DW



EGR/SOIL/ EGR_SOIL_GDL BGC.GDT 12/01/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-43

LOCATION : PLATINUM GULCH

CO-ORDINATES (m): 459,658.1E - 7,099,106.6N
 GROUND ELEVATION (m) : 1,034.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 1.52

START DATE : 02 Aug 11
 FINISH DATE : 02 Aug 11
 FINAL DEPTH (m) : 23.8
 DEPTH TO TOP OF ROCK (m) : 0.9
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa				MOISTURE CONTENT & SPT N			
							★ % FINES		● SPT (blows/300mm)		RECOVERY		W _p %	
							20	40	60	80	20	40	60	80
0				GRAVEL (GP) Some sand, trace silt, maximum particle size = 60 mm, elongated, subangular to subrounded, blue grey, iron staining, some cementation, clasts are metasedimentary. [HIGHLY TO COMPLETELY WEATHERED METASEDIMENTARY ROCK]										
1				Rock encountered at 0.90 m depth. Refer to rock log.										
2														
3														
4														
5														
6														
7														
8														
9														
10														
11														
12														

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/01/12

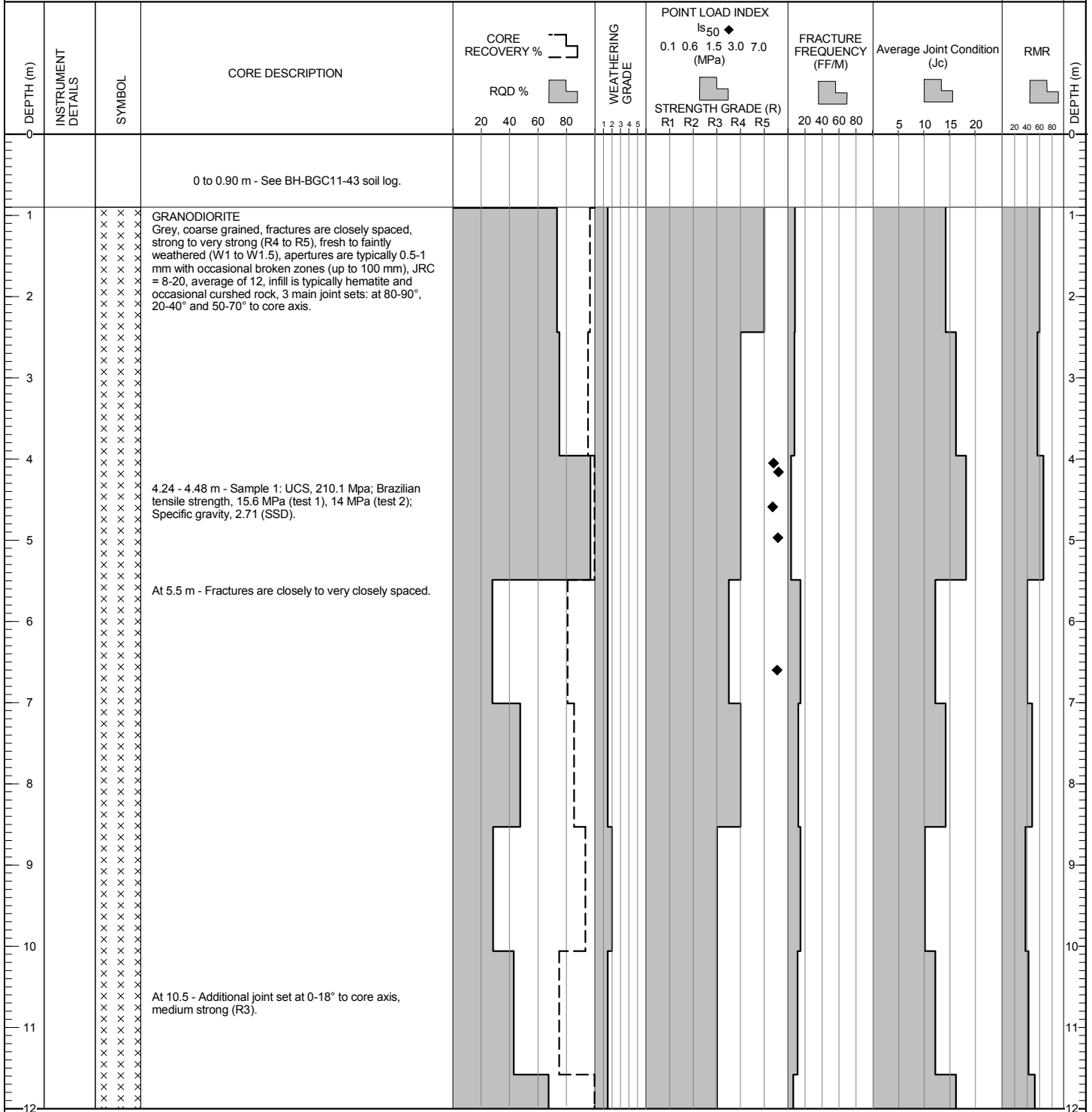


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 459,658.1E - 7,099,106.6N
 GROUND ELEVATION (m) : 1,034.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 1.52

START DATE : 02 Aug 11
 FINISH DATE : 02 Aug 11
 FINAL DEPTH (m) : 23.8
 DEPTH TO TOP OF ROCK (m) : 0.9
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

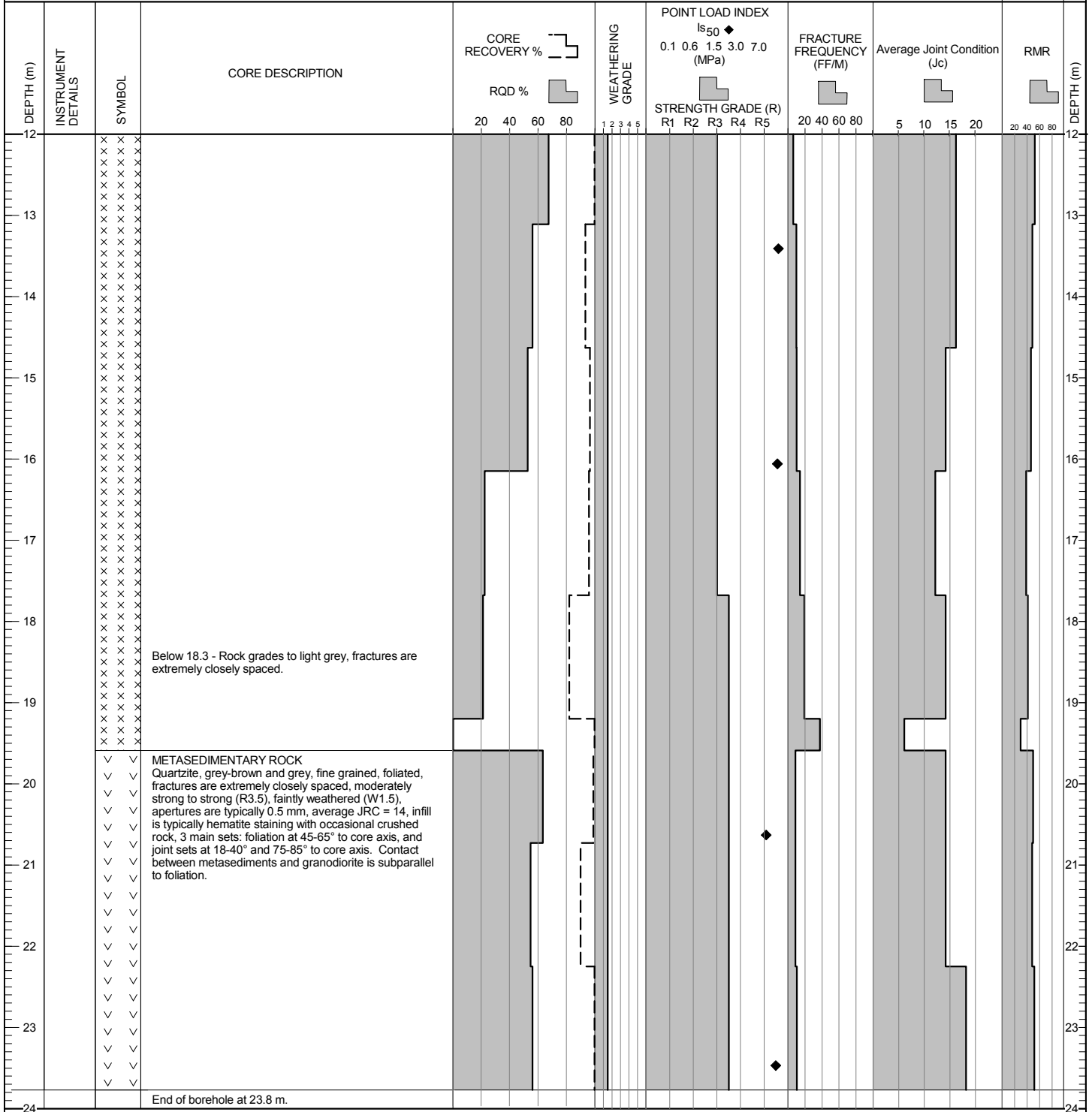


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CO-ORDINATES (m) 459,658.1E - 7,099,106.6N
 GROUND ELEVATION (m) : 1,034.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 1.52

START DATE : 02 Aug 11
 FINISH DATE : 02 Aug 11
 FINAL DEPTH (m) : 23.8
 DEPTH TO TOP OF ROCK (m) : 0.9
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



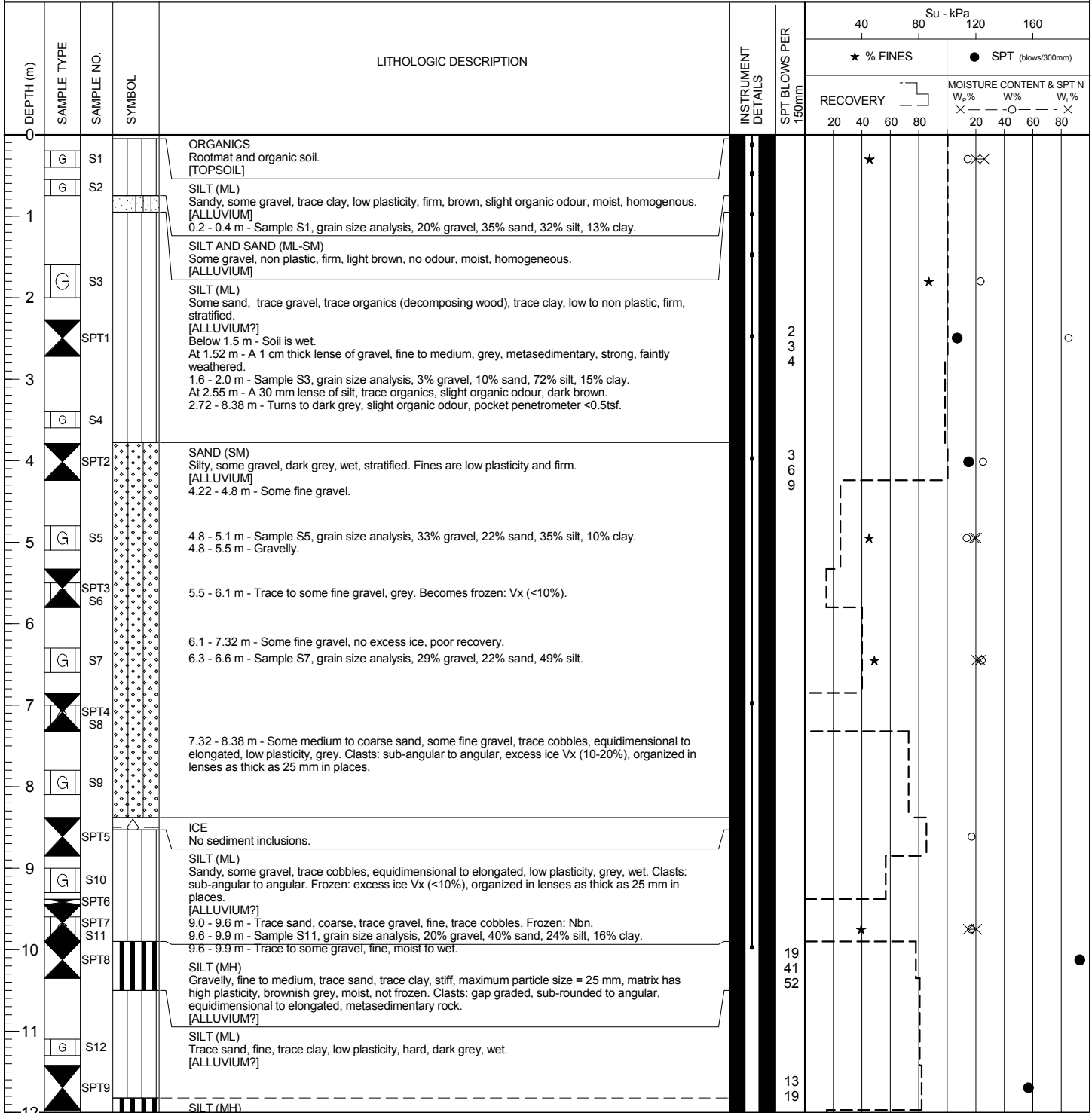
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 458,690.4E - 7,100,547.3N
 GROUND ELEVATION (m) : 830.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 03 Aug 11
 FINISH DATE : 03 Aug 11
 FINAL DEPTH (m) : 14.7
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT
 REVIEWED BY : PQ/DW



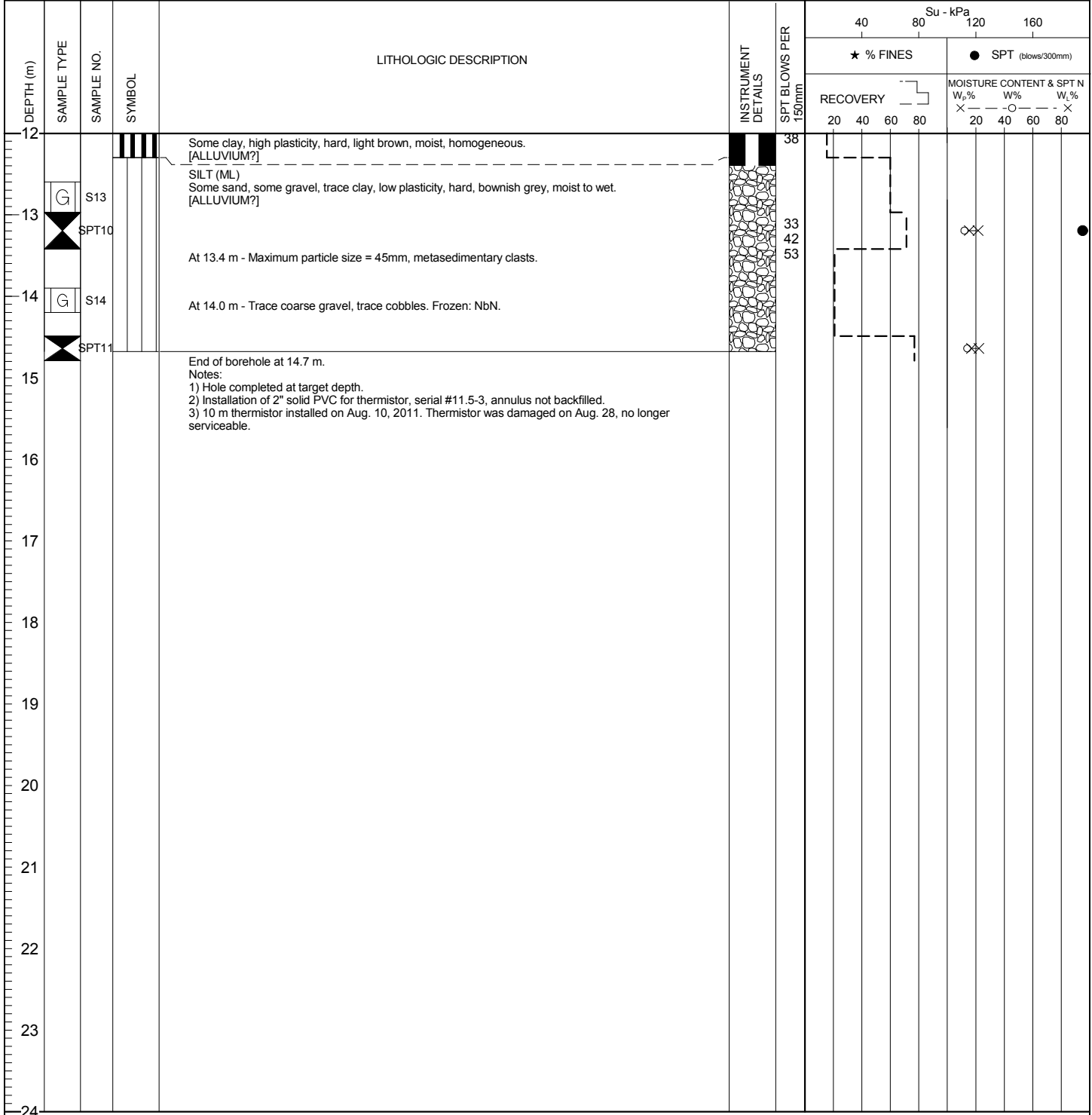
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EGR (SOIL) EGR_SOIL.GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 458,690.4E - 7,100,547.3N
 GROUND ELEVATION (m) : 830.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 03 Aug 11
 FINISH DATE : 03 Aug 11
 FINAL DEPTH (m) : 14.7
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-45

LOCATION : PLATINUM GULCH

CO-ORDINATES (m): 460,556.2E - 7,099,109.5N
 GROUND ELEVATION (m) : 1,354.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 3.05

START DATE : 03 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 20.7
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa							
							★ % FINES	● SPT (blows/300mm)	MOISTURE CONTENT & SPT N					
							RECOVERY		W _p %	W%	W _L %			
							20	40	60	80	20	40	60	80
0				0.0 - 3.05 m - No recovery. For soil description refer to log of TP-BGC11-102, excavated on this drill pad.										
1														
2														
3				Rock encountered at 3.05 m depth. Refer to rock log.										
4														
5														
6														
7														
8														
9														
10														
11														
12														

EGR/SOIL/ EGP_SOIL_GDL BGC.GDT 12/9/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-45

LOCATION : PLATINUM GULCH

CO-ORDINATES (m) 460,556.2E - 7,099,109.5N
 GROUND ELEVATION (m) : 1,354.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 03 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 20.7
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



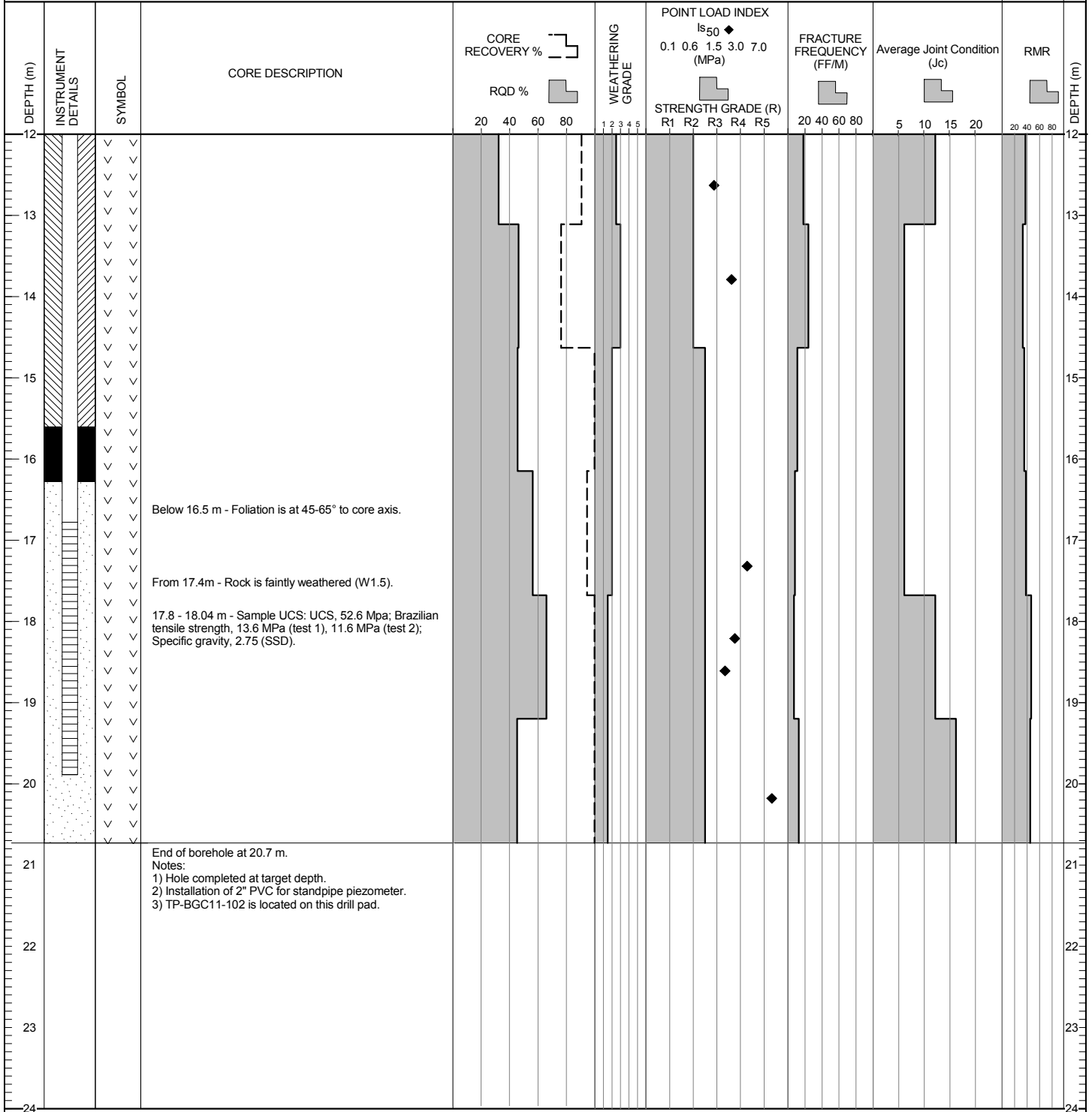
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,556.2E - 7,099,109.5N
 GROUND ELEVATION (m) : 1,354.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 03 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 20.7
 DEPTH TO TOP OF ROCK (m) : 3.1
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006


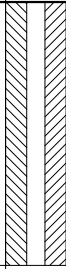
DRILL HOLE # BH-BGC11-46

LOCATION : PLATINUM GULCH

CO-ORDINATES (m): 460,262.7E - 7,099,004.0N
 GROUND ELEVATION (m) : 1,246.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 10.91

START DATE : 04 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 20.1
 DEPTH TO TOP OF ROCK (m) : 2.4
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa						
							★ % FINES		● SPT (blows/300mm)				
							RECOVERY		MOISTURE CONTENT & SPT N				
							20	40	60	80	W _p %	W _l %	W _u %
0				COBBLES Granodiorite and quartzite cobbles. [FILL/COLLUVIUM?]									
2.4				Rock encountered at 2.44 m depth. Refer to rock log.									
1													
2													
3													
4													
5													
6													
7													
8													
9													
10													
11													
12													

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/9/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

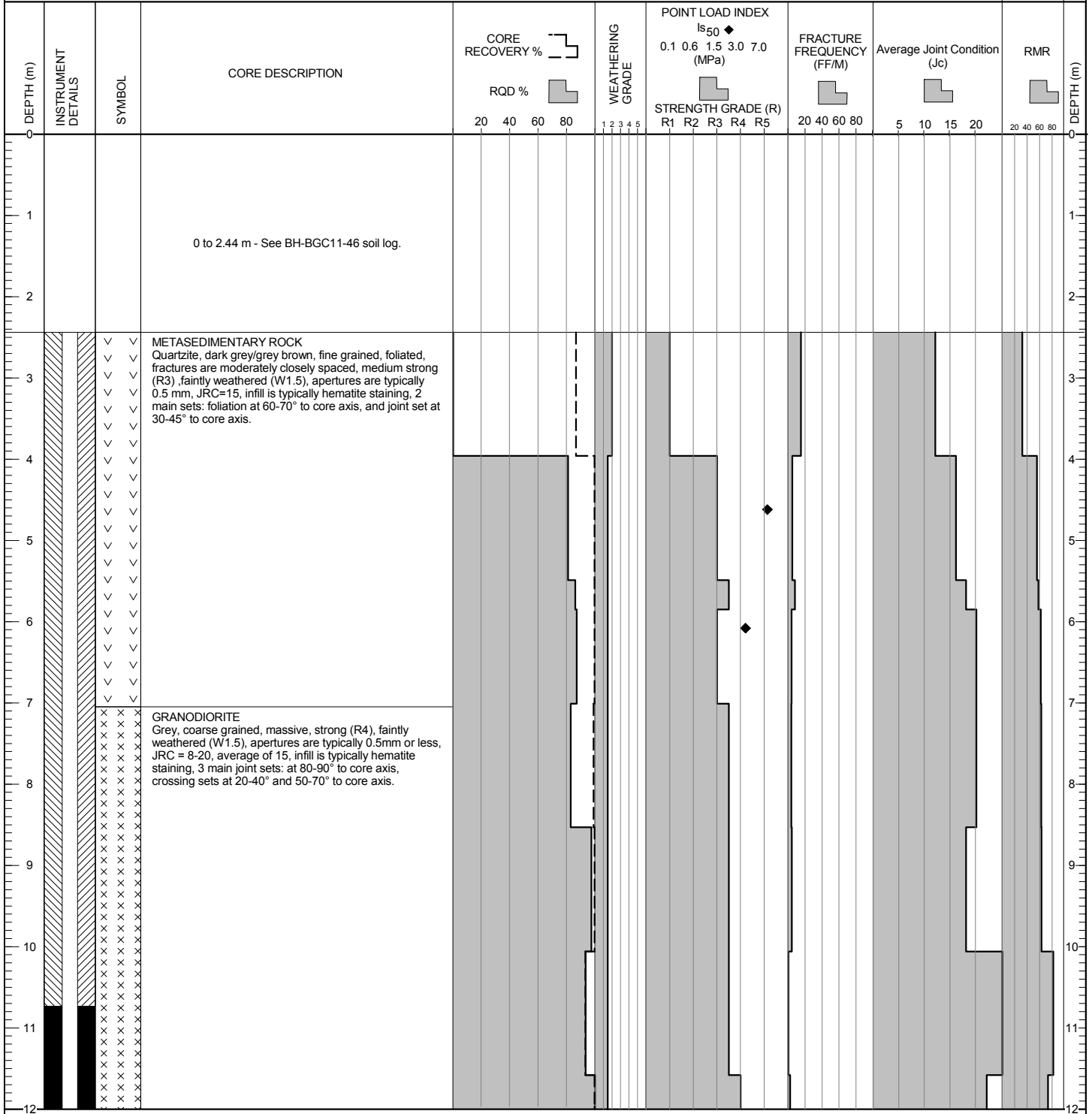
DRILL HOLE # BH-BGC11-46

LOCATION : PLATINUM GULCH

CO-ORDINATES (m) 460,262.7E - 7,099,004.0N
 GROUND ELEVATION (m) : 1,246.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 10.91

START DATE : 04 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 20.1
 DEPTH TO TOP OF ROCK (m) : 2.4
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



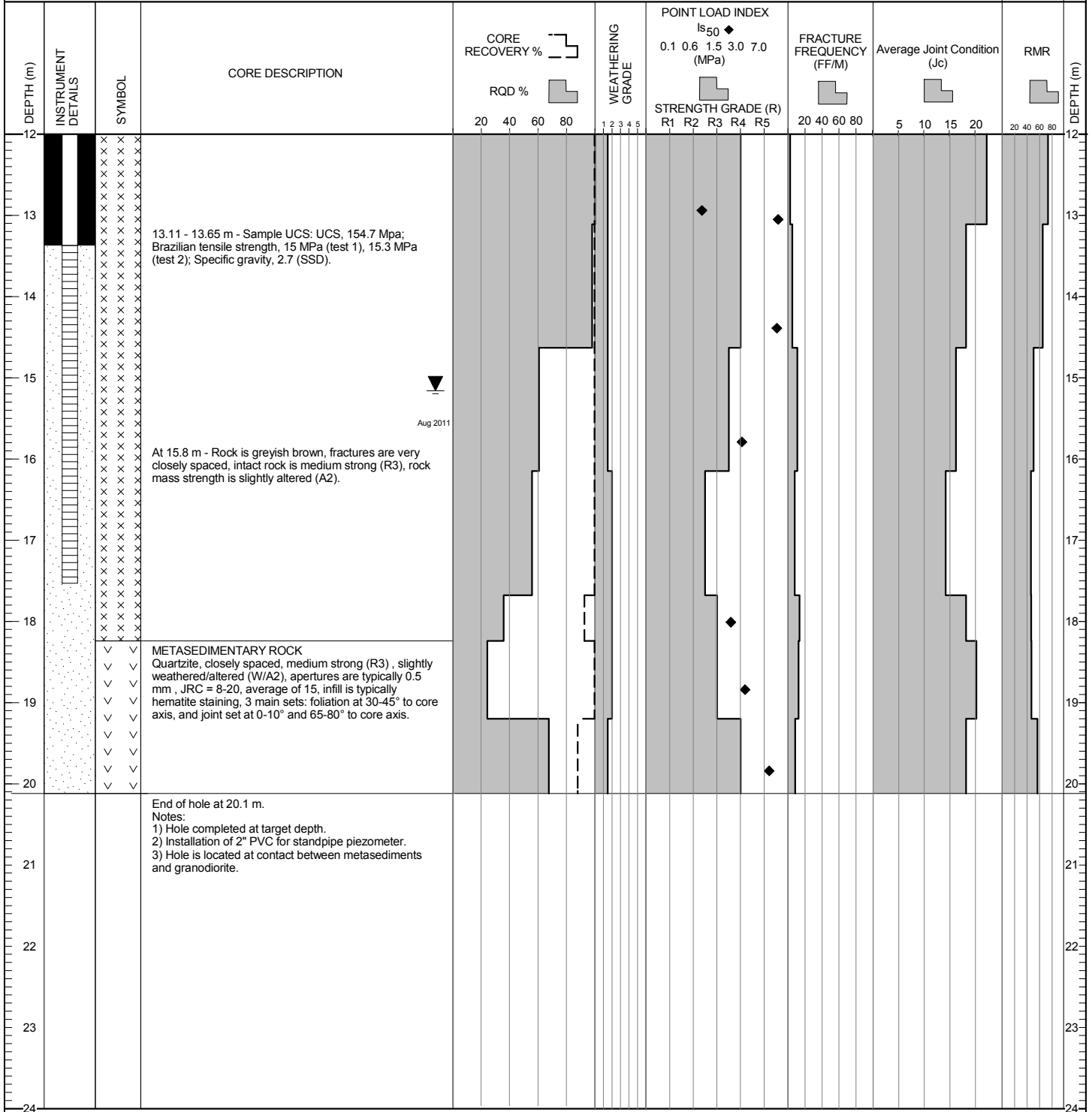
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ESP (ROCK) ESP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,262.7E - 7,099,004.0N
 GROUND ELEVATION (m) : 1,246.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 10.91

START DATE : 04 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 20.1
 DEPTH TO TOP OF ROCK (m) : 2.4
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

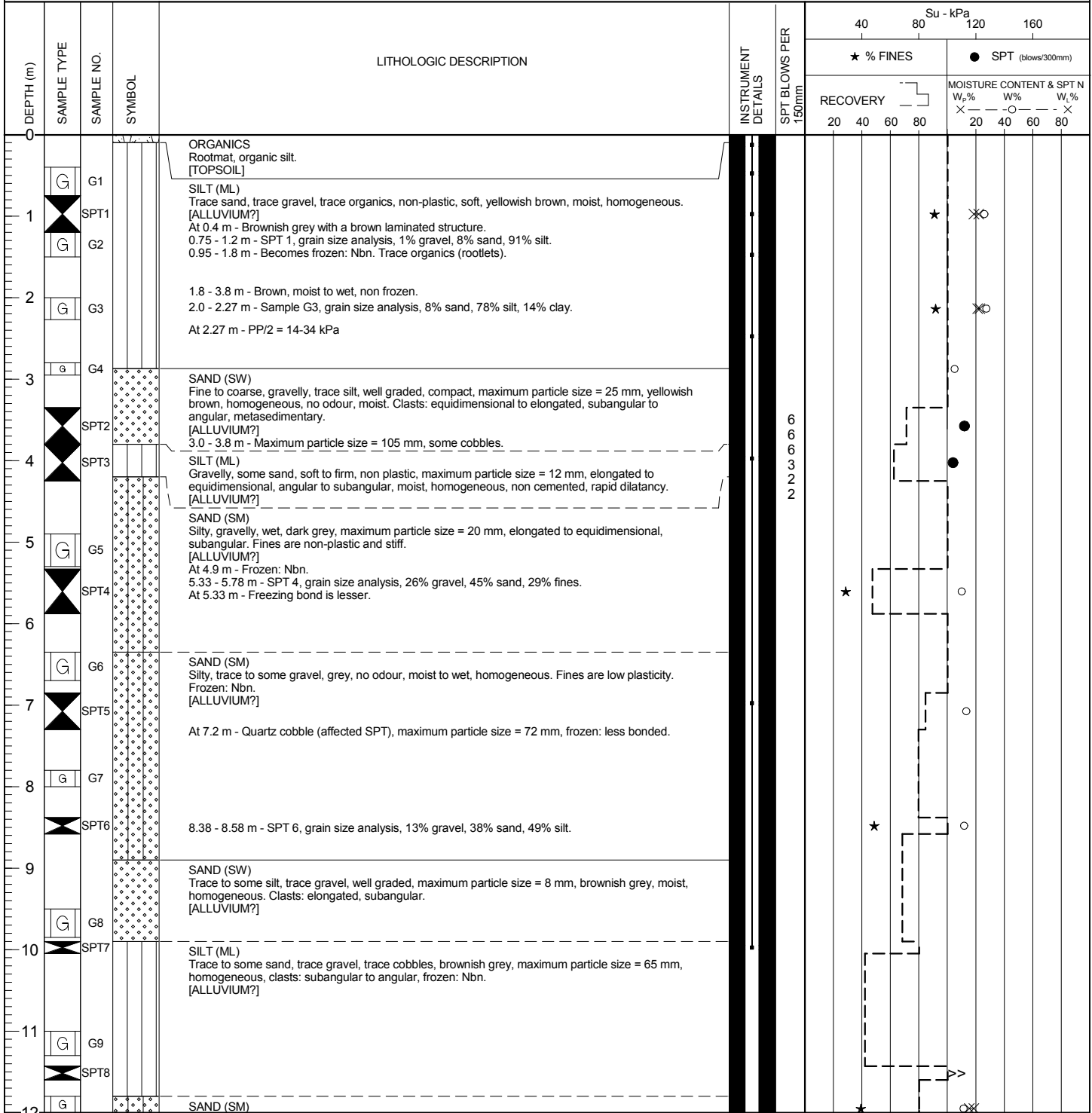


ESP (ROCK) ESP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 458,811.9E - 7,100,698.9N
 GROUND ELEVATION (m) : 841.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/HSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 04 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 16.0
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT/AKU
 REVIEWED BY : PQ/DW



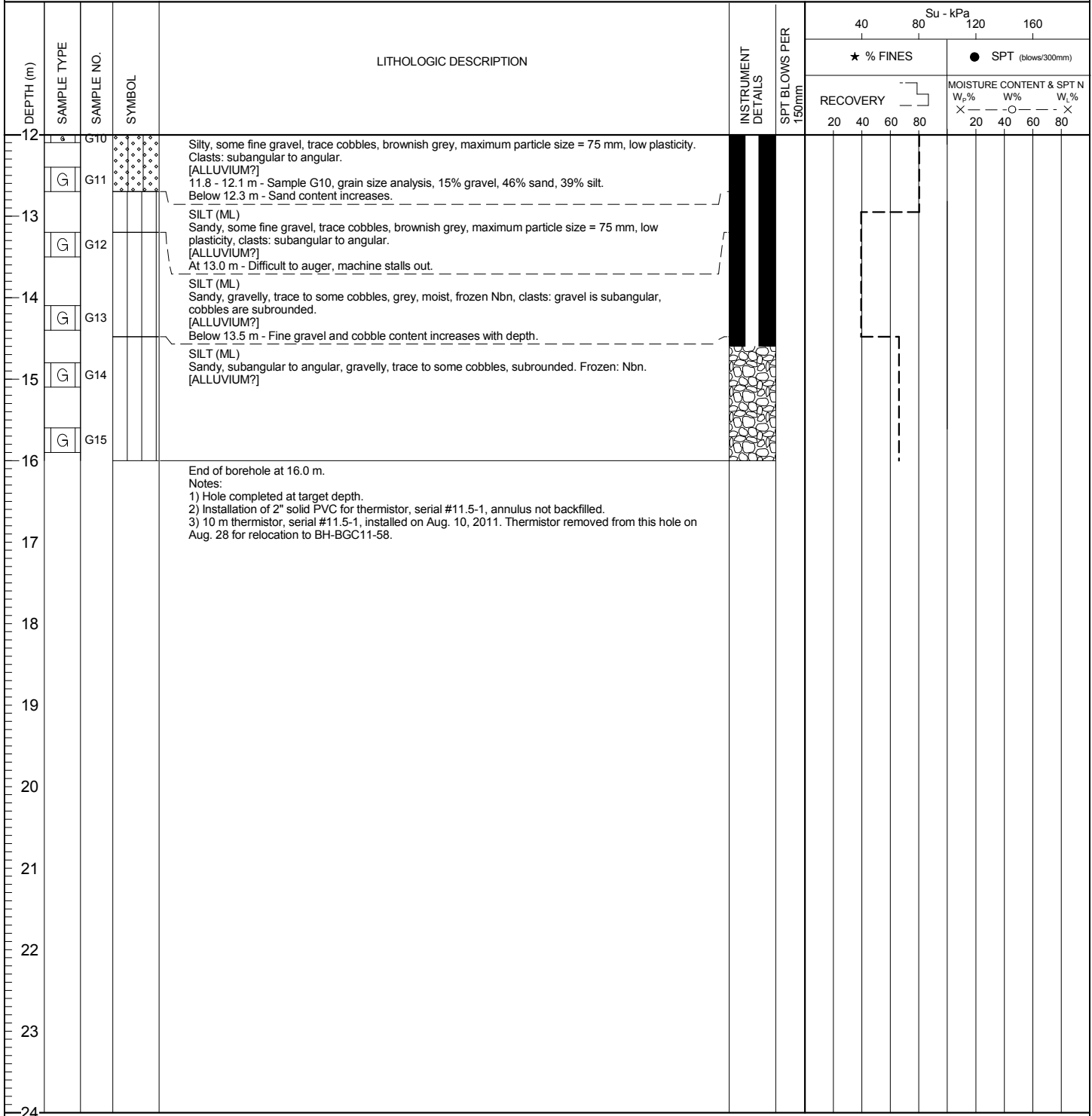
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EGR(SOIL) EGR_SOIL_GDL BGC-GDT 1/20/12

CO-ORDINATES (m): 458,811.9E - 7,100,698.9N
 GROUND ELEVATION (m) : 841.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/HSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASSED TO (m) :

START DATE : 04 Aug 11
 FINISH DATE : 04 Aug 11
 FINAL DEPTH (m) : 16.0
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : LGT/AKU
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

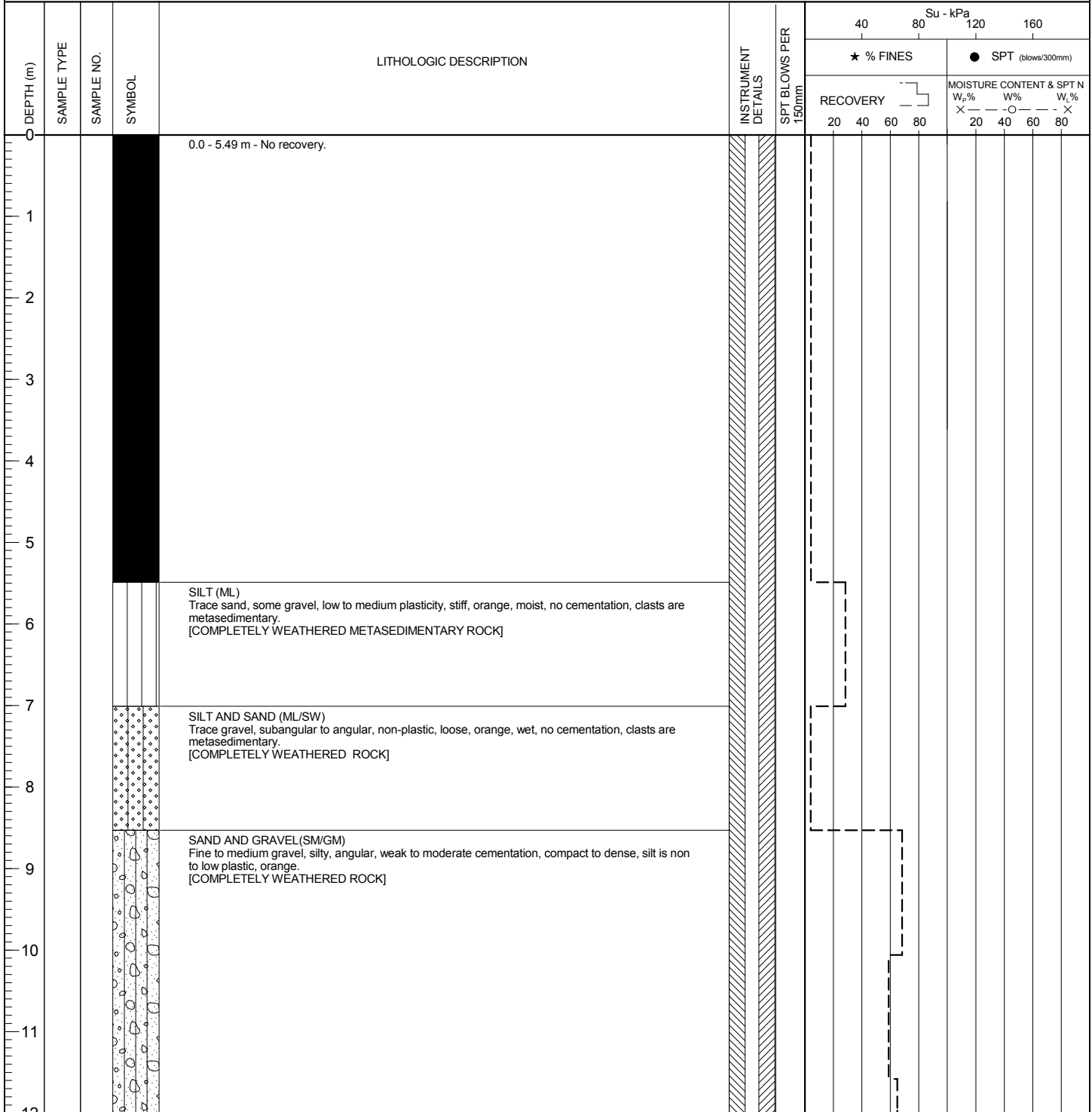
DRILL HOLE # BH-BGC11-48

LOCATION : PLATINUM GULCH

CO-ORDINATES (m): 459,936.0E - 7,099,118.1N
 GROUND ELEVATION (m) : 1,140.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 3.50

START DATE : 04 Aug 11
 FINISH DATE : 05 Aug 11
 FINAL DEPTH (m) : 32.9
 DEPTH TO TOP OF ROCK (m) : 13.1
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



(Continued on next page)

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-48

LOCATION : PLATINUM GULCH

CO-ORDINATES (m): 459,936.0E - 7,099,118.1N
 GROUND ELEVATION (m) : 1,140.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 3.50

START DATE : 04 Aug 11
 FINISH DATE : 05 Aug 11
 FINAL DEPTH (m) : 32.9
 DEPTH TO TOP OF ROCK (m) : 13.1
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa												
							★ % FINES	● SPT (blows/300mm)	MOISTURE CONTENT & SPT N										
							RECOVERY		W _p %			W _l %							
							20	40	60	80	20	40	60	80	20	40	60	80	
12																			
13				Rock encountered at 13.10 m depth. Refer to rock log.															
14																			
15																			
16																			
17																			
18																			
19																			
20																			
21																			
22																			
23																			
24																			

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/01/12

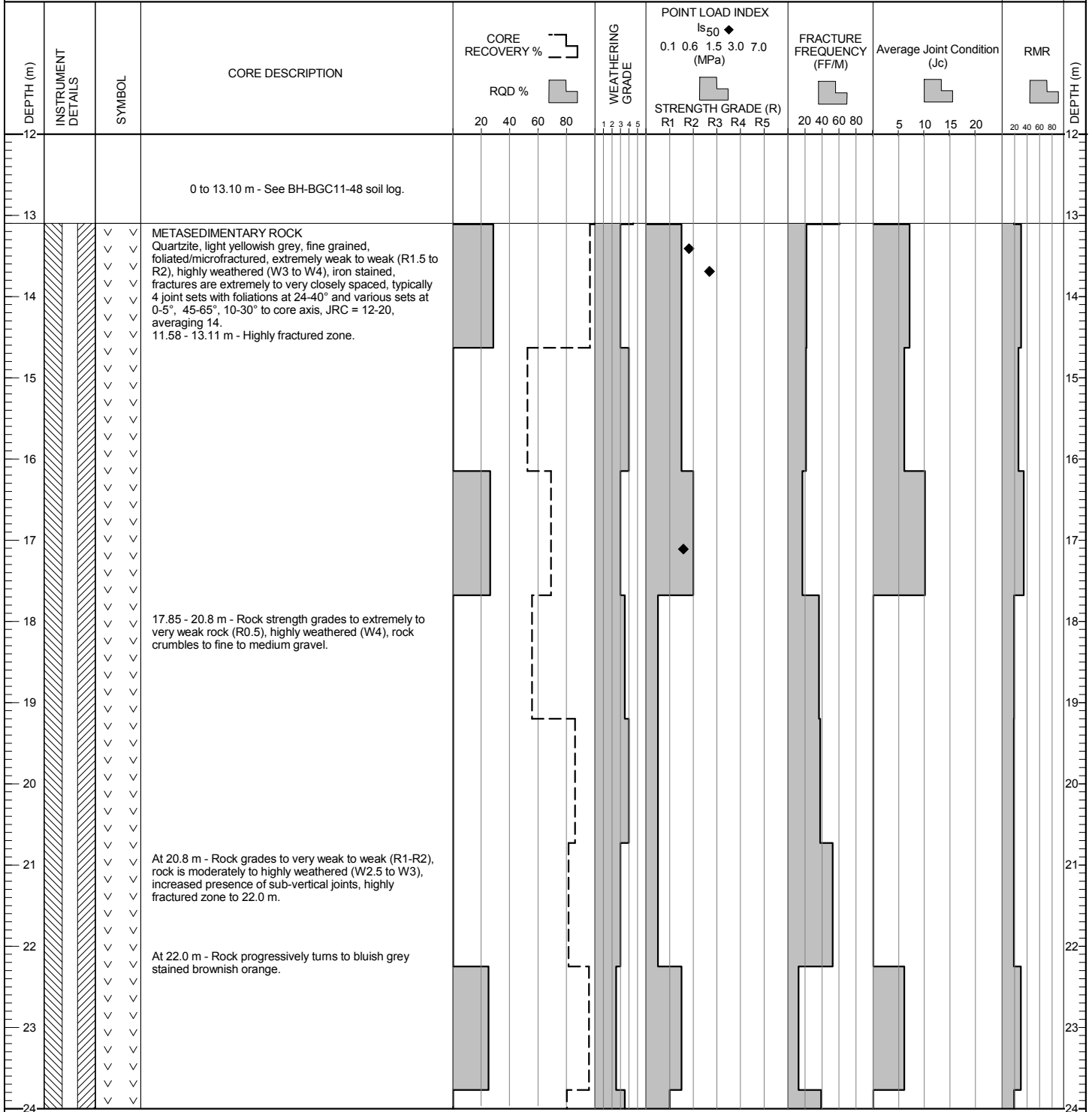


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 459,936.0E - 7,099,118.1N
 GROUND ELEVATION (m) : 1,140.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.5

START DATE : 04 Aug 11
 FINISH DATE : 05 Aug 11
 FINAL DEPTH (m) : 32.9
 DEPTH TO TOP OF ROCK (m) : 13.1
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



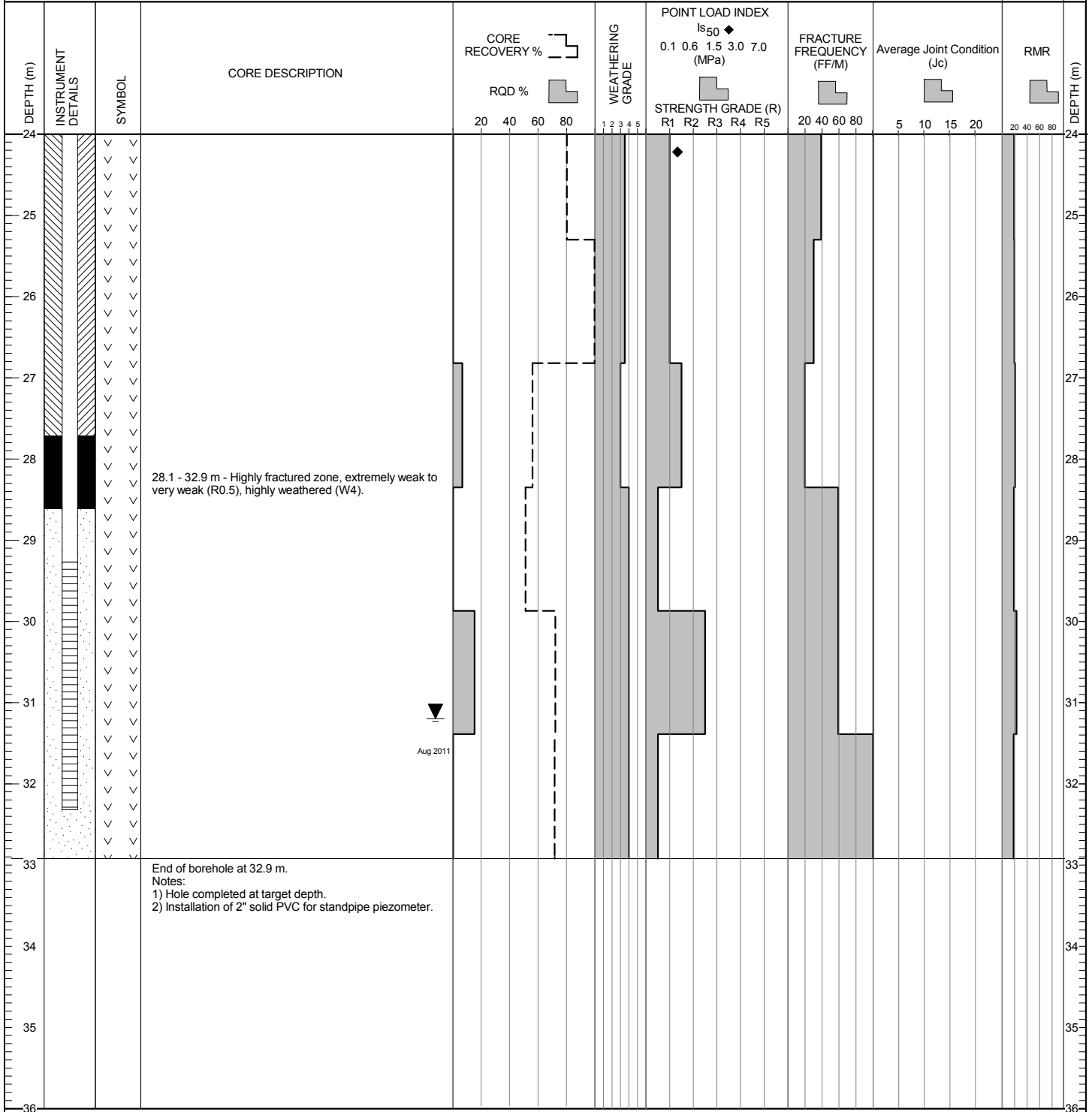
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,936.0E - 7,099,118.1N
 GROUND ELEVATION (m) : 1,140.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.5

START DATE : 04 Aug 11
 FINISH DATE : 05 Aug 11
 FINAL DEPTH (m) : 32.9
 DEPTH TO TOP OF ROCK (m) : 13.1
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

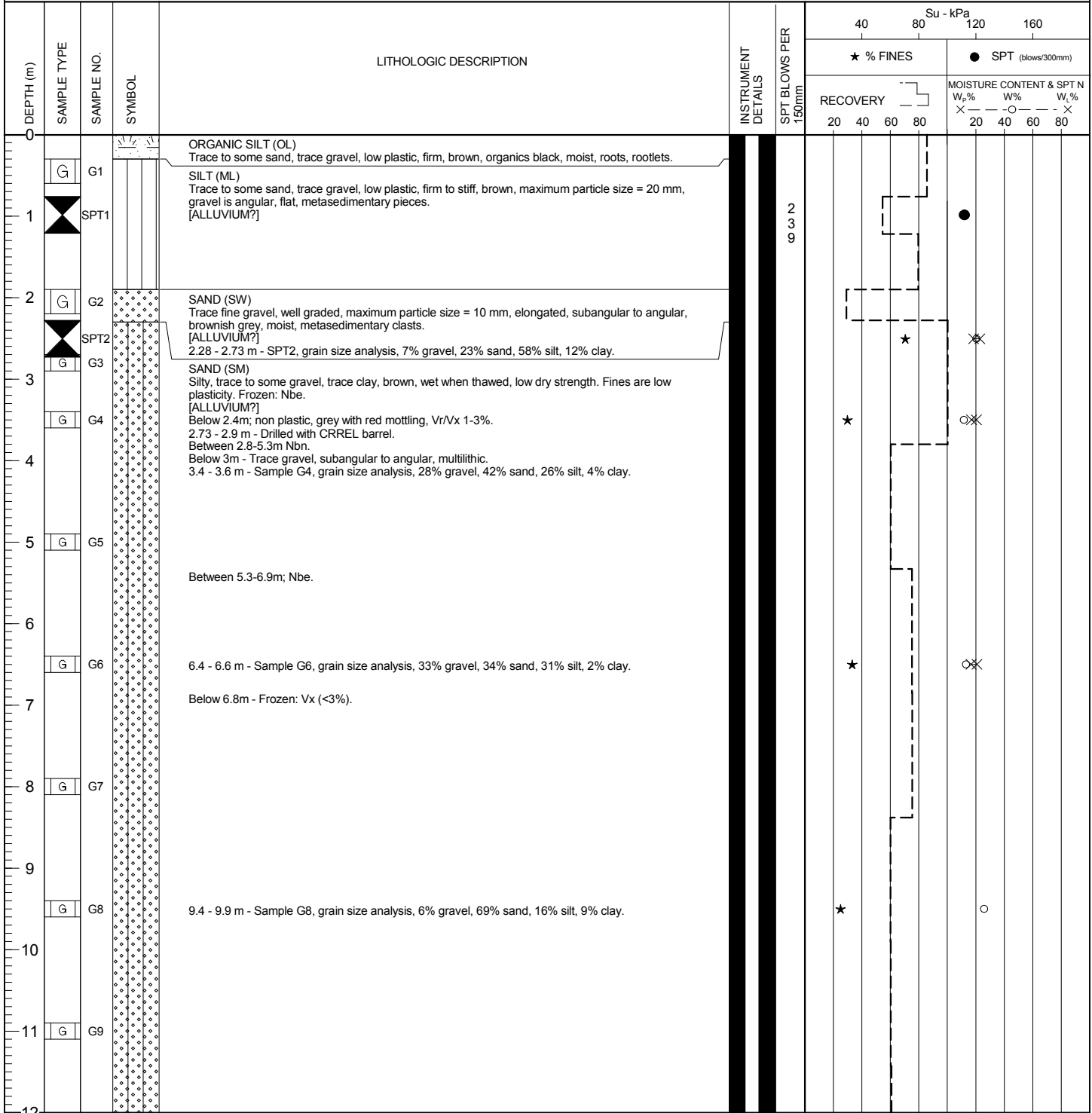


EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) : 458,746.5E - 7,100,635.9N
 GROUND ELEVATION (m) : 835.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 05 Aug 11
 FINISH DATE : 05 Aug 11
 FINAL DEPTH (m) : 16.0
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



(Continued on next page)

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/01/12

CO-ORDINATES (m) : 458,746.5E - 7,100,635.9N
 GROUND ELEVATION (m) : 835.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 05 Aug 11
 FINISH DATE : 05 Aug 11
 FINAL DEPTH (m) : 16.0
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa								
							★ % FINES		● SPT (blows/300mm)						
							RECOVERY		MOISTURE CONTENT & SPT N						
							20	40	60	80	W _p %	W ₅ %	W ₁ %	SPT N	
12	G	G10	(Symbol: dots)	12.4 - 12.6 m - Sample G10, grain size analysis, 22% gravel, 36% sand, 29% silt, 13% clay.											
13															
14	G	G11	(Symbol: dots)												
15				Below 15m - Semi-frozen.											
16	G	G12	(Symbol: dots)												
17				End of borehole at 16.0 m. Notes: 1) Hole completed at target depth. 2) Installation of 2" solid PVC for thermistor. 3) 10 m thermistor installed on Aug. 10, 2011, serial # 11.5-2, annulus of PVC not backfilled; thermistor damaged and removed on August 26, 2011.											
18															
19															
20															
21															
22															
23															
24															

EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/2/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-50

LOCATION : STUTTLE GULCH

CO-ORDINATES (m) : 459,791.3E - 7,099,997.6N
 GROUND ELEVATION (m) : 1,058.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -69
 TREND (°) : 89

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 4.57

START DATE : 06 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 5.2
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa				MOISTURE CONTENT & SPT N				
							★ % FINES		● SPT (blows/300mm)		RECOVERY		W _p %		W _L %
							20	40	60	80	20	40	60	80	
0				0.0 - 3.66 m - No recovery. Drilled with casing. For soil description refer to log of TP-BGC11-59, excavated on this drill pad.											
4				GRAVEL (GW) Subrounded to subangular, medisedimentary. [COLLUVIUM]											
5.2				Rock encountered at 5.20 m depth. Refer to rock log.											

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/2/12

CO-ORDINATES (m) 459,791.3E - 7,099,997.6N
 GROUND ELEVATION (m) : 1,058.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -69
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m): 4.57

START DATE : 06 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 5.2
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW

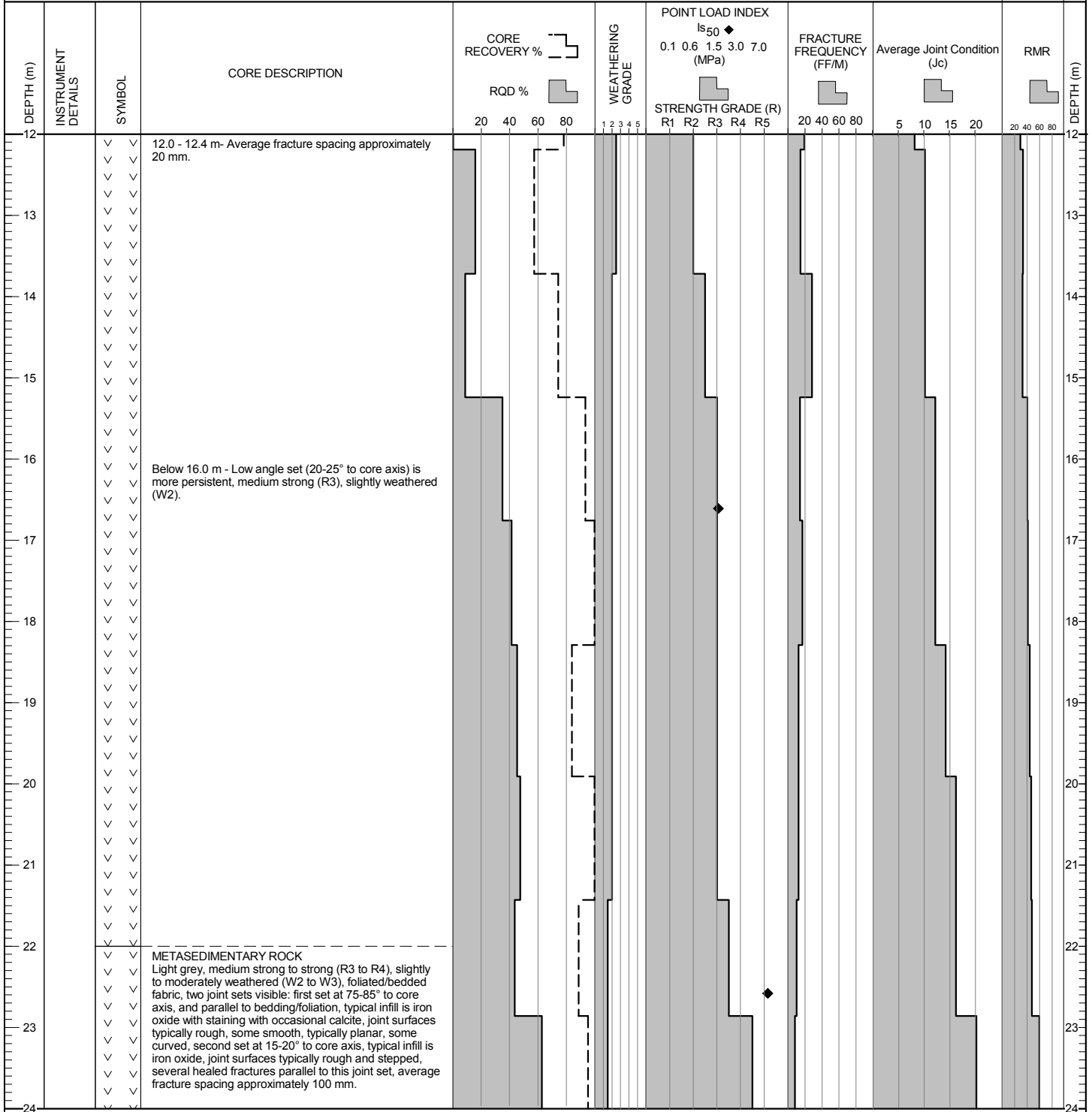
DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50}					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
0			0 to 5.20 m - See BH-BGC11-50 soil log.												0
5.2			METASEDIMENTARY ROCK Dark bluish grey, weak (R2), moderately weathered (W3), foliated/bedded fabric, 2 joint sets: first set parallel to foliation at 70-80° to core axis, typically iron oxide infill with occasional broken rock, talc and calcite, joint surfaces typically planar and rough, second set at approximately 20-25° to core axis, typically iron oxide infill, joint surfaces typically planar and rough, occasionally stepped as breaks occurred perpendicular to foliation/bedding, some healed fractures parallel to this joint set, average fracture spacing approximately 20 mm. Below 6.0 m - Average fracture spacing approximately 50 mm. Below 7.0 m - Colour changes to light grey. 7.6 - 10.7 m - Zone of poor recovery, metasedimentary gravel and cobbles were recovered.												5.2
9.1			9.1 - 10.7 m - Highly fractured zone.												9.1
12															12

(CONTINUED ON NEXT PAGE)

CO-ORDINATES (m) 459,791.3E - 7,099,997.6N
 GROUND ELEVATION (m) : 1,058.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -69
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 4.57

START DATE : 06 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 5.2
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW



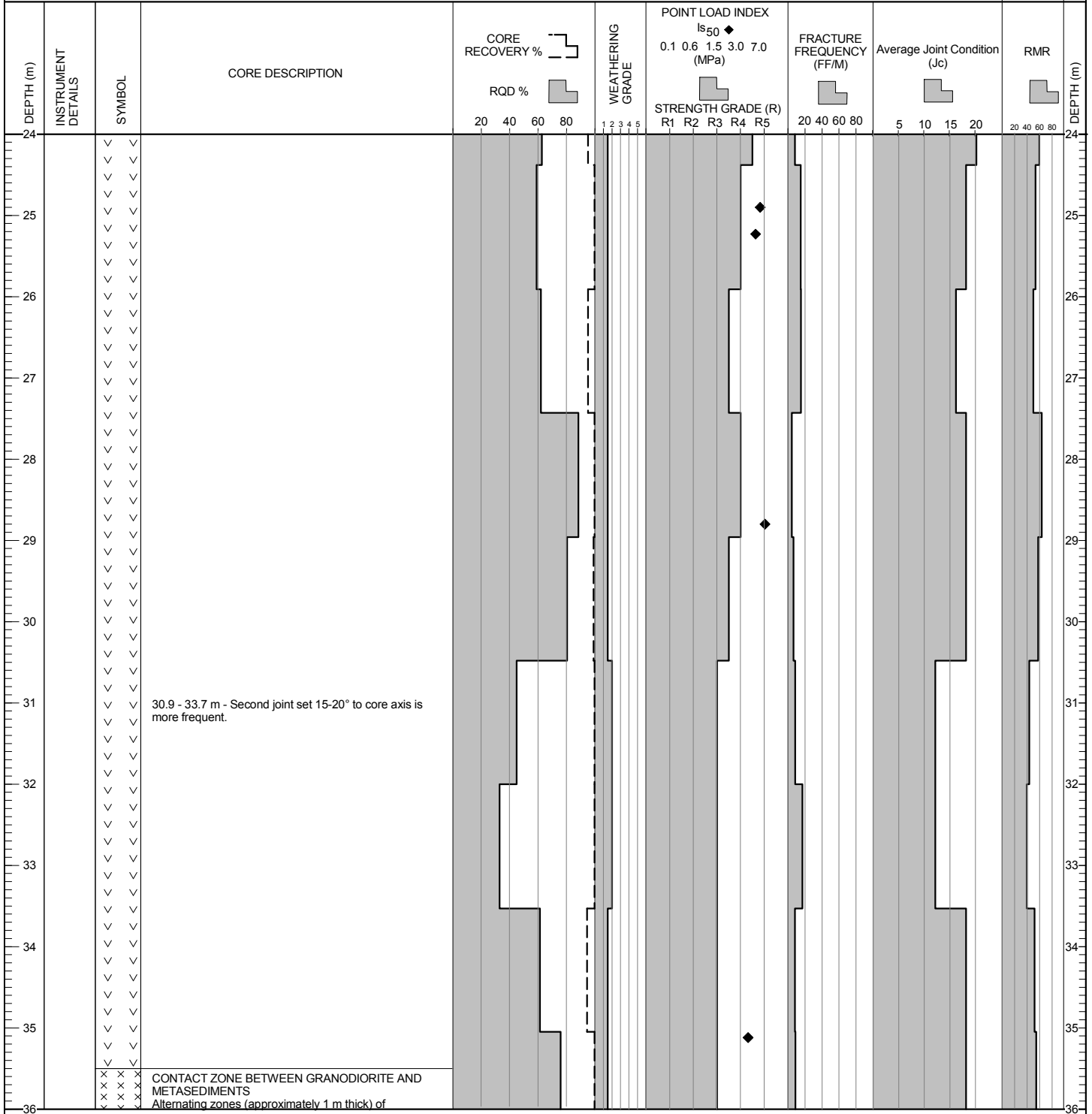
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ESP (ROCK) ESP-ROCK.GDL BGC-GDT 1/20/12

CO-ORDINATES (m) 459,791.3E - 7,099,997.6N
 GROUND ELEVATION (m) : 1,058.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -69
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 4.57

START DATE : 06 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 5.2
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW



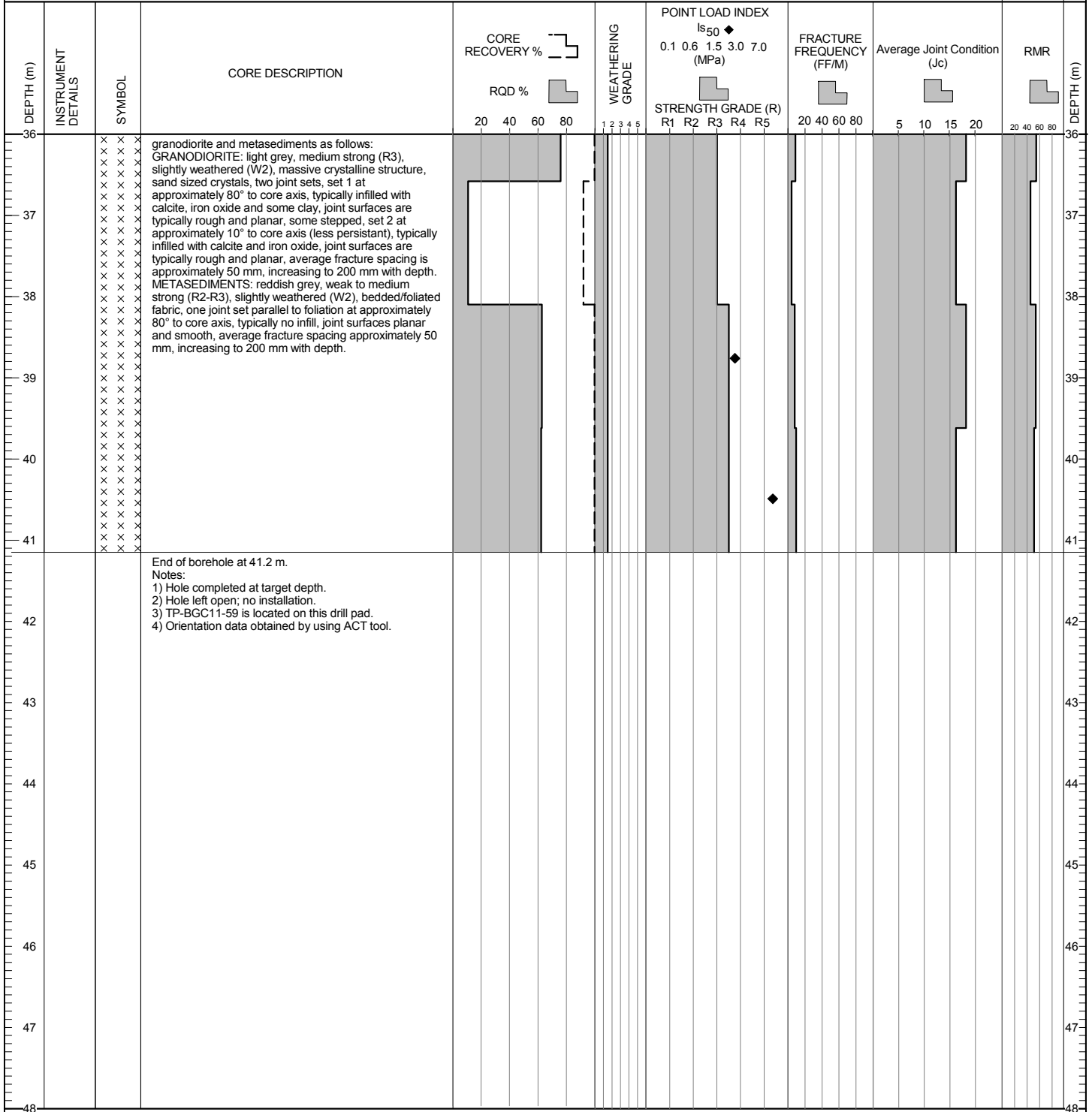
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,791.3E - 7,099,997.6N
 GROUND ELEVATION (m) : 1,058.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -69
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 4.57

START DATE : 06 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 5.2
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW

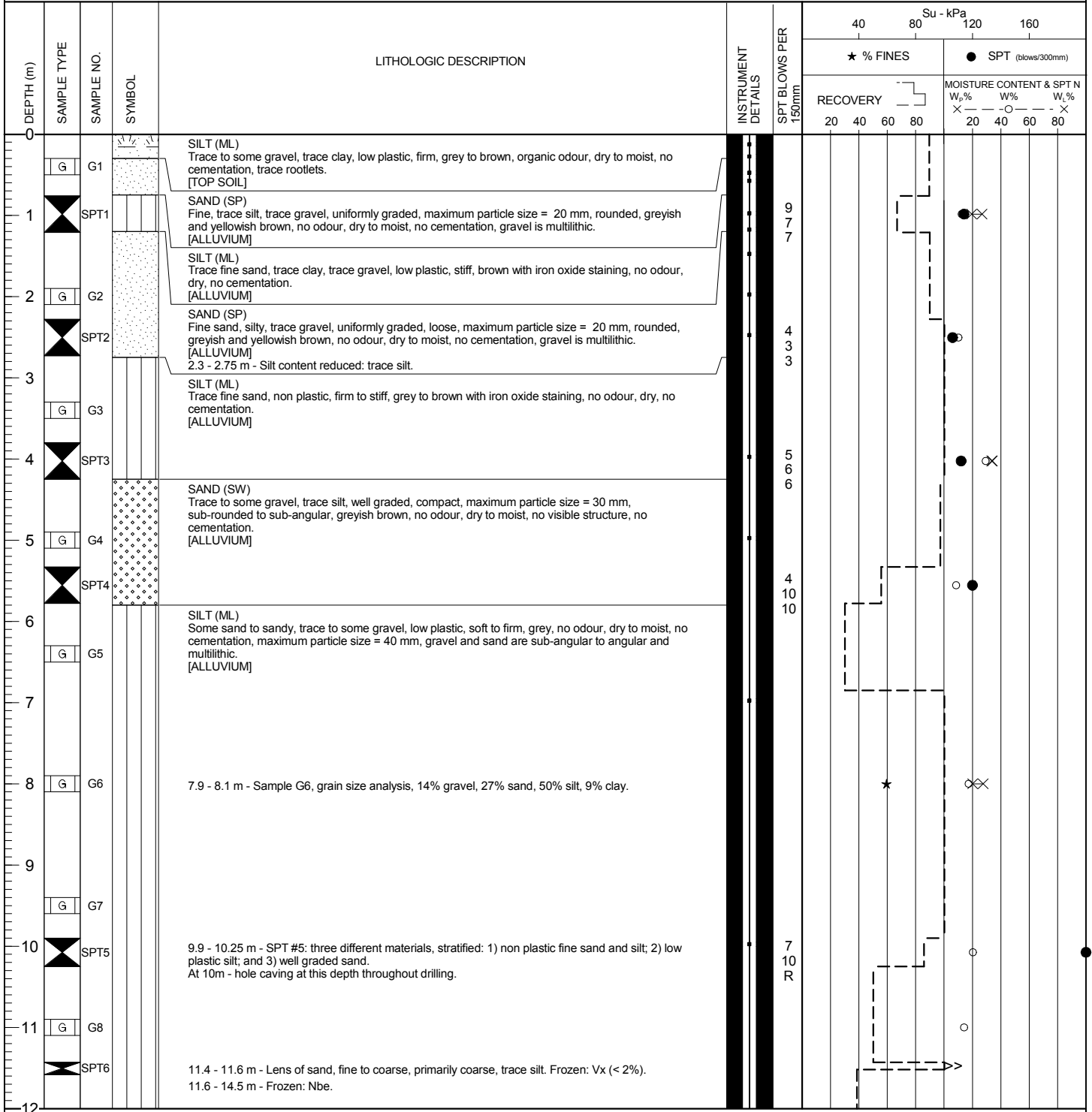


ESP (ROCK) ESP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 458,643.5E - 7,100,743.6N
 GROUND ELEVATION (m) : 818.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 06 Aug 11
 FINISH DATE : 06 Aug 11
 FINAL DEPTH (m) : 25.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



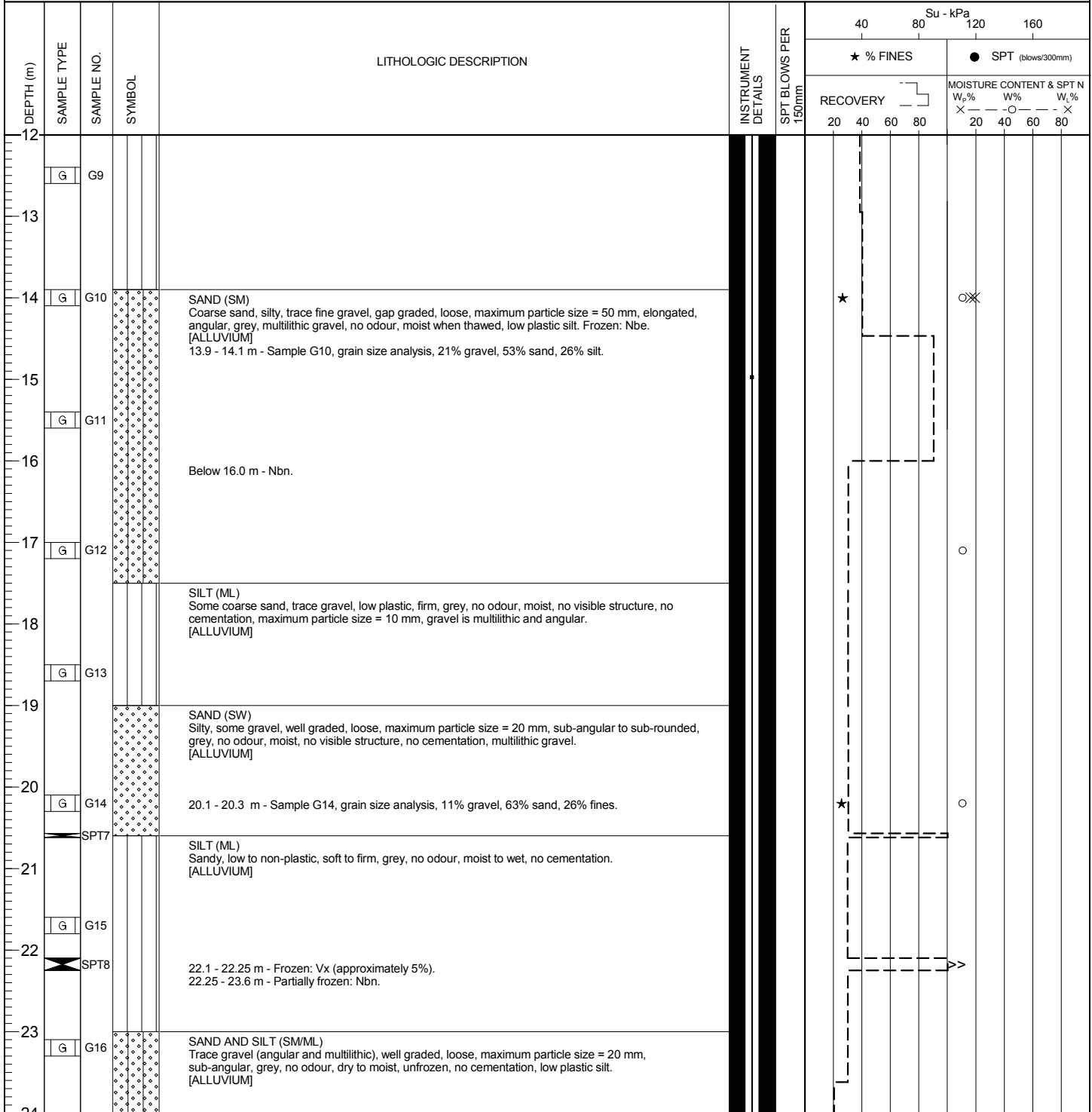
(Continued on next page)

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

CO-ORDINATES (m) : 458,643.5E - 7,100,743.6N
 GROUND ELEVATION (m) : 818.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 06 Aug 11
 FINISH DATE : 06 Aug 11
 FINAL DEPTH (m) : 25.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



(Continued on next page)

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 458,643.5E - 7,100,743.6N
 GROUND ELEVATION (m) : 818.4m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 06 Aug 11
 FINISH DATE : 06 Aug 11
 FINAL DEPTH (m) : 25.2
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa									
							★ % FINES		● SPT (blows/300mm)							
							RECOVERY		MOISTURE CONTENT & SPT N							
							20	40	60	80	W _p %	W ₉₀ %	W _L %	SPT N		
24																
25	G	G17		End of borehole at 25.2 m. Notes: 1) Hole completed at target depth. 2) Installation of 2" solid PVC for thermistor. 3) 10 m thermistor installed on Aug. 10, 2011, serial #11.5-2, annulus not backfilled. Thermistor removed from this hole on Aug. 25 for relocation to BH-BGC11-57 and replaced with a 25 m thermistor, serial #26.5-2, annulus not backfilled.												
26																
27																
28																
29																
30																
31																
32																
33																
34																
35																
36																

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/2/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

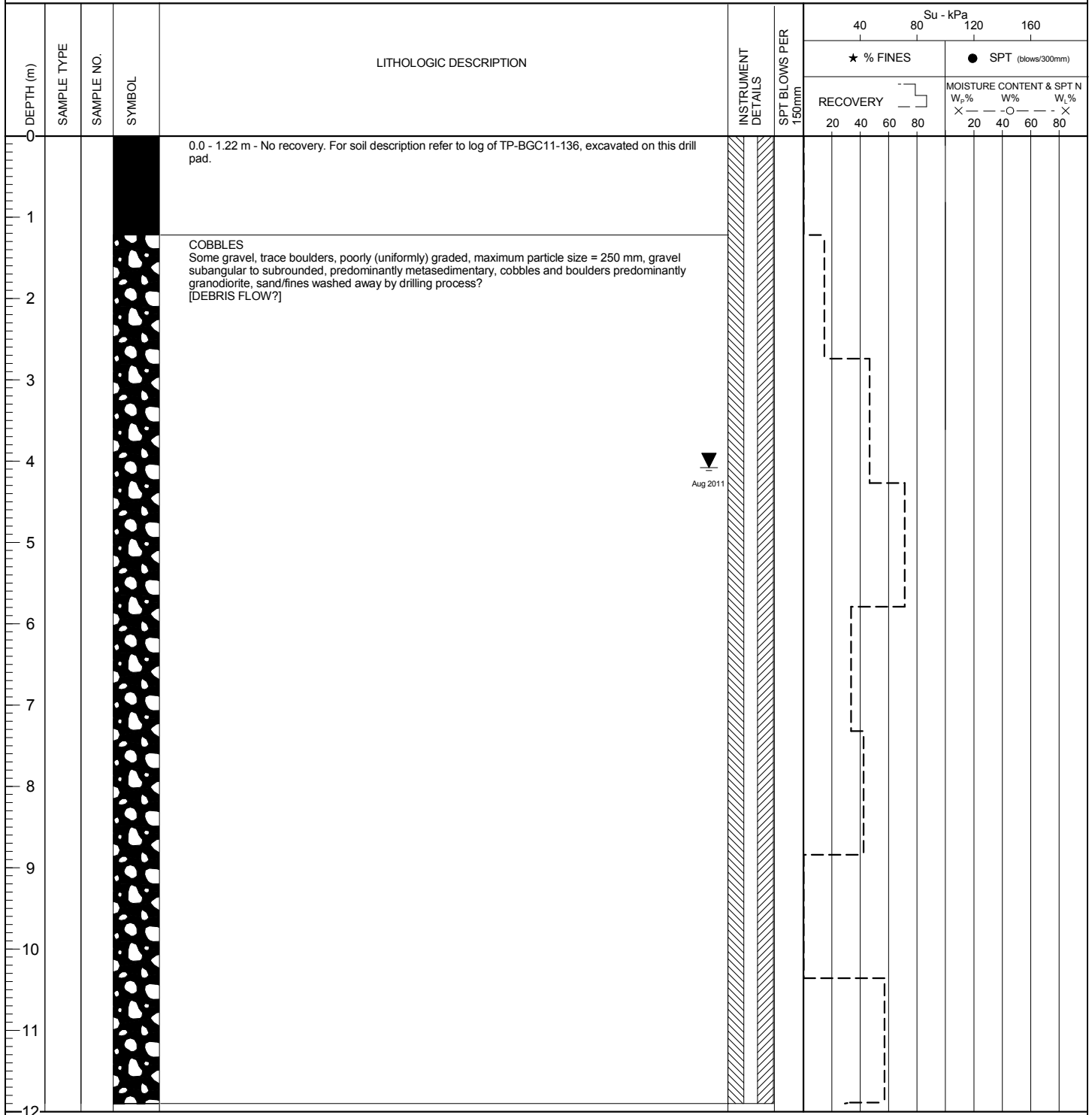
DRILL HOLE # BH-BGC11-52

LOCATION : MIDDLE REACH DUBLIN GULCH

CO-ORDINATES (m): 459,891.2E - 7,101,316.4N
 GROUND ELEVATION (m) : 909.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 12.19

START DATE : 07 Aug 11
 FINISH DATE : 08 Aug 11
 FINAL DEPTH (m) : 22.6
 DEPTH TO TOP OF ROCK (m) : 11.9
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW



▼
Aug 2011

Rock encountered at 11.90 m depth.
 Refer to rock log.

EGR/SOIL/ EGR_SOIL_GDL BGC.GDT 12/2/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

DRILL HOLE # BH-BGC11-52

LOCATION : MIDDLE REACH DUBLIN GULCH

CO-ORDINATES (m) 459,891.2E - 7,101,316.4N
 GROUND ELEVATION (m) : 909.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 12.19

START DATE : 07 Aug 11
 FINISH DATE : 08 Aug 11
 FINAL DEPTH (m) : 22.6
 DEPTH TO TOP OF ROCK (m) : 11.9
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
0															0
1															1
2															2
3															3
4															4
5															5
6			0 to 11.90 m - See BH-BGC11-52 soil log.												6
7															7
8															8
9															9
10															10
11															11
12															12

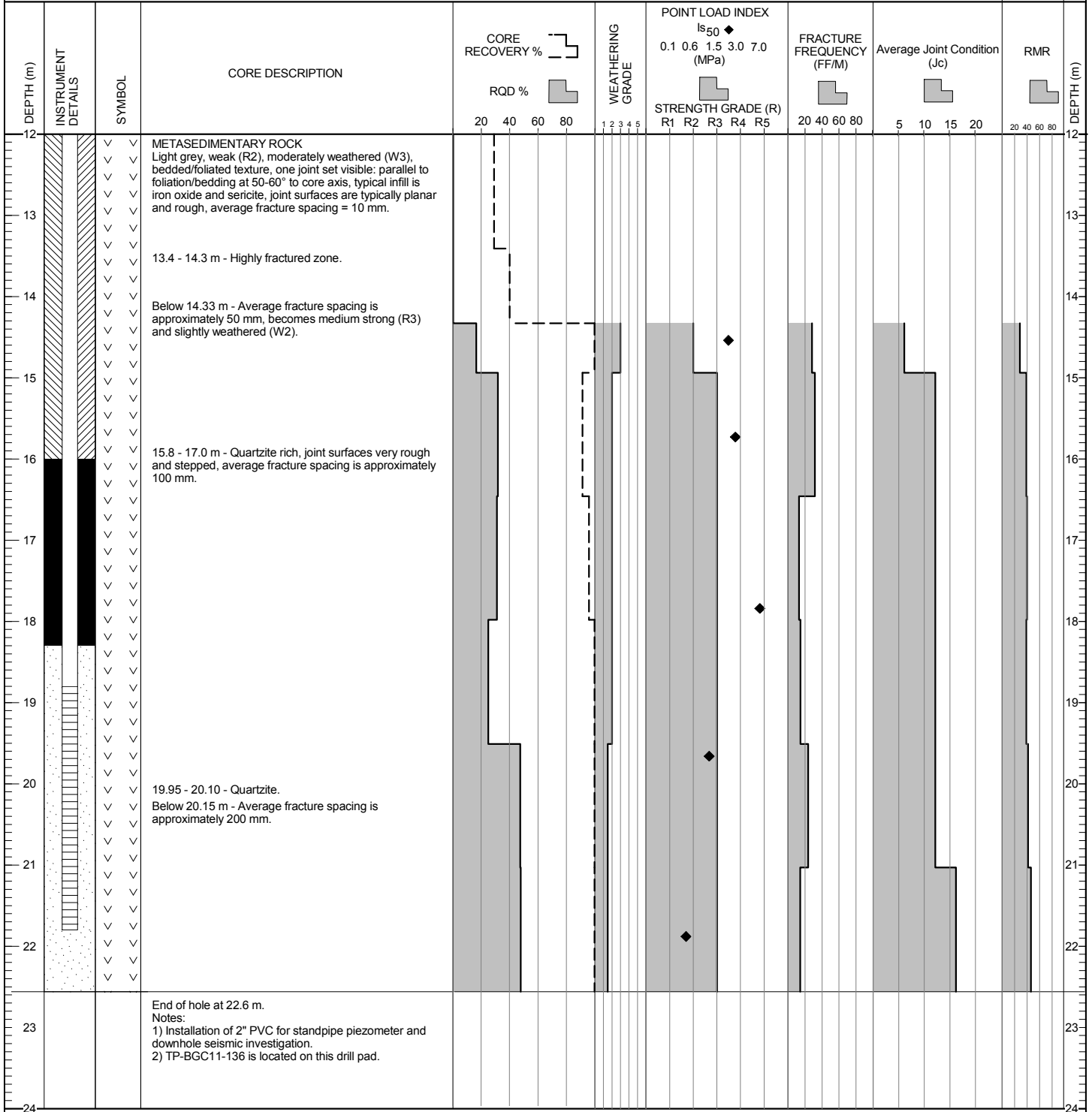
(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,891.2E - 7,101,316.4N
 GROUND ELEVATION (m) : 909.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 12.19

START DATE : 07 Aug 11
 FINISH DATE : 08 Aug 11
 FINAL DEPTH (m) : 22.6
 DEPTH TO TOP OF ROCK (m) : 11.9
 LOGGED BY : SP/JD/KH
 REVIEWED BY : PQ/DW



PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

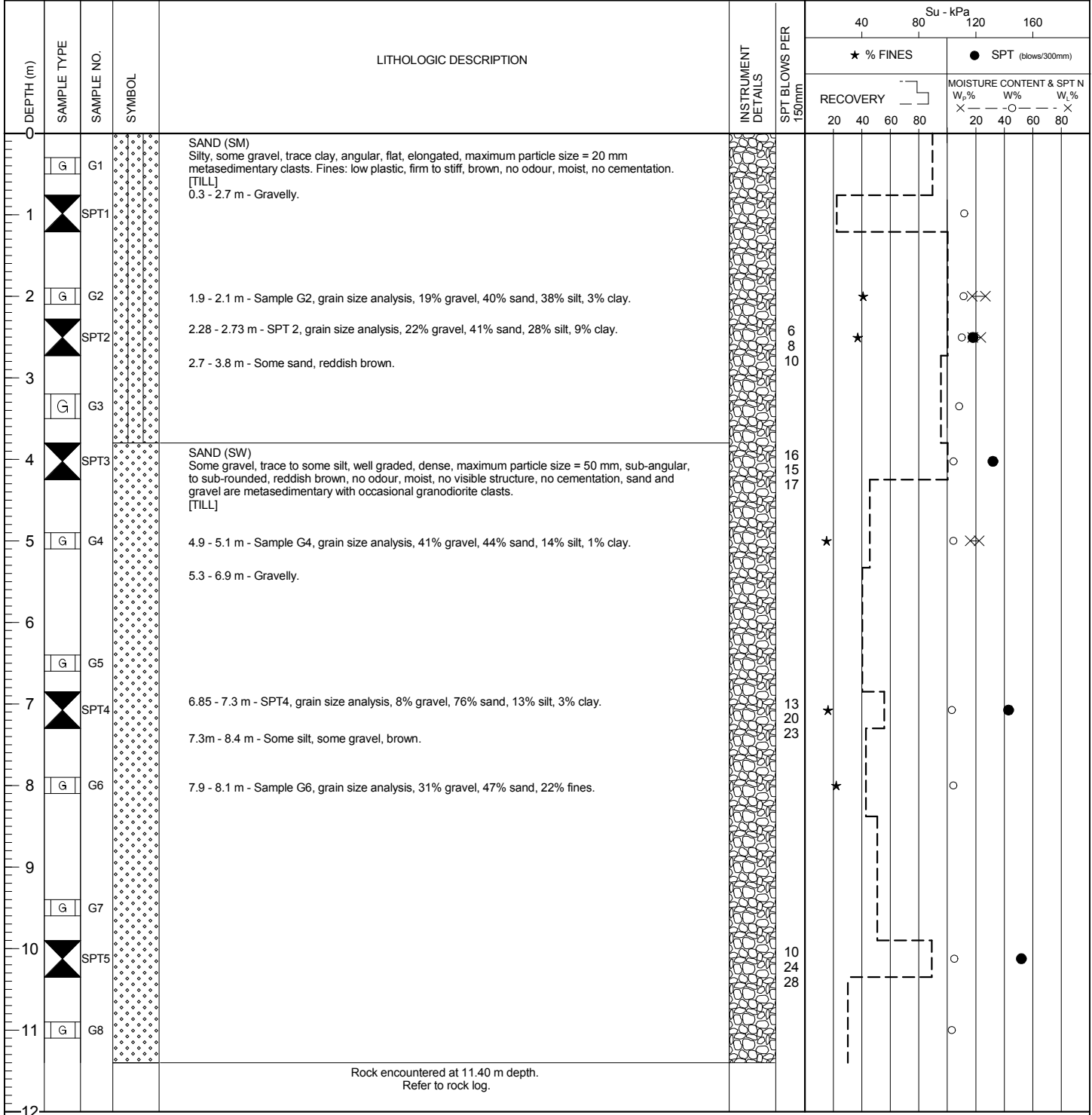
DRILL HOLE # BH-BGC11-53

LOCATION : EAGLE PUP

CO-ORDINATES (m) : 459,483.6E - 7,100,993.0N
 GROUND ELEVATION (m) : 875.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 07 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 14.5
 DEPTH TO TOP OF ROCK (m) : 11.4
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGP/SOIL/EGP_SOIL_GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-53

LOCATION : EAGLE PUP

CO-ORDINATES (m) 459,483.6E - 7,100,993.0N
 GROUND ELEVATION (m) : 875.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 07 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 14.5
 DEPTH TO TOP OF ROCK (m) : 11.4
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
0															0
1															1
2															2
3															3
4															4
5															5
6			0 to 11.40 m - See BH-BGC11-53 soil log.												6
7															7
8															8
9															9
10															10
11															11
12		✓ ✓ ✓	SAND (SM) Some silt, some gravel, angular, elongated, metasedimentary, well graded, dense, maximum particle												12

(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

DRILL HOLE # BH-BGC11-53

LOCATION : EAGLE PUP

CO-ORDINATES (m) 459,483.6E - 7,100,993.0N
 GROUND ELEVATION (m) : 875.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASSED TO (m):

START DATE : 07 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 14.5
 DEPTH TO TOP OF ROCK (m) : 11.4
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				RQD %			R1	R2	R3	R4	R5				
12		✓	size = 20 mm, angular, orange, no odour, dry, no visible structure, no cementation, moderately weathered (W3) metasedimentary rock. Note - rock is broken down by drilling process and logged as soil.	-	-	-	-	-	-	-	-	-	-	-	12
13															
14															
15															
16															
15			End of borehole at 14.5 m. Notes: 1) Hole completed at target depth. 2) Backfilled with cuttings.												15
16															
17															
18															
19															
20															
21															
22															
23															
24															

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

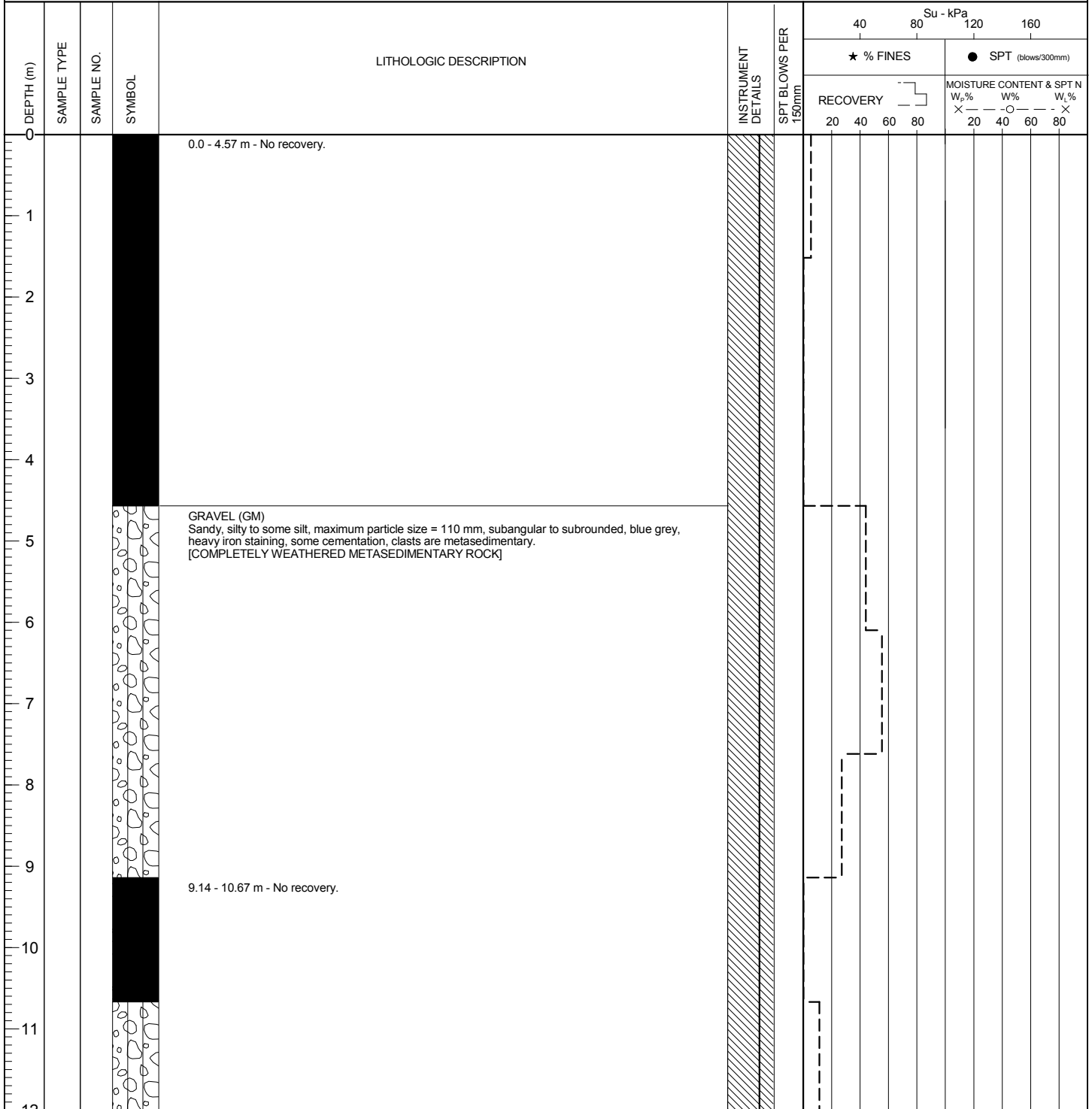
DRILL HOLE # BH-BGC11-54

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m): 458,886.2E - 7,101,245.6N
 GROUND ELEVATION (m) : 883.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -75
 TREND (°) : 4

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) :

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 19.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



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EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

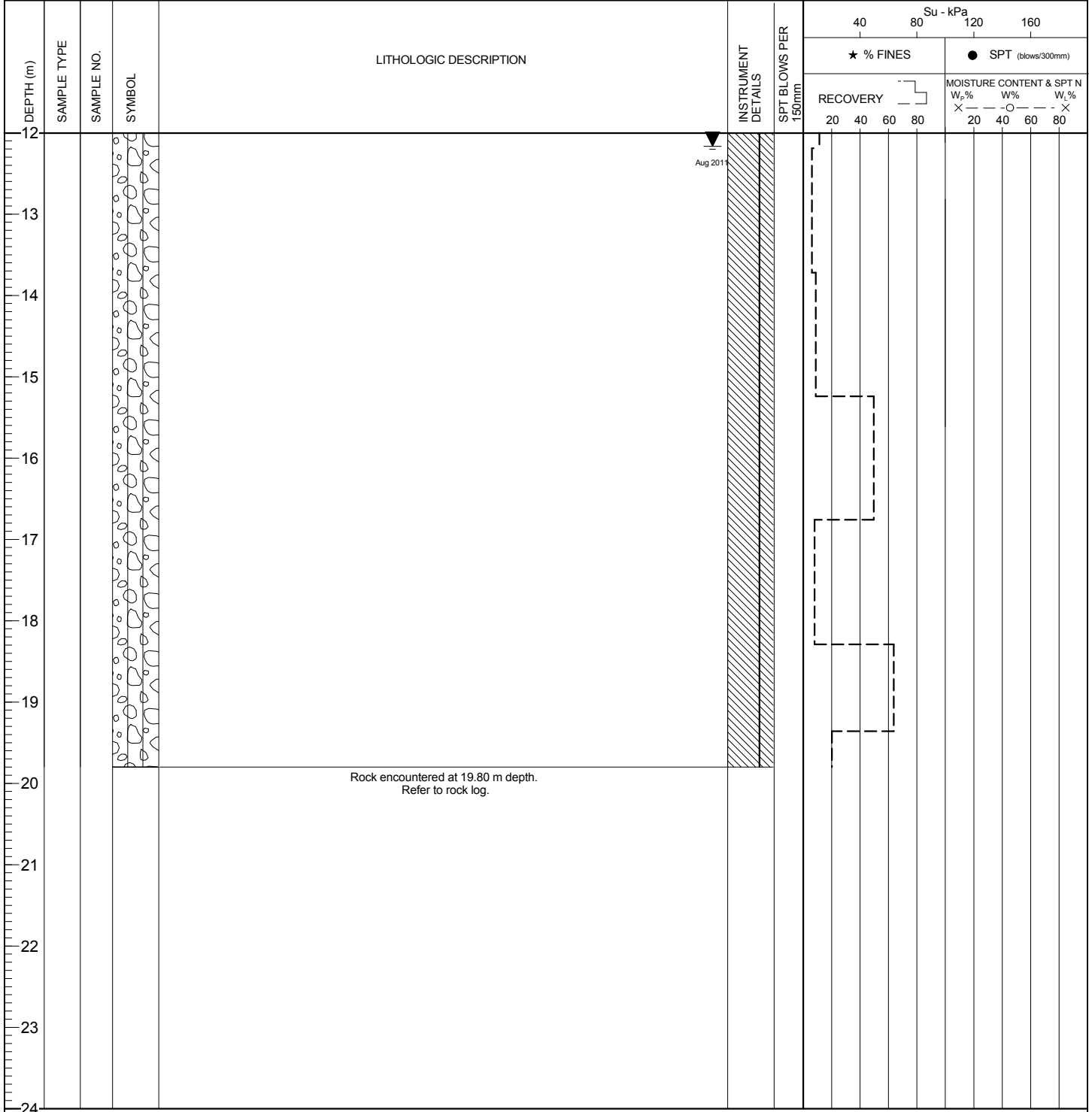
DRILL HOLE # BH-BGC11-54

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m): 458,886.2E - 7,101,245.6N
 GROUND ELEVATION (m) : 883.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -75
 TREND (°) : 4

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) :

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 19.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



EGR/SOIL/ EGP_SOIL_GDL BGC.GDT 12/01/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-54

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m) 458,886.2E - 7,101,245.6N
 GROUND ELEVATION (m) : 883.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -75
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 19.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %				WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)				RMR	DEPTH (m)					
				20	40	60	80		R1	R2	R3	R4	R5		20	40	60	80			5	10	15	20	20
12																								12	
			Aug 2011																						
13																									13
14																									14
15																									15
16			0 to 19.80 m - See BH-BGC11-54 soil log.																						16
17																									17
18																									18
19																									19
20		✓	METASEDIMENTARY ROCK																						20
		✓	Quartzite, grey/greenish grey, fine grained, foliated,																						
		✓	intact rock is very weak (R1), rockmass crumbles apart																						
		✓	when disturbed, moderately weathered (W2.5), fractures																						
		✓	are extremely closely to very closely spaced, apertures																						
		✓	are generally 1-4 mm with broken zones, infill is typically																						
		✓	crushed and decomposed rock with hematite staining, 3																						
		✓	main sets: 55-68° (foliation), and two joint sets at 40-50°																						
		✓	and 15-21° to core axis, JRC typically = 12-20.																						
21		✓																							21
22		✓																							22
23		✓																							23
24		✓																							24

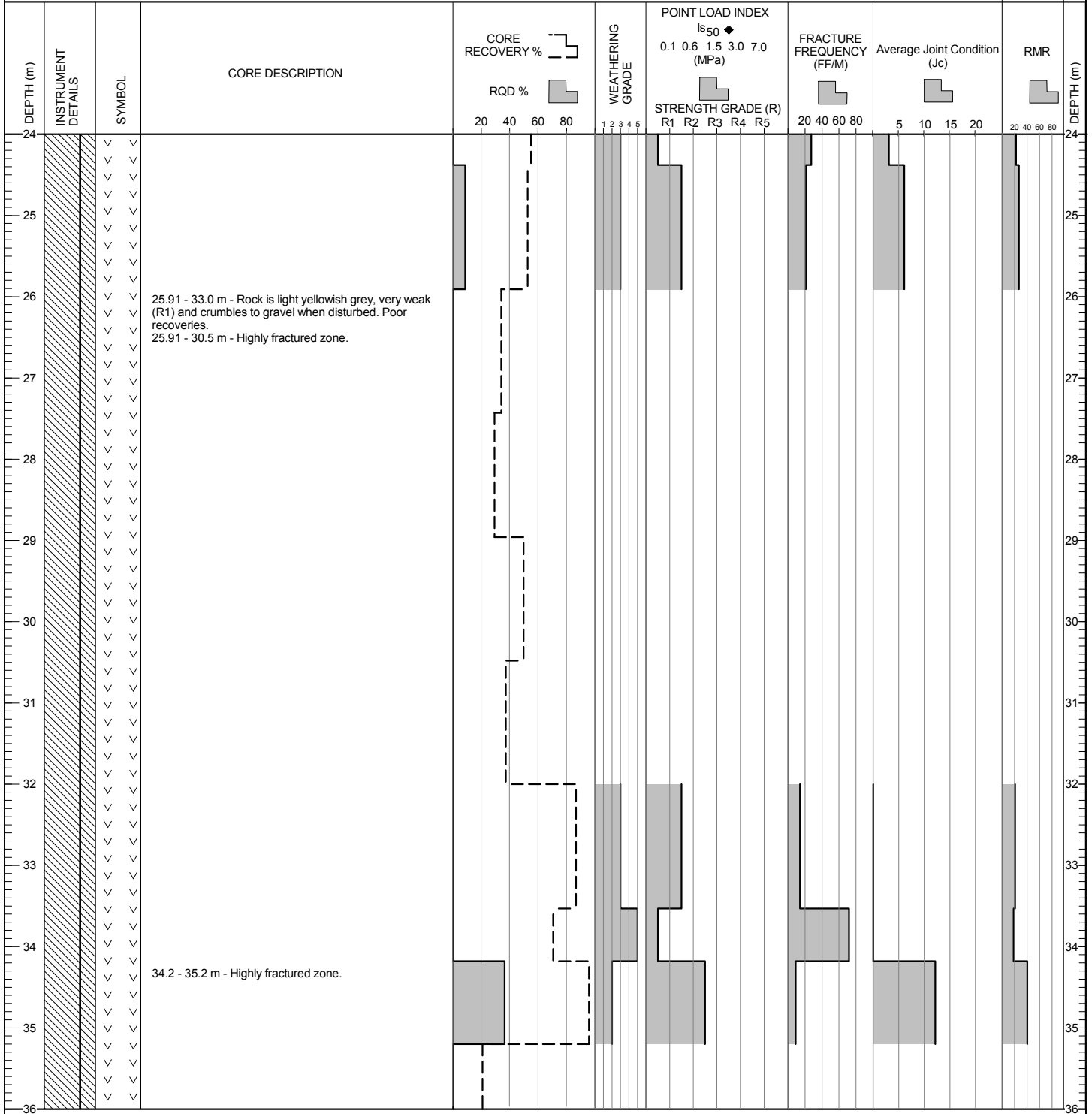
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 458,886.2E - 7,101,245.6N
 GROUND ELEVATION (m) : 883.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -75
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 19.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW

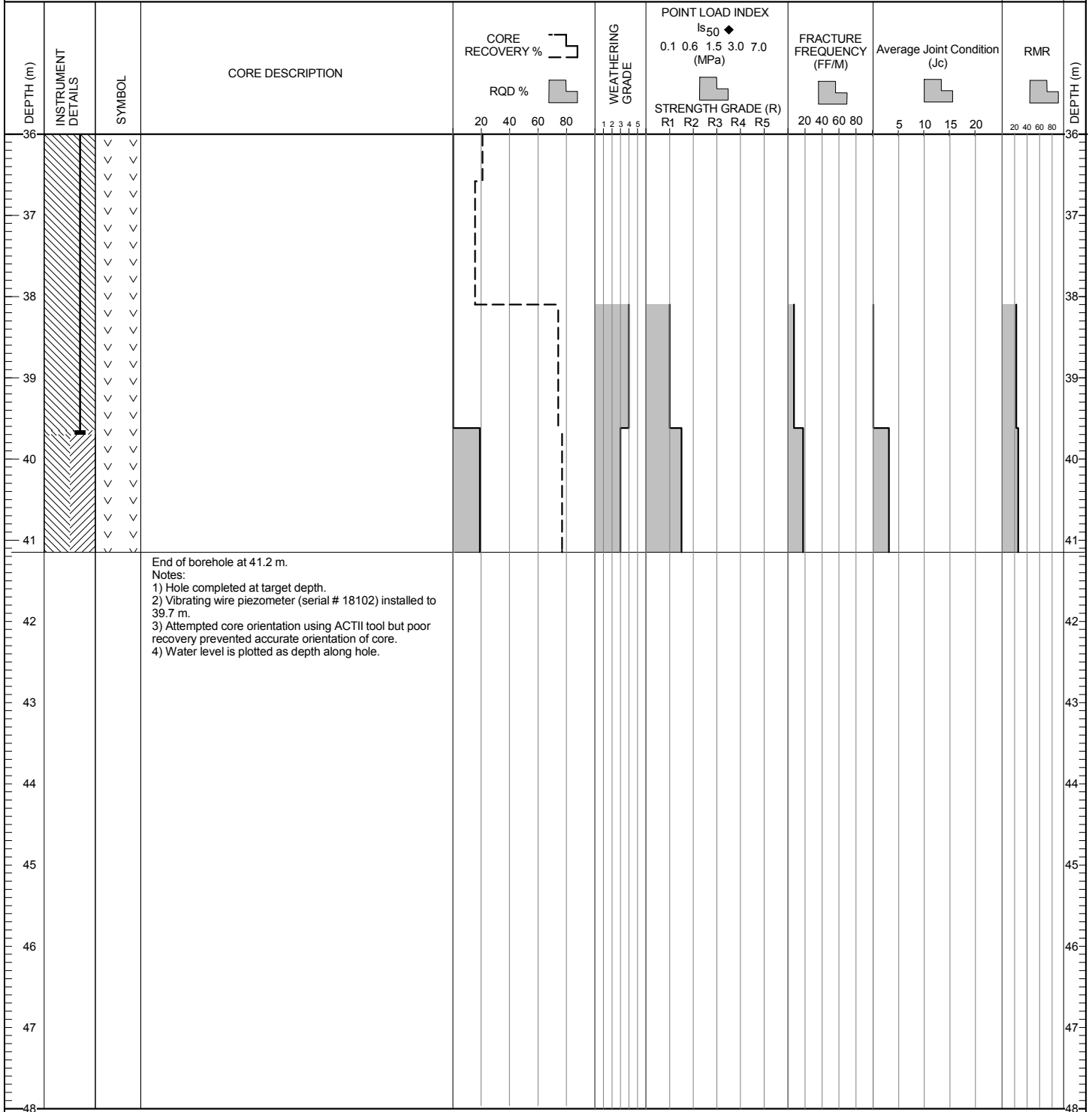


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CO-ORDINATES (m) 458,886.2E - 7,101,245.6N
 GROUND ELEVATION (m) : 883.8m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -75
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 41.2
 DEPTH TO TOP OF ROCK (m) : 19.8
 LOGGED BY : SP/JD
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

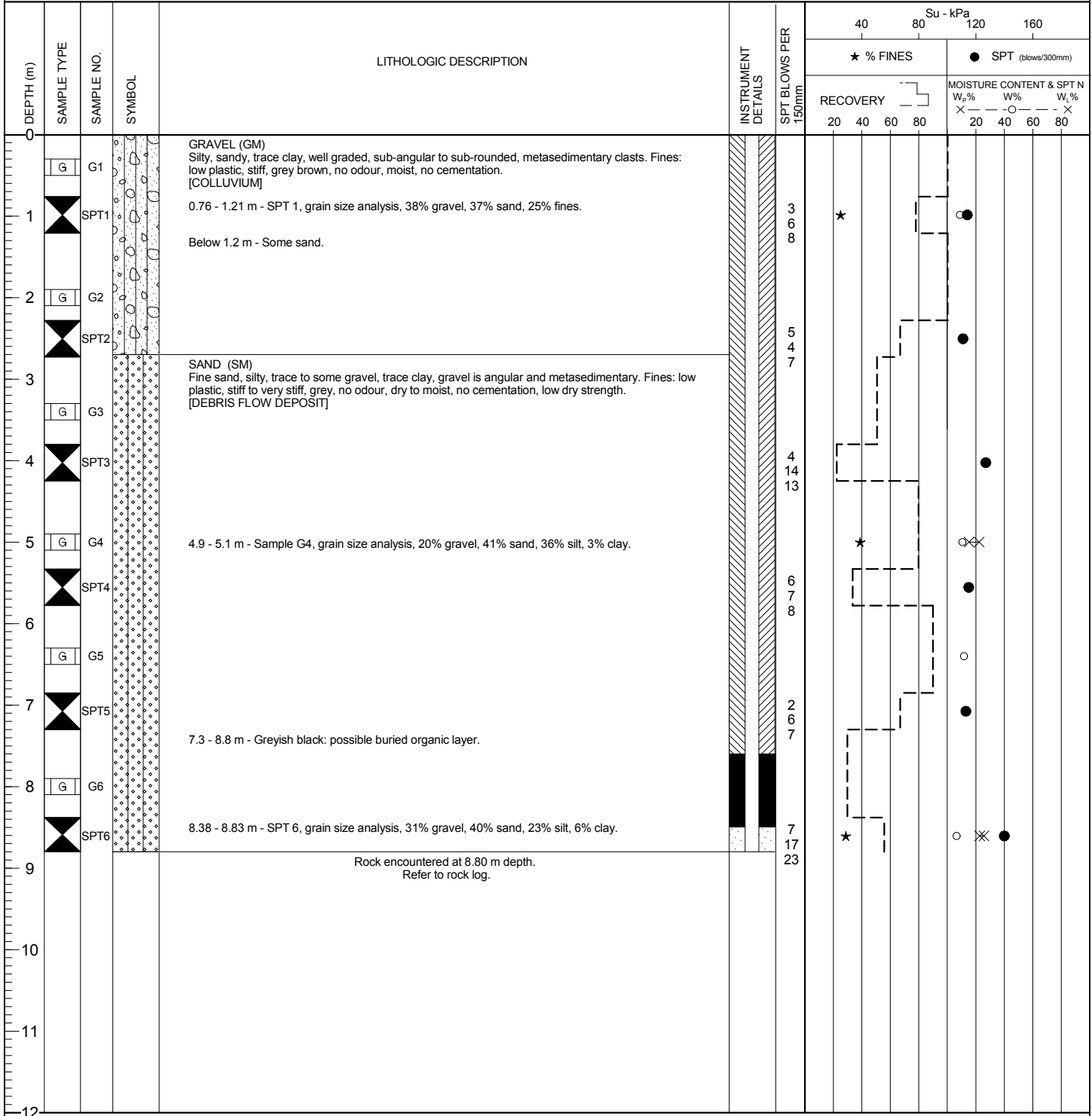
DRILL HOLE # BH-BGC11-55

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,440.6E - 7,100,918.4N
 GROUND ELEVATION (m) : 881.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 07 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 14.5
 DEPTH TO TOP OF ROCK (m) : 8.8
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

DRILL HOLE # BH-BGC11-55

LOCATION : STUTTLE GULCH

CO-ORDINATES (m) 459,440.6E - 7,100,918.4N
 GROUND ELEVATION (m) : 881.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 07 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 14.5
 DEPTH TO TOP OF ROCK (m) : 8.8
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50}					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
0															0
1															1
2															2
3															3
4															4
5			0 to 8.80 m - See BH-BGC11-55 soil log.												5
6															6
7															7
8															8
9		✓ ✓	SILT (ML)												9
10		✓ ✓	Some sand to sandy, some gravel, low to non plastic, hard (based on auger difficulty), yellowish brown, no odour, dry, no cementation, moderately weathered (W3) metasedimentary rock. Clasts: metasedimentary, angular, elongated, talc present on clast surfaces. Note - rock is broken down by drilling process and logged as soil.												10
11		✓ ✓													11
12		✓ ✓													12

(CONTINUED ON NEXT PAGE)

CO-ORDINATES (m) 459,440.6E - 7,100,918.4N
 GROUND ELEVATION (m) : 881.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 07 Aug 11
 FINISH DATE : 07 Aug 11
 FINAL DEPTH (m) : 14.5
 DEPTH TO TOP OF ROCK (m) : 8.8
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				RQD %			0.1	0.6	1.5	3.0	7.0				
12															12
13			▼ DRY Aug 2011												13
14															14
15			End of borehole at 14.5 m. Notes: 1) Hole completed at target depth. 2) Installation of 2" PVC for standpipe piezometer.												15
16															16
17															17
18															18
19															19
20															20
21															21
22															22
23															23
24															24

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

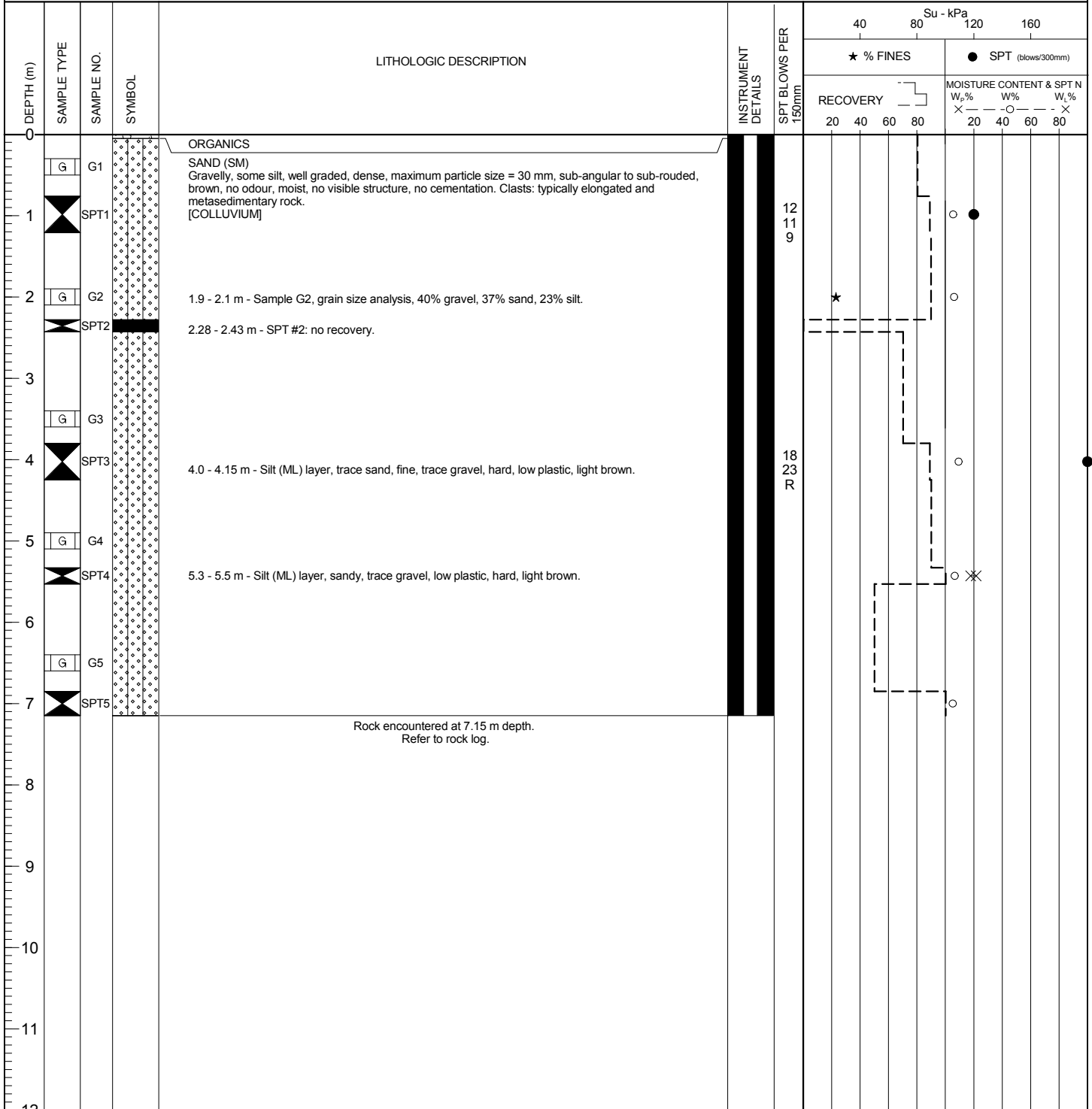
DRILL HOLE # BH-BGC11-56

LOCATION : STEINER AREA

CO-ORDINATES (m): 458,774.8E - 7,099,822.7N
 GROUND ELEVATION (m) : 845.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 13.7
 DEPTH TO TOP OF ROCK (m) : 7.2
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/01/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

DRILL HOLE # BH-BGC11-56

LOCATION : STEINER AREA

CO-ORDINATES (m) 458,774.8E - 7,099,822.7N
 GROUND ELEVATION (m) : 845.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASSED TO (m):

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 13.7
 DEPTH TO TOP OF ROCK (m) : 7.2
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
0			0 to 7.15 m - See BH-BGC11-56 soil log.												0
1															1
2															2
3															3
4															4
5															5
6															6
7															7
8			SILT (ML) Sandy, trace gravel, low plastic, hard, light brown, no odour, dry, no visible structure, no cementation, sand and gravel are angular, and metasedimentary rock, moderately weathered (W3) metasedimentary rock. Note - rock is broken down by drilling process and logged as soil.												8
9															9
10															10
11															11
12															12

(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 458,774.8E - 7,099,822.7N
 GROUND ELEVATION (m) : 845.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 08 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 13.7
 DEPTH TO TOP OF ROCK (m) : 7.2
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
12				20	40	1									12
13				20	40	2									
14			End of borehole at 13.7 m. Notes: 1) Hole terminated due to refusal of auger on rock. 2) Installation of 2" solid PVC for potential thermistor.												14
15															15
16															16
17															17
18															18
19															19
20															20
21															21
22															22
23															23
24															24

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

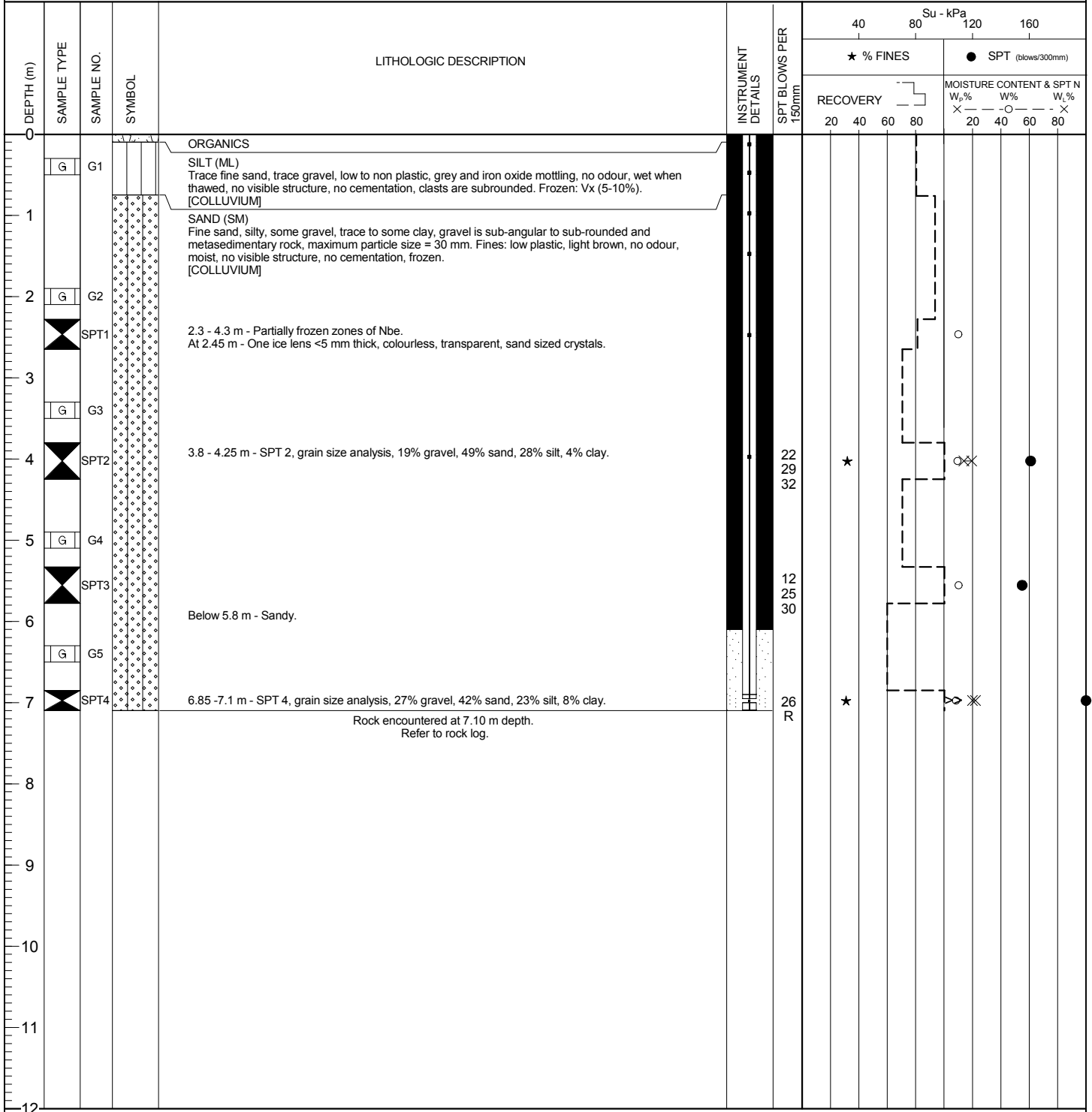
DRILL HOLE # BH-BGC11-57

LOCATION : STEINER AREA

CO-ORDINATES (m): 458,798.6E - 7,099,978.2N
 GROUND ELEVATION (m) : 859.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 09 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 12.1
 DEPTH TO TOP OF ROCK (m) : 7.1
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/01/12

DRILL HOLE # BH-BGC11-57

LOCATION : STEINER AREA

CO-ORDINATES (m) 458,798.6E - 7,099,978.2N
 GROUND ELEVATION (m) : 859.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASSED TO (m):

START DATE : 09 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 12.1
 DEPTH TO TOP OF ROCK (m) : 7.1
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)				RMR	DEPTH (m)							
				CORE RECOVERY %	RQD %		Is ₅₀ (MPa)						Average Joint Condition (Jc)												
				20	40	60	80	1	2	3	4	5	20	40	60	80	5	10	15	20	20	40	60	80	
0			0 to 7.10 m - See BH-BGC11-57 soil log.																						0
7.1																									7.1
8.33			SILT (ML) Some sand, some gravel, low to non-plastic, hard (based on augering difficulty), light brown, no odour, dry to moist, no visible structure, no cementation, gravel and sand are angular metasedimentary rock, some break by finger pressure and others broken by SPT action, maximum particle size = 20 mm, moderately weathered (W3), metasedimentary rock. Note - rock is broken down by drilling process and logged as soil. Below 8.3 m - Dry. 8.33 m - 8.43 m - SPT #5, refusal in first six inches.																						8.33
9.9			9.9 m - 10.2 m - SPT#6, 17/38/refusal in final six inches. Below 10.2 m - Gravelly, some sand, flat and elongated.																						9.9
10.2																									10.2
12																									12

(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 458,798.6E - 7,099,978.2N
 GROUND ELEVATION (m) : 859.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 09 Aug 11
 FINISH DATE : 09 Aug 11
 FINAL DEPTH (m) : 12.1
 DEPTH TO TOP OF ROCK (m) : 7.1
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

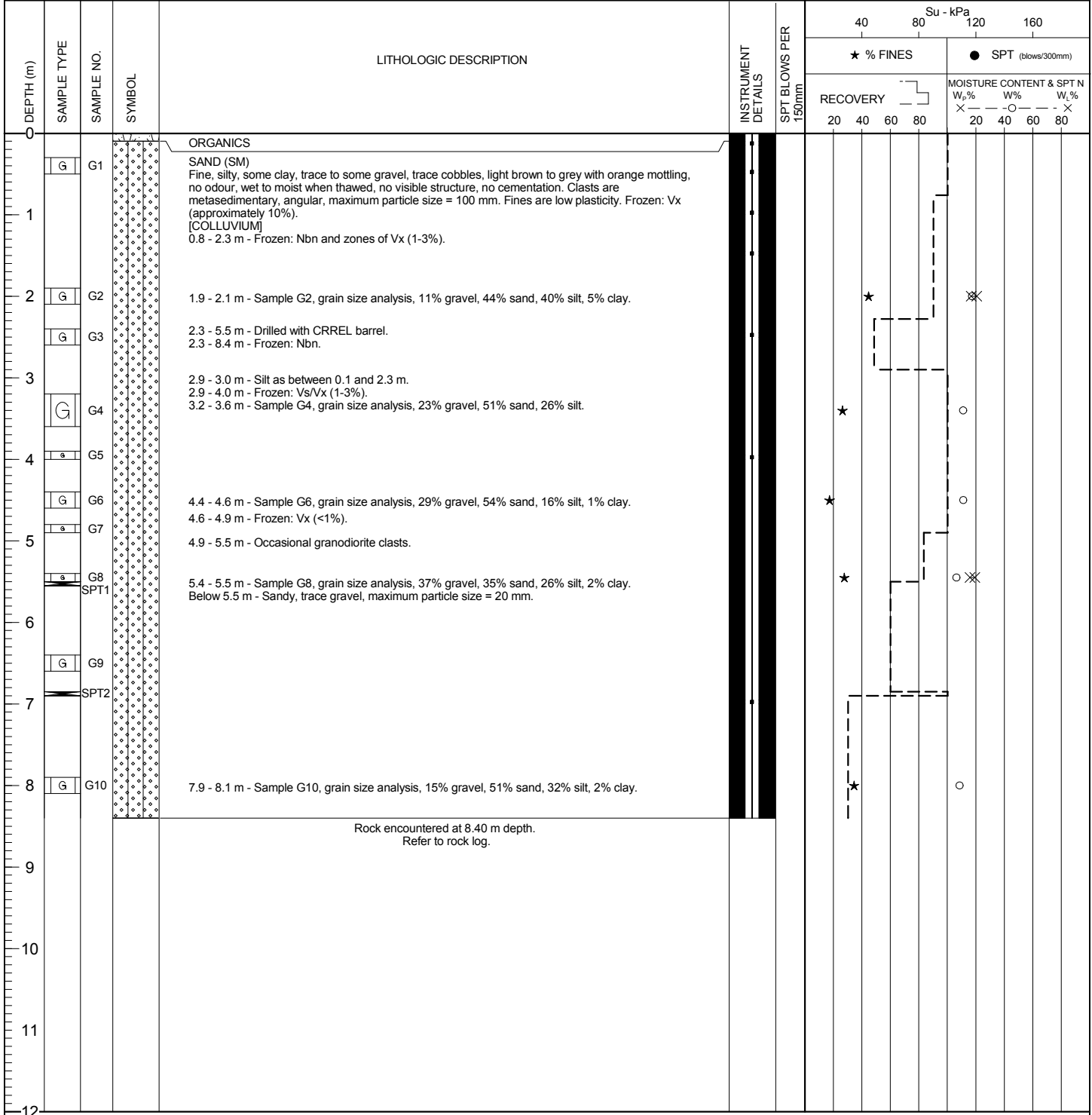
DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
12			End of borehole at 12.1 m. Notes: 1) Hole terminated due to refusal of auger. 2) Installation of 2" PVC for standpipe piezometer. 3) 10 m thermistor installed on Aug. 26, 2011, serial #11.5-2, annulus not backfilled.												12
13															13
14															14
15															15
16															16
17															17
18															18
19															19
20															20
21															21
22															22
23															23
24															24

EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 458,799.8E - 7,100,119.4N
 GROUND ELEVATION (m) : 859.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 10 Aug 11
 FINISH DATE : 10 Aug 11
 FINAL DEPTH (m) : 10.8
 DEPTH TO TOP OF ROCK (m) : 8.4
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 458,799.8E - 7,100,119.4N
 GROUND ELEVATION (m) : 859.0m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 10 Aug 11
 FINISH DATE : 10 Aug 11
 FINAL DEPTH (m) : 10.8
 DEPTH TO TOP OF ROCK (m) : 8.4
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50}					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
0															0
1															1
2															2
3															3
4															4
5			0 to 8.40 m - See BH-BGC11-58 soil log.												5
6															6
7															7
8															8
9		✓	SILT (ML)												9
10		✓	Some sand, trace gravel, low to non plastic, hard (based on augering difficulty), light brown, no odour, dry, no visible structure, no cementation, gravel and sand are metasedimentary and angular, slight powdery texture, moderately weathered (W3), metasedimentary rock. Note - rock is broken down by drilling process and logged as soil.												10
11		✓	End of hole at 10.8 m. Notes: 1) Hole terminated due to refusal of auger. 2) Installation of 2" solid PVC for thermistor. 3) 10 m thermistor installed on Aug. 29, 2011, serial #11.5-1, annulus not backfilled.												11
12															12

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

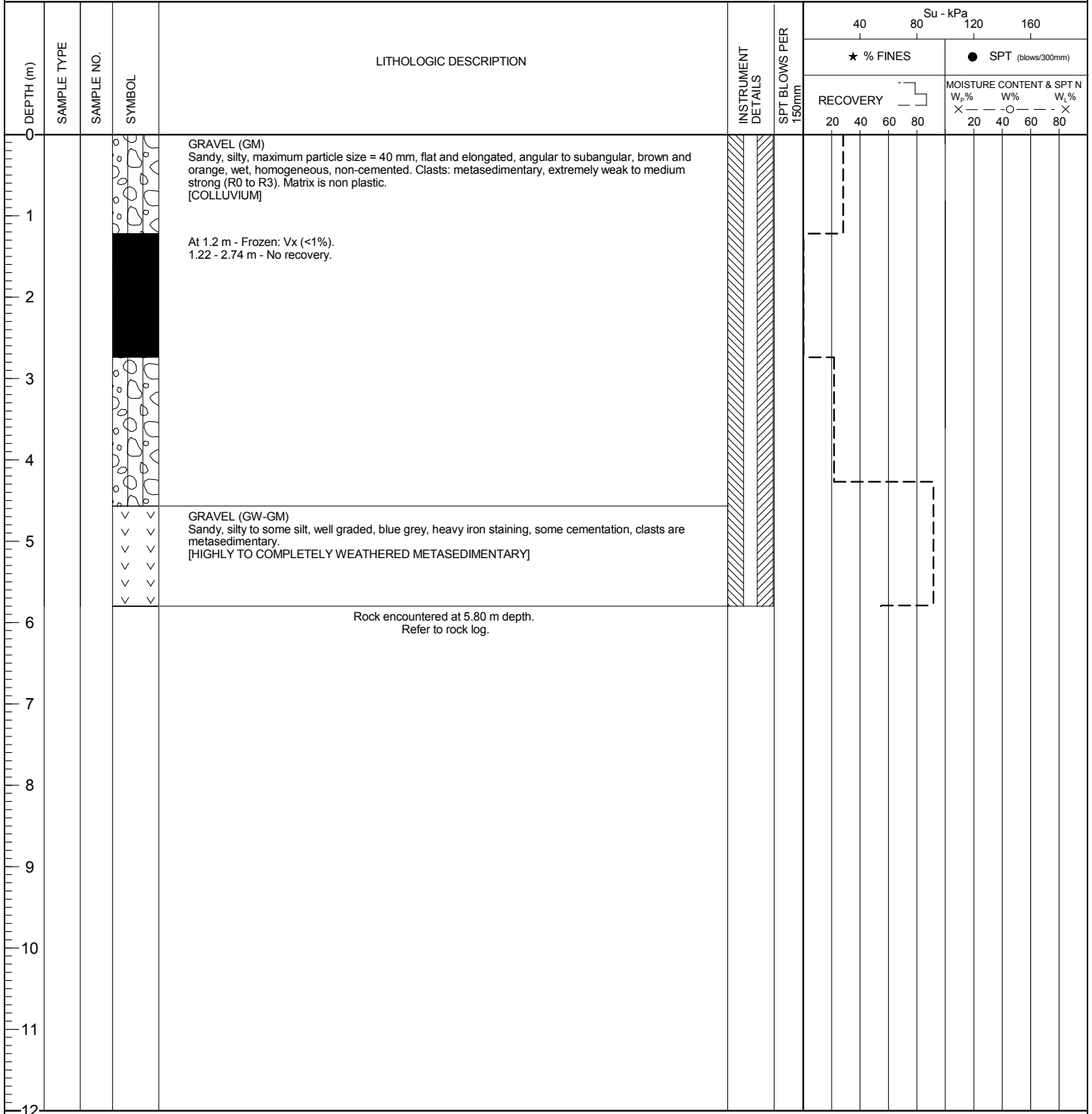
DRILL HOLE # BH-BGC11-59

LOCATION : ANN GULCH

CO-ORDINATES (m): 459,113.1E - 7,101,239.4N
 GROUND ELEVATION (m) : 883.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 5.79

START DATE : 10 Aug 11
 FINISH DATE : 11 Aug 11
 FINAL DEPTH (m) : 30.2
 DEPTH TO TOP OF ROCK (m) : 5.8
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/01/12

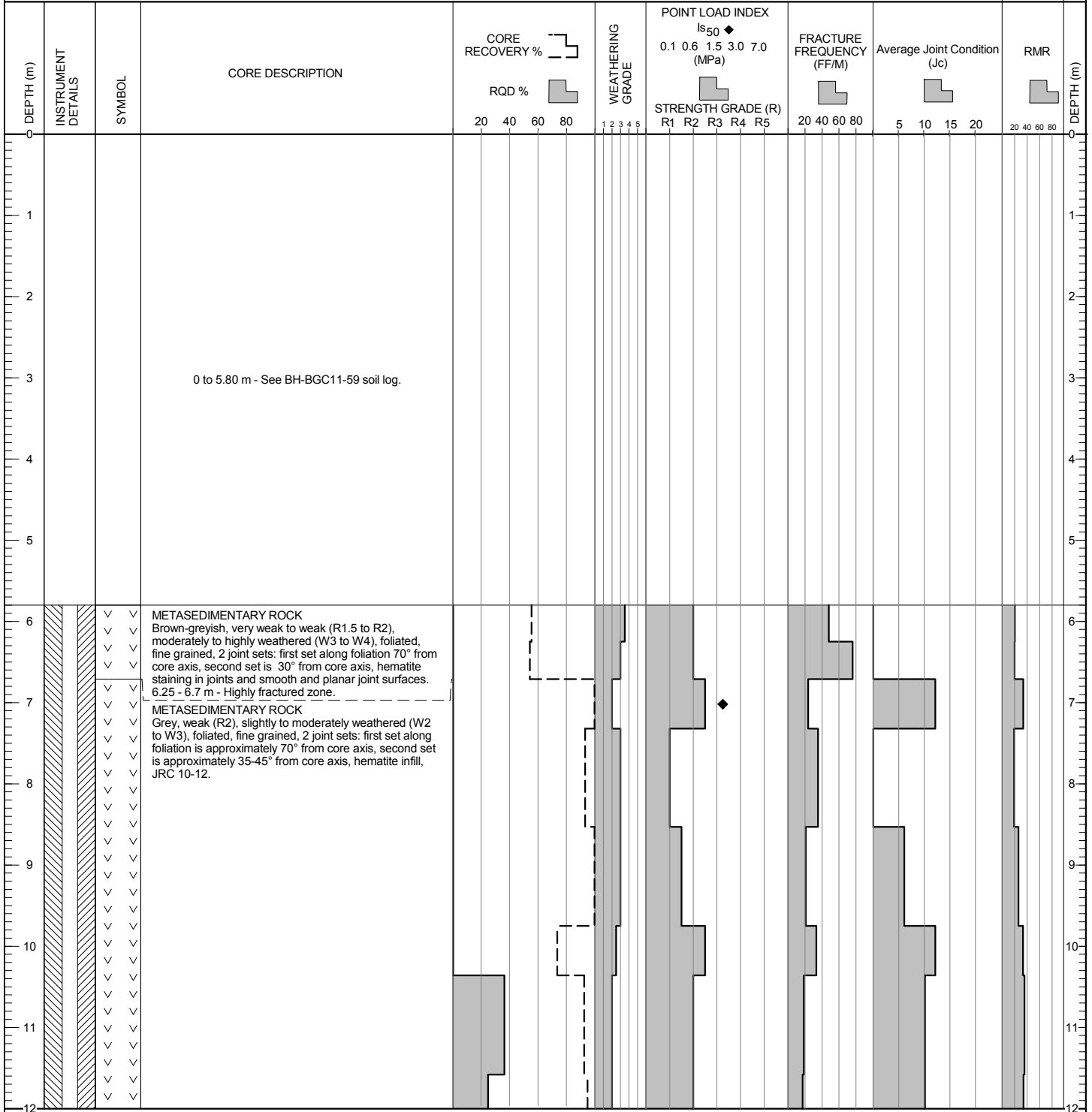


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 459,113.1E - 7,101,239.4N
 GROUND ELEVATION (m) : 883.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 5.79

START DATE : 10 Aug 11
 FINISH DATE : 11 Aug 11
 FINAL DEPTH (m) : 30.2
 DEPTH TO TOP OF ROCK (m) : 5.8
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW

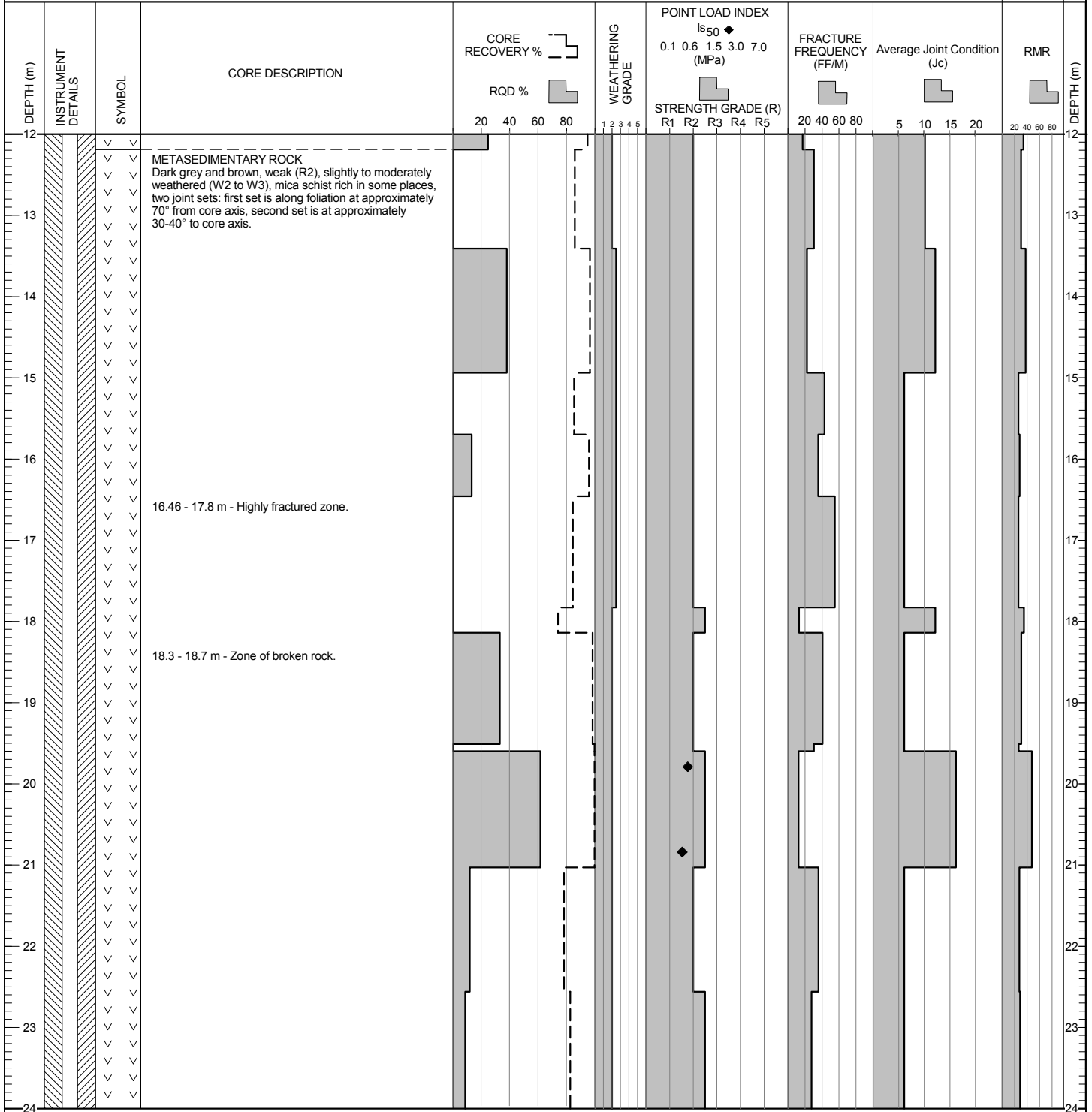


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CO-ORDINATES (m) 459,113.1E - 7,101,239.4N
 GROUND ELEVATION (m) : 883.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 5.79

START DATE : 10 Aug 11
 FINISH DATE : 11 Aug 11
 FINAL DEPTH (m) : 30.2
 DEPTH TO TOP OF ROCK (m) : 5.8
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW

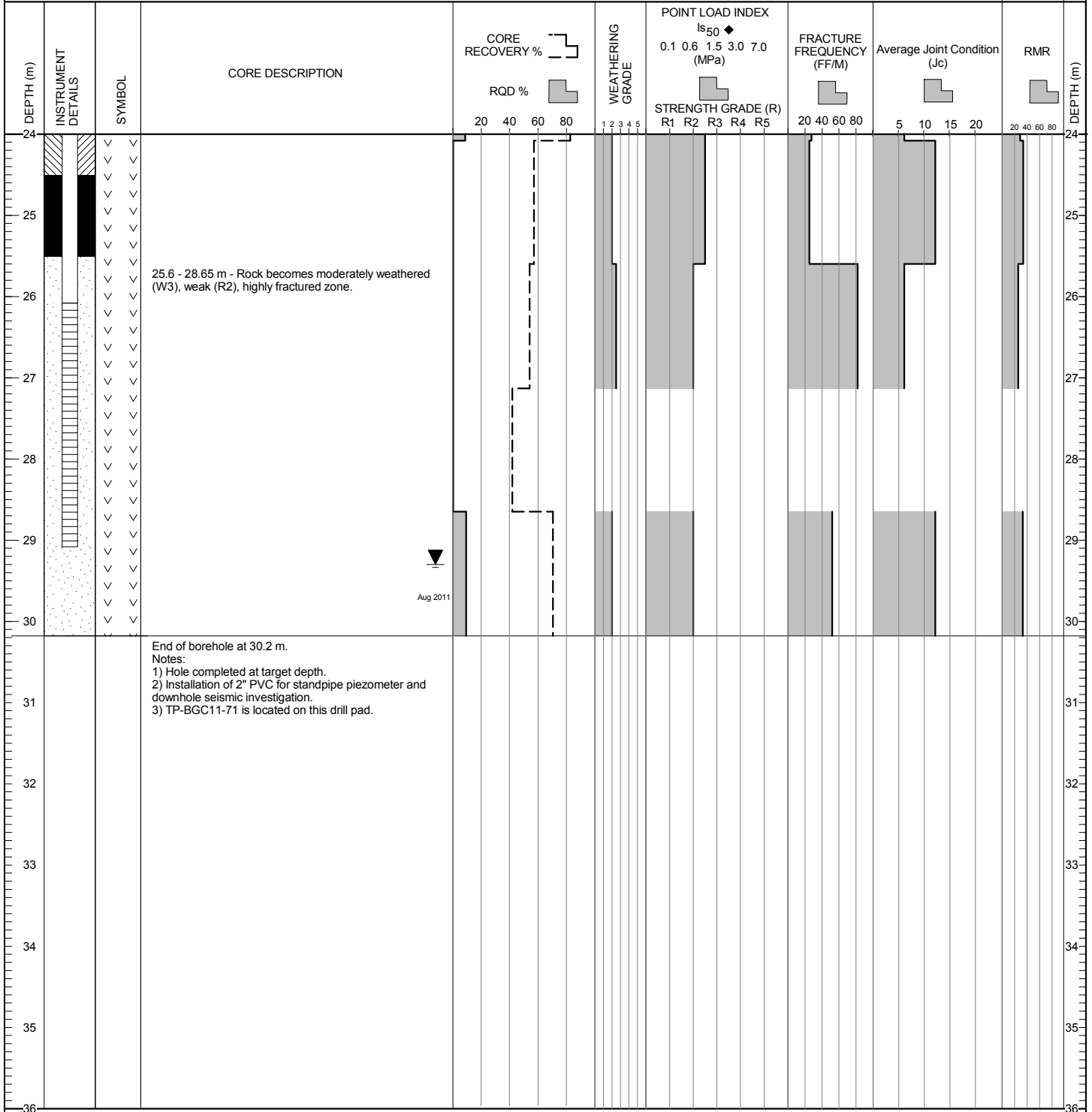


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CO-ORDINATES (m) 459,113.1E - 7,101,239.4N
 GROUND ELEVATION (m) : 883.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 5.79

START DATE : 10 Aug 11
 FINISH DATE : 11 Aug 11
 FINAL DEPTH (m) : 30.2
 DEPTH TO TOP OF ROCK (m) : 5.8
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/2/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

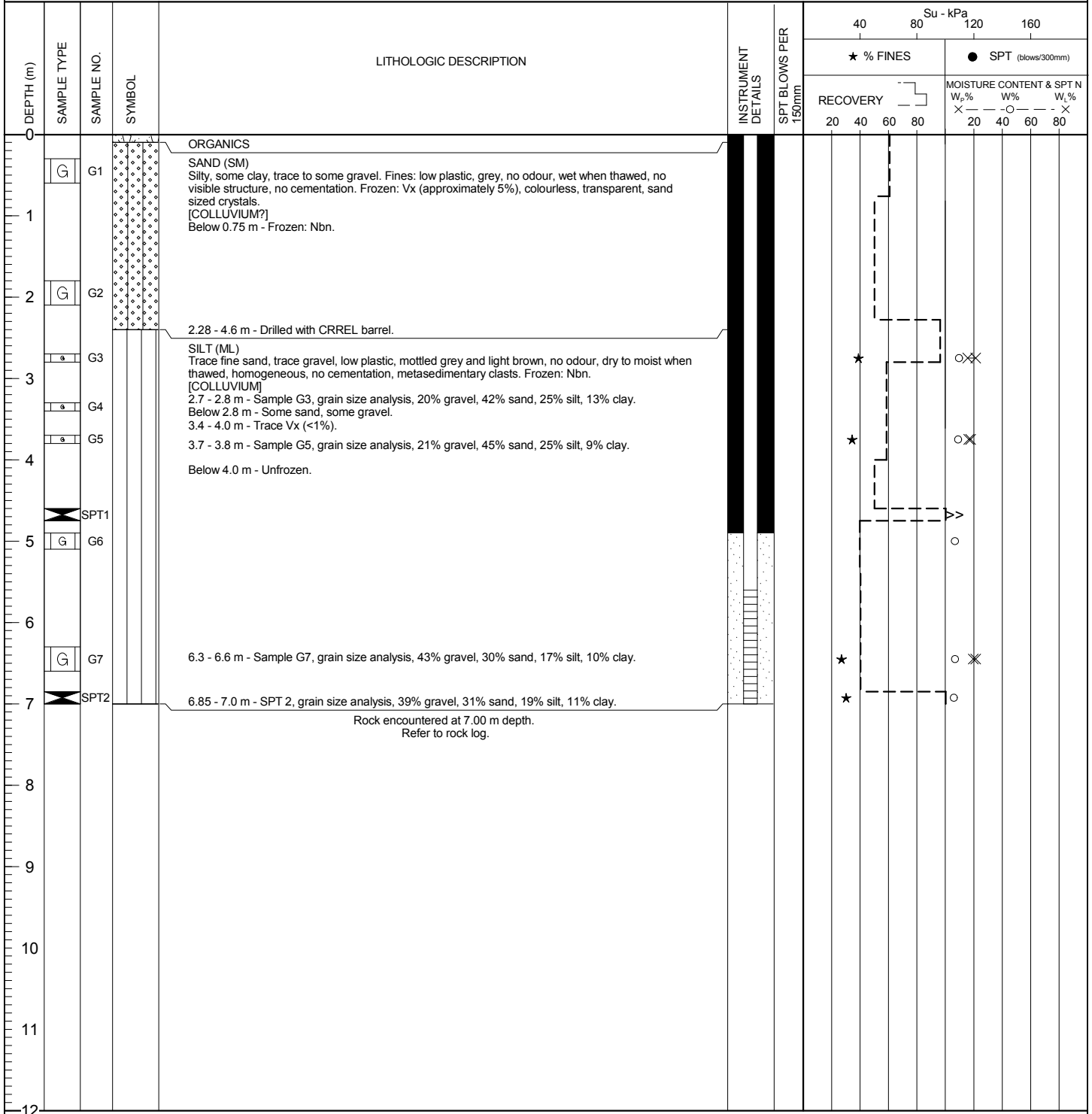
DRILL HOLE # BH-BGC11-60

LOCATION : STEINER AREA

CO-ORDINATES (m): 458,795.6E - 7,100,041.6N
 GROUND ELEVATION (m) : 859.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 10 Aug 11
 FINISH DATE : 10 Aug 11
 FINAL DEPTH (m) : 9.2
 DEPTH TO TOP OF ROCK (m) : 7.0
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGR (SOIL) EGR_SOIL_GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 458,795.6E - 7,100,041.6N
 GROUND ELEVATION (m) : 859.2m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE SIZE :
 FLUID :
 CASSED TO (m):

START DATE : 10 Aug 11
 FINISH DATE : 10 Aug 11
 FINAL DEPTH (m) : 9.2
 DEPTH TO TOP OF ROCK (m) : 7.0
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)				RMR	DEPTH (m)					
							Is ₅₀	R1	R2	R3	R4		R5										
0				20	40	60	80	1	2	3	4	5						20	40	60	80	0	
1																							1
2																							2
3																							3
4																							4
5																							5
6																							6
7																							7
8			SILT (ML) Sandy, some gravel, low plastic, hard (based on augering difficulty), light reddish brown, no odour, dry, no visible structure, no cementation, talc present on clast surfaces, angular and metasedimentary rock clasts, moderately weathered (W3), metasedimentary rock. Note - rock is broken down by drilling process and logged as soil.																				8
9																							9
10			End of borehole at 9.2 m. Notes: 1) Hole terminated due to refusal of auger. 2) Installation of 2" PVC for standpipe piezometer. 3) TP-BGC11-85 is located on this drill pad.																				10
11																							11
12																							12

EGP (ROCK) EGP-ROCK-GDL BGC-GDT 12/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-61

LOCATION : EAGLE PUP

CO-ORDINATES (m): 460,367.0E - 7,100,013.0N
 GROUND ELEVATION (m) : 1,152.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 11 Aug 11
 FINISH DATE : 11 Aug 11
 FINAL DEPTH (m) : 2.7
 DEPTH TO TOP OF ROCK (m) : 0.8
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa			
							★ % FINES		● SPT (blows/300mm)	
						RECOVERY	MOISTURE CONTENT & SPT N			
						20 40 60 80	W _p %	W _l %	W _u %	
						20 40 60 80	20 40 60 80	20 40 60 80	20 40 60 80	
0				DRILL PAD FILL						
0.8		G1		SAND (SM) Some silt, some gravel, well graded, maximum particle size = 10 mm, subangular to subrounded, grey, wet when thawed, no cementation. Frozen: Vx (approximately 10%). [COLLUVIUM]						
0.75	Rock encountered at 0.75 m depth. Refer to rock log.									
1										
2										
3										
4										
5										
6										
7										
8										
9										
10										
11										
12										

EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/20/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 460,367.0E - 7,100,013.0N
 GROUND ELEVATION (m) : 1,152.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 11 Aug 11
 FINISH DATE : 11 Aug 11
 FINAL DEPTH (m) : 2.7
 DEPTH TO TOP OF ROCK (m) : 0.8
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)				RMR	DEPTH (m)
				RQD %			R1	R2	R3	R4	R5							
0			0 to 0.75 m - See BH-BGC11-61 soil log.													0		
1			SAND (SP) Fine sand, trace silt, trace gravel, maximum particle size = 20 mm, angular, light brown, no odour, dry, no cementation, granodiorite clasts, moderately weathered (W3) granodiorite. Note - rock is broken down by drilling process and logged as soil.													1		
2																2		
3			End of borehole at 2.7 m. Notes: 1) Hole terminated due to refusal of auger. 2) Backfilled with cuttings. 3) TP-BGC11-106 and BH-BGC11-66 are located on this drill pad.													3		
4																4		
5																5		
6																6		
7																7		
8																8		
9																9		
10																10		
11																11		
12																12		

ESP (ROCK) ESP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-62

LOCATION : STUTTLE GULCH

CO-ORDINATES (m): 459,785.9E - 7,100,334.1N
 GROUND ELEVATION (m) : 1,017.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) :

START DATE : 11 Aug 11
 FINISH DATE : 12 Aug 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : JD/SP/KH
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa							
							★ % FINES		● SPT (blows/300mm)					
							RECOVERY		MOISTURE CONTENT & SPT N					
							20	40	60	80	W _p %	W ₉₀ %	W _L %	
0				GRAVEL (GP) Some cobbles, poorly (uniformly) graded, subangular, metasedimentary clasts. [COLLUVIUM]										
1														
1.5				Rock encountered at 1.50 m depth. Refer to rock log.										
2														
3														
4														
5														
6														
7														
8														
9														
10														
11														
12														

EGR/SOIL/ EGR_SOIL_GDL BGC.GDT 12/2/12

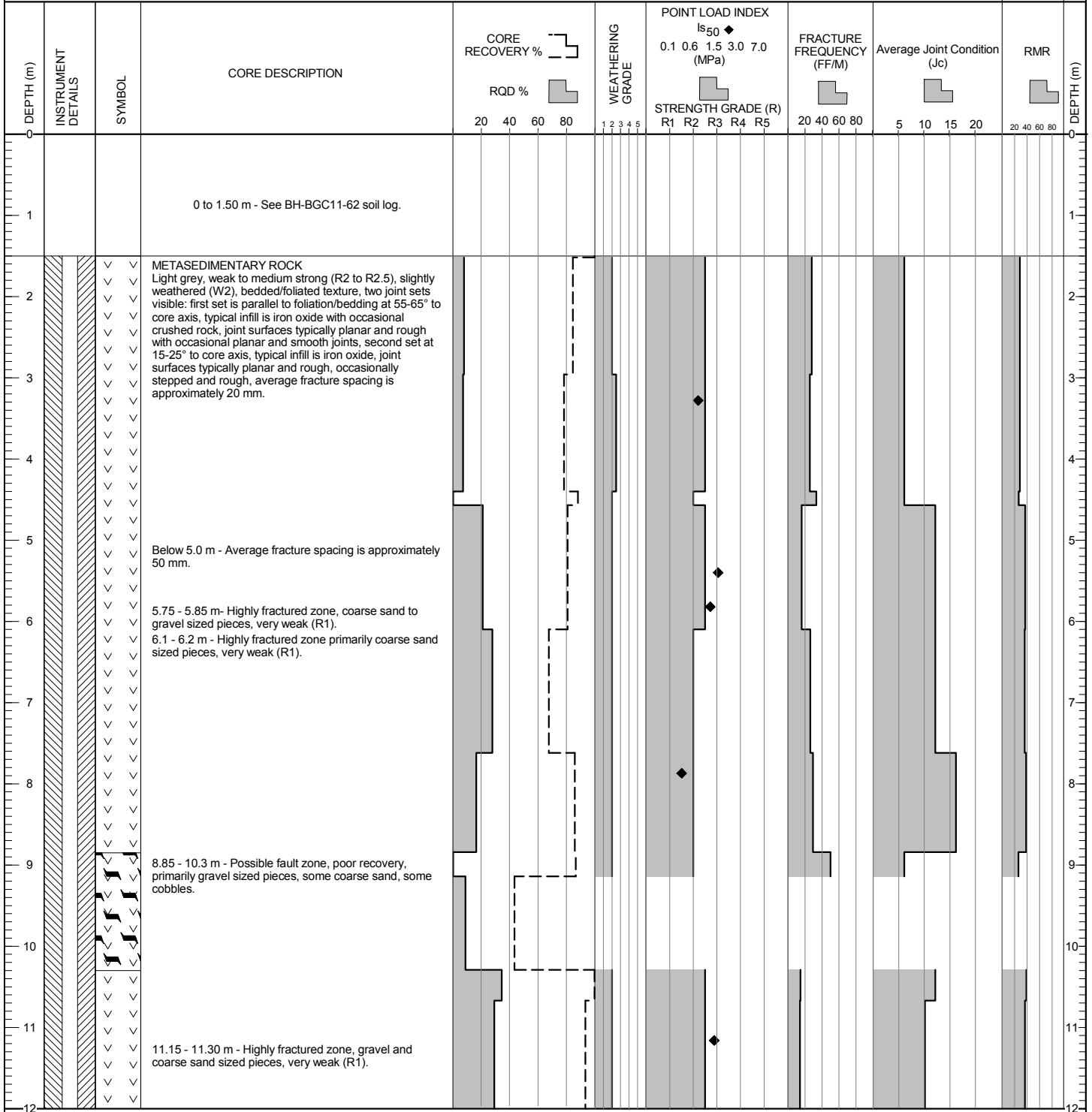


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 459,785.9E - 7,100,334.1N
 GROUND ELEVATION (m) : 1,017.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 11 Aug 11
 FINISH DATE : 12 Aug 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : JD/SP/KH
 REVIEWED BY : PQ/DW



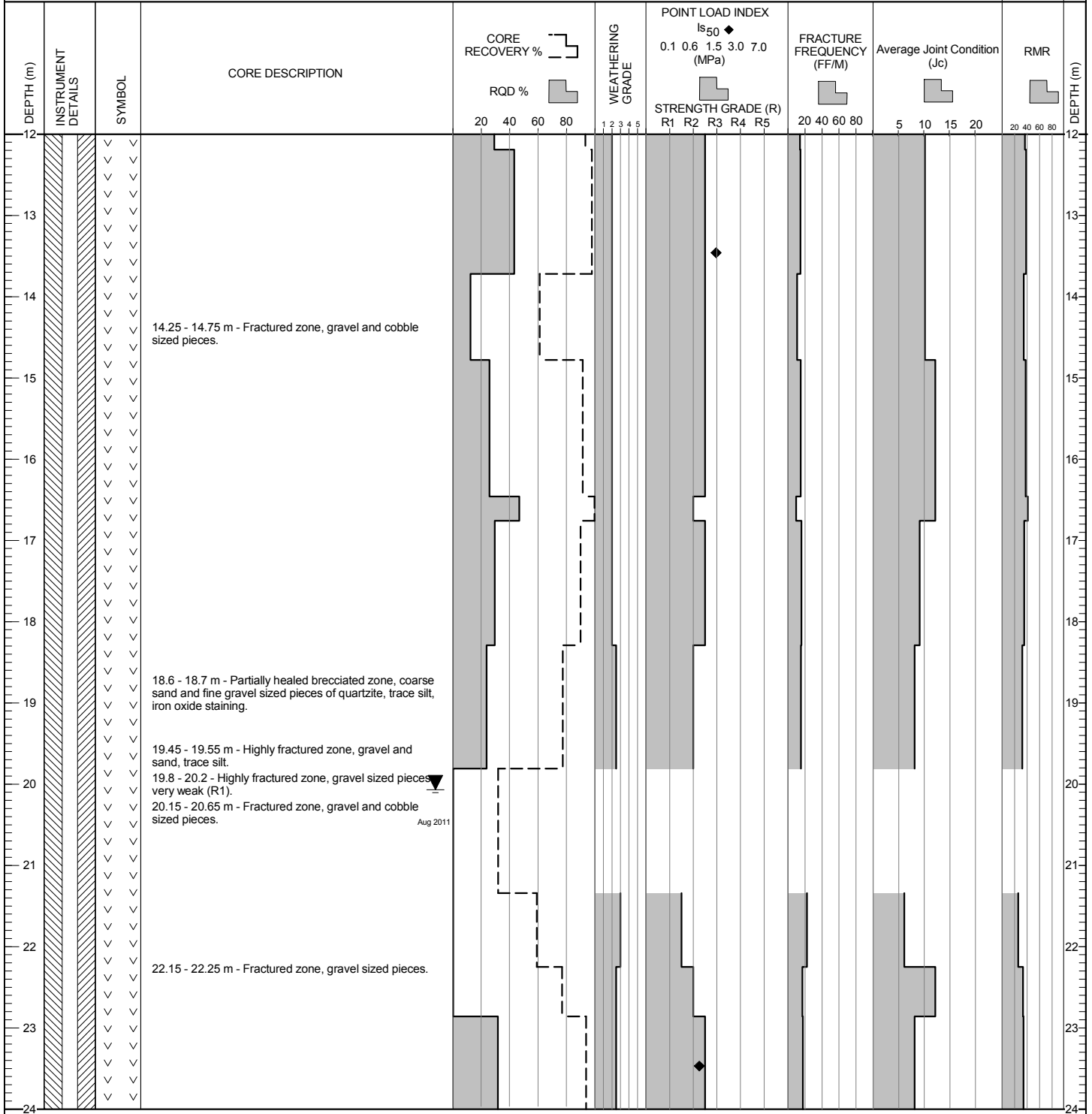
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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 459,785.9E - 7,100,334.1N
 GROUND ELEVATION (m) : 1,017.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 11 Aug 11
 FINISH DATE : 12 Aug 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : JD/SP/KH
 REVIEWED BY : PQ/DW



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EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/2/12

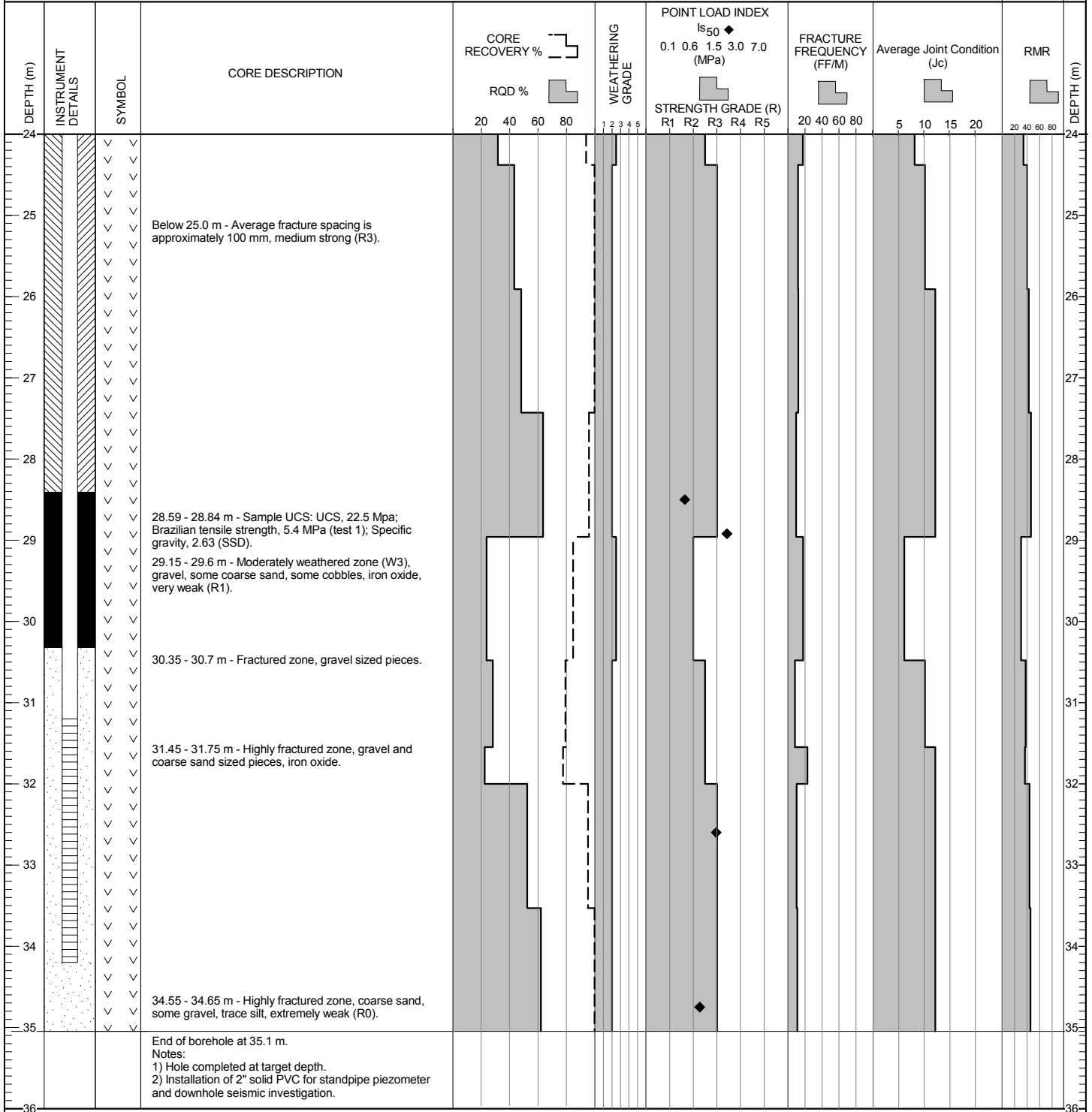
DRILL HOLE # BH-BGC11-62

LOCATION : STUTTLE GULCH

CO-ORDINATES (m) 459,785.9E - 7,100,334.1N
 GROUND ELEVATION (m) : 1,017.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 11 Aug 11
 FINISH DATE : 12 Aug 11
 FINAL DEPTH (m) : 35.1
 DEPTH TO TOP OF ROCK (m) : 1.5
 LOGGED BY : JD/SP/KH
 REVIEWED BY : PQ/DW

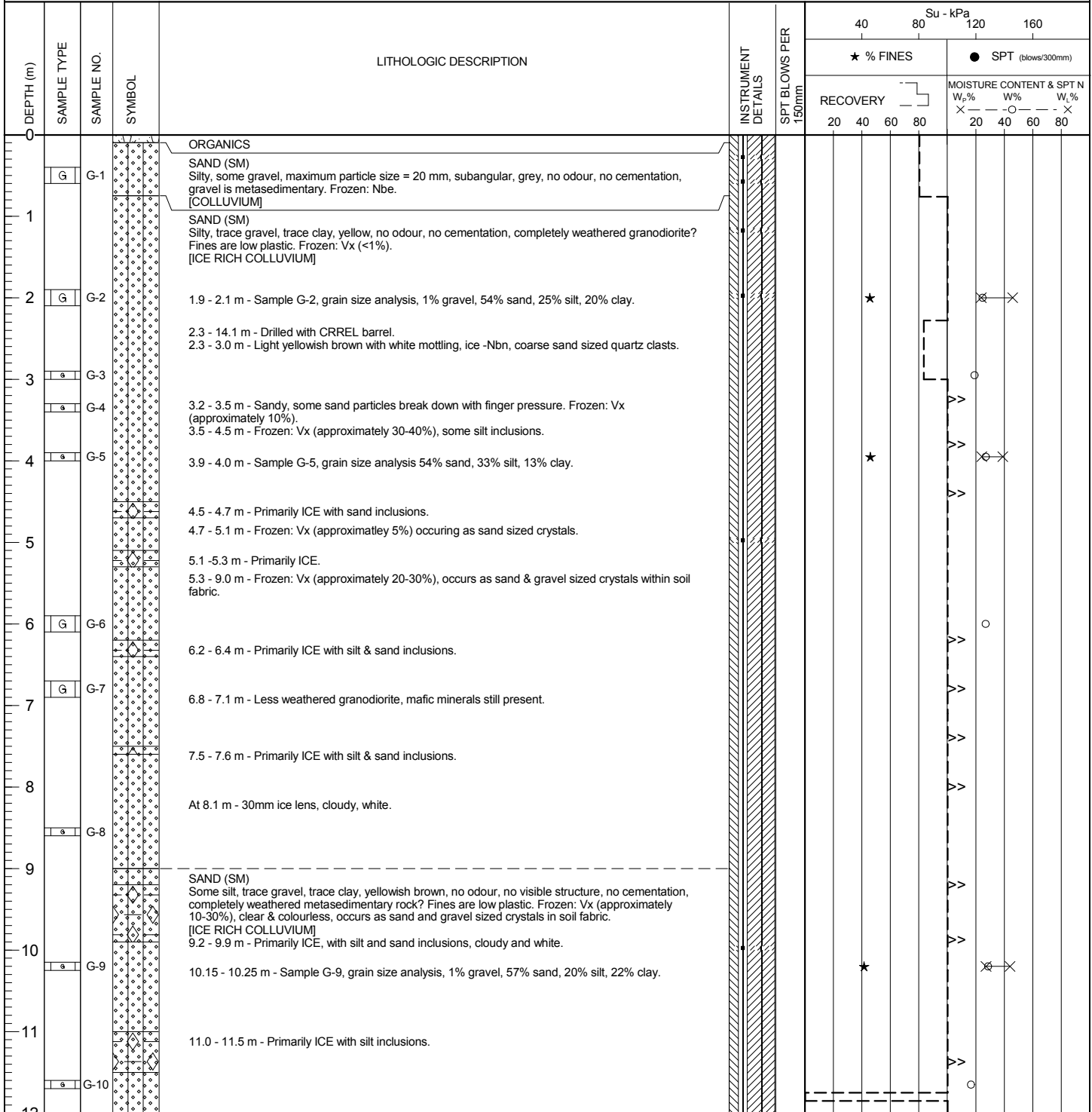


EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 460,302.9E - 7,100,114.1N
 GROUND ELEVATION (m) : 1,100.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 12 Aug 11
 FINISH DATE : 13 Aug 11
 FINAL DEPTH (m) : 26.7
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



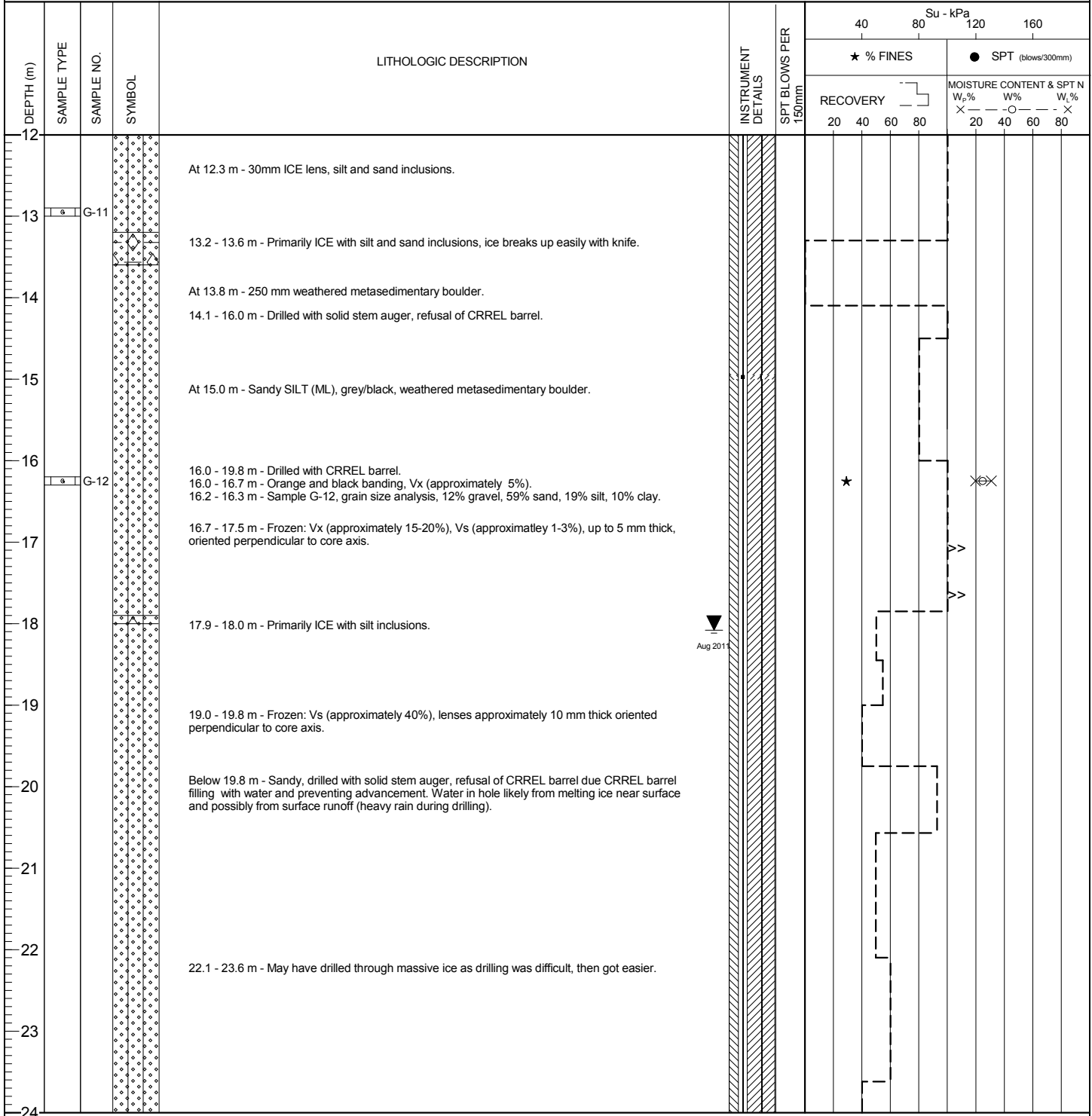
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EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

CO-ORDINATES (m): 460,302.9E - 7,100,114.1N
 GROUND ELEVATION (m) : 1,100.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 12 Aug 11
 FINISH DATE : 13 Aug 11
 FINAL DEPTH (m) : 26.7
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



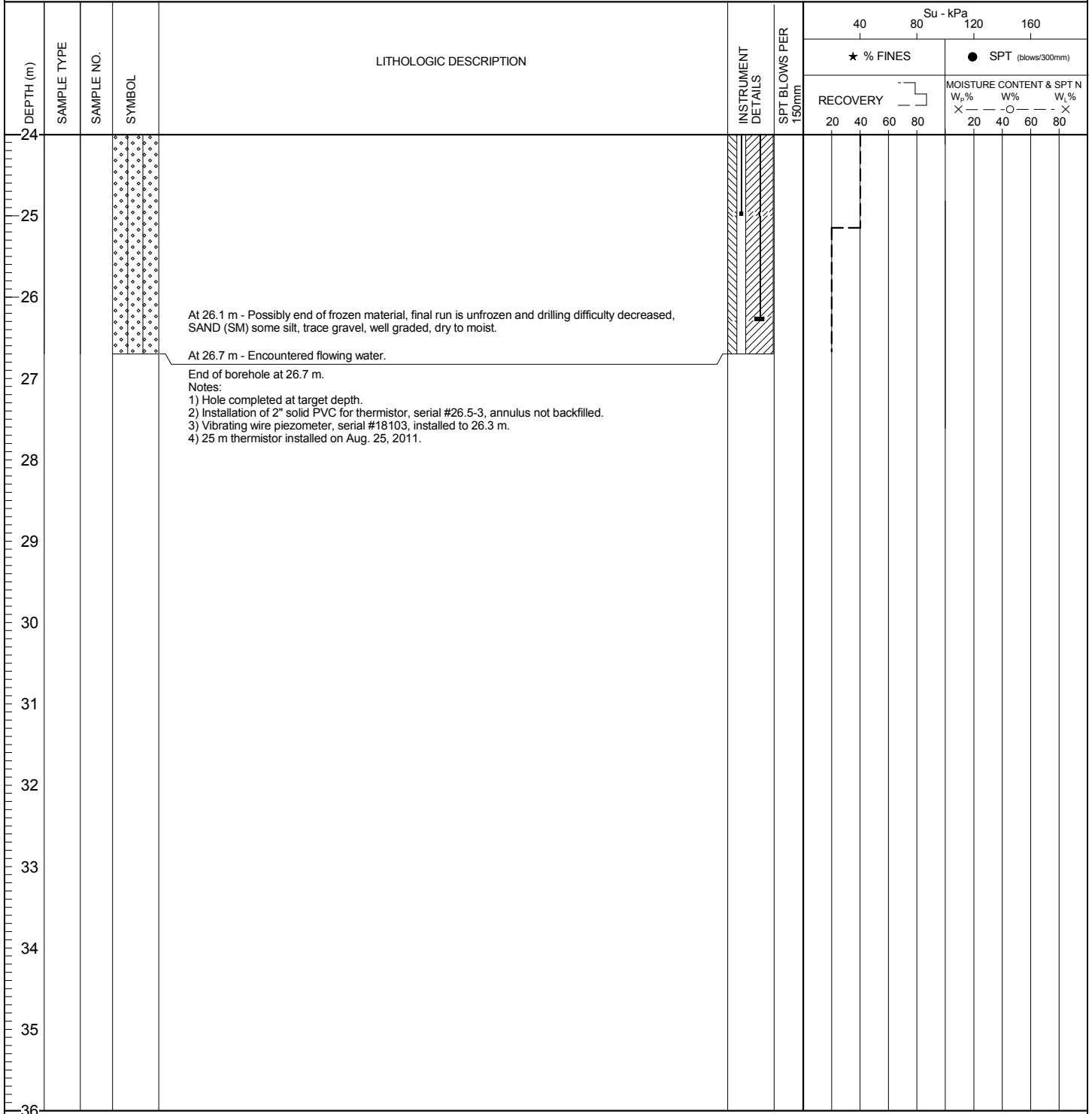
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EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/2/12

CO-ORDINATES (m): 460,302.9E - 7,100,114.1N
 GROUND ELEVATION (m) : 1,100.7m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 12 Aug 11
 FINISH DATE : 13 Aug 11
 FINAL DEPTH (m) : 26.7
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 12/21/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

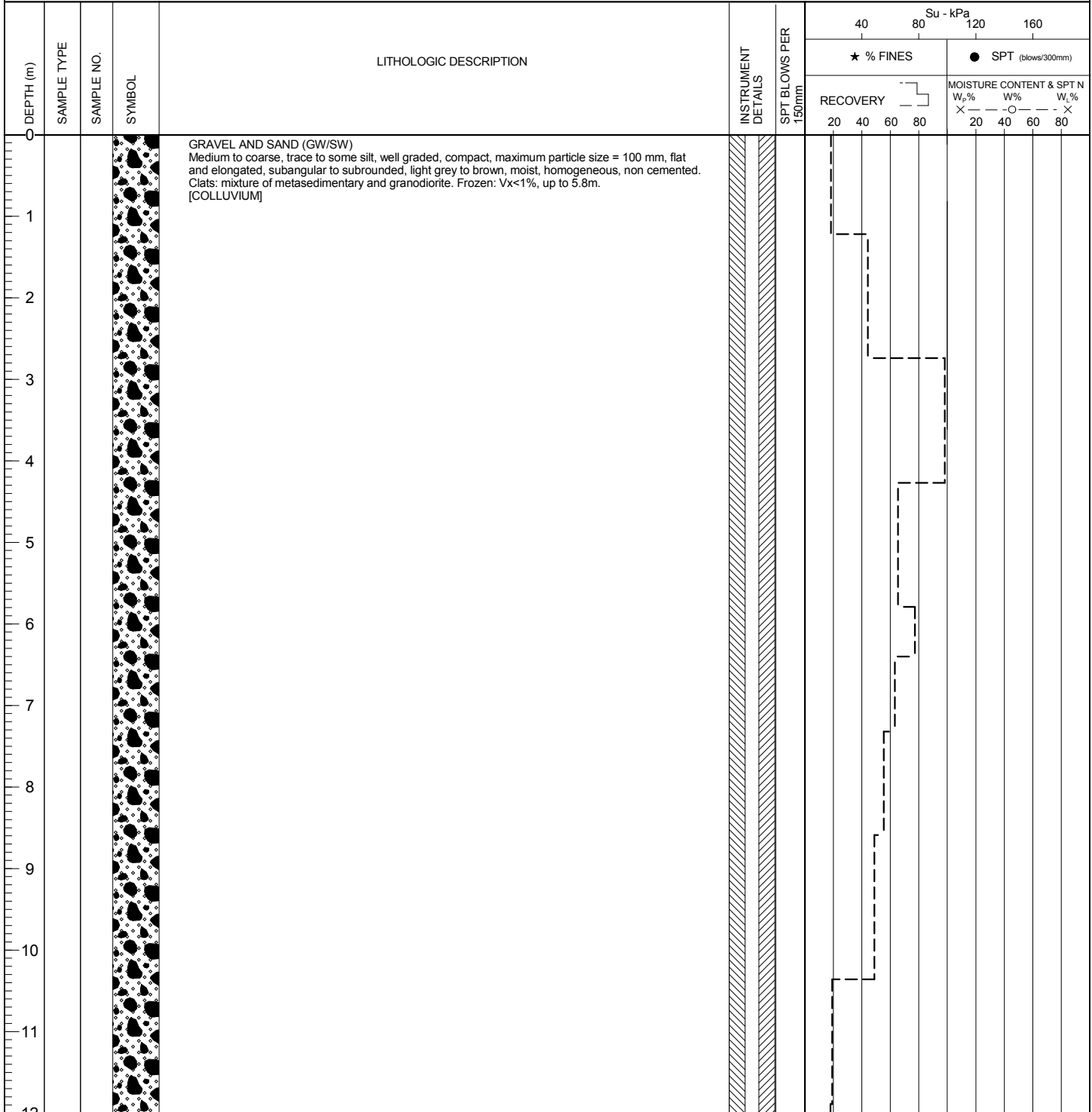
DRILL HOLE # BH-BGC11-64

LOCATION : EAGLE PUP

CO-ORDINATES (m): 460,182.4E - 7,100,262.9N
 GROUND ELEVATION (m) : 1,044.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) :

START DATE : 13 Aug 11
 FINISH DATE : 14 Aug 11
 FINAL DEPTH (m) : 48.8
 DEPTH TO TOP OF ROCK (m) : 16.4
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



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EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/2012

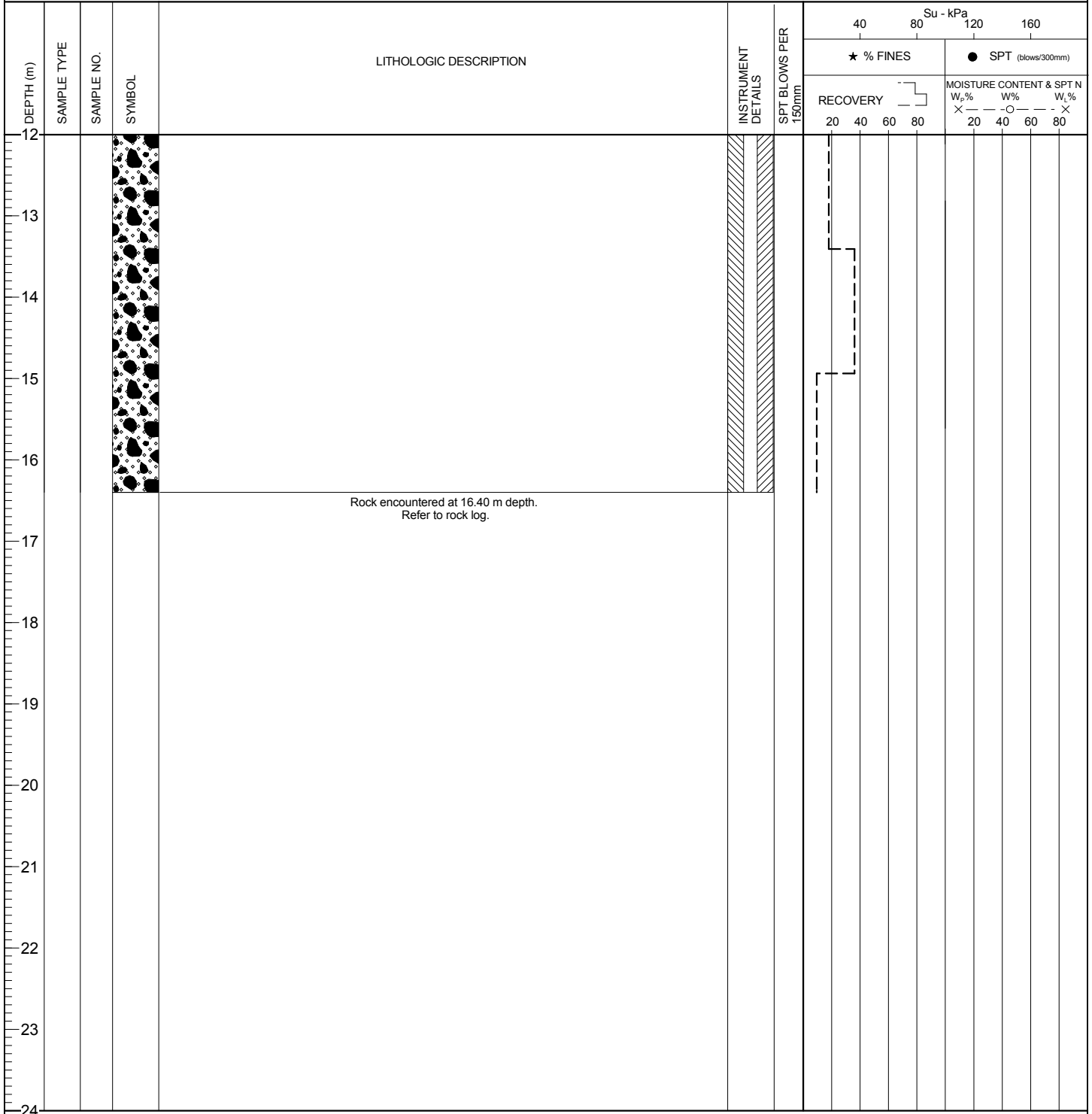


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m): 460,182.4E - 7,100,262.9N
 GROUND ELEVATION (m) : 1,044.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) :

START DATE : 13 Aug 11
 FINISH DATE : 14 Aug 11
 FINAL DEPTH (m) : 48.8
 DEPTH TO TOP OF ROCK (m) : 16.4
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



EGR/SOIL/EGP_SOIL_GDL BGC.GDT 12/9/12

CO-ORDINATES (m) 460,182.4E - 7,100,262.9N
 GROUND ELEVATION (m) : 1,044.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASED TO (m):

START DATE : 13 Aug 11
 FINISH DATE : 14 Aug 11
 FINAL DEPTH (m) : 48.8
 DEPTH TO TOP OF ROCK (m) : 16.4
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		R1	R2	R3	R4	R5				
12															12
13															13
14			0 to 16.40 m - See BH-BGC11-64 soil log.												14
15															15
16															16
17			GRANODIORITE ROCK Light grey, highly to completely altered/weathered (A4 to A5), extremely weak to weak (R0 to R1), massive, crystalline, medium grained, two joint sets: first set is approximately 50° to core axis, second set is approximately 90° to core axis, hematite and clay infill, JRC is approximately 2-4.												17
18			18.0 - 28.7 m - Highly fractured zone.												18
19															19
20															20
21															21
22															22
23															23
24															24

(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,182.4E - 7,100,262.9N
 GROUND ELEVATION (m) : 1,044.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 13 Aug 11
 FINISH DATE : 14 Aug 11
 FINAL DEPTH (m) : 48.8
 DEPTH TO TOP OF ROCK (m) : 16.4
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



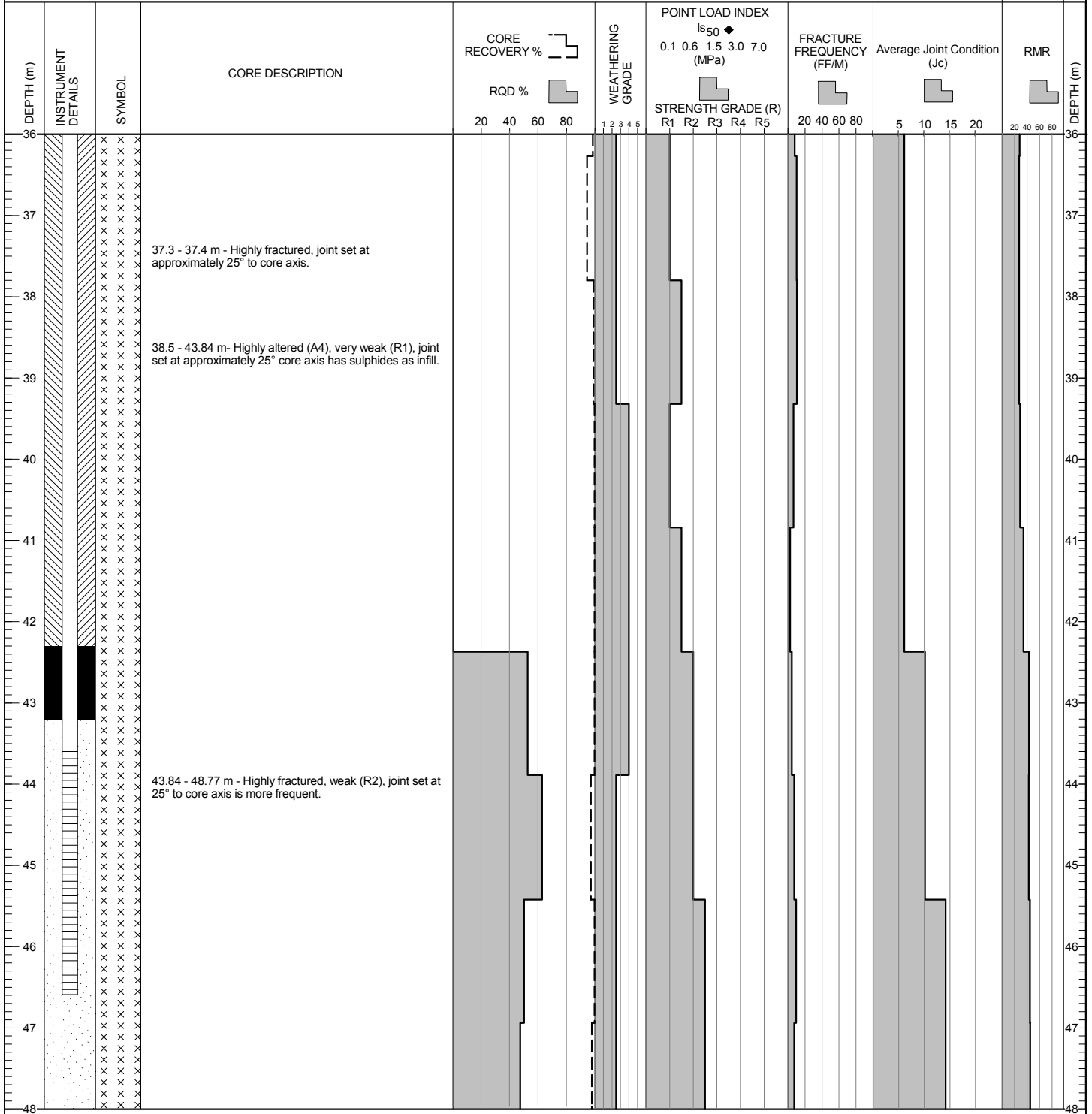
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,182.4E - 7,100,262.9N
 GROUND ELEVATION (m) : 1,044.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 13 Aug 11
 FINISH DATE : 14 Aug 11
 FINAL DEPTH (m) : 48.8
 DEPTH TO TOP OF ROCK (m) : 16.4
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



(CONTINUED ON NEXT PAGE)

EGP (ROCK) EGP-ROCK.GDL BGC.GDT 1/20/12

DRILL HOLE # BH-BGC11-64

LOCATION : EAGLE PUP

CO-ORDINATES (m) 460,182.4E - 7,100,262.9N
 GROUND ELEVATION (m) : 1,044.1m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 13 Aug 11
 FINISH DATE : 14 Aug 11
 FINAL DEPTH (m) : 48.8
 DEPTH TO TOP OF ROCK (m) : 16.4
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I _{s50} ♦ (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)										
				CORE RECOVERY %	RQD %		R1	R2	R3	R4	R5														
48	XXXXXX XXXXXX XXXXXX XXXXXX	XXXXXX XXXXXX XXXXXX XXXXXX		20	40	60	80	1	2	3	4	5	20	40	60	80	5	10	15	20	20	40	60	80	48
49			End of borehole at 48.8 m. Notes: 1) Hole completed at target depth. 2) Installation of 2" PVC for standpipe piezometer. 3) TP-BGC11-107 is located on this drill pad.																						49
50																									50
51																									51
52																									52
53																									53
54																									54
55																									55
56																									56
57																									57
58																									58
59																									59
60																									60

ESP (ROCK) ESP ROCK.GDL BGC.GDT 12/01/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-65

LOCATION : DUBLIN GULCH VALLEY BOTTOM

CO-ORDINATES (m): 458,771.0E - 7,100,939.0N
 GROUND ELEVATION (m) : 825.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger HSA
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 14 Aug 11
 FINISH DATE : 14 Aug 11
 FINAL DEPTH (m) : 6.9
 DEPTH TO TOP OF ROCK (m) :
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	Su - kPa							
						★ % FINES		● SPT (blows/300mm)					
						RECOVERY		MOISTURE CONTENT & SPT N					
						20	40	60	80	W _p %	W ₅ %	W ₁₀ %	
0				SAND AND GRAVEL (SW-GW) Trace boulders, trace cobbles, trace to some silt, well graded, compact, maximum particle size = 400 mm, (visible on surface in area of drill pad), subangular to subrounded, greyish brown, dry to moist, no visible structure, no cementation, gravel/cobbles/boulders are metasedimentary and granodiorite. [PLACER TAILINGS] 0.76 - 1.21 m - SPT 1, grain size analysis, 26% gravel, 60% sand, 14% fines.		10							
1	▲	SPT1				14	★				○	●	
2	▲	SPT2		1.52 - 1.97 m - SPT 2, grain size analysis, 40% gravel, 48% sand, 12% fines.		8	★				○	●	
3	▲	SPT3		2.28 - 2.73 m - SPT 3, grain size analysis, 53% gravel, 38% sand, 9% fines.		4	★				○	●	
4	▲	SPT4		3.04 - 3.49 m - SPT 4, grain size analysis, 63% gravel, 29% sand, 8% fines.		6	★				○	●	
5	▲	SPT5		3.8 - 4.25 m - SPT 5, grain size analysis, 76% gravel, 19% sand, 5% fines.		10	★				○	●	
6	▲	SPT6		4.56 - 5.01 m - SPT 6, grain size analysis, 40% gravel, 44% sand, 16% fines.		15	★				○	●	
7	▲	SPT7		5.33 - 5.75 m - SPT 7, grain size analysis, 64% gravel, 26% sand, 10% fines.		12	★				○		
8	▲	SPT8		At 5.7 m - Wet; possibly groundwater level.									
9	▲	SPT9		6.09 - 6.54 m - SPT 8, grain size analysis, 53% gravel, 38% sand, 9% fines.			★				○		
10				End of borehole at 6.9 m. Notes: 1) Hole terminated due to refusal of augers & SPT. 2) Backfilled with cuttings. 3) A second attempt was made approximately 1.0 m west, refusal met at 3.8 m. 4) Adjacent diamond hole BH-BGC11-32, located approximately 10 m west and 2-3 m lower. 5) Description based on SPT recovery and material visible on drill pad. No recovery between SPTs as drilling was completed with hollow stem augers which were left in the ground as casing during SPT testing.									

EGR (SOIL) EGR_SOIL_GDL BGC.GDT 1/20/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

DRILL HOLE # BH-BGC11-66

LOCATION : EAGLE PUP

CO-ORDINATES (m): 460,360.2E - 7,100,011.1N
 GROUND ELEVATION (m) : 1,151.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) :

START DATE : 15 Aug 11
 FINISH DATE : 16 Aug 11
 FINAL DEPTH (m) : 33.5
 DEPTH TO TOP OF ROCK (m) : 3.9
 LOGGED BY : JD/SP
 REVIEWED BY : PQ/DW

DEPTH (m)	SAMPLE TYPE	SAMPLE NO.	SYMBOL	LITHOLOGIC DESCRIPTION	INSTRUMENT DETAILS	SPT BLOWS PER 150mm	Su - kPa								
							★ % FINES		● SPT (blows/300mm)						
							RECOVERY		MOISTURE CONTENT & SPT N						
							20	40	60	80	W _p %	W ₁₀ %	W ₂₀ %	W _L %	
0				GRAVEL (GW) Cobbly, sandy, some boulders, wet, yellowish brown, homogeneous. Clasts: granodiorite, elongated to equidimensional, angular to subangular, strong to very strong (R4 to R5), fresh to faintly weathered (W1 to W1.5). Frozen: Vx. [COLLUVIUM]											
1															
2															
3				GRAVEL (GW) Sandy, maximum particle size = 50 mm. Clasts: elongated to equidimensional, angular to subangular, moderately weathered granodiorite, very weak to weak (R1 to R2), highly weathered (W4), wet when thawed, yellowish brown, stratified/laminated. Frozen: Vx with visible ice infilling some of the fractures. [HIGHLY TO COMPLETELY WEATHERED ROCK]											
4				Rock encountered at 3.90 m depth. Refer to rock log.											
5															
6															
7															
8															
9															
10															
11															
12															

EGR (SOIL) EGR_SOIL_GDL BGC.GDT 12/20/12

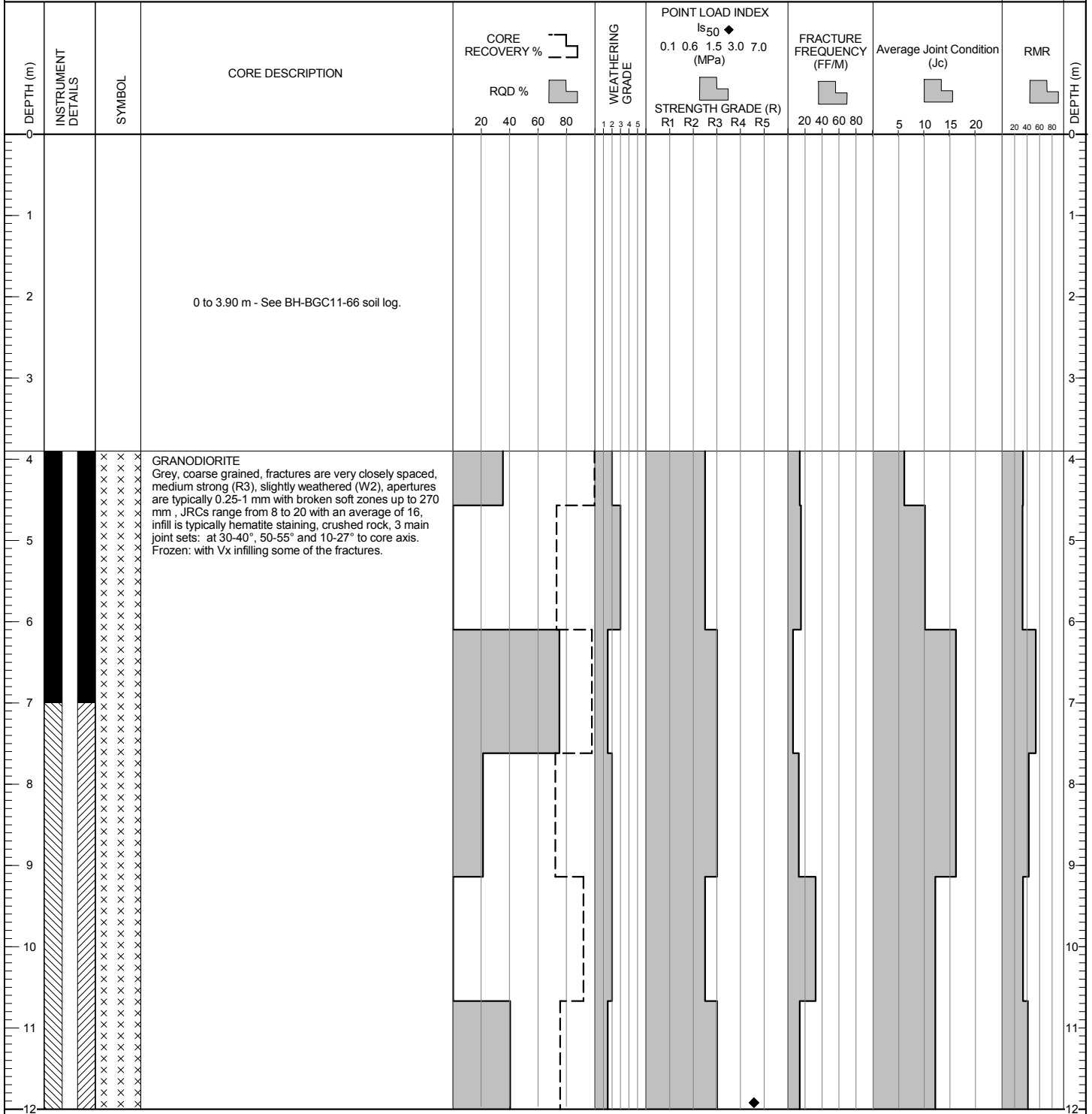


CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 460,360.2E - 7,100,011.1N
 GROUND ELEVATION (m) : 1,151.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 15 Aug 11
 FINISH DATE : 16 Aug 11
 FINAL DEPTH (m) : 33.5
 DEPTH TO TOP OF ROCK (m) : 3.9
 LOGGED BY : JD/SP
 REVIEWED BY : PQ/DW



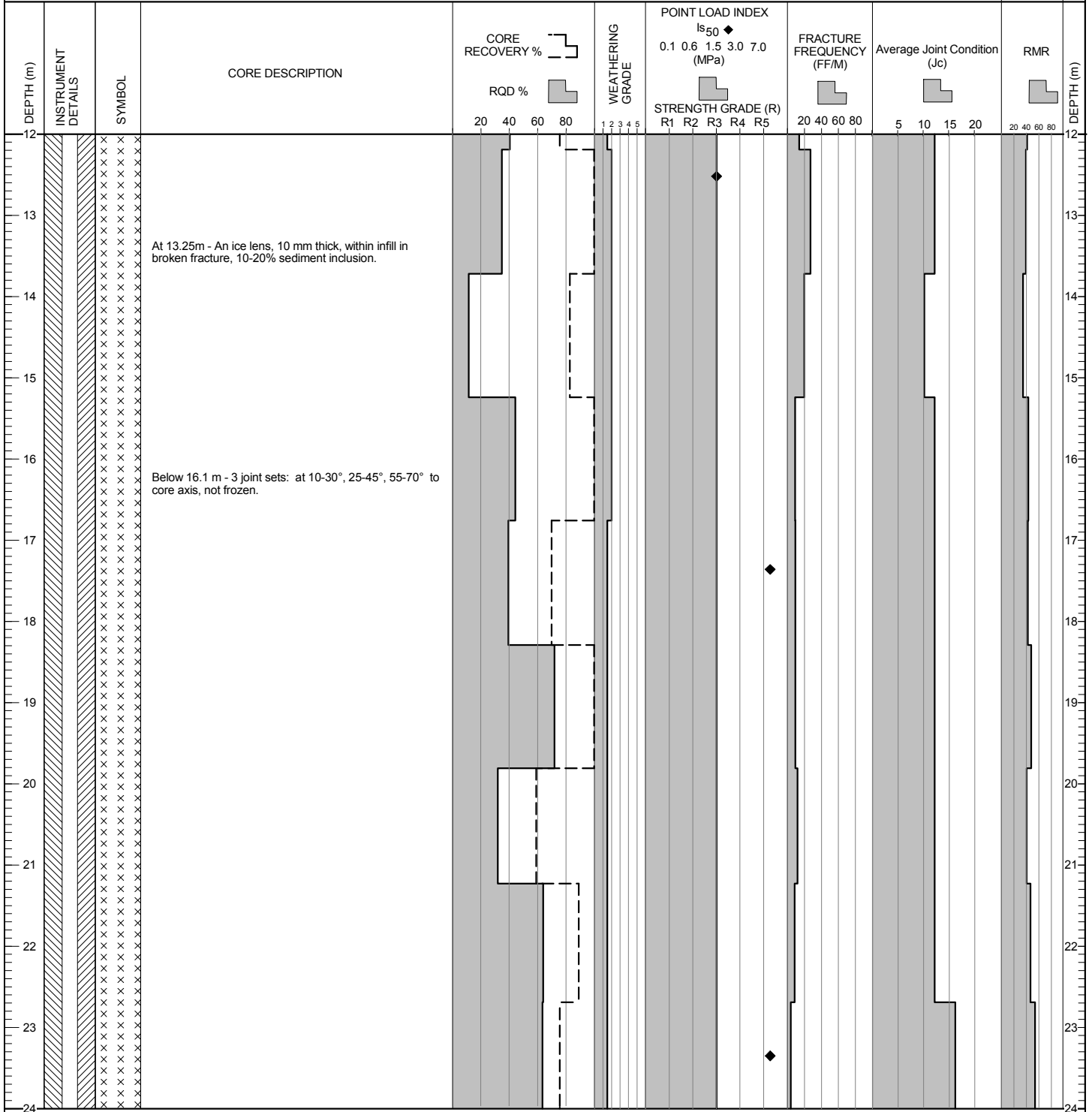
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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

CO-ORDINATES (m) 460,360.2E - 7,100,011.1N
 GROUND ELEVATION (m) : 1,151.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 15 Aug 11
 FINISH DATE : 16 Aug 11
 FINAL DEPTH (m) : 33.5
 DEPTH TO TOP OF ROCK (m) : 3.9
 LOGGED BY : JD/SP
 REVIEWED BY : PQ/DW

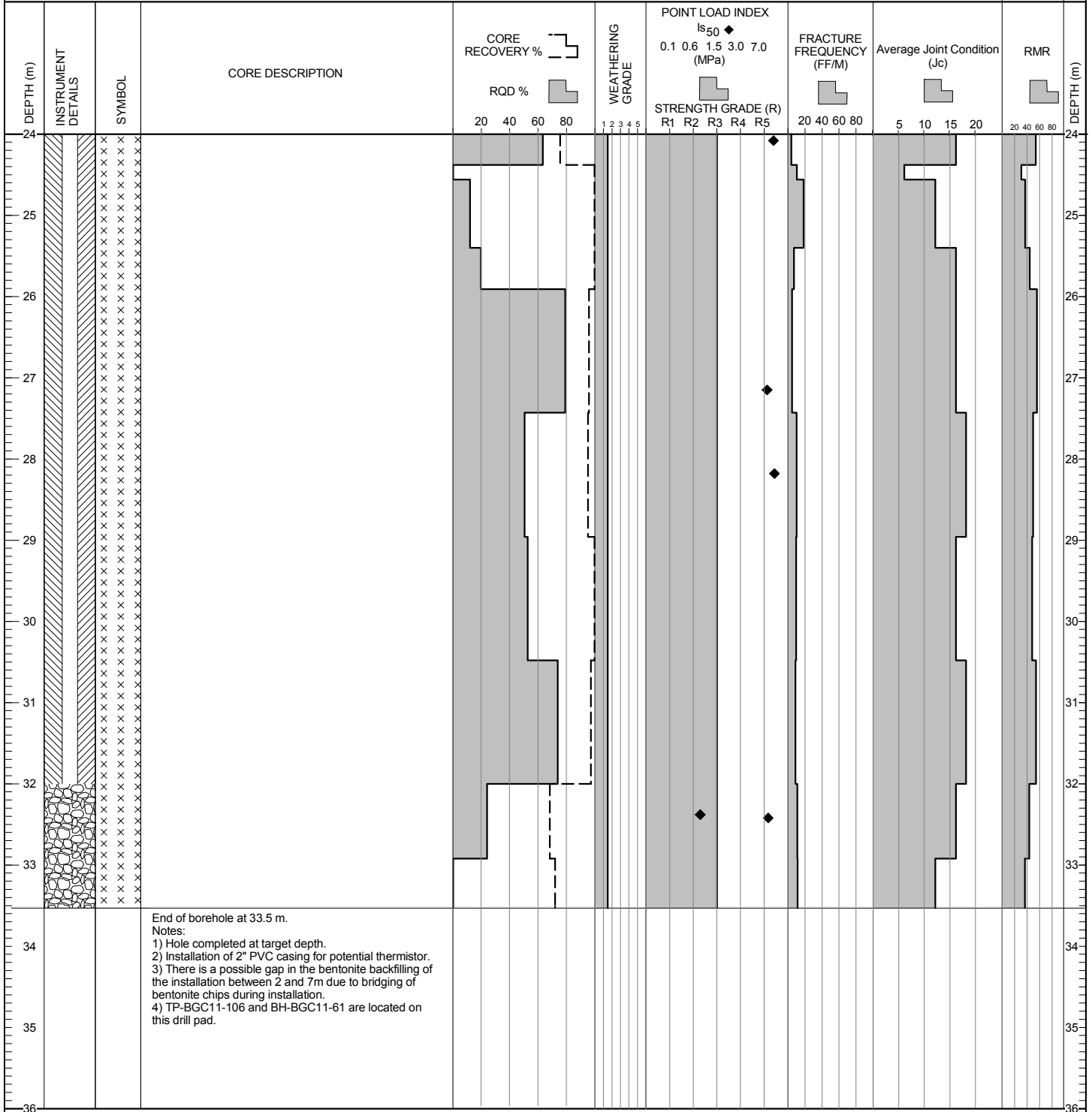


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CO-ORDINATES (m) 460,360.2E - 7,100,011.1N
 GROUND ELEVATION (m) : 1,151.9m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m):

START DATE : 15 Aug 11
 FINISH DATE : 16 Aug 11
 FINAL DEPTH (m) : 33.5
 DEPTH TO TOP OF ROCK (m) : 3.9
 LOGGED BY : JD/SP
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

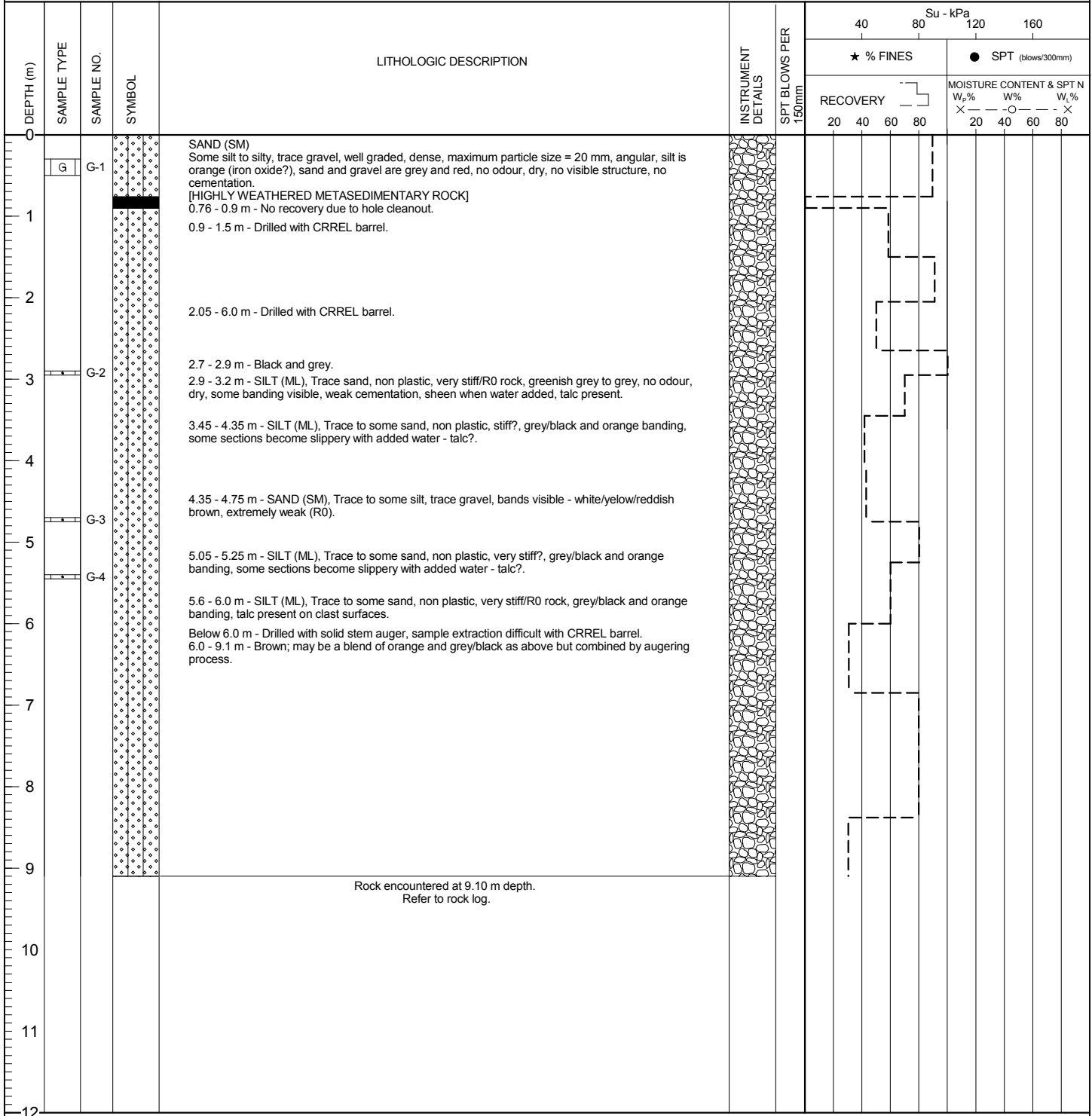
DRILL HOLE # BH-BGC11-67

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m): 458,823.0E - 7,101,148.0N
 GROUND ELEVATION (m) : 862.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE : N/A
 FLUID : N/A
 CASED TO (m) :

START DATE : 15 Aug 11
 FINISH DATE : 15 Aug 11
 FINAL DEPTH (m) : 9.9
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : KH
 REVIEWED BY : PQ/DW



EGR (SOIL) EGP_SOIL_GDL BGC.GDT 1/20/12

DRILL HOLE # BH-BGC11-67

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m) 458,823.0E - 7,101,148.0N
 GROUND ELEVATION (m) : 862.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 15 Aug 11
 FINISH DATE : 15 Aug 11
 FINAL DEPTH (m) : 9.9
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I _{s50} ♦ 0.1 0.6 1.5 3.0 7.0 (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)				RMR	DEPTH (m)				
				RQD %			R1	R2	R3	R4	R5		20	40	60	80			5	10	15	20
0																						0
1																						1
2																						2
3																						3
4																						4
5			0 to 9.10 m - See BH-BGC11-67 soil log.																			5
6																						6
7																						7
8																						8
9																						9
10		✓ ✓ ✓ ✓ ✓	SAND (SW) Trace silt, trace gravel, well graded, dense?, maximum particle size = 10 mm, angular, some flat gravel, light orange, no odour, dry, no visible structure, no cementation, slight powdery texture, moderately weathered (W3) metasedimentary rock. Note - rock is broken down by drilling process and logged as soil.																			10
11			End of borehole at 9.9 m. Notes: 1) Hole terminated due to refusal of auger. 2) Backfilled with cuttings. 3) TP-BGC11-103 is located on this drill pad. 4) 4 DCPT tests completed within 4 m of this hole, met refusal at 2.5 m, 3.22m, 0.45 m and 0.6 m. 5) Drill pad approximately 5.0 m below ground surface in plate load testing pit (PT-BGC11-01), BH-BGC11-69																			11
12																						12

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EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

DRILL HOLE # BH-BGC11-67

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m) 458,823.0E - 7,101,148.0N
 GROUND ELEVATION (m) : 862.0m
 SURVEY METHOD : Handheld GPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : MARL 4CT
 DRILLING CONTRACTOR : Midnight Sun Drilling
 DRILL METHOD : Auger SSA/CRREL
 CORE SIZE :
 FLUID :
 CASED TO (m):

START DATE : 15 Aug 11
 FINISH DATE : 15 Aug 11
 FINAL DEPTH (m) : 9.9
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : KH
 REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %		WEATHERING GRADE	POINT LOAD INDEX I_{s50} (MPa)					FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				CORE RECOVERY %	RQD %		0.1	0.6	1.5	3.0	7.0				
12			(rotary diamond) was drilled adjacent to this hole.												12
13															13
14															14
15															15
16															16
17															17
18															18
19															19
20															20
21															21
22															22
23															23
24															24

EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

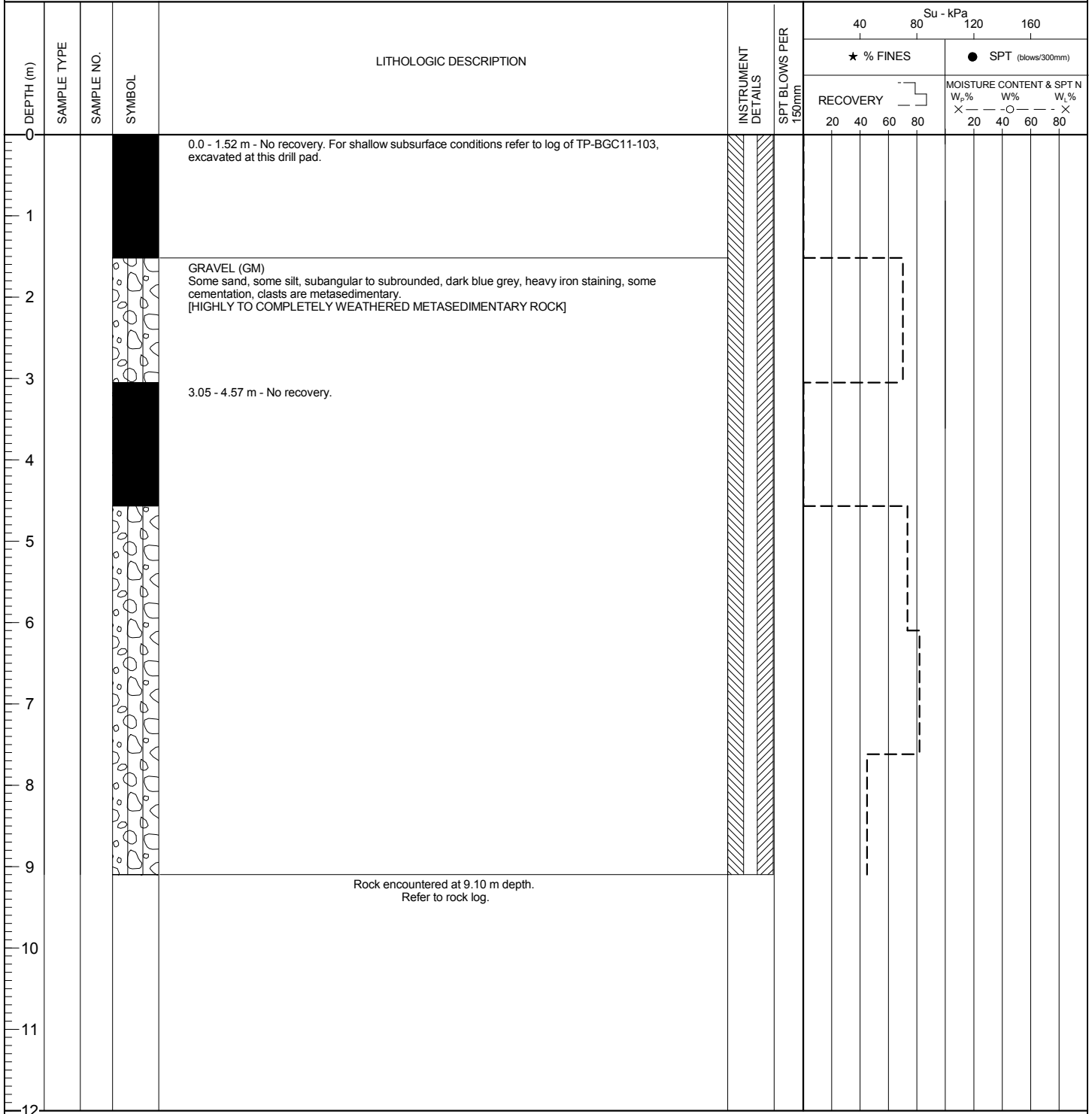
DRILL HOLE # BH-BGC11-69

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m): 458,829.3E - 7,101,144.3N
 GROUND ELEVATION (m) : 861.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : N/A

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE : HQ3
 FLUID : Water/Polymer
 CASED TO (m) : 3.05

START DATE : 17 Aug 11
 FINISH DATE : 17 Aug 11
 FINAL DEPTH (m) : 21.3
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



Rock encountered at 9.10 m depth.
 Refer to rock log.

EGR(SOIL) EGP_SOIL_GDL BGC.GDT 12/2/12



CLIENT: Victoria Gold Corporation
 PRINT DATE: 1/20/2012

CO-ORDINATES (m) 458,829.3E - 7,101,144.3N

DRILL DESIGNATION : CS-1000

START DATE : 17 Aug 11

GROUND ELEVATION (m) : 861.6m

DRILLING CONTRACTOR : Lyncorp

FINISH DATE : 17 Aug 11

SURVEY METHOD : DGPS

DRILL METHOD : Rotary Diamond

FINAL DEPTH (m) : 21.3

DATUM : UTM NAD 83

CORE SIZE : HQ3

DEPTH TO TOP OF ROCK (m) : 9.1

PLUNGE (°) : -90

FLUID : Water/Polymer

LOGGED BY : JD/SP/DW

TREND (°) : n/a

CASED TO (m): 3.05

REVIEWED BY : PQ/DW

DEPTH (m)	INSTRUMENT DETAILS	SYMBOL	CORE DESCRIPTION	CORE RECOVERY %	WEATHERING GRADE	POINT LOAD INDEX I _{s50} (MPa)	STRENGTH GRADE (R)	FRACTURE FREQUENCY (FF/M)	Average Joint Condition (Jc)	RMR	DEPTH (m)
				20 40 60 80	1 2 3 4 5	0.1 0.6 1.5 3.0 7.0 R1 R2 R3 R4 R5		20 40 60 80	5 10 15 20	20 40 60 80	
0											0
1											1
2											2
3											3
4											4
5			0 to 9.10 m - See BH-BGC11-69 soil log.								5
6											6
7											7
8											8
9											9
10		v v	METASEDIMENTARY ROCK Dark grey and brown, extremely weak to very weak (R0 to R1.5), moderately to highly weathered (W3 to W4), foliated, fine grained, 2 joint sets: one parallel to foliation at 45-50° from core axis, second set is at 20-25° from core axis. 10.0 - 12.26 m - Weak (R2), moderately weathered (W3).								10
11		v v									11
12		v v									12

(CONTINUED ON NEXT PAGE)

PROJECT: Eagle Gold Project - Infrastructure
 PROJECT NO.: 0792-006

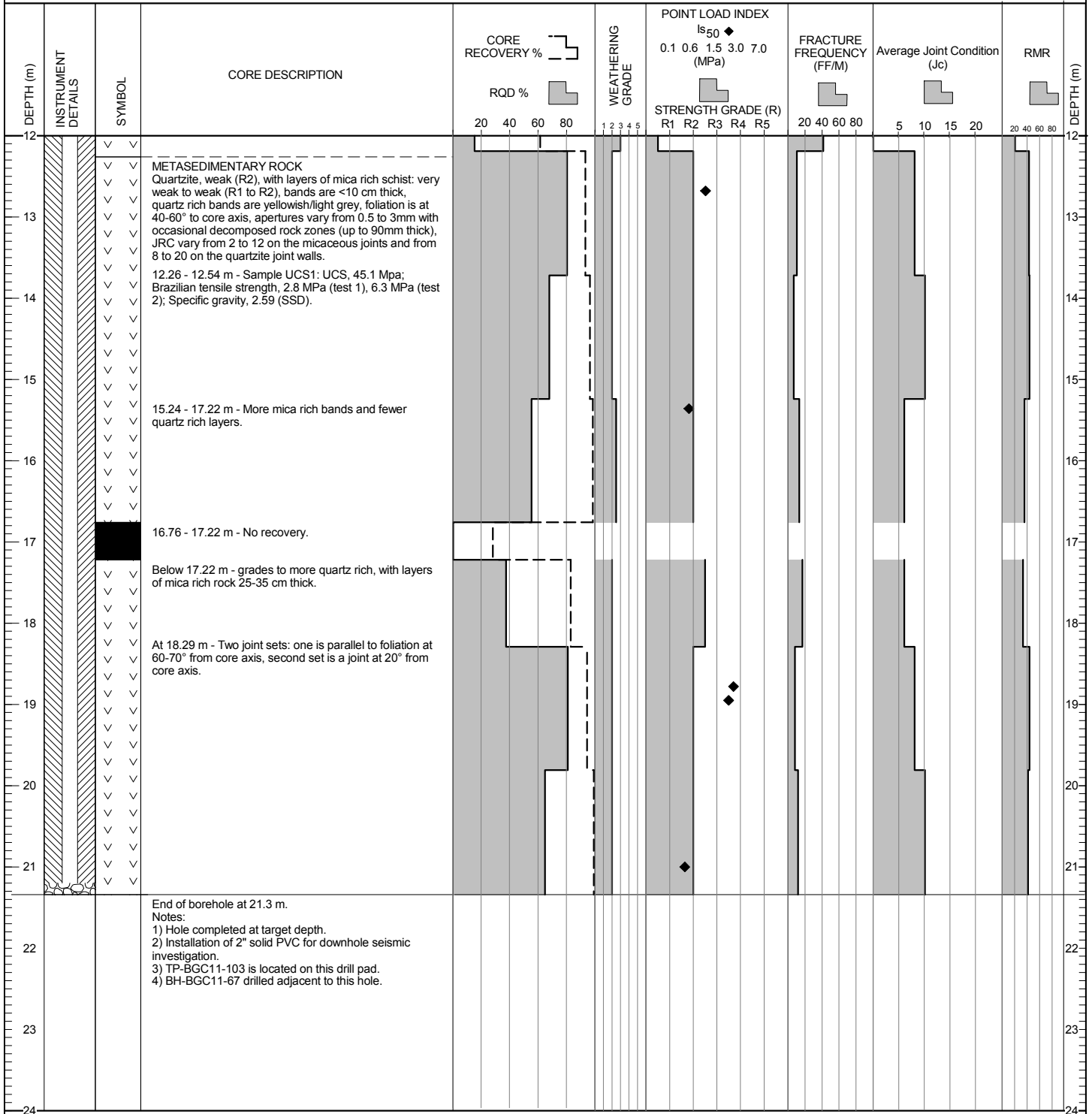
DRILL HOLE # BH-BGC11-69

LOCATION : WEST FLANK TIN DOME

CO-ORDINATES (m) 458,829.3E - 7,101,144.3N
 GROUND ELEVATION (m) : 861.6m
 SURVEY METHOD : DGPS
 DATUM : UTM NAD 83
 PLUNGE (°) : -90
 TREND (°) : n/a

DRILL DESIGNATION : CS-1000
 DRILLING CONTRACTOR : Lyncorp
 DRILL METHOD : Rotary Diamond
 CORE SIZE : HQ3
 FLUID : Water/Polymer
 CASSED TO (m): 3.05

START DATE : 17 Aug 11
 FINISH DATE : 17 Aug 11
 FINAL DEPTH (m) : 21.3
 DEPTH TO TOP OF ROCK (m) : 9.1
 LOGGED BY : JD/SP/DW
 REVIEWED BY : PQ/DW



EGP (ROCK) EGP_ROCK.GDL BGC.GDT 1/20/12

APPENDIX F BOREHOLE PHOTOS



BH-BGC11-24 Box 1 – 0.00 m to 5.41 m



BH-BGC11-24 Box 2 – 5.41 m to 9.30 m



BH-BGC11-24 Box 3 – 9.30 m to 13.30 m



BH-BGC11-24 Box 4 – 13.30 m to 18.99 m



BH-BGC11-24 Box 5 – 18.99 m to 20.88 m



BH-BGC11-25 Box 1 – 0.00 m to 4.91 m



BH-BGC11-25 Box 2 – 4.91 m to 9.10 m



BH-BGC11-25 Box 3 – 9.10 m to 13.11 m



BH-BGC11-25 Box 4 – 13.11 m to 17.98 m



BH-BGC11-25 Box 5 – 17.98 m to 20.42 m



BH-BGC11-26 Box 1 – 0.00 m to 10.21 m



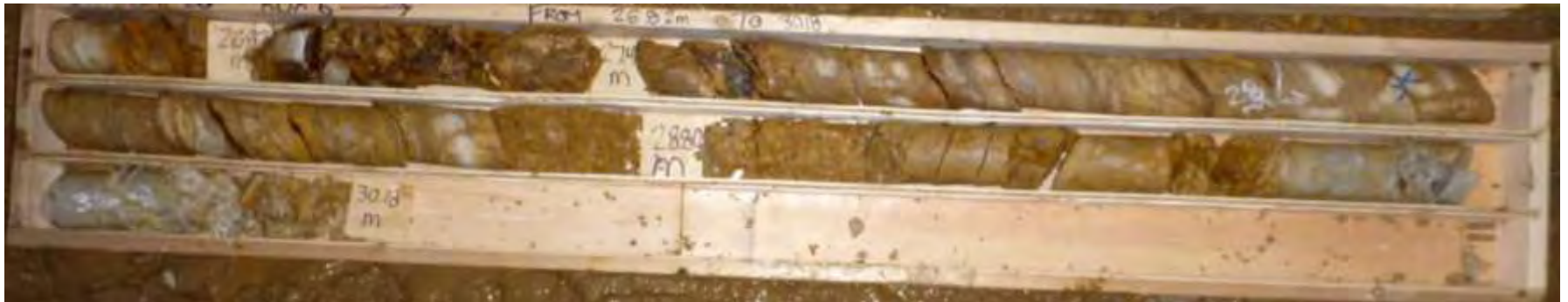
BH-BGC11-26 Box 2 – 10.21 m to 15.38 m



BH-BGC11-26 Box 3 – 15.38 m to 21.34 m



BH-BGC11-26 Box 4 – 21.34 m to 26.82 m



BH-BGC11-26 Box 5 – 26.82 m to 30.18 m



BH-BGC11-27 Box 1 – 0.00 m to 14.02 m



BH-BGC11-27 Box 2 – 14.02 m to 18.29 m



BH-BGC11-27 Box 3 – 18.29 m to 22.30 m



BH-BGC11-27 Box 4 – 22.30 m to 26.52 m



BH-BGC11-28 Box 1 – 0.00 m to 12.19 m



BH-BGC11-28 Box 2 – 12.19 m to 18.14 m



BH-BGC11-28 Box 3 – 18.14 m to 22.10 m



BH-BGC11-28 Box 4 – 22.10 m to 26.21 m



BH-BGC11-28 Box 5 – 26.21 m to 32.00 m



BH-BGC11-28 Box 6 – 32.00 m to 40.84 m



BH-BGC11-29 Box 1 – 0.00 m to 15.54 m



BH-BGC11-29 Box 2 – 15.54 m to 22.83 m



BH-BGC11-29 Box 3 – 22.83 m to 26.72 m



BH-BGC11-29 Box 4 – 26.72 m to 30.69 m



BH-BGC11-30 Box 1 – 0.00 m to 18.29 m



BH-BGC11-30 Box 2 – 18.29 m to 30.85 m



BH-BGC11-30 Box 3 – 30.85 m to 35.05 m



BH-BGC11-31 Box 1 – 0.00 m to 11.94 m



BH-BGC11-31 Box 2 – 11.94 m to 22.86 m



BH-BGC11-31 Box 3 – 22.86 m to 35.05 m



BH-BGC11-32 Box 1 – 0.00 m to 19.95 m



BH-BGC11-32 Box 2 – 19.95 m to 24.38 m



BH-BGC11-33 Box 1 – 0.00 m to 16.46 m



BH-BGC11-33 Box 2 – 16.46 m to 27.43 m



BH-BGC11-33 Box 3 – 27.43 m to 39.01 m



BH-BGC11-34 Box 1 – 0.00 m to 21.34 m



BH-BGC11-34 Box 2 – 21.34 m to 27.62 m



BH-BGC11-34 Box 3 – 27.62 m to 32.10 m



BH-BGC11-34 Box 4 – 32.10 m to 36.86 m



BH-BGC11-34 Box 5 – 36.86 m to 38.10 m



BH-BGC11-35 Box 1 – 0.00 m to 19.81 m



BH-BGC11-35 Box 2 – 19.81 m to 30.20 m



BH-BGC11-35 Box 3 – 30.20 m to 34.70 m



BH-BGC11-35 Box 4 – 34.70 m to 45.72 m



BH-BGC11-35 Box 5 – 45.72 m to 50.29 m



BH-BGC11-36 Box 1 – 0.00 m to 6.44 m



BH-BGC11-36 Box 2 – 6.44 m to 12.19 m



BH-BGC11-36 Box 3 – 12.19 m to 16.10 m



BH-BGC11-36 Box 4 – 16.10 m to 22.55 m



BH-BGC11-36 Box 5 – 22.55 m to 28.54 m



BH-BGC11-36 Box 6 – 28.54 m to 34.14 m



BH-BGC11-36 Box 7 – 34.14 m to 38.36 m



BH-BGC11-36 Box 8 – 38.36 m to 42.52 m



BH-BGC11-36 Box 9 – 42.52 m to 46.47 m



BH-BGC11-36 Box 10 – 46.47 m to 49.83 m



BH-BGC11-36 Box 11 – 49.83 m to 50.29 m



BH-BGC11-37 Box 1 – 0.00 m to 7.16 m



BH-BGC11-37 Box 2 – 7.16 m to 11.11 m



BH-BGC11-37 Box 3 – 11.11 m to 16.08 m



BH-BGC11-37 Box 4 – 16.08 m to 22.00 m



BH-BGC11-37 Box 5 – 22.00 m to 26.53 m



BH-BGC11-37 Box 6 – 26.53 m to 31.42 m



BH-BGC11-37 Box 7 – 31.42 m to 36.90 m



BH-BGC11-37 Box 8 – 36.90 m to 42.37 m



BH-BGC11-37 Box 9 – 42.37 m to 43.59 m



BH-BGC11-38 Box 1 – 0.00 m to 16.15 m



BH-BGC11-38 Box 2 – 16.15 m to 21.97 m



BH-BGC11-38 Box 3 – 21.97 m to 26.50 m



BH-BGC11-38 Box 4 – 26.50 m to 31.14 m



BH-BGC11-38 Box 5 – 31.14 m to 35.80 m



BH-BGC11-38 Box 6 – 35.80 m to 40.83 m



BH-BGC11-38 Box 7 – 40.83 m to 50.00 m



BH-BGC11-38 Box 8 – 50.00 m to 50.52 m



BH-BGC11-39 SPT 1 – 1.15 m to 1.60 m



BH-BGC11-39 SPT 2 – 1.60 m to 2.05 m






BH-BGC11-39 SPT 3 – 2.28 m to 2.73 m



BH-BGC11-39 SPT 4 – 2.75 m to 3.20 m

	
<p>BH-BGC11-39 SPT 5 – 4.60 m to 5.05 m</p>	<p>BH-BGC11-39 SPT 6 – 5.53 m to 5.98 m</p>
	
<p>BH-BGC11-39 SPT 7 – 6.22 m to 6.67 m</p>	<p>BH-BGC11-39 SPT 8 – 7.16 m to 7.61 m</p>

	
<p>BH-BGC11-39 SPT 9 – 7.75 m to 8.20 m</p>	<p>BH-BGC11-39 SPT 10 – 8.71 m to 9.16 m</p>
	
<p>BH-BGC11-39 SPT 11 – 9.25 m to 9.70 m</p>	



BH-BGC11-40A Box 1 – 0.00 m to 9.48 m



BH-BGC11-40A Box 2 – 9.48 m to 13.53 m



BH-BGC11-40A Box 3 – 13.53 m to 17.56 m



BH-BGC11-40A Box 4 – 17.56 m to 21.90 m



BH-BGC11-40A Box 5 – 21.90 m to 25.82 m



BH-BGC11-40A Box 6 – 25.82 m to 29.88 m



BH-BGC11-40A Box 7 – 29.88 m to 33.19 m



BH-BGC11-40B Box 1 – 8.69 m to 13.72 m



BH-BGC11-40B Box 2 – 13.72 m to 18.29 m



BH-BGC11-40B Box 3 – 18.29 m to 22.56 m



BH-BGC11-40B Box 4 – 22.56 m to 26.77 m



BH-BGC11-40B Box 5 – 26.77 m to 30.80 m



BH-BGC11-40B Box 6 – 30.80 m to 34.73 m



BH-BGC11-40B Box 7 – 34.73 m to 38.70 m



BH-BGC11-40B Box 8 – 38.70 m to 42.67 m



BH-BGC11-40B Box 9 – 42.67 m to 45.72 m






BH-BGC11-41 Run 1 – 0.00 m to 1.53 m

				
BH-BGC11-42	Run 1 – 0.0 m to 0.75 m		BH-BGC11-42	Run 2 – 0.75 m to 2.27 m
				
BH-BGC11-42	Run 3 – 2.27 m to 3.80 m		BH-BGC11-42	Run 4 – 3.80 m to 5.32 m

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BH-BGC11-42	Run 5 – 5.32 m to 6.84 m		BH-BGC11-42	Run 6 – 6.84 m to 8.36 m	
					
BH-BGC11-42	Run 7 – 8.36 m to 9.88 m		BH-BGC11-42	Run 8 – 9.88 m to 11.43 m	

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BH-BGC11-42 Run 9 – 11.43 m to 11.75 m	BH-BGC11-42 Run 10 – 11.75 m to 12.50 m
	
BH-BGC11-42 Run 11 – 12.50 m to 13.11 m	BH-BGC11-42 Run 12 – 13.11 m to 13.71 m



BH-BGC11-42 Run 13 – 13.71 m to 14.16 m





BH-BGC11-42 Run 14 – 14.16 m to 14.73 m



BH-BGC11-42 Run 15 – 14.73 m to 15.40 m



BH-BGC11-42 Run 16 – 15.40 m to 15.85 m

	
<p>BH-BGC11-42 Run 17 – 15.85 m to 17.53 m</p>	<p>BH-BGC11-42 Run 18 – 17.53 m to 18.0 m</p>
	
<p>BH-BGC11-42 Run 19 – 18.0 m to 18.30 m</p>	<p>BH-BGC11-42 Run 20 – 18.30 m to 19.05 m</p>

					
BH-BGC11-42	Run 21 – 19.05 m to 20.57 m		BH-BGC11-42	Run 21 – 20.57 m to 22.04 m	
					
BH-BGC11-42	Run 23 – 22.04 m to 23.62 m		BH-BGC11-42	Run 24 – 23.62 m to 25.15 m	

	
BH-BGC11-42 Run 25 – 25.15 m to 26.67 m	BH-BGC11-42 Run 26 – 26.67 m to 28.19 m



BH-BGC11-43 Box 1 – 0.00 m to 4.48 m



BH-BGC11-43 Box 2 – 4.48 m to 8.96 m



BH-BGC11-43 Box 3 – 8.96 m to 13.54 m




BH-BGC11-43 Box 4 – 13.54 m to 18.15 m



BH-BGC11-43 Box 5 – 18.15 m to 22.37 m



BH-BGC11-43 Box 6 – 22.37 m to 23.77 m

	
<p>BH-BGC11-44 Run 1 – 0.0 m to 0.75 m</p>	<p>BH-BGC11-44 Run 2 – 0.75 m to 2.27 m</p>
	
<p>BH-BGC11-44 Run 3 – 2.72 m to 3.79 m</p>	<p>BH-BGC11-44 Run 4 – 4.24 m to 5.33 m</p>

	
<p>BH-BGC11-44 Run 5 – 5.80 m to 6.85 m</p>	<p>BH-BGC11-44 Run 6 – 7.32 m to 8.38 m</p>
	
<p>BH-BGC11-44 Run 7 – 8.85 m to 9.38 m</p>	<p>BH-BGC11-44 Run 8 – 9.38 m to 9.90 m</p>

	
<p>BH-BGC11-44 Run 9 – 10.35 m to 11.42 m</p>	<p>BH-BGC11-44 Run 10 – 11.97 m to 12.35 m</p>
	
<p>BH-BGC11-44 Run 11 – 13.42 m to 14.49 m</p>	<p>BH-BGC11-44 SPT 1 – 2.27 m to 2.72 m</p>



BH-BGC11-44 SPT 2 – 3.79 m to 4.24 m






BH-BGC11-44 SPT 3 – 5.33 m to 5.80 m



BH-BGC11-44 SPT 5 – 8.38 m to 8.85 m



BH-BGC11-44 SPT 8 – 9.90 m to 10.35 m

	
<p>BH-BGC11-44 SPT 9 – 11.42 m to 11.97 m</p>	<p>BH-BGC11-44 SPT 10 – 12.97 m to 13.42 m</p>
	
<p>BH-BGC11-44 SPT 11 – 14.49 m to 14.79 m</p>	



BH-BGC11-45 Box 1 – 3.05 m to 7.93 m



BH-BGC11-45 Box 2 – 7.93 m to 12.22 m



BH-BGC11-45 Box 3 – 12.22 m to 16.76 m



BH-BGC11-45 Box 4 – 16.76 m to 20.73 m



BH-BGC11-46 Box 1 – 0.00 m to 6.21 m



BH-BGC11-46 Box 2 – 6.21 m to 10.36 m



BH-BGC11-46 Box 3 – 10.36 m to 14.40 m



BH-BGC11-46 Box 4 – 14.40 m to 18.31 m



BH-BGC11-46 Box 5 – 18.31 m to 20.12 m

	
<p>BH-BGC11-47 Run 1 – 0.00 m to 0.75 m</p>	<p>BH-BGC11-47 Box 1 – 1.2 m to 2.27 m</p>
	
<p>BH-BGC11-47 Run 2 – 2.30 m to 2.95 m</p>	<p>BH-BGC11-47 Run 4 – 2.95 m to 3.35 m</p>

	
<p>BH-BGC11-47 Run 5 – 4.25 m to 5.33 m</p>	<p>BH-BGC11-47 Run 6 – 5.78 m to 6.85 m</p>
	
<p>BH-BGC11-47 Run 7 – 7.30 m to 8.38 m</p>	<p>BH-BGC11-47 Run 9 – 8.58 m to 9.90 m</p>

					
BH-BGC11-47	Run 10 – 10.05 m to 11.43 m		BH-BGC11-47	Run 11 – 11.60 m to 12.95 m	
					
BH-BGC11-47	Run 12 – 12.95 m to 14.48 m		BH-BGC11-47	Run 13 – 14.48 m to 16.0 m	

	
BH-BGC11-47 SPT 1 – 0.75 m to 1.20 m	BH-BGC11-47 SPT 2 – 3.35 m to 3.80 m
	
BH-BGC11-47 SPT 3 – 3.80 m to 4.25 m	BH-BGC11-47 SPT 4 – 5.33 m to 5.88 m

 <p>BH-BGC11-47 SPT #5 6.85 to 7.30 m 4/8/11 LGT 0792006 - E&P.</p>	 <p>BH-BGC11-47 SPT #6 8.38 → 8.83 m. 4/8/11 LGT 0792006 - E&P.</p>
<p>BH-BGC11-47 SPT 5 – 6.85 m to 7.30 m</p>	<p>BH-BGC11-47 SPT 6 – 8.38 m to 8.83 m</p>
 <p>BH BGC11-47 SPT #7 9.90 → 10.05 4/8/11 LGT 0792006 - E&P.</p>	 <p>BH-BGC11-47 SPT #8 From 11.43 m to 11.70 m 4/8/11 LGT/ARV. 0792006 - E&P.</p>
<p>BH-BGC11-47 SPT 7 – 9.90 m to 10.05 m</p>	<p>BH-BGC11-47 SPT 8 – 11.43 m to 11.70 m</p>



BH-BGC11-48 Box 1 – 0.00 m to 13.11 m



BH-BGC11-48 Box 2 – 13.11 m to 18.96 m



BH-BGC11-48 Box 3 – 18.96 m to 23.38 m



BH-BGC11-48 Box 4 – 23.98 m to 28.00 m



BH-BGC11-48 Box 5 – 28.00 m to 32.92 m



BH-BGC11-49 Run 1 – 0.00 m to 0.76 m



BH-BGC11-49 Run 2 – 1.23 m to 2.28 m



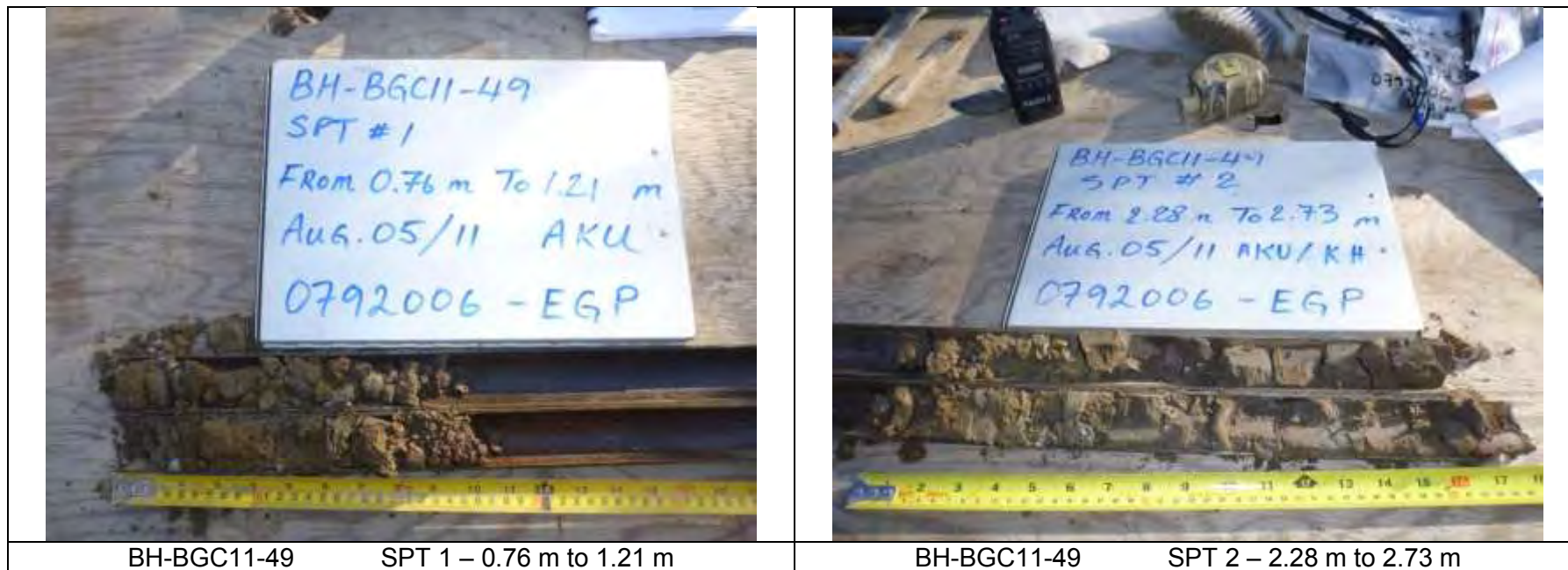
BH-BGC11-49 Run 3 – 2.73 m to 2.90 m



BH-BGC11-49 Run 4 – 2.90 m to 3.80 m

	
<p>BH-BGC11-49 Run 5 – 3.80 m to 5.33 m</p>	<p>BH-BGC11-49 Run 6 – 5.33 m to 6.85 m</p>
	
<p>BH-BGC11-49 Run 7 – 6.85 m to 8.38 m</p>	<p>BH-BGC11-49 Run 8 – 8.38 m to 9.90 m</p>

			
BH-BGC11-49	Run 9 – 9.90 m to 11.43 m	BH-BGC11-49	Run 10 – 11.43 m to 12.95 m
			
BH-BGC11-49	Run 11 – 12.95 m to 14.47 m	BH-BGC11-49	Run 12 – 14.47 m to 16.0 m





BH-BGC11-50 Box 1 – 3.66 m to 11.53 m



BH-BGC11-50 Box 2 – 11.53 m to 16.50 m



BH-BGC11-50 Box 3 – 16.50 m to 20.67 m



BH-BGC11-50 Box 4 – 20.67 m to 24.93 m



BH-BGC11-50 Box 5 – 24.93 m to 29.11 m



BH-BGC11-50 Box 6 – 29.11 m to 33.28 m



BH-BGC11-50 Box 7 – 33.28 m to 37.41 m



BH-BGC11-50 Box 8 – 37.41 m to 41.15 m

			
BH-BGC11-51	Run 1 – 0.00 m to 0.76 m	BH-BGC11-51	Run 2 – 1.21 m to 2.28 m
			
BH-BGC11-51	Run 3 – 2.73 m to 3.80 m	BH-BGC11-51	Run 4 – 4.25 m to 5.33 m

	
<p>BH-BGC11-51 Run 5 – 5.78 m to 6.85 m</p>	<p>BH-BGC11-51 Run 6 – 6.85 m to 8.38 m</p>
	
<p>BH-BGC11-51 Run 7 – 8.38 m to 9.90 m</p>	<p>BH-BGC11-51 Run 8 – 10.25 m to 11.43 m</p>

 A worker in a red jumpsuit and blue hard hat stands next to a yellow drilling rig. A vertical yellow measuring tape is positioned against the rig. A white label is on the ground in front of the tape.		 A worker in a red jumpsuit stands next to a yellow drilling rig. A vertical yellow measuring tape is positioned against the rig. A white label is on the ground in front of the tape.	
BH-BGC11-51	Run 9 – 11.58 m to 12.95 m	BH-BGC11-51	Run 10 – 12.95 m to 14.47 m
 A worker in a red jumpsuit stands next to a yellow drilling rig. A vertical yellow measuring tape is positioned against the rig. A white label is on the ground in front of the tape.		 A worker in a red jumpsuit stands next to a yellow drilling rig. A vertical yellow measuring tape is positioned against the rig. A white label is on the ground in front of the tape.	
BH-BGC11-51	Run 11 – 14.47 m to 16.00 m	BH-BGC11-51	Run 12 – 16.00 m to 17.52 m

			
BH-BGC11-51	Run 13 – 17.52 m to 19.05 m	BH-BGC11-51	Run 14 – 19.05 m to 20.57 m
			
BH-BGC11-51	Run 15 – 20.62 m to 22.10 m	BH-BGC11-51	Run 16 – 22.25 m to 23.62 m

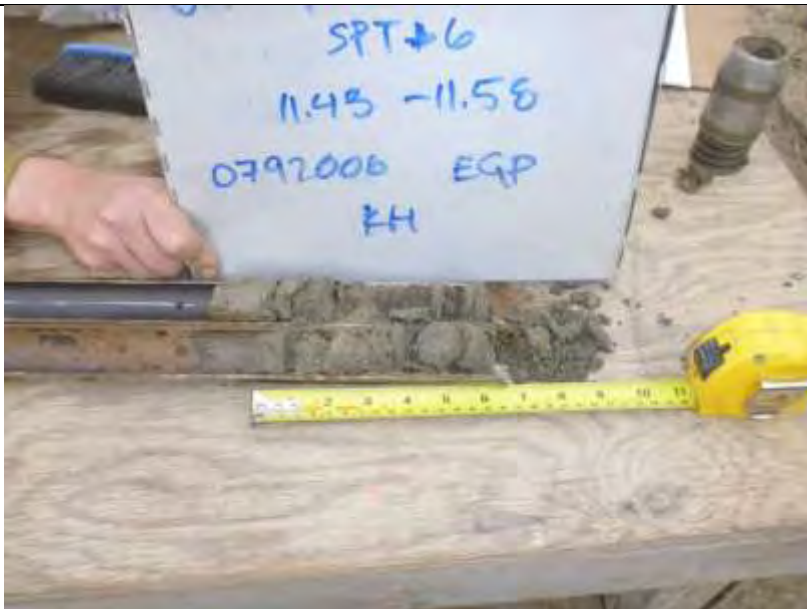
	
<p>BH-BGC11-51 Run 17 – 23.62 m to 25.15 m</p>	<p>BH-BGC11-51 SPT 1 – 0.76 m to 1.21 m</p>
	
<p>BH-BGC11-51 SPT 2 – 2.28 m to 2.73 m</p>	<p>BH-BGC11-51 SPT 3 – 3.80 m to 4.25 m</p>



BH-BGC11-51 SPT 4 – 5.33 m to 5.78 m



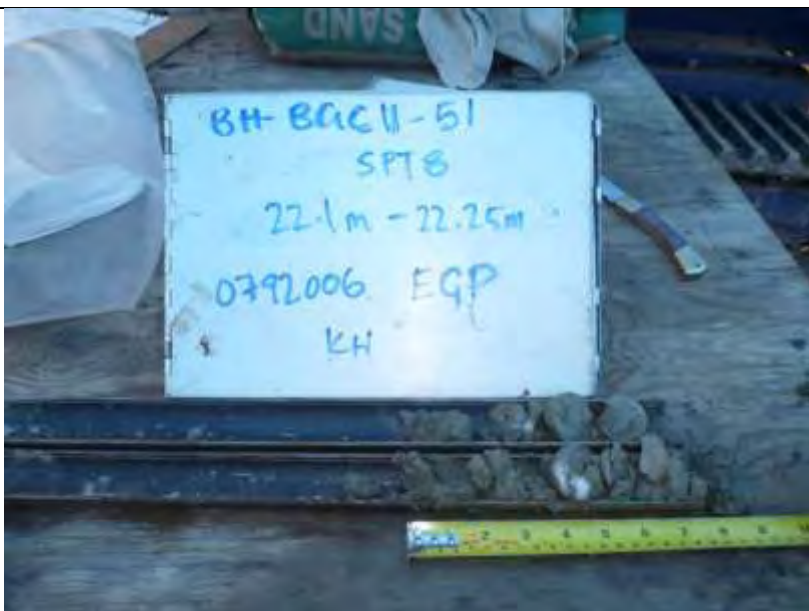
BH-BGC11-51 SPT 5 – 9.90 m to 10.25 m



BH-BGC11-51 SPT 6 – 11.43 m to 11.58 m



BH-BGC11-51 SPT 6 – 20.57 m to 20.62 m



BH-BGC11-51 SPT 8 – 22.1 m to 22.25 m



BH-BGC11-52 Box 1 – 1.22 m to 11.09 m



BH-BGC11-52 Box 2 – 11.09 m to 16.85 m



BH-BGC11-52 Box 3 – 16.85 m to 21.03 m



BH-BGC11-52 Box 4 – 21.03 m to 22.56 m

 A photograph showing a yellow drilling rig on the left and a person's legs in grey pants on the right. A yellow measuring tape is held vertically against the borehole. A white sign on the ground contains handwritten text: "BH-BGC11-53", "Run 1", "0.00 m to 0.76 m".		 A photograph of a person in a blue safety vest and grey pants standing next to a yellow drilling rig. A yellow measuring tape is held vertically against the borehole. A white sign on the ground contains handwritten text: "BH-BGC11-53", "Run 2", "1.21 m to 2.28 m".	
BH-BGC11-53	Run 1 – 0.00 m to 0.76 m	BH-BGC11-53	Run 2 – 1.21 m to 2.28 m
 A photograph of a person in a blue safety vest and grey pants standing next to a yellow drilling rig. A yellow measuring tape is held vertically against the borehole. A white sign on the ground contains handwritten text: "BH-BGC11-53", "Run 3", "2.73 m to 3.80 m".		 A photograph of a person in a blue safety vest and grey pants standing next to a yellow drilling rig. A yellow measuring tape is held vertically against the borehole. A white sign on the ground contains handwritten text: "BH-BGC11-53", "Run 4", "4.25 m to 5.33 m".	
BH-BGC11-53	Run 3 – 2.73 m to 3.80 m	BH-BGC11-53	Run 4 – 4.25 m to 5.33 m

			
BH-BGC11-53	Run 5 – 5.33 m to 6.85 m	BH-BGC11-53	Run 6 – 7.30 m to 8.37 m
			
BH-BGC11-53	Run 7 – 8.37 m to 9.90 m	BH-BGC11-53	Run 8 – 10.35 m to 11.42 m

	
<p>BH-BGC11-53 Run 9 – 11.42 m to 12.95 m</p>	<p>BH-BGC11-53 Run 10 – 12.95 m to 14.47 m</p>
	
<p>BH-BGC11-53 SPT 1 – 0.76 m to 1.21 m</p>	<p>BH-BGC11-53 SPT 2 – 2.28 m to 2.73 m</p>

	
<p>BH-BGC11-53 SPT 3 – 3.80 m to 4.25 m</p>	<p>BH-BGC11-53 SPT 4 – 6.85 m to 7.3 m</p>
	
<p>BH-BGC11-53 SPT 5 – 9.90 m to 10.35 m</p>	



BH-BGC11-54 Box 1 – 0.00 m to 18.29 m



BH-BGC11-54 Box 2 – 18.29 m to 24.60 m



BH-BGC11-54 Box 3 – 24.60 m to 32.94 m



BH-BGC11-54 Box 4 – 32.94 m to 39.71 m



BH-BGC11-54 Box 5 – 39.71 m to 41.15 m

			
BH-BGC11-55	Run 1 – 0.00 m to 0.76 m	BH-BGC11-55	Run 2 – 1.21 m to 2.27 m
			
BH-BGC11-55	Run 3 – 2.73 m to 3.80 m	BH-BGC11-55	Run 4 – 4.25 m to 5.33 m

			
BH-BGC11-55	Run 5 – 5.78 m to 6.85 m	BH-BGC11-55	Run 6 – 7.30 m to 8.38 m
			
BH-BGC11-55	Run 7 – 8.83 m to 9.90 m	BH-BGC11-55	Run 8 – 10.35 m to 11.42 m

	
<p>BH-BGC11-55 Run 9 – 11.42 m to 12.95 m</p>	<p>BH-BGC11-55 Box 1 – 12.95 m to 14.47 m</p>
	
<p>BH-BGC11-55 SPT 1 – 0.76 m to 1.21 m</p>	<p>BH-BGC11-55 SPT 2 – 2.28 m to 2.73 m</p>





	
<p>BH-BGC11-55 SPT 3 – 3.80 m to 4.25 m</p>	<p>BH-BGC11-55 SPT 4 – 5.33 m to 5.78 m</p>
	
<p>BH-BGC11-55 SPT 5 – 6.85 m to 7.30 m</p>	<p>BH-BGC11-55 SPT 6 – 8.38 m to 8.83 m</p>

 A photograph showing a soil sample in a tray. A white label with handwritten text is placed on top of the tray. The text on the label reads: "0792-006", "BH-BGC11-55", "SPT 7", "9.90m-10.35m", and "KH". A yellow measuring tape is visible at the bottom of the tray, showing a scale from approximately 10 to 15 centimeters. The soil sample is a light brown, sandy material.	
BH-BGC11-55	SPT 7 – 9.90 m to 10.35 m

			
BH-BGC11-56	Run 1 – 0.00 m to 0.76 m	BH-BGC11-56	Run 2 – 1.21 m to 2.28 m
			
BH-BGC11-56	Run 3 – 2.43 m to 3.80 m	BH-BGC11-56	Run 4 – 4.25 m to 5.33 m

			
BH-BGC11-56	Run 5 – 5.53 m to 6.85 m	BH-BGC11-56	Run 6 – 7.15 m to 8.38 m
			
BH-BGC11-56	Run 7 – 8.53 m to 9.90 m	BH-BGC11-56	Run 8 – 9.90 m to 11.42 m

	
<p>BH-BGC11-56 Run 9 – 11.42 m to 12.95 m</p>	<p>BH-BGC11-56 Run 10 – 12.95 m to 13.70 m</p>
	
<p>BH-BGC11-56 SPT 1 – 0.76 m to 1.21 m</p>	<p>BH-BGC11-56 SPT 2 – 2.28 m to 2.43 m</p>

	
BH-BGC11-56 SPT 3 – 3.80 m to 4.25 m	BH-BGC11-56 SPT 4 – 5.33 m to 5.53 m
	
BH-BGC11-56 SPT 5 – 6.85 m to 7.15 m	BH-BGC11-56 SPT 6 – 8.38 m to 8.53 m

	
<p>BH-BGC11-57 Run 1 – 0.00 m to 0.76 m</p>	<p>BH-BGC11-57 Run 2 – 0.76 m to 2.28 m</p>
	
<p>BH-BGC11-57 Run 3 – 2.65 m to 3.90 m</p>	<p>BH-BGC11-57 Run 4 – 4.25 m to 5.33 m</p>

			
BH-BGC11-57	Run 5 – 5.78 m to 6.85 m	BH-BGC11-57	Run 6 – 7.10 m to 8.43 m
			
BH-BGC11-57	Run 7 – 8.53 m to 9.90 m	BH-BGC11-57	Run 8 – 10.20 m to 11.47 m



BH-BGC11-57 Run 9 – 11.47 m to 12.10 m



BH-BGC11-57 SPT 1 – 2.28 m to 2.65 m



BH-BGC11-57 SPT 2 – 3.80 m to 4.25 m



BH-BGC11-57 SPT 3 – 5.33 m to 5.78 m

	
BH-BGC11-57 SPT 4 – 6.85 m to 7.10 m	BH-BGC11-57 SPT 5 – 8.43 m to 8.53 m
	
BH-BGC11-57 SPT 6 – 9.90 m to 10.20 m	

	
<p>BH-BGC11-58 Run 1 – 0.00 m to 0.76 m</p>	<p>BH-BGC11-58 Run 2 – 0.76 m to 2.28 m</p>
	
<p>BH-BGC11-58 Run 3 – 2.28 m to 2.90 m</p>	<p>BH-BGC11-58 Run 4 – 2.90 m to 3.40 m</p>



BH-BGC11-58 Run 5 – 3.40 m to 4.00 m



BH-BGC11-58 Run 6 – 4.00 m to 4.60 m



BH-BGC11-58 Run 7 – 4.60 m to 4.90 m



BH-BGC11-58 Run 8 – 4.90 m to 5.50 m

					
BH-BGC11-58	Run 9 – 5.50 m to 6.85 m		BH-BGC11-58	Run 10 – 6.90 m to 8.43 m	
					
BH-BGC11-58	Run 11 – 8.43 m to 9.90 m		BH-BGC11-58	Run 12 – 9.90 m to 10.70 m	



BH-BGC11-58 SPT 2 – 6.85 m to 6.90 m



BH-BGC11-59 Box 1 – 0.00 m to 7.58 m



BH-BGC11-59 Box 2 – 7.58 m to 11.58 m



BH-BGC11-59 Box 3 – 11.58 m to 15.70 m



BH-BGC11-59 Box 4 – 15.70 m to 19.56 m



BH-BGC11-59 Box 5 – 19.56 m to 23.95 m



BH-BGC11-59 Box 6 – 23.95 m to 30.18 m

	
<p>BH-BGC11-60 Run 1 – 0.00 m to 0.76 m</p>	<p>BH-BGC11-60 Run 2 – 0.76 m to 2.28 m</p>
	
<p>BH-BGC11-60 Run 3 – 2.28 m to 2.80 m</p>	<p>BH-BGC11-60 Run 4 – 2.80 m to 3.40 m</p>

	
<p>BH-BGC11-60 Run 5 – 3.40 m to 4.00 m</p>	<p>BH-BGC11-60 Run 6 – 4.00 m to 4.60 m</p>
	
<p>BH-BGC11-60 Run 7 – 4.75 m to 5.33 m</p>	<p>BH-BGC11-60 Run 8 – 5.33 m to 6.85 m</p>

	
<p>BH-BGC11-60 Run 9 – 7.00 m to 8.38 m</p>	<p>BH-BGC11-60 Run 10 – 8.38 m to 9.15 m</p>
	
<p>BH-BGC11-60 SPT 1 – 4.60 m to 4.75 m</p>	<p>BH-BGC11-60 SPT 2 – 6.85 m to 7.00 m</p>

		
BH-BGC11-61 Run 1 – 0.00 m to 0.76m	BH-BGC11-61 Run 2 – 0.76m to 2.28 m	
		
BH-BGC11-61 Run 3 – 2.28 m to 2.70 m		



BH-BGC11-62 Box 1 – 0.00 m to 5.47 m



BH-BGC11-62 Box 2 – 5.47 m to 10.32 m



BH-BGC11-62 Box 3 – 10.32 m to 14.78 m



BH-BGC11-62 Box 4 – 14.78 m to 19.33 m



BH-BGC11-62 Box 5 – 19.33 m to 24.59 m



BH-BGC11-62 Box 6 – 24.59 m to 28.84 m



BH-BGC11-62 Box 7 – 28.84 m to 33.42 m



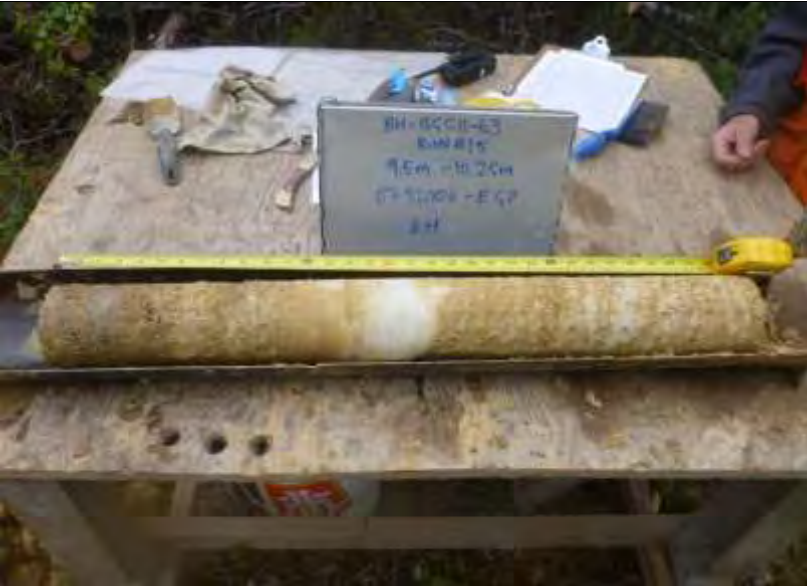



BH-BGC11-62 Box 8 – 33.42 m to 35.05 m

	
<p>BH-BGC11-63 Run 1 – 0.00 m to 0.76 m</p>	<p>BH-BGC11-63 Run 2 – 0.76 m to 2.28 m</p>
	
<p>BH-BGC11-63 Run 3 – 2.28 m to 3.00 m</p>	<p>BH-BGC11-63 Run 4 – 3.00 m to 3.50 m</p>

	
<p>BH-BGC11-63 Run 5 – 3.50 m to 4.10 m</p>	<p>BH-BGC11-63 Run 6 – 4.10 m to 4.70 m</p>
	
<p>BH-BGC11-63 Run 7 – 4.70 m to 5.30 m</p>	<p>BH-BGC11-63 Run 8 – 5.30 m to 5.90 m</p>

	
BH-BGC11-63 Run 9 – 5.90 m to 6.50 m	BH-BGC11-63 Run 10 – 6.50 m to 7.10 m
	
BH-BGC11-63 Run 11 – 7.10 m to 7.70 m	BH-BGC11-63 Run 12 – 7.70 m to 8.30 m

	
<p>BH-BGC11-63 Run 13 – 8.30 m to 8.90 m</p>	<p>BH-BGC11-63 Run 14 – 8.90 m to 9.50 m</p>
	
<p>BH-BGC11-63 Run 15 – 9.50 m to 10.25 m</p>	<p>BH-BGC11-63 Run 16 – 10.25 m to 11.00 m</p>



BH-BGC11-63 Run 17 – 11.00 m to 11.75 m



BH-BGC11-63 Run 19 – 11.85 m to 12.60 m



BH-BGC11-63 Run 20 – 12.60 m to 13.30 m



BH-BGC11-63 Run 21 – 13.30 m to 14.10 m

	
<p>BH-BGC11-63 Run 23 – 14.48 m to 16.00 m</p>	<p>BH-BGC11-63 Run 24 – 16.00 m to 16.70 m</p>
	
<p>BH-BGC11-63 Run 25 – 16.70 m to 17.45 m</p>	<p>BH-BGC11-63 Run 26 – 17.45 m to 17.85 m</p>



BH-BGC11-63 Run 27 – 17.85 m to 18.45 m



BH-BGC11-63 Run 28 – 18.45 m to 19.00 m



BH-BGC11-63 Run 29 – 19.00 m to 19.75 m



BH-BGC11-63 Run 30 – 19.75 m to 20.57 m

			
BH-BGC11-63	Run 31 – 20.57 m to 22.10 m	BH-BGC11-63	Run 32 – 22.10 m to 23.62 m
			
BH-BGC11-63	Run 33 – 23.62 m to 25.15 m	BH-BGC11-63	Run 34 – 25.15 m to 26.67 m



BH-BGC11-64 Box 1 – 0.00 m to 6.40 m



BH-BGC11-64 Box 2 – 6.40 m to 16.46 m



BH-BGC11-64 Box 3 – 16.46 m to 20.97 m



BH-BGC11-64 Box 4 – 20.97 m to 24.91 m



BH-BGC11-64 Box 5 – 24.91 m to 28.65 m



BH-BGC11-64 Box 6 – 28.65 m to 32.99 m



BH-BGC11-64 Box 7 – 32.99 m to 36.75 m



BH-BGC11-64 Box 8 – 36.75 m to 40.84 m



BH-BGC11-64 Box 9 – 40.84 m to 44.74 m



BH-BGC11-64 Box 10 – 44.74 m to 48.77 m

	
<p>BH-BGC11-65 Run 1 – 0.00 m to 0.76 m</p>	<p>BH-BGC11-65 SPT 1 – 0.76 m to 1.21 m</p>
	
<p>BH-BGC11-65 SPT 2 – 1.52 m to 1.97 m</p>	<p>BH-BGC11-65 SPT 3 – 2.28 m to 2.73 m</p>



BH-BGC11-65 SPT 4 – 3.04 m to 3.49 m



BH-BGC11-65 SPT 5 – 3.80 m to 4.25 m



BH-BGC11-65 SPT 6 – 4.56 m to 5.01 m



BH-BGC11-65 SPT 7 – 5.33 m to 5.78 m





BH-BGC11-66 Box 1 – 0.00 m to 5.04 m



BH-BGC11-66 Box 2 – 5.04 m to 9.57 m



BH-BGC11-66 Box 3 – 9.57 m to 13.72 m



BH-BGC11-66 Box 4 – 13.72 m to 18.48 m



BH-BGC11-66 Box 5 – 18.48 m to 23.24 m



BH-BGC11-66 Box 6 – 23.24 m to 27.17 m






BH-BGC11-66 Box 7 – 27.17 m to 31.19 m



BH-BGC11-66 Box 8 – 31.19 m to 33.53 m

	
<p>BH-BGC11-67 Run 1 – 0.00 m to 0.76 m</p>	<p>BH-BGC11-67 Run 2 – 0.76 m to 0.90 m</p>
	
<p>BH-BGC11-67 Run 3 – 0.90 m to 1.50 m</p>	<p>BH-BGC11-67 Run 4 – 1.50 m to 2.05 m</p>

	
<p>BH-BGC11-67 Run 5 – 2.05 m to 2.65 m</p>	<p>BH-BGC11-67 Run 6 – 2.65 m to 2.95 m</p>
	
<p>BH-BGC11-67 Run 7 – 2.95 m to 3.45 m</p>	<p>BH-BGC11-67 Run 8 – 3.45 m to 4.05 m</p>

	
<p>BH-BGC11-67 Run 9 – 4.05 m to 4.75 m</p>	<p>BH-BGC11-67 Run 10 – 4.75 m to 5.25 m</p>
	
<p>BH-BGC11-67 Run 11 – 5.25 m to 6.00 m</p>	<p>BH-BGC11-67 Run 13 – 6.00 m to 6.85 m</p>

				
BH-BGC11-67	Run 14 – 6.85 m to 8.36 m		BH-BGC11-67	Run 15 – 8.36 m to 9.90 m



BH-BGC11-69 Box 1 – 0.00 m to 8.62 m



BH-BGC11-69 Box 2 – 8.62 m to 13.14 m



BH-BGC11-69 Box 3 – 13.14 m to 17.63 m



BH-BGC11-69 Box 4 – 17.63 m to 21.34 m

APPENDIX G PENETRATION TESTING

PENETRATION TESTING

1.0 INTRODUCTION

This Appendix describes the methods and observations of three specific types of in-situ penetration testing completed in support of geotechnical investigations for mine site infrastructure.

2.0 TEST LOCATIONS

Three types of penetration testing were completed as part of the 2011 geotechnical investigation program. Standard Penetration Testing (SPT) was completed in 12 of the 16 auger holes completed. Dynamic Cone Penetration Testing (DCPT) was completed adjacent to BH-BGC11-67/69. Wildcat Penetrometer testing, which is completed using a hand-operated penetration testing device, was completed to depths of up to 5 m at 25 locations. Test locations are illustrated on Drawing 06, and Table G.1 summarizes all penetration testing completed.

Table G.1 Location of Penetration Testing

Location	Northing ^{1,2} (m)	Easting ^{1,2} (m)	Elevation ³ (masl)	Type of Penetration Testing
BH-BGC11-39	7101056	458462	804	SPT
BH-BGC11-44	7100547	458690	830	SPT
BH-BGC11-47	7100698	458811	841	SPT
BH-BGC11-49	7100635	458764	835	SPT
BH-BGC11-51	7100743	458643	818	SPT
BH-BGC11-53	7100993	459483	875	SPT
BH-BGC11-55	7100918	459440	881	SPT
BH-BGC11-56	7099822	458774	845	SPT
BH-BGC11-57	7099978	458798	859	SPT
BH-BGC11-58	7100119	458799	859	SPT
BH-BGC11-60	7100041	458795	859	SPT
BH-BGC11-65	7100939	458771	825	SPT
BH-BGC11-67/69	7101148	458823	867	DCPT
near TP-BGC11-90	7100513	459695	977	Wildcat Penetrometer
near BH-BGC10-24	7101012	458500	801	Wildcat Penetrometer
near BH-BGC10-13	7100942	458844	824	Wildcat Penetrometer
near BH-BGC10-14	7100980	458580	808	Wildcat Penetrometer
near BH-BGC10-22	7101098	458391	794	Wildcat Penetrometer
near TP-BGC11-62	7099924	459251	972	Wildcat Penetrometer
near TP-BGC11-69 (test A)	7099457	458915	878	Wildcat Penetrometer
near TP-BGC11-69 (test B)	7099457	458915	878	Wildcat Penetrometer
Placer Tailings (PQ12-A)	7101199	459747	891	Wildcat Penetrometer
Placer Tailings (PQ12-B)	7101199	459747	891	Wildcat Penetrometer
Placer Tailings (PQ13)	7101284	459832	902	Wildcat Penetrometer
Placer Tailings (PQ7-A)	7101061	459307	860	Wildcat Penetrometer
Placer Tailings (PQ7-B)	7101061	459307	860	Wildcat Penetrometer
Placer Tailings (PQ6-A)	7101008	459196	842	Wildcat Penetrometer
Placer Tailings (PQ6-B)	7101008	459196	842	Wildcat Penetrometer

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Location	Northing ^{1,2} (m)	Easting ^{1,2} (m)	Elevation ³ (masl)	Type of Penetration Testing
near TP-BGC11-94 (test A)	7101517	459980	937	Wildcat Penetrometer
near TP-BGC11-94 (test B)	7101517	459980	937	Wildcat Penetrometer
near BH-BGC11-53	7100984	459486	890	Wildcat Penetrometer
near BH-BGC11-55 (test A)	7100913	459448	889	Wildcat Penetrometer
near BH-BGC11-55 (test B)	7100913	459448	889	Wildcat Penetrometer
near TP-BGC11-103 (test A)	7101147	458821	866	Wildcat Penetrometer
near TP-BGC11-103 (test B)	7101148	458828	866	Wildcat Penetrometer
near TP-BGC11-105	7101199	458935	878	Wildcat Penetrometer
PQ14 (test A)	7100285	459722	1009	Wildcat Penetrometer
PQ14 (test B)	7100285	459722	1009	Wildcat Penetrometer

1. Boreholes surveyed by Underhill in August and September 2011 using a differential GPS, with the exception of BH-BGC11-39, 65 and 67 and which were surveyed by BGC staff using a handheld GPS. Test pits and PQ locations were surveyed by BGC staff using a handheld GPS.
2. UTM coordinates in NAD 83, zone 8N.
3. masl – metres above sea level

3.0 SPT TESTING

Standard Penetration Test (SPT) sampling involves an open sample tube driven into the formation at the base of a borehole using repeated blows of a steel weight that impacts a jar assembly or anvil at the top of the drill rods to cause the sampler to penetrate a standard distance, usually 45 cm, into the formation. The number of blows needed to achieve this penetration is taken as an indicator of the density or consistency of the formation material. The standard procedure for penetration testing, i.e. the Standard Penetration Test (SPT), utilizes a sampler with an internal diameter of 34.9 mm (1 3/8”) and divides the blowcount into three segments (150 mm each) and considers only the two last segments (final 300 mm) for calculation of the blowcount, or N value. The first 150 mm segment is ignored, as it often represents penetration of the sampler through sloughed or disturbed material at the bottom of the hole rather than intact in-situ materials. In geological formations containing a significant proportion of coarse fragments, the sampler will often meet refusal on a large clast, and the resulting high blowcount can lead to a false indication of a high density deposit.

The SPT-tested geological formations in the Eagle Gold project are known from past investigations to contain gravel, cobbles and boulders. In an attempt to identify interference by coarse materials, SPT blowcounts were typically recorded for each 25 mm interval during the SPT tests, rather than every 150 mm. Where interference was known to have occurred from observation of bouncing of the drill string, or as evidenced by low recovery in the sampler, the raw blowcounts are considered suspect, and estimated N values are extrapolated from the initial per 25 mm penetration values. In this situation, an ‘equivalent’ N value was estimated for the initial portion of the test by multiplying the sum of the meaningful per 25 mm blow counts over the selected test interval by a factor that yielded a projected blowcount for the full 300 mm. This methodology was followed only for samples with more than 150 mm of recovery and only blows within the last 300 mm were considered. All the SPT testing was done following the per 25 mm methodology except for three tests (BH-BGC11-51, SPT 3; BH-BGC11-51, SPT 6; and BH-BGC11-65, SPT 3) where materials were

too soft to allow per 25 mm blowcounts, and in these cases blowcounts were recorded over the standard 150 mm increments.

Energy measurements were made to determine the hammer efficiency. The sum of blows between 150 mm and 450 mm was corrected to 60% hammer efficiency to yield $(N)_{60}$.

SPT tests that were completed in frozen ground were omitted from the analysis.

The blow count details for all SPT tests, including blows per 25 mm, are shown in Table G.2. Plots of cumulative blowcounts versus recovery are found in Figures G-1 to G-10. Out of a total of 77 SPT tests, 46 are considered valid. Of the 77 tests, 7 were completed in frozen ground and are not considered valid, 19 tests had less than 150 mm of recovery and are not considered valid and 5 were influenced by coarse clasts prior to reaching 150 mm of penetration and are not considered valid. Raw blow counts for invalid tests are included below, but are not shown on borehole logs (Appendix E).

Table G.2 SPT Testing Results

Hole	SPT #	Depth From (m)	Depth To (m)	Blow Counts per 25 mm Increment																		N - Raw	N – Corrected for Gravel Effects	N ₆₀	Comments
				1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18				
BH-BGC11-39	SPT1	1.15	1.6	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1	1		
BH-BGC11-39	SPT2	1.6	2.05	1	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	2	2	2		
BH-BGC11-39	SPT3	2.28	2.73	1	0	5	6	4	3	4	3	5	5	5	4	2	3	3	3	4	3	44	44	56	
BH-BGC11-39	SPT4	2.75	3.2	1	8	19	7	6	4	4	4	3	2	9	9	7	4	5	4	5	3	59	39	50	
BH-BGC11-39	SPT5	4.6	5.05	1	4	2	3	3	3	3	5	6	9	9	8	6	7	4	5	4	5	71	48	62	
BH-BGC11-39	SPT6	5.53	5.98	1	1	1	2	2	1	3	2	2	2	2	2	3	2	2	3	3	3	29	27	35	
BH-BGC11-39	SPT7	6.22	6.67	1	1	1	2	2	1	2	2	2	2	3	2	2	3	2	2	3	2	27	29	37	
BH-BGC11-39	SPT8	7.16	7.61	1	1	2	2	2	2	3	2	2	2	2	2	3	2	3	2	3	2	28	28	36	
BH-BGC11-39	SPT9	7.75	8.2	1	1	2	4	3	5	3	4	4	5	4	4	6	5	4	5	4	52	52	67		
BH-BGC11-39	SPT10	8.71	9.14	1	1	2	2	2	1	3	2	2	2	2	3	2	3	3	3	3	3	30	30	39	
BH-BGC11-39	SPT11	9.25	9.7	1	1	2	2	2	2	3	2	3	3	3	3	2	3	3	3	3	3	34	34	44	
BH-BGC11-44	SPT1	2.27	2.72	1	0	0	0	0	1	1	0	0	1	0	1	0	1	1	1	0	7	7	9		
BH-BGC11-44	SPT2	3.79	4.24	1	0	1	0	1	0	0	1	1	1	2	1	2	1	2	1	2	1	15	15	19	
BH-BGC11-44	SPT3	5.33	5.8	1	0	0	0	0	0	0	1	0	0	1	0	0	0	1	0	1	0	4	-	-	less than 150 mm recovery
BH-BGC11-44	SPT4	6.85	7.32	1	0	0	0	0	0	0	1	0	0	0	1	1	0	1	0	1	1	6	-	-	less than 150 mm recovery
BH-BGC11-44	SPT5	8.38	8.83	1	1	3	5	4	2	4	4	3	4	3	3	5	5	4	6	7	6	54	-	-	frozen - invalid test
BH-BGC11-44	SPT6	9.38	9.45	1	1	1	R															R	-	-	less than 150 mm recovery
BH-BGC11-44	SPT7	9.4	9.85	4	40	6	7	6	6	6	6	6	8	6	6	10	6	8	5	9	6	82	-	-	less than 150 mm recovery
BH-BGC11-44	SPT8	9.9	10.35	3	3	3	4	2	4	3	3	4	12	10	9	6	9	13	10	7	7	93	40	51	
BH-BGC11-44	SPT9	11.42	11.97	2	2	2	2	2	3	3	3	3	4	3	3	6	6	8	6	6	6	57	57	73	
BH-BGC11-44	SPT10	12.97	13.42	5	6	4	8	5	5	5	6	9	6	6	10	8	9	10	7	7	12	95	77	99	
BH-BGC11-44	SPT11	14.49	14.79	1	0	0	1	0	1	5	6	6	5	5	6	R						R	-	-	gravel effects at 150 mm
BH-BGC11-47	SPT1	0.75	1.2	1	0	0	1	0	0	1	0	0	1	2	2	1	2	2	1	1	2	15	-	-	Frozen
BH-BGC11-47	SPT2	3.35	3.8	1	1	2	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	12	12	15	
BH-BGC11-47	SPT3	3.8	4.25	1	1	0	1	0	0	1	0	0	0	1	0	0	1	0	1	0	0	4	4	5	
BH-BGC11-47	SPT4	5.33	5.88	1	0	1	1	1	1	1	1	0	1	1	1	1	0	1	2	1	1	11	-	-	Frozen
BH-BGC11-47	SPT5	6.85	7.3	1	2	1	3	2	2	4	6	5	4	4	5	4	10	10	11	8	8	79	-	-	Frozen
BH-BGC11-47	SPT6	8.38	8.58	1	2	2	3	7	7	11	13											R	-	-	gravel effects before 150 mm
BH-BGC11-47	SPT7	9.9	10.05	1	2	6	45	24	29													R	-	-	less than 150 mm recovery
BH-BGC11-47	SPT8	11.43	11.605	4	4	5	15	10	17	25												R	-	-	gravel effects before 150 mm
BH-BGC11-49	SPT1	0.76	1.22	1	0	0	0	1	0	0	1	0	1	0	1	1	1	1	2	2	2	12	8	10	
BH-BGC11-49	SPT2	2.28	2.73	2	2	4	4	4	6	4	4	3	2	2	2	3	3	3	3	3	2	34	-	-	Frozen
BH-BGC11-51	SPT1	0.76	1.21	1	2	1	2	1	2	1	2	1	1	1	1	1	1	1	2	1	1	14	14	18	
BH-BGC11-51	SPT2	2.28	2.73	1	1	0	1	0	1	1	0	0	0	1	1	0	1	0	1	1	0	6	6	8	
BH-BGC11-51	SPT3	3.8	4.25																			12	12	15	N=12, blow counts not recorded by 25 mm
BH-BGC11-51	SPT4	5.33	5.78	0	0	1	1	1	1	0	1	3	3	2	1	1	1	1	2	2	3	20	20	26	
BH-BGC11-51	SPT5	9.9	10.25	1	1	1	2	1	2	1	1	2	2	1	3	4	4	3	10			R	17	22	
BH-BGC11-51	SPT6	11.43	11.52																			R	-	-	N = R, blow counts not recorded by 25 mm, less than 150 mm of recovery
BH-BGC11-51	SPT7	20.57	20.62	1	2	R																R	-	-	less than 150 mm recovery
BH-BGC11-51	SPT8	22.1	22.25	15	R																	R	-	-	Frozen
BH-BGC11-53	SPT1	0.76	1.21	0	1	1	1	1	1	1	0	0	1	0	1	0	1	0	1	1	1	7	-	-	less than 150 mm recovery
BH-BGC11-53	SPT2	2.28	2.73	0	1	1	1	2	1	1	2	1	2	1	1	2	1	2	1	3		18	18	23	
BH-BGC11-53	SPT3	3.8	4.25	0	1	3	4	3	5	3	2	3	2	2	3	3	2	3	2	3	4	32	32	41	
BH-BGC11-53	SPT4	6.85	7.3	1	1	2	2	3	4	3	3	3	4	4	3	2	4	3	5	5	4	43	43	55	
BH-BGC11-53	SPT5	9.9	10.35	1	1	1	2	2	3	3	5	3	5	4	4	2	3	3	4	8	8	52	43	55	
BH-BGC11-55	SPT1	0.76	1.21	0	0	1	0	1	1	1	1	1	1	1	1	1	2	1	1	1	2	14	14	18	

Hole	SPT #	Depth From (m)	Depth To (m)	Blow Counts per 25 mm Increment																		N - Raw	N – Corrected for Gravel Effects	N ₆₀	Comments
				1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18				
BH-BGC11-55	SPT2	2.28	2.73	1	0	1	1	1	1	0	1	1	0	1	1	1	1	2	1	1	11	11	14		
BH-BGC11-55	SPT3	3.8	4.25	0	1	0	1	1	1	1	1	2	4	2	4	2	2	2	2	3	27	27	35		
BH-BGC11-55	SPT4	5.33	5.78	1	0	1	2	1	1	2	1	1	1	1	2	1	1	1	2	1	15	15	19		
BH-BGC11-55	SPT5	6.85	7.3	0	0	0	1	0	1	0	1	0	1	2	2	1	1	1	1	2	13	13	17		
BH-BGC11-55	SPT6	8.38	8.83	1	1	1	2	1	1	1	1	3	4	4	4	4	4	4	4	3	40	12	15		
BH-BGC11-55	SPT7	9.9	10.35	1	1	1	3	4	6	6	6	3	5	6	6	7	7	6	5	5	67	67	86		
BH-BGC11-56	SPT1	0.76	1.21	2	2	2	2	2	2	2	2	1	3	2	1	2	1	2	1	2	20	20	26		
BH-BGC11-56	SPT2	2.28	2.43	1	1	1	2	1	R											R	-	-	less than 150 mm recovery		
BH-BGC11-56	SPT3	3.8	4.25	1	1	3	5	5	3	4	4	4	6	3	2	4	4	5	R	R	48	62			
BH-BGC11-56	SPT4	5.33	5.53	1	3	3	4	4	7	7	R									R	-	-	gravel effects before 150 mm		
BH-BGC11-56	SPT5	6.85	7.15	1	0	1	2	2	2	5	5	5	7	9	6	R				R	-	-	gravel effects before 150 mm		
BH-BGC11-56	SPT6	8.38	8.53	0	1	1	7	10	R											R	-	-	less than 150 mm recovery		
BH-BGC11-57	SPT1	2.28	2.65	0	0	0	2	1	2	3	7	7	4	5	6	6	5	R		R	-	-	frozen		
BH-BGC11-57	SPT2	3.8	4.25	2	2	3	4	5	6	4	4	5	5	6	5	7	5	5	7	4	4	61	61	78	
BH-BGC11-57	SPT3	5.33	5.78	2	1	1	1	4	3	4	2	4	5	5	5	5	3	4	6	7	55	55	71		
BH-BGC11-57	SPT4	6.85	7.1	2	1	3	6	7	7	7	9	9	R							R	100	100	N ₆₀ maximum of 100, calculated N ₆₀ is 128		
BH-BGC11-57	SPT5	8.43	8.53	1	1	2	4	R												R	-	-	less than 150 mm recovery		
BH-BGC11-57	SPT6	9.9	10.2	0	1	1	2	4	9	5	5	8	5	7	8	R				R	76	98	gravelly sample		
BH-BGC11-58	SPT1	5.5	5.5	1	R															R	-	-	less than 150 mm recovery		
BH-BGC11-58	SPT2	6.85	6.9	1	2	R														R	-	-	less than 150 mm recovery		
BH-BGC11-60	SPT1	4.6	4.75	3	4	8	7	8	10	R										R	-	-	less than 150 mm penetration		
BH-BGC11-60	SPT2	6.85	7	0	1	1	2	5	8	8	R									R	-	-	less than 150 mm recovery		
BH-BGC11-65	SPT1	0.76	1.21	1	1	2	2	2	2	2	2	3	3	2	2	3	3	2	2	1	3	28	28	36	
BH-BGC11-65	SPT2	1.52	1.97	1	1	2	1	2	1	2	3	1	2	1	2	1	1	2	1	1	2	19	19	24	
BH-BGC11-65	SPT3	2.28	2.73																		13	13	17	N=13, blow counts not recorded by 25 mm	
BH-BGC11-65	SPT4	3.04	3.49	1	1	1	1	1	1	1	2	1	2	2	2	2	1	1	2	2	1	19	19	24	
BH-BGC11-65	SPT5	3.8	4.15	2	2	2	3	2	1	1	2	6	7	7	7	10	R			R	-	-	less than 150 mm recovery		
BH-BGC11-65	SPT6	4.56	5.01	4	4	2	1	2	2	3	2	2	2	2	2	2	2	2	2	2	25	25	32		
BH-BGC11-65	SPT7	5.33	5.78	3	3	2	2	1	3	3	2	2	1	2	1	1	1	2	1	1	2	19	-	less than 150 mm recovery	
BH-BGC11-65	SPT8	6.09	6.54	3	5	5	3	3	4	3	2	3	3	1	2	3	3	3	3	2	4	32	-	less than 150 mm recovery	
BH-BGC11-65	SPT9	6.85	6.9	1	2	R														R	-	-	less than 150 mm recovery		

2.1 SPT Energy Measurements

All SPT testing was completed using the auger rig (MARL M4CT) which was equipped with an automatic trip hammer weighing 140 lbs (63.5 kg) and having a drop height of 30 inches (760 mm).

The SPT tests for BH-BGC11-65 were monitored for hammer impact energy using an SPT Analyzer manufactured by Pile Dynamics Inc (PDI) of Cleveland, Ohio. The analyzer system consisted of a short length of instrumented sampling rod inserted in the rod string just below the jar assembly, and a field data recorder. The instrumented rods included two strain gauges permanently bonded to the rod, and two accelerometers bolted to the rod. The signals collected from the strain transducers and accelerometers were then converted by the SPT Analyzer to force and velocity records. Every SPT energy measurement was done using an AW-size instrumented rod.

Force and velocity measurements were recorded for each blow, and the SPT analyzer instantly calculated an average Energy Transfer Ratio (ETR) using the force-velocity method (ASTM D 4633-05). The ETR is given as a percent of the theoretical free fall potential energy of the standard 63.5 kg SPT hammer. Energy data was averaged for every 150 mm SPT increment and the average energy values for the two increments over 150 mm to 450 mm were also averaged to give C_E , the hammer energy correction factor, to correct the blow count value to standard 60% hammer efficiency.

C_E was calculated using the following equation:

$$C_E = \frac{ETR}{60}$$

The average energy measurement for BH-BGC11-65 was 77% of the theoretical free fall potential energy for the SPT tests. Table G.3 summarizes the values obtained.

Table G.3 Measured SPT Energy Transfer Ratios

Test	Depth (m)	Energy Transfer Ratio (ETR)%	
		Mean	Standard Deviation
SPT 1	0.76 – 1.21	78.7	3.6
SPT 2	1.52 – 1.97	79.1	2.9
SPT 4	3.04 – 3.49	77	3.6
SPT 5	3.8 – 4.15	78.4	1.5
SPT 6	4.56 – 5.01	77	1.6
SPT 7	5.33 – 5.78	75	1.5
SPT 8	6.09 – 6.54	76.8	1.6
SPT 9	6.85 – 6.9	75.6	2.2
All SPTs		77.3	2.6

Since the same hammer and equipment were used for all SPT tests, the same energy correction factor was applied to all equivalent SPT N values to calculate SPT $(N)_{60}$ values. These values are shown in figures G-11 through G-20 and in Table G.2.

4.0 DYNAMIC CONE PENETRATION TESTING (DCPT)

Cone penetration testing is conducted using a 60 mm diameter cone driven into the formation at the base of a borehole using repeated blows of a steel weight that slides on the drill rods and impacts a jar assembly or anvil to cause the cone to penetrate a standard distance, usually 300 mm, into the formation. The number of blows needed to achieve this penetration is taken as an indicator of the density or consistency of the formation material.

DCPT testing was completed with the auger rig and the same automatic trip hammer as was used for SPT testing. The DCPT tests were also monitored using SPT analyzer. Blows were counted in 150 mm increments.

A total of four DCPT tests were completed adjacent to BH-BGC11-67/69 and plate load test PT-BGC11-01. Refusal was met in these four tests at 2.5 m, 3.2 m, 0.5 m and 0.6 m.

The average energy measurement for DCPT test A was 81%. The analyzer malfunctioned for the other three DCPT tests. All raw blow counts are presented in Figure G-21 and Table G.4.

Table G.4 DCPT data

Depth		Blows per 150 mm			
From (m)	To (m)	Test A	Test B	Test C	Test D
0.00	0.15	9	11	17	21
0.15	0.30	10	14	43	38
0.30	0.46	18	19	89	75
0.46	0.61	25	30	25 (for no advancement)	131
0.61	0.76	32	36		25 (for no advancement)
0.76	0.91	35	42		
0.91	1.07	51	35		
1.07	1.22	48	32		
1.22	1.37	42	51		
1.37	1.52	29	61		
1.52	1.68	36	60		
1.68	1.83	71	56		
1.83	1.98	59	*		
1.98	2.13	50	62		
2.13	2.29	62	23		
2.29	2.44	62	12		
2.44	2.59	45	19		
2.59	2.74	27 (for no advancement)	23		
2.74	2.90		35		
2.90	3.05		26		
3.05	3.20		31		
3.20	3.35		86		
3.35	3.51		105		

*Lost count of blows.

5.0 WILDCAT PENETROMETER TESTING

Wildcat penetrometer testing is conducted using a hand operated penetration testing tool manufactured by Triggs Technologies of Willoughby Hills, Ohio, USA (Triggs Technologies 2007). The device consists of a safety hammer, rods, a sacrificial cone tip and a lubricating system. The falling portion of the hammer weighs 15.9 kgs (35 lbs) and the drop height is 0.377 m (15 inches). The rods are 1 m long and 2.8 cm (1.1 inches) in diameter. The rods are grooved every 10 cm to allow blow counts to be recorded in 10 cm increments. The sacrificial cone tip is 3.5 cm (1.4 inches) in diameter, has a 90° apex and 10 cm² projected area. The lubrication system allows polymer to fill the annulus between the rods and the soil, limiting the hammer energy lost to friction.

Wildcat penetration tests are completed by driving the sacrificial tip into the ground, using the safety hammer. Blows are counted for each 10 cm increment. Additional rods are added to the assembly as penetration continues. Polymer slurry is injected into the assembly or the directly into the annulus between the rods and the ground throughout testing to limit friction between the rods and the ground. Testing is completed when refusal is met (considered to be 50 blows in one 10 cm increment).

A total of 25 Wildcat penetrometer tests were completed (Table G.5). Of the 23 tests, 22 were terminated at refusal and three were terminated at the maximum length of rods available (5 m). Blow counts with depth are plotted in figures G-22 to G-38 and tabulated in Table G.6.

Table G.5 Wildcat Penetrometer Test Locations

Test Number	Test Location	Northing (m)	Easting (m)	Elevation (m)	Test Date	Tested By	Final Depth of Test (m)	Reason for End of Test	Comments
1	near TP-BGC11-90	7100513	459695	977	15/07/2011	LT	3	Refusal	
2	near BH-BGC10-24	7101012	458500	801	15/07/2011	LT/EB	5	Maximum length of rods	0.5 m of granular material present above natural ground as a result of drill pad construction
3	near BH-BGC10-13	7100942	458844	824	17/07/2011	EC	0.3	Refusal on cobbles/boulders	
4	near BH-BGC10-14	7100980	458580	808	17/07/2011	EC	1.3	Refusal	
5	near BH-BGC10-22	7101098	458391	794	17/07/2011	EC	4.9	Maximum length of rods	
6	near TP-BGC11-62	7099924	459251	972	19/07/2011	EC/EB	2.47	Refusal	removed 0.2 m of organics prior to testing
7	near TP-BGC11-69 (test A)	7099457	458915	878	19/07/2011	EC	1	Refusal	
8	near TP-BGC11-69 (test B)	7099457	458915	878	19/07/2011	EC	4.21	Refusal on cobbles/boulders	removed 0.2 m of organics prior to testing
9	Placer Tailings (PQ12-A)	7101199	459747	891	23/07/2011	EC	0.3	Refusal on cobbles/boulders	
10	Placer Tailings (PQ12-B)	7101199	459747	891	23/07/2011	EC	0.3	Refusal on cobbles/boulders	
11	Placer Tailings (PQ13)	7101284	459832	902	23/07/2011	EC	4.9	Maximum length of rods	
12	Placer Tailings (PQ7-A)	7101061	459307	860	23/07/2011	EC	0.1	Refusal on cobbles/boulders	

Test Number	Test Location	Northing (m)	Easting (m)	Elevation (m)	Test Date	Tested By	Final Depth of Test (m)	Reason for End of Test	Comments
13	Placer Tailings (PQ7-B)	7101061	459307	860	23/07/2011	EC	0.3	Refusal on cobbles/boulders	
14	Placer Tailings (PQ6-A)	7101008	459196	842	23/07/2011	EC	0.1	Refusal on cobbles/boulders	
15	Placer Tailings (PQ6-B)	7101008	459196	842	23/07/2011	EC	0.14	Refusal on cobbles/boulders	
16	near TP-BGC11-94 (test A)	7101517	459980	937	23/07/2011	EC	0.7	Refusal on cobbles/boulders	removed 0.25 m of organics prior to testing, frozen ground between 0.1 m and 2.0 m
17	near TP-BGC11-94 (test B)	7101517	459980	937	23/07/2011	EC	2.3	Refusal	removed 0.25 m of organics prior to testing, frozen ground between 0.1 m and 2.0 m
18	near BH-BGC11-53	7100984	459486	890	15/08/2011	LT	1.6	Refusal	
19	near BH-BGC11-55 (test A)	7100913	459448	889	15/08/2011	LT	1.1	Refusal	
20	near BH-BGC11-55 (test B)	7100913	459448	889	15/08/2011	LT	0.87	Refusal	
21	near TP-BGC11-103 (test A)	7101147	458821	866	15/08/2011	LT	0.35	Refusal	
22	near TP-BGC11-103 (test B)	7101148	458828	866	15/08/2011	LT	0.9	Refusal	
23	near TP-BGC11-105	7101199	458935	878	15/08/2011	LT	1	Refusal	

Test Number	Test Location	Northing (m)	Easting (m)	Elevation (m)	Test Date	Tested By	Final Depth of Test (m)	Reason for End of Test	Comments
24	PQ-14 (test A)	7100285	459722	1009	25/08/2011	PQ	0.01	Refusal	near Plate Load Test area 3 at proposed secondary crushers
25	PQ-14 (test B)	7100285	459722	1009	25/08/2011	PQ	0.01	Refusal	near Plate Load Test area 3 at proposed secondary crushers

Table G.6 Wildcat Penetrometer Blow Counts

Test #	1	2	3	4	5
Location	Near TP-BGC11-90	Near BH-BGC10-24	Near BH-BGC10-13	Near BH-BGC10-14	Near BH-BGC10-22
Material Tested	0.15 m to 1.15 m SAND (SP), 1.15 m to 4 m SILT (MH)	Placer Tailings	Placer Tailings, GRAVEL and SAND (GW/SW)	Placer Tailings, 0 m to 2 m GRAVEL and SAND (GW/SW), 2 m to 9.5 m SILT and SAND	Placer Tailings, 0 m to 4.7 m SAND and SILT (SM/ML)
Depth (m)	Blows per 100 mm				
0.1	4	1	3	7	2
0.2	5	7	20	8	4
0.3	4	9	50	13	6
0.4	4	10		15	6
0.5	9	9		22	7
0.6	9	9		16	5
0.7	9	7		21	5
0.8	11	4		20	6
0.9	8	3		20	5
1	9	3		19	4
1.1	10	2		30	3
1.2	7	2		50	2
1.3	8	2		50	4
1.4	11	2			3
1.5	10	2			4
1.6	9	2			3
1.7	6	1			3
1.8	8	2			4
1.9	8	3			3
2	7	3			3
2.1	11	2			4
2.2	13	3			3
2.3	13	4			4
2.4	10	2			6
2.5	9	2			4
2.6	10	3			4
2.7	9	3			5
2.8	6	3			5
2.9	11	3			5
3	25	2			5
3.1		1			5
3.2		4			4
3.3		4			5
3.4		4			4
3.5		4			5
3.6		5			5
3.7		4			5
3.8		4			6
3.9		4			5
4		2			6
4.1		5			4
4.2		4			5
4.3		4			5
4.4		4			5
4.5		5			5
4.6		4			4
4.7		5			4
4.8		6			4
4.9		8			4
5		12			

Test #	6	7	8	9	10
Location	Near TP-BGC11-62	Near TP-BGC11-69 (test A)	Near TP-BGC11-69 (test B)	Placer Tailings PQ-12 (test A)	Placer Tailings PQ-12 (test B)
Material Tested	0.2 m to 1 m Cobbly Silt, 1 m to 6.2 m Sandy Silt,		0.2 m to 2.6 m Silt, 2.6 m to 3.2 m Gravelly silt, 3.2 m to 4 m disintegrated bedrock, 4 m to 6.5 m very blocky bedrock		Placer Tailings
Depth (m)	Blows per 100 mm				
0.1	1	1	1	11	16
0.2	1	1 blow for 20 cm	1	12	8
0.3	2	2	1	17	Refusal
0.4	3	1	1		
0.5	6	2	1		
0.6	5	3	4		
0.7	6	7	2		
0.8	7	14	12		
0.9	11	12	10		
1	15	45	11		
1.1	14		8		
1.2	27		9		
1.3	20		7		
1.4	24		5		
1.5	25		8		
1.6	15		15		
1.7	11		12		
1.8	14		15		
1.9	12		27		
2	17		15		
2.1	18		14		
2.2	15		8		
2.3	25		8		
2.4	42		8		
2.5	51		9		
2.6			9		
2.7			13		
2.8			23		
2.9			22		
3			23		
3.1			20		
3.2			23		
3.3			23		
3.4			20		
3.5			20		
3.6			22		
3.7			25		
3.8			26		
3.9			19		
4			21		
4.1			35		
4.2			25		
4.3			40		

Test #	11	12	13	14	15
Location	Placer Tailings PQ-13	Placer Tailings PQ-7 (test A)	Placer Tailings PQ-7 (test B)	Placer Tailings PQ-6 (test A)	Placer Tailings PQ-6 (test B)
Material Tested	0.15 m to 1.15 m SAND (SP), 1.15 m to 4 m SILT (MH)	Placer Tailings		Placer Tailings	
Depth (m)	Blows per 100 mm				
0.1	1	5	12	50	37
0.2	2		15		35
0.3	2		20		
0.4	1				
0.5	1				
0.6	2				
0.7	2				
0.8	2				
0.9	5				
1	2				
1.1	3				
1.2	1				
1.3	3				
1.4	4				
1.5	3				
1.6	3				
1.7	7				
1.8	9				
1.9	4				
2	2				
2.1	4				
2.2	3				
2.3	4				
2.4	3				
2.5	4				
2.6	2				
2.7	4				
2.8	4				
2.9	3				
3	3				
3.1	7				
3.2	4				
3.3	6				
3.4	4				
3.5	5				
3.6	8				
3.7	10				
3.8	6				
3.9	6				
4	9				
4.1	10				
4.2	8				
4.3	8				
4.4	10				
4.5	10				
4.6	10				
4.7	15				
4.8	13				
4.9	14				

Test #	16	17	18	19	20
Location	Near TP-BGC11-94 (test A)	Near TP – BGC11-94 (test B)	Near BH-BGC11-53	Near BH-BGC11-55 (test A)	Near BH-BGC11-55 (test B)
Material Tested	0.2 m to 2.0 m Silt, 2.0 m to 5.0 m fine Sand		0 m to 3.8 m Silt	0 m to 8.8 m Silt	
Depth (m)	Blows per 100 mm				
0.1	16	5	1	1	0
0.2	25	21	3	3	0
0.3	22	43	1	2	0
0.4	23	54	4	2	2
0.5	19	70	8	3	4
0.6	66	50	5	3	13
0.7	48	69	5	6	10
0.8		67	4	6	36
0.9		34	6	5	50
1		24	5	29	
1.1		35	22	45+	
1.2		8	18		
1.3		8	24		
1.4		7	19		
1.5		10	16		
1.6		15	50		
1.7		18			
1.8		30			
1.9		22			
2		18			
2.1		21			
2.2		25			
2.3		22			
2.4		14			
2.5		12			
2.6		90			

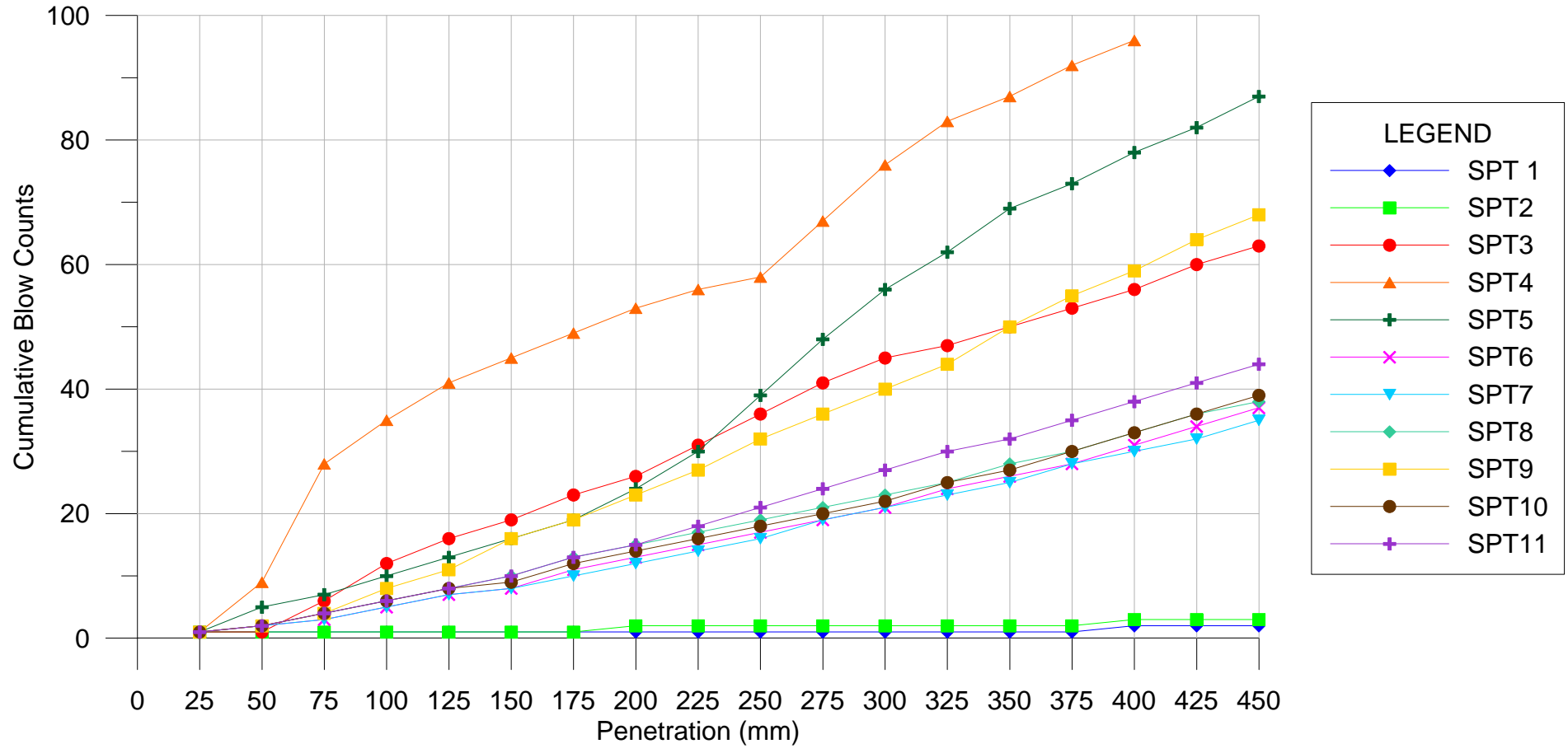
Test #	21	22	23	24	25
Location	Near TP-BGC11-103 (test A)	Near TP – BGC11-103 (test B)	Near TP-BGC11-105	PQ14 (test A)	PQ14 (test B)
Material Tested	0.2 m to 3.5 m Silt		0.2 m to 2.5 m Sand	Metasedimentary Rock	
Depth (m)	Blows per 100 mm				
0.1	13	50	7	50	50
0.2	18		15		
0.3	20		29		
0.4	50		30		
0.5			30		
0.6			36		
0.7			28		
0.8			28		
0.9			25		
1			50		

6.0 REFERENCES

ASTM D 4633-05. Standard Test Method for Energy Measurement for Dynamic Penetrometers.

Triggs Technologies Inc. Manual 14 for the Wildcat Dynamic Cone Penetrometer. May 4, 2007.

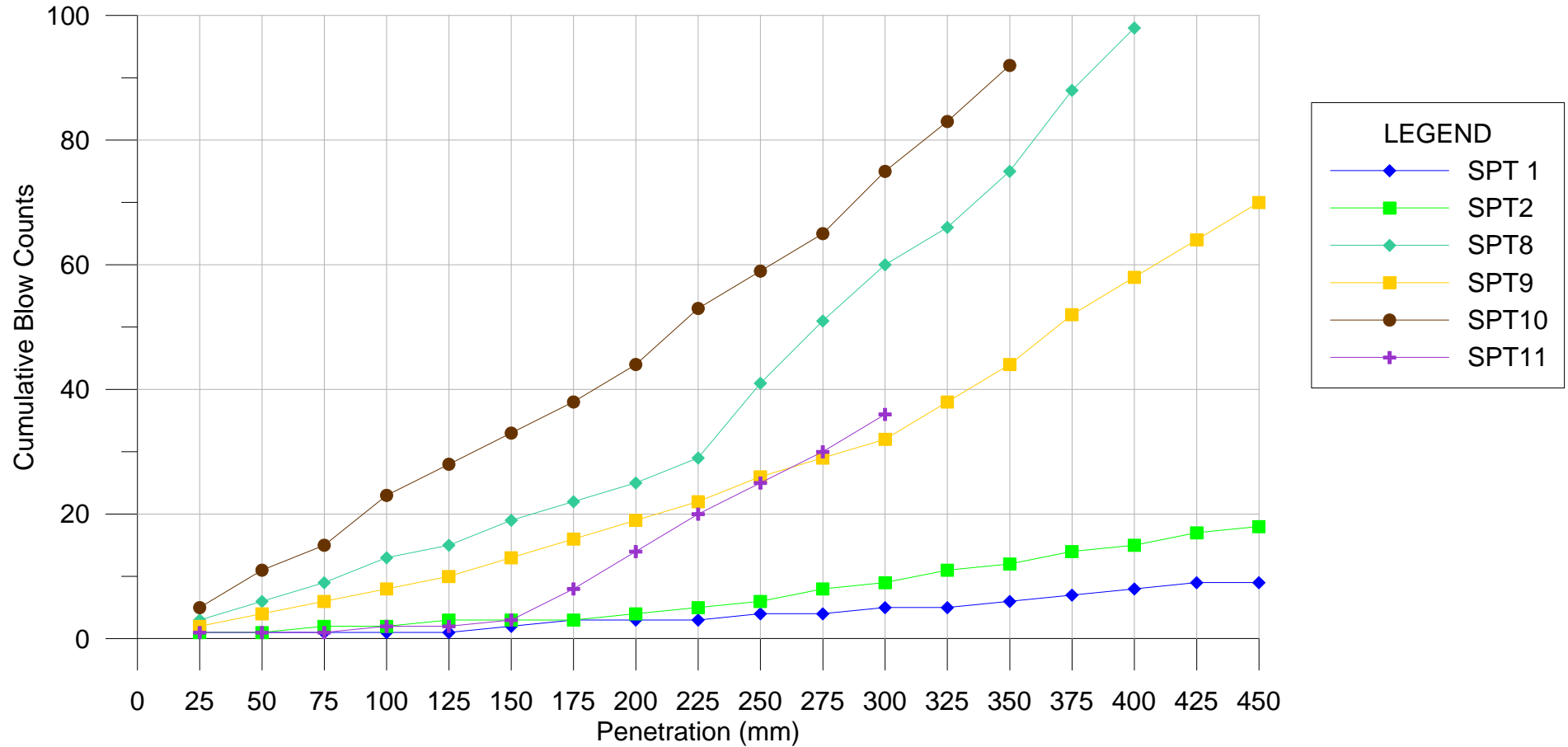
FIGURES



Note: Cumulative blow counts greater than 100 are not plotted.

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		CLIENT: VICTORIA GOLD CORP.	PROJECT No. 0792-006	FIG No. G-01	REV. 0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.



NOTE: SPT 3, 4, 6 and 7 had less than six inches of recovery and are not considered.
 NOTE: SPT 5 was frozen and is not considered.
 NOTE: Cumulative blow counts greater than 100 are not plotted.


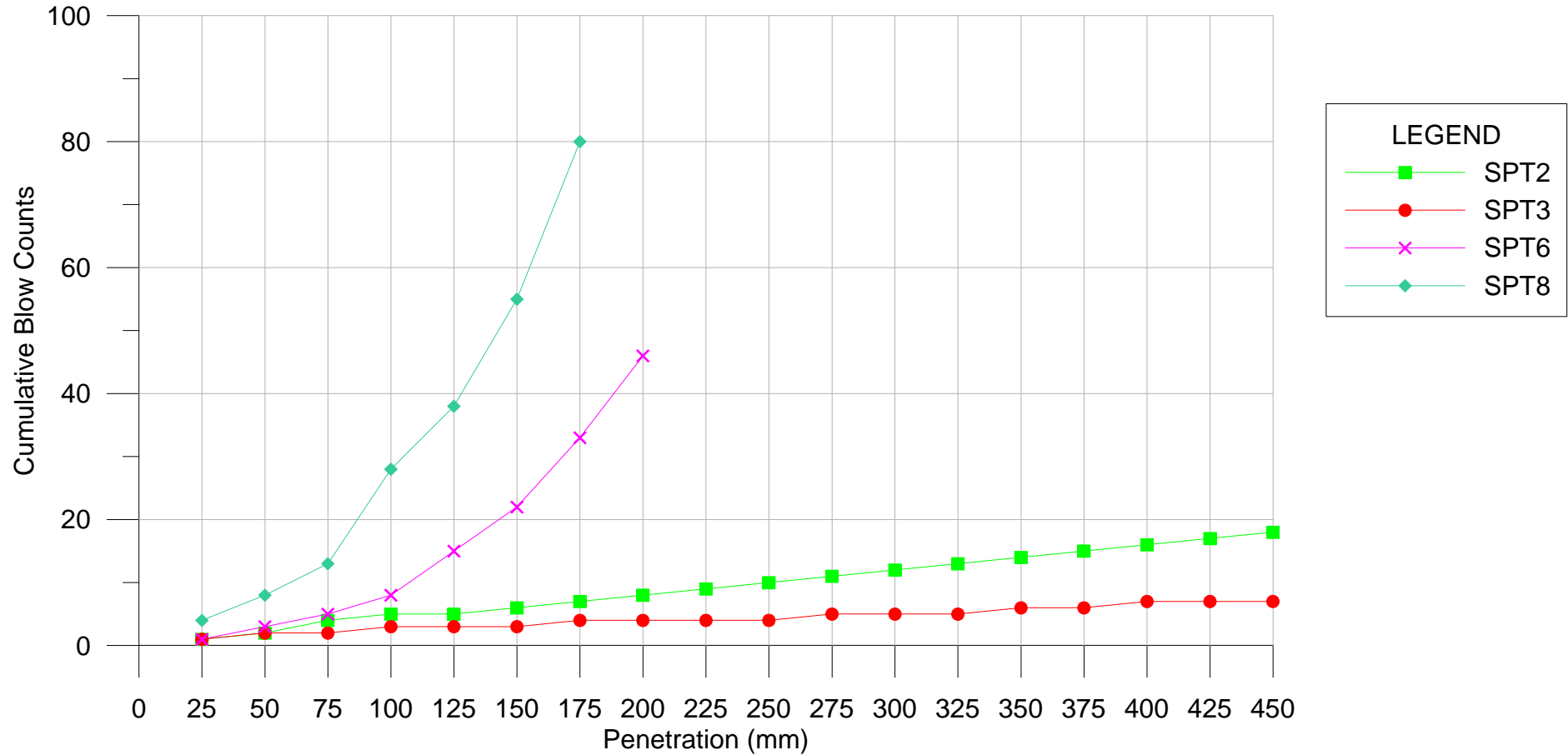
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		CLIENT: VICTORIA GOLD CORP.	PROJECT No. 0792-006	FIG No. G-02	REV. 0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.



Note: SPT 1, 4 and 5 were frozen and are not considered.
 Note: SPT 7 had less than six inches of recovery and is not considered.


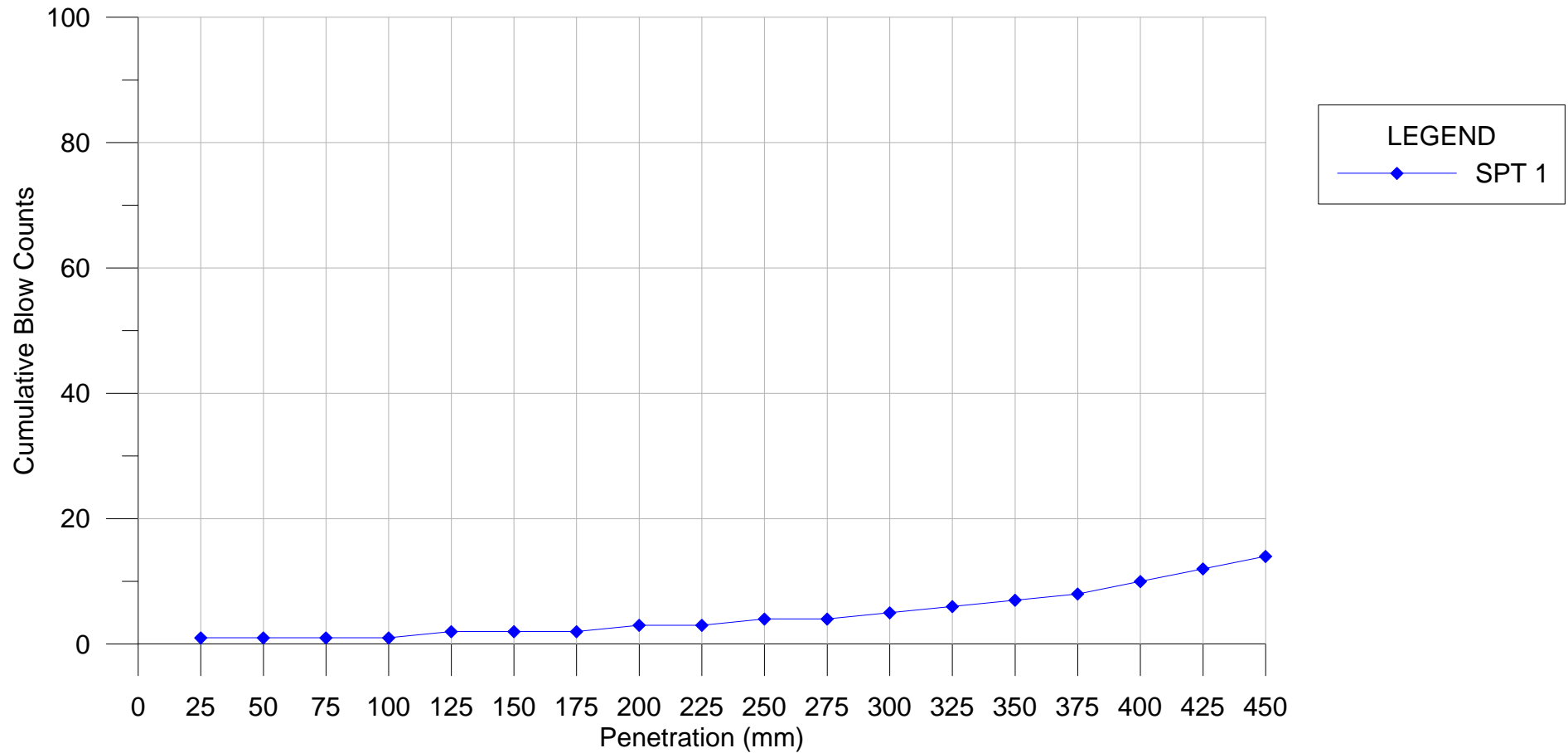

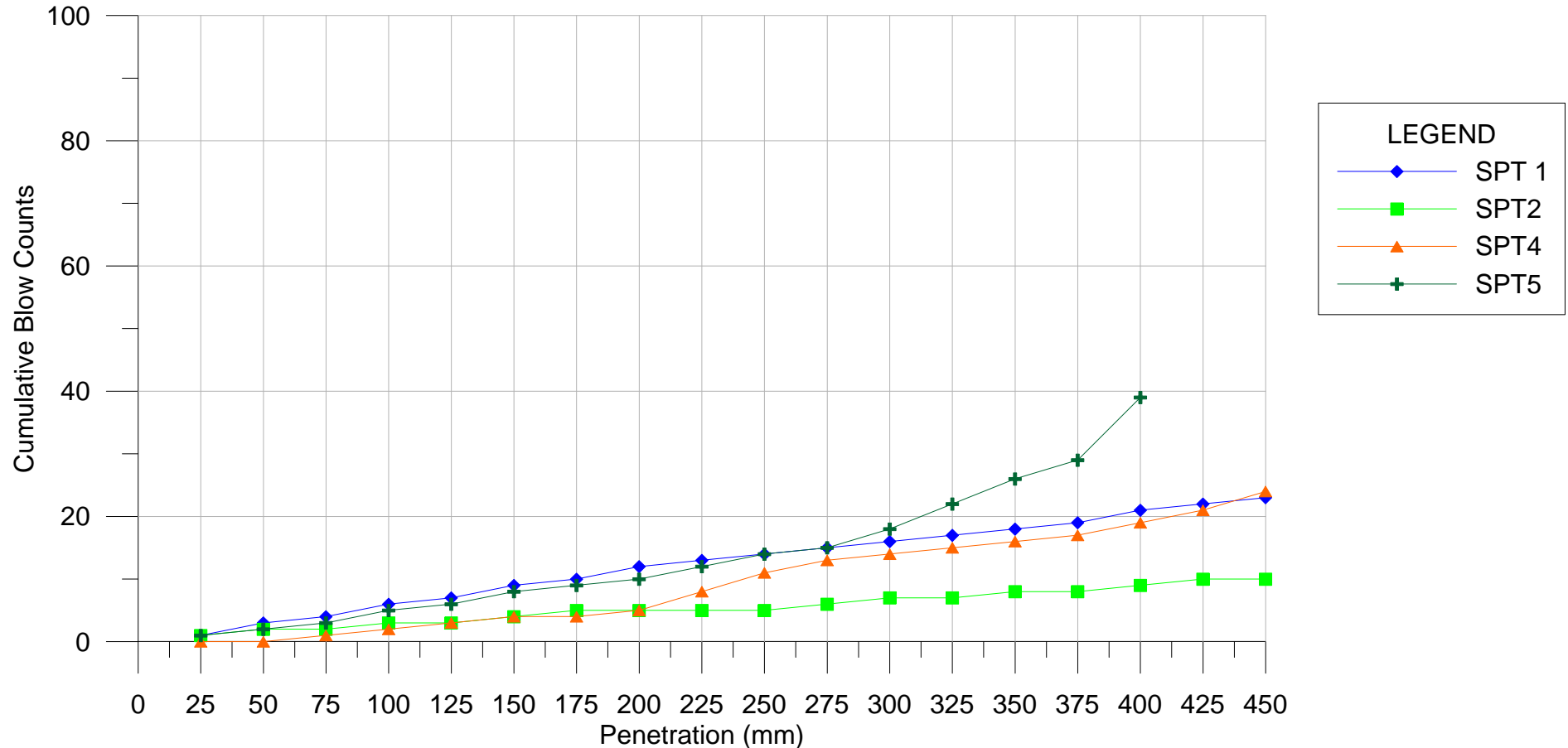
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		TITLE: CUMULATIVE SPT BLOW COUNTS BH-BGC11-47			
CLIENT: VICTORIA GOLD CORP.		PROJECT No. 0792-006	FIG No. G-03	REV. 0	

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.



Note: SPT 2 was frozen and is not considered.

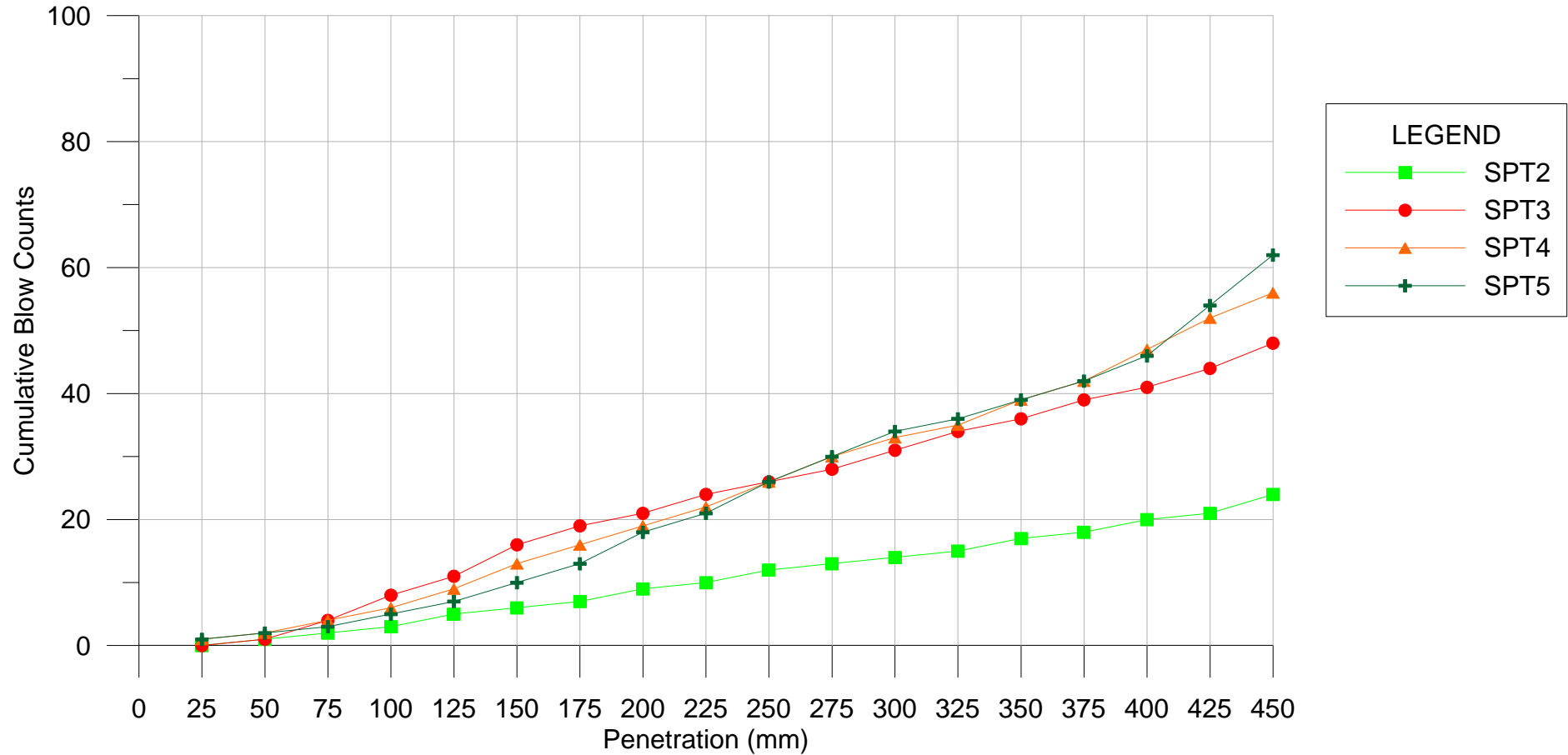
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		CLIENT: VICTORIA GOLD CORP.	PROJECT No. 0792-006	FIG No. G-04	REV. 0



Note: SPT3 and SPT 6 blow counts were recorded in six inch increments.
 Note: SPT7 had less than six inches of recovery and is not considered.
 Note: SPT8 was completed in frozen material and is not considered.

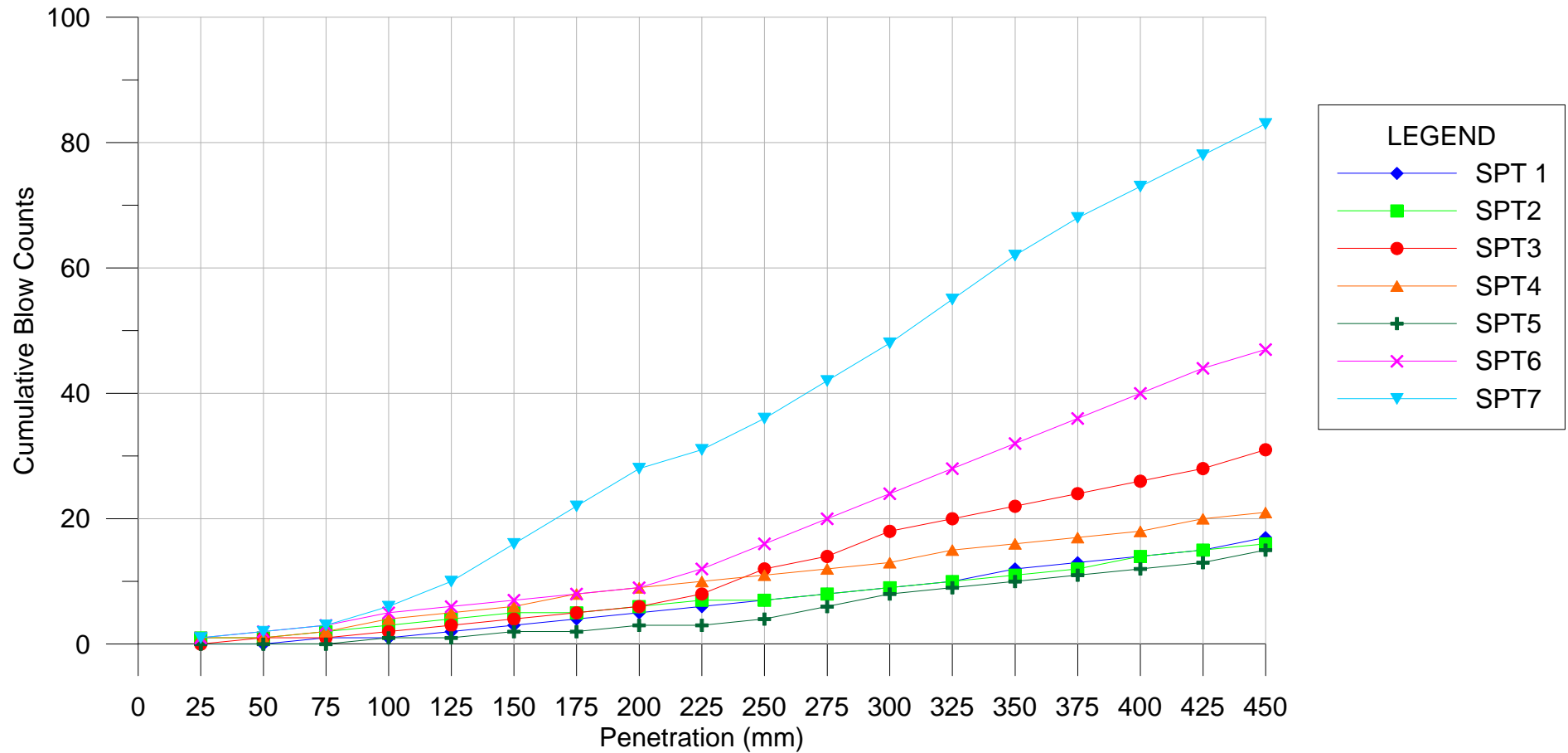
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		CLIENT: VICTORIA GOLD CORP.	PROJECT No. 0792-006	FIG No. G-05	REV. 0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.



Note: SPT 1 had less than six inches of recovery and is not considered.

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CLIENT: VICTORIA GOLD CORP.		PROJECT No. 0792-006	FIG No. G-06	REV. 0	




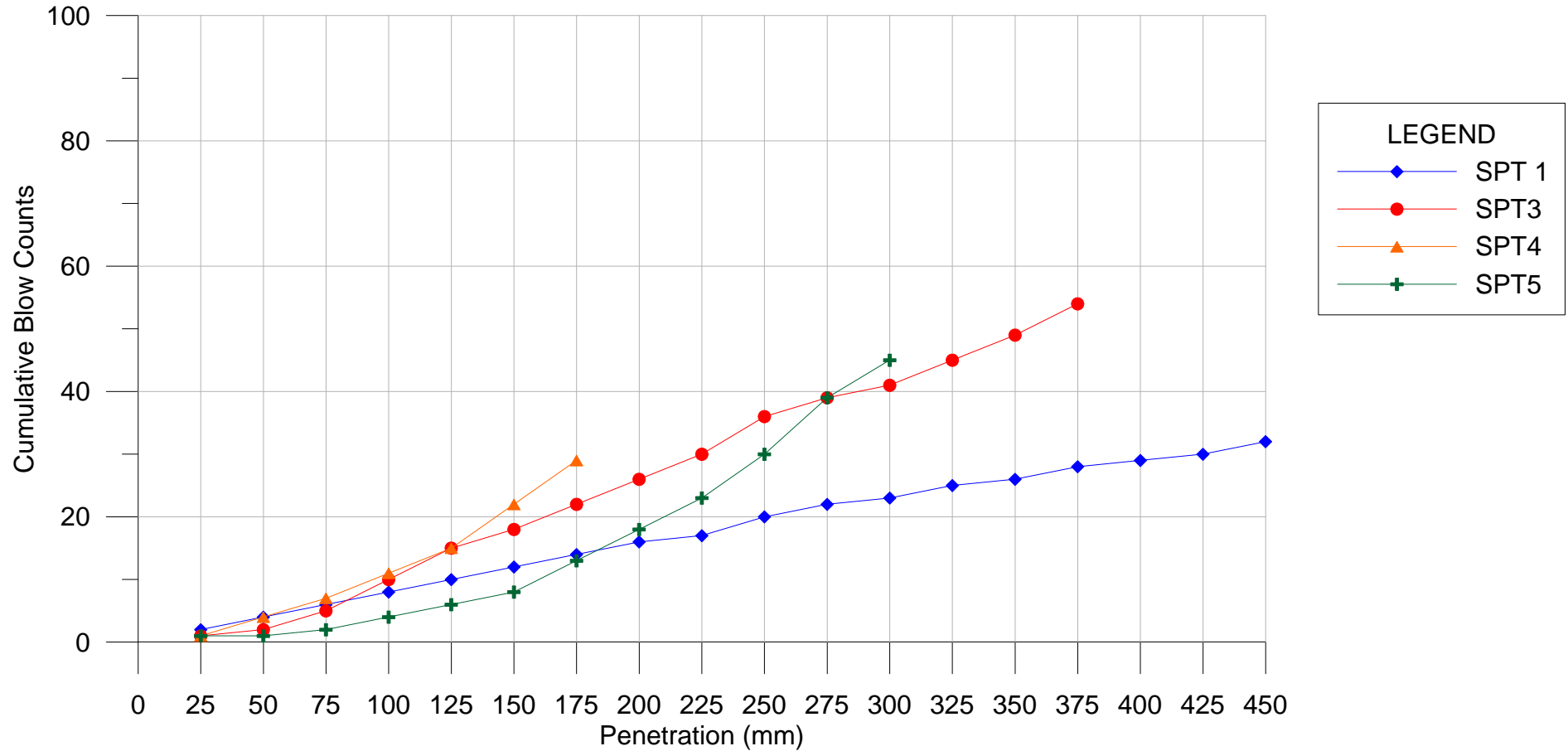
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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.



Note: SPT2 had no recovery and is not considered.
 Note: SPT6 had less than six inches of recovery and is not considered.


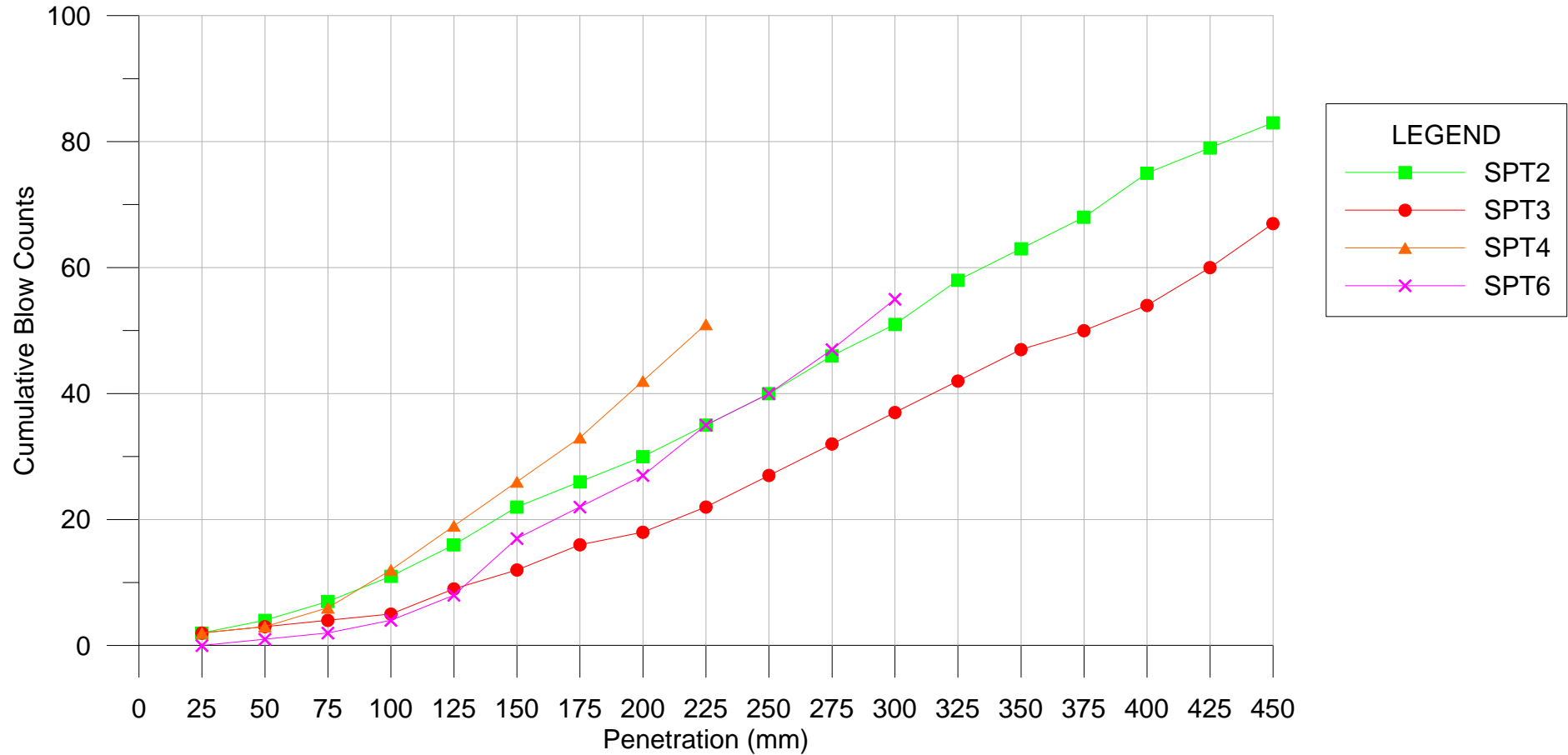
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CLIENT: VICTORIA GOLD CORP.		PROJECT No. 0792-006	FIG No. G-08	REV. 0	

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.



Note: SPT1 was frozen and is not considered.
 Note: SPT5 had less than six inches of penetration so is not considered.


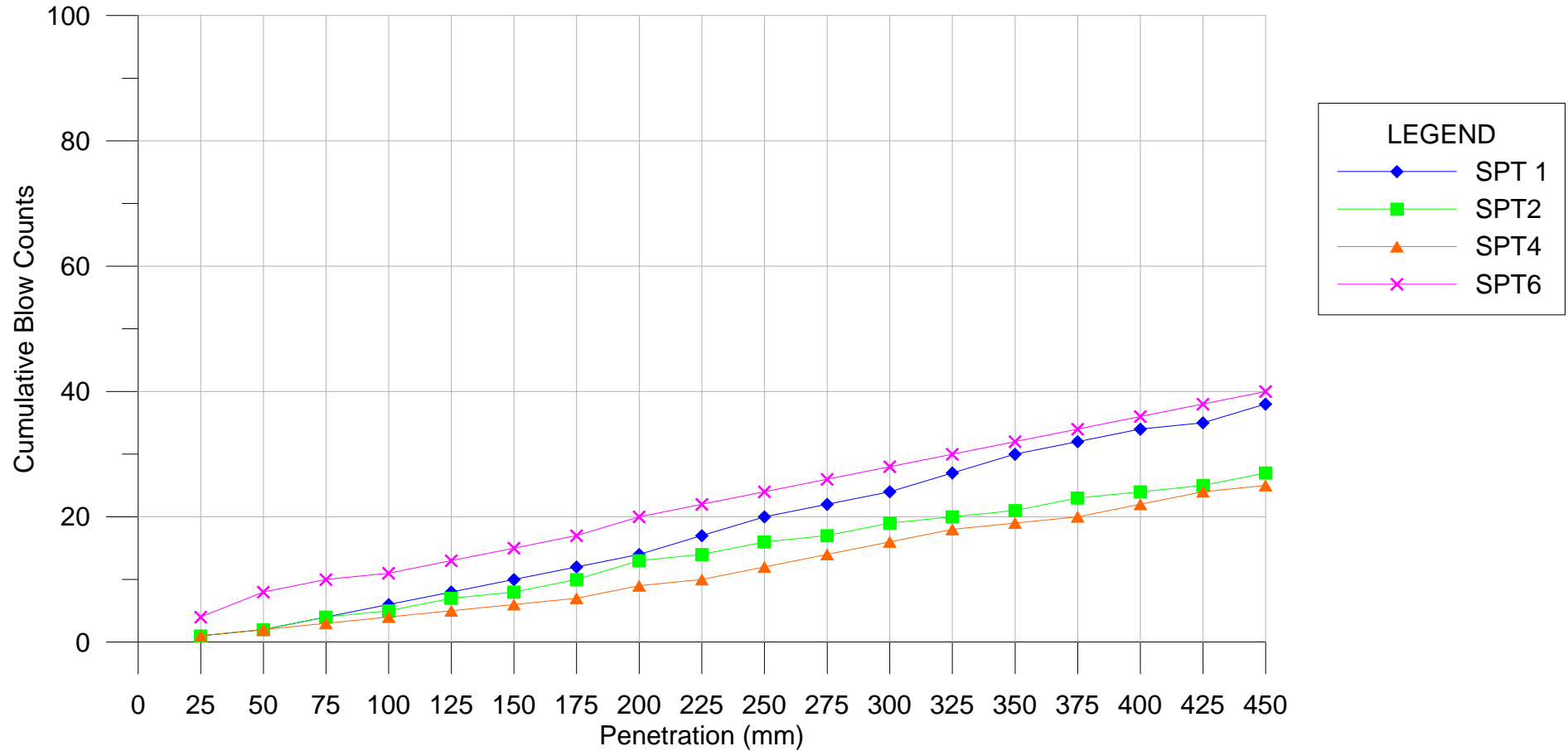
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CLIENT: VICTORIA GOLD CORP.		PROJECT No. 0792-006	FIG No. G-09	REV. 0	

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.



Note: SPT3 blow counts were recorded in six inch increments are not included in this plot.
 Note SPT 5, 7, 8 and 9 had less than six inches of recovery and are not considered.


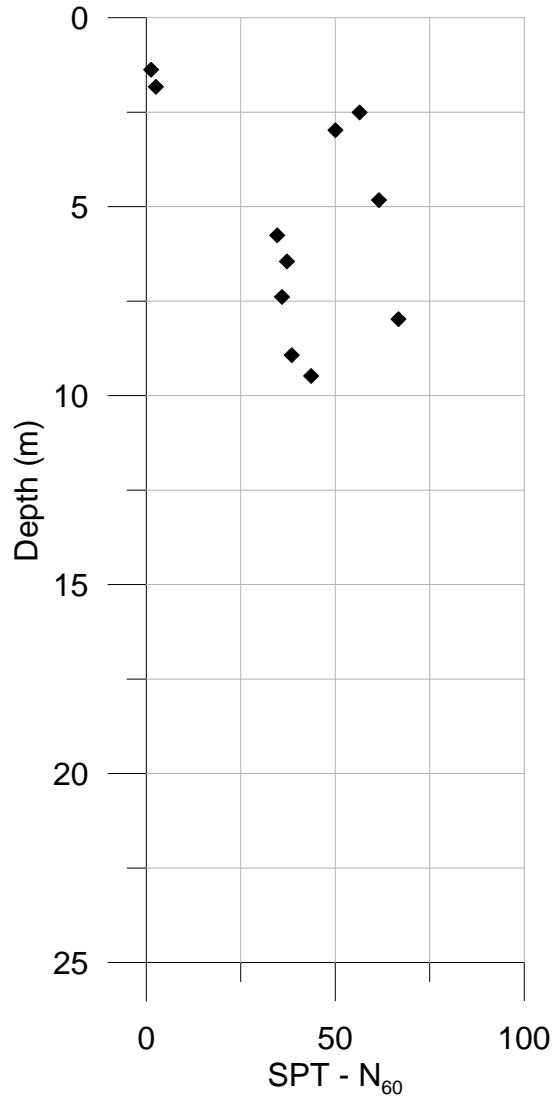
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CLIENT: VICTORIA GOLD CORP.	PROJECT No. 0792-006	FIG No. G-10	REV. 0		

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT". DATED JAN 2012.

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CLIENT: VICTORIA GOLD CORP.

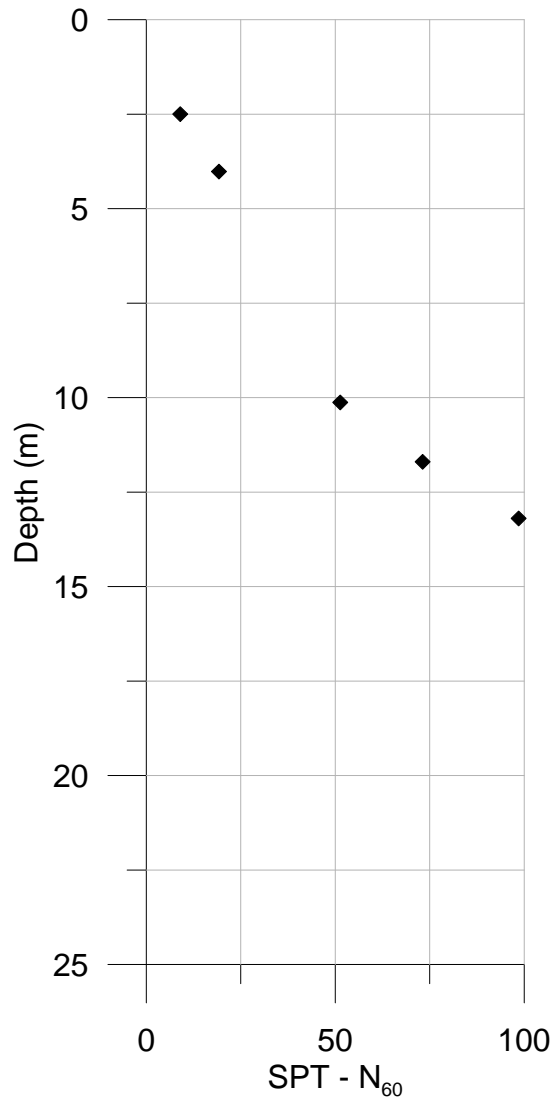
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TITLE: SPT N ₆₀ BH-BGC11-39		
PROJECT No.	FIG No.	REV.
0792-006	G-11	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

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DATE:	JAN 2012	CHECKED:	DW
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CLIENT: VICTORIA GOLD CORP.

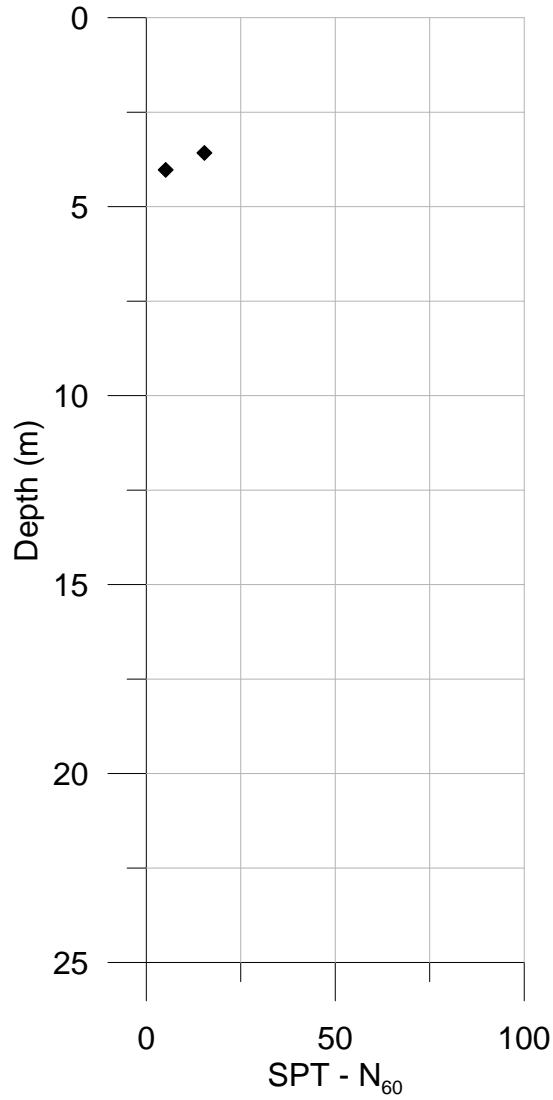
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TITLE: SPT N ₆₀ BH-BGC11-44		
PROJECT No.	FIG No.	REV.
0792-006	G-12	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

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CLIENT: VICTORIA GOLD CORP.

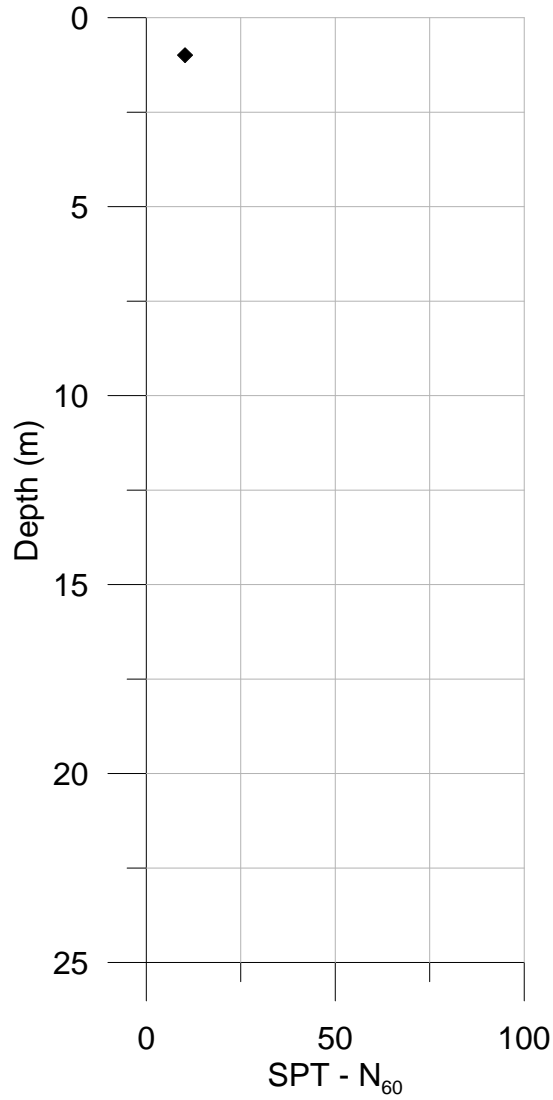
PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

TITLE: SPT N ₆₀ BH-BGC11-47		
PROJECT No.	FIG No.	REV.
0792-006	G-13	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

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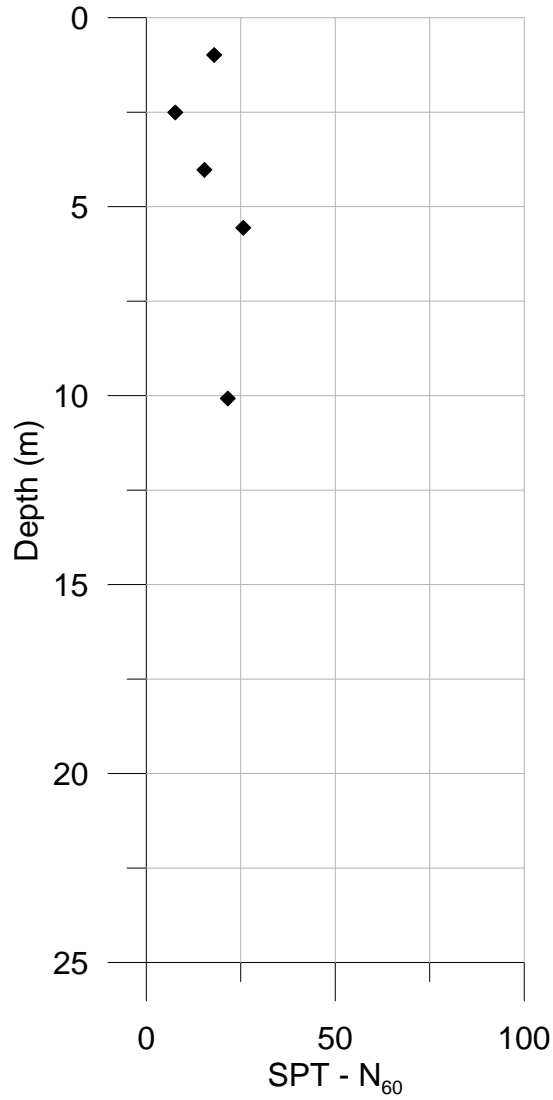
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

TITLE: SPT N ₆₀ BH-BGC11-49		
PROJECT No.	FIG No.	REV.
0792-006	G-14	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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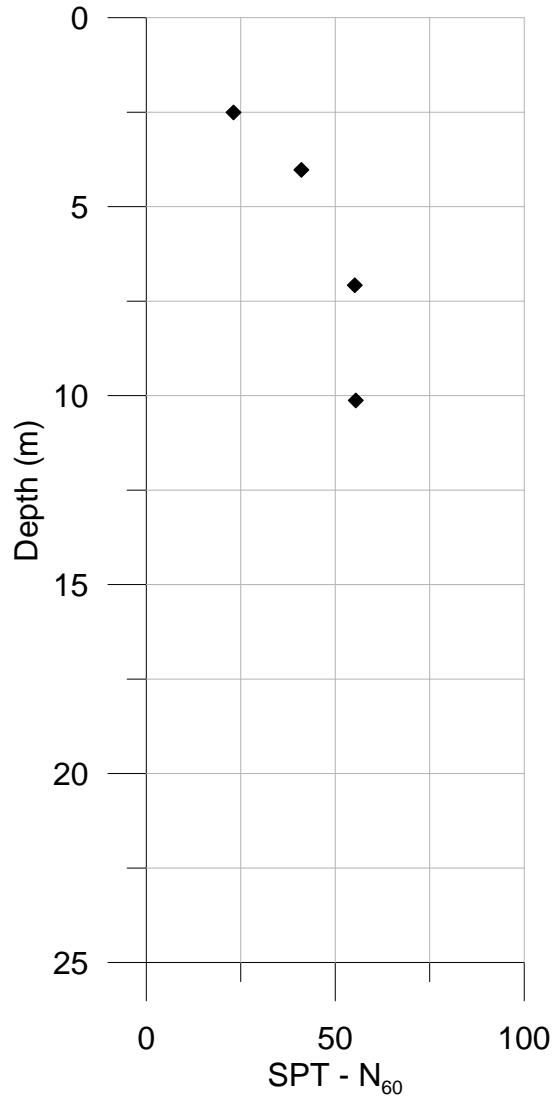
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
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TITLE: SPT N ₆₀ BH-BGC11-51		
PROJECT No.	FIG No.	REV.
0792-006	G-15	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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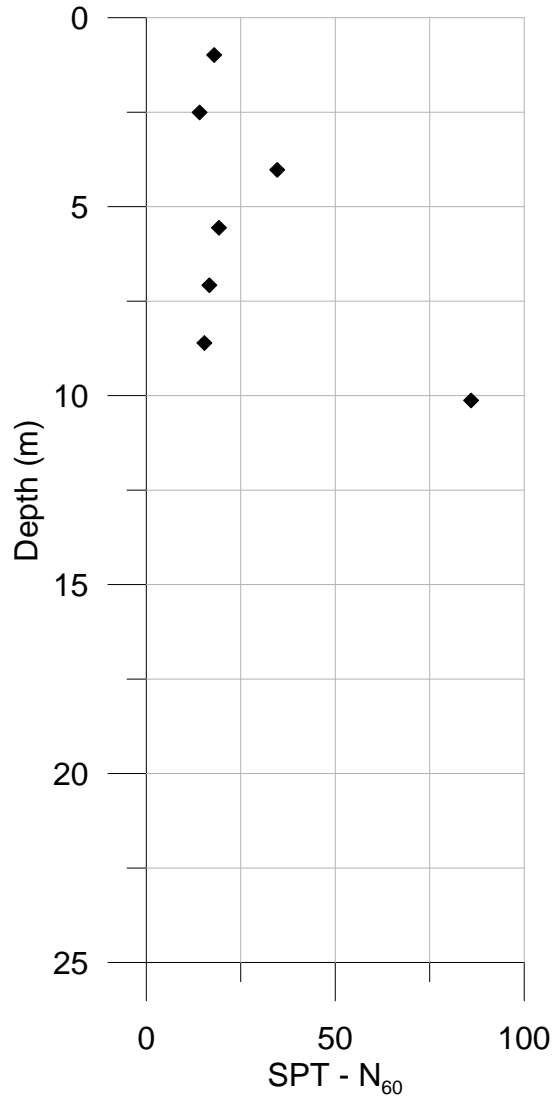
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
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TITLE: SPT N ₆₀ BH-BGC11-53		
PROJECT No.	FIG No.	REV.
0792-006	G-16	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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CLIENT: VICTORIA GOLD CORP.

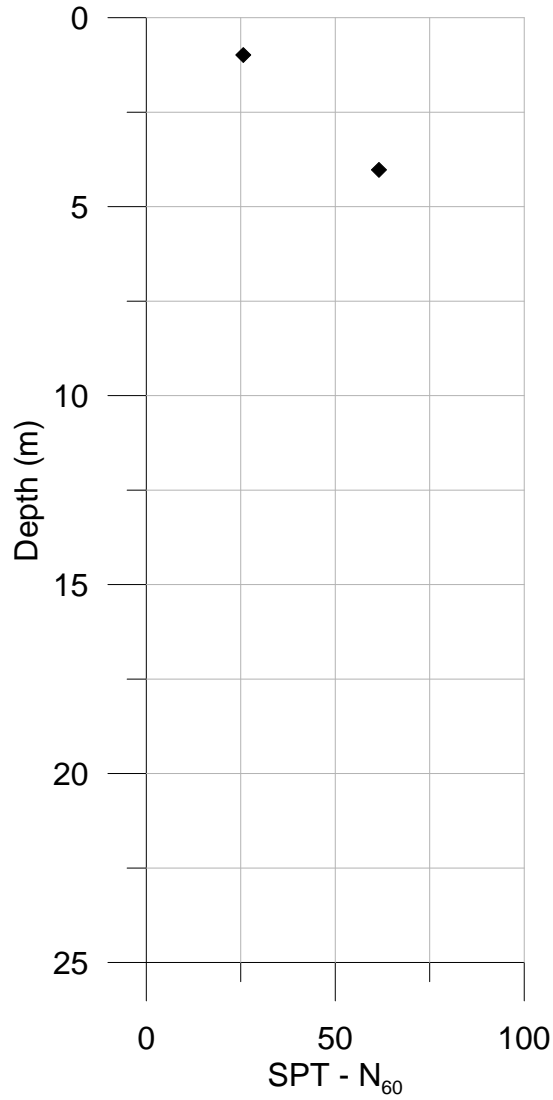
PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
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TITLE: SPT N ₆₀ BH-BGC11-55		
PROJECT No.	FIG No.	REV.
0792-006	G-17	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

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CLIENT: VICTORIA GOLD CORP.

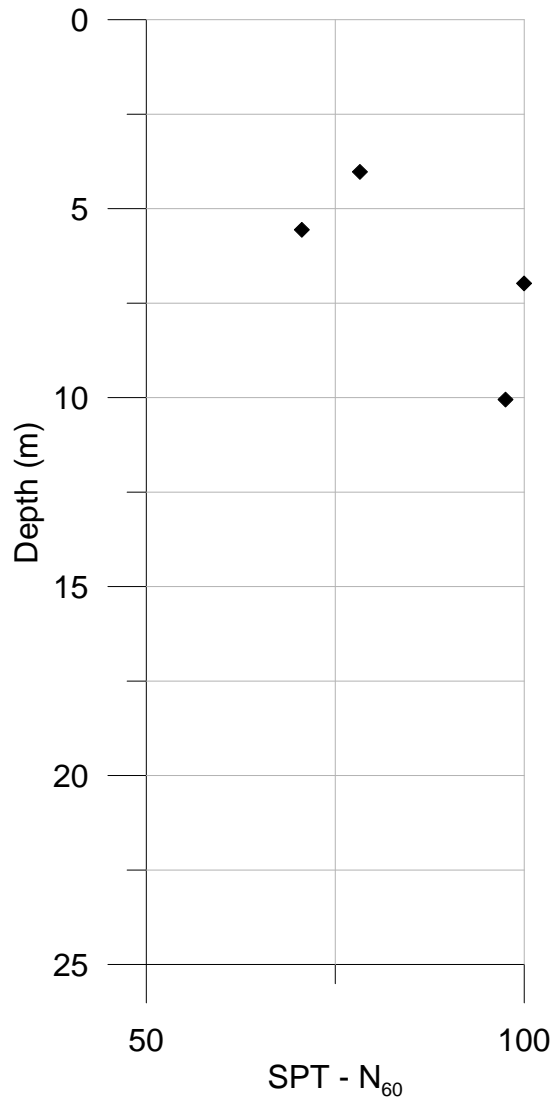
PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

TITLE: SPT N ₆₀ BH-BGC11-56		
PROJECT No.	FIG No.	REV.
0792-006	G-18	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

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Note: Calculated N₆₀ values exceeding 100 are truncated to 100.


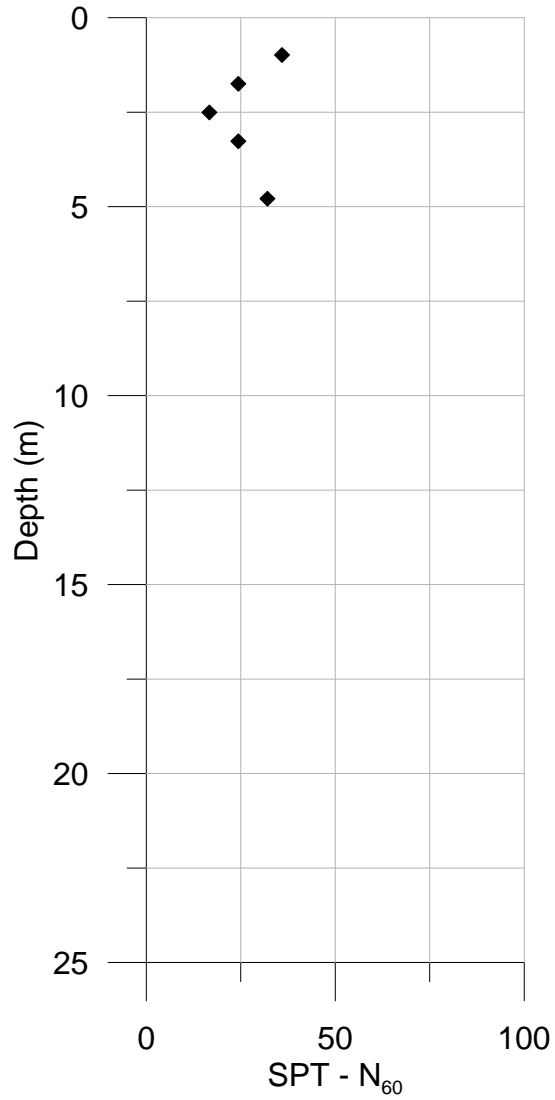
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CLIENT:		PROJECT:			
VICTORIA GOLD CORP.		2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT			
 BGC ENGINEERING INC. AN APPLIED EARTH SCIENCES COMPANY		TITLE:			
		SPT N ₆₀ BH-BGC11-57			
PROJECT No.		FIG No.		REV.	
0792-006		G-19		0	

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

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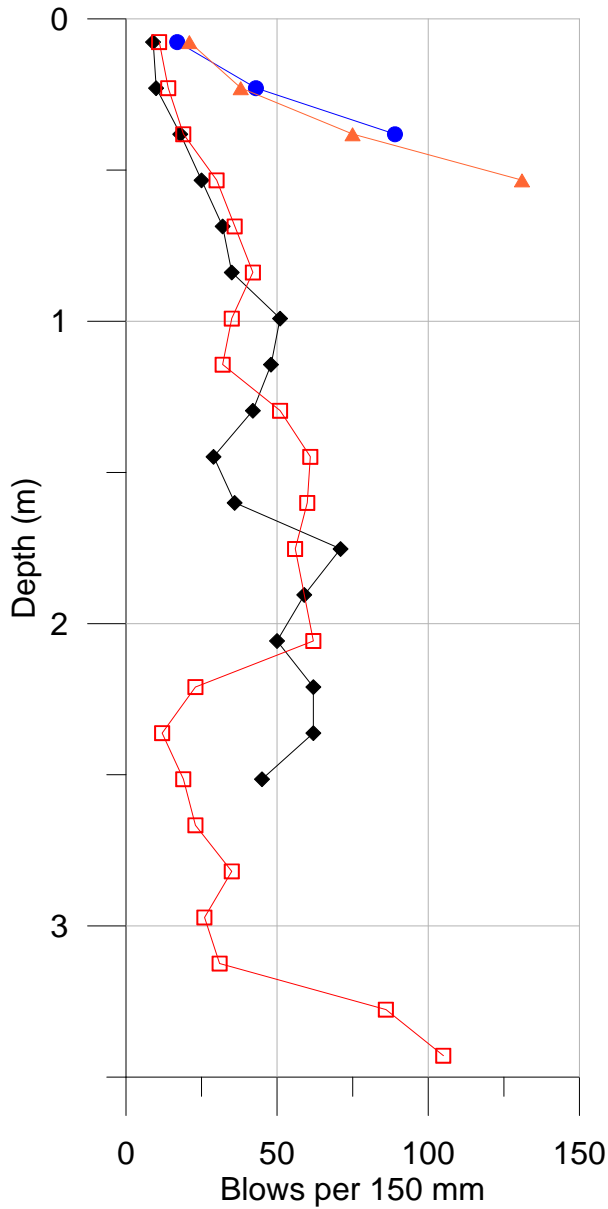
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

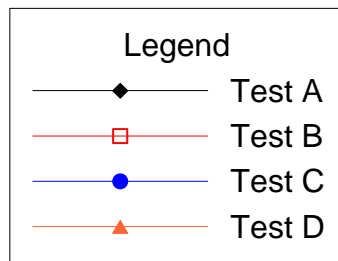
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TITLE: SPT N ₆₀ BH-BGC11-65		
PROJECT No.	FIG No.	REV.
0792-006	G-20	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



- Notes:
1. Tests conducted within 3 m of each other adjacent to BH-BGC11-67.
 2. Located adjacent to plate load test PT-BGC11-01 and BH-BGC11-69.



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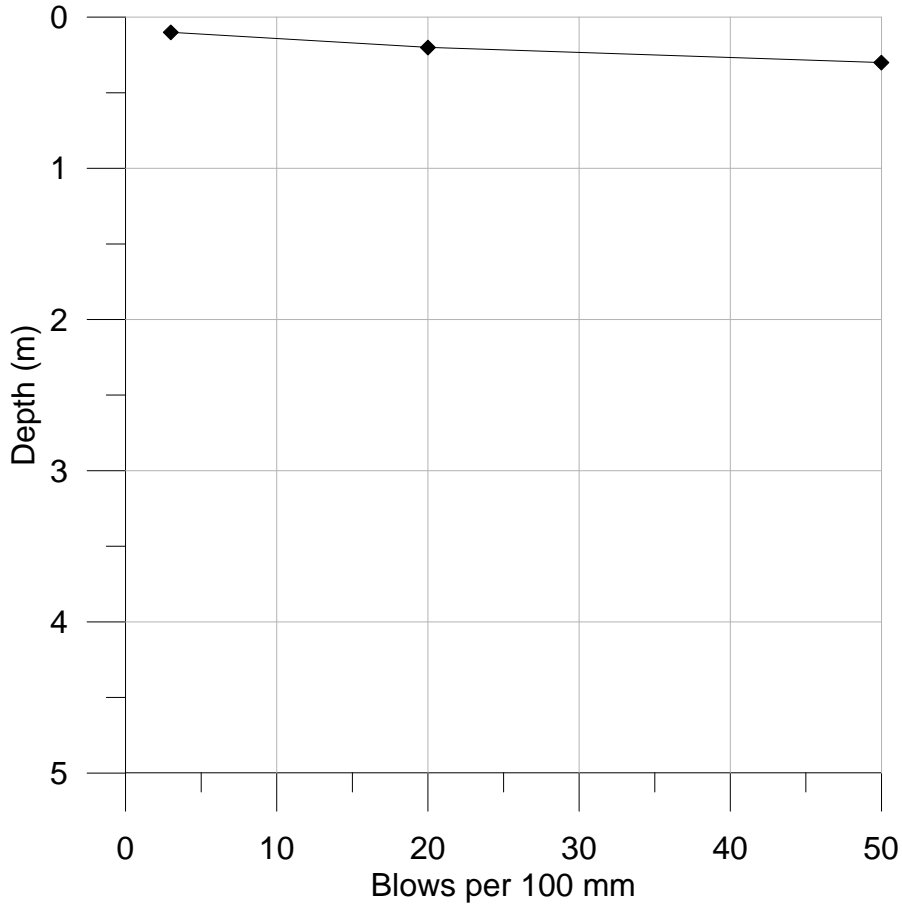
SCALE:	NTS	DESIGN:	KH
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DRAWN:	KH	APPROVED:	PQ

CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: DCPT TESTING AT BH-BGC11-67		
PROJECT No.	FIG No.	REV.
0792-006	G-21	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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CLIENT: VICTORIA GOLD CORP.

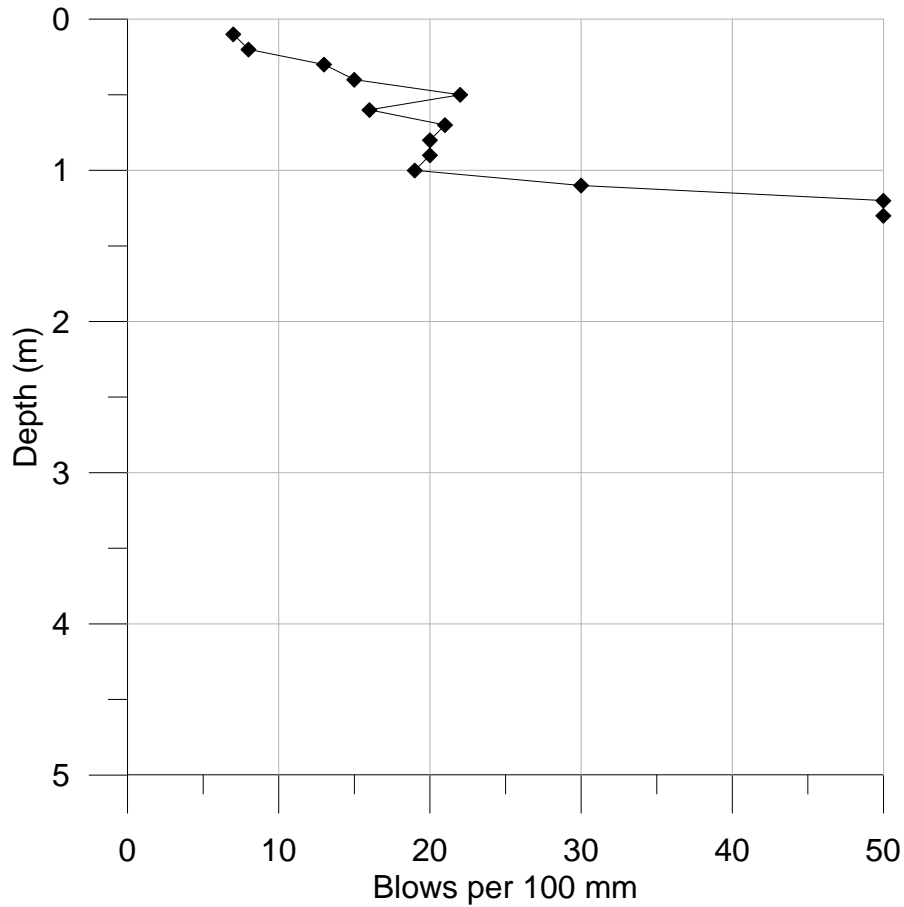
PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
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TITLE: Wildcat Penetrometer Testing near BH-BGC10-13		
PROJECT No.	FIG No.	REV.
0792-006	G-22	0

N:\BGC\Projects\0792_Victoria Gold\006 EG Infrastructure 2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix G Penetration Testing\WildCat\grapher.plots

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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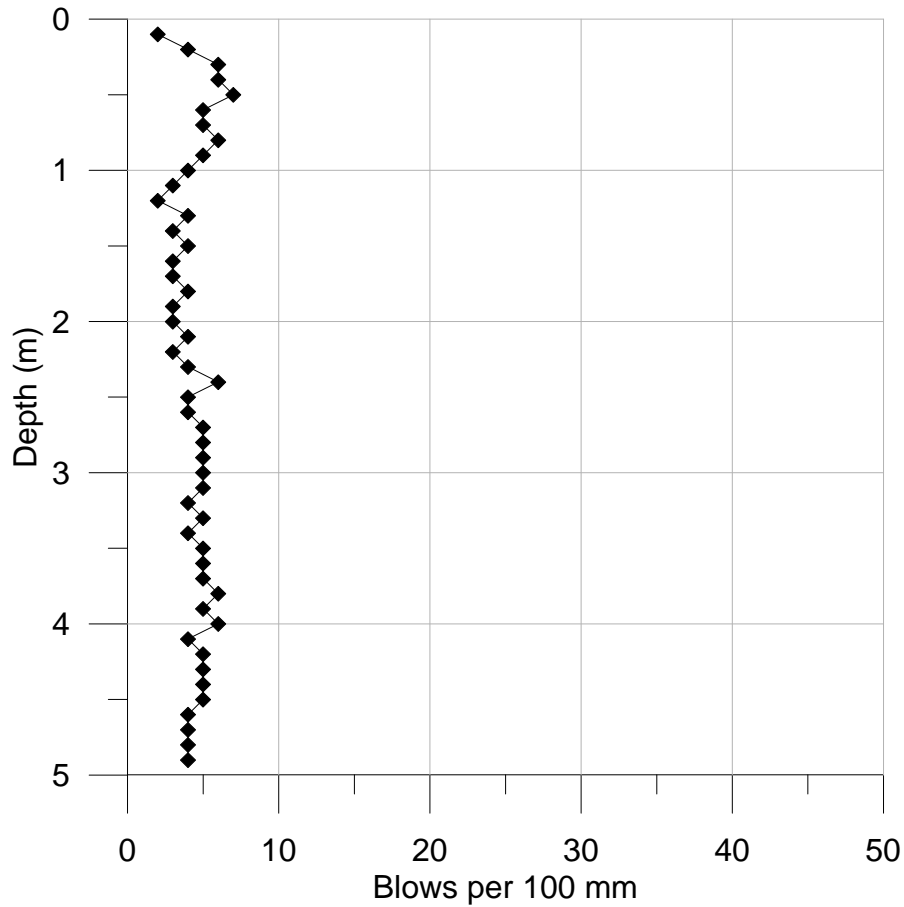
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: Wildcat Penetrometer Testing near BH-BGC10-14		
PROJECT No.	FIG No.	REV.
0792-006	G-23	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

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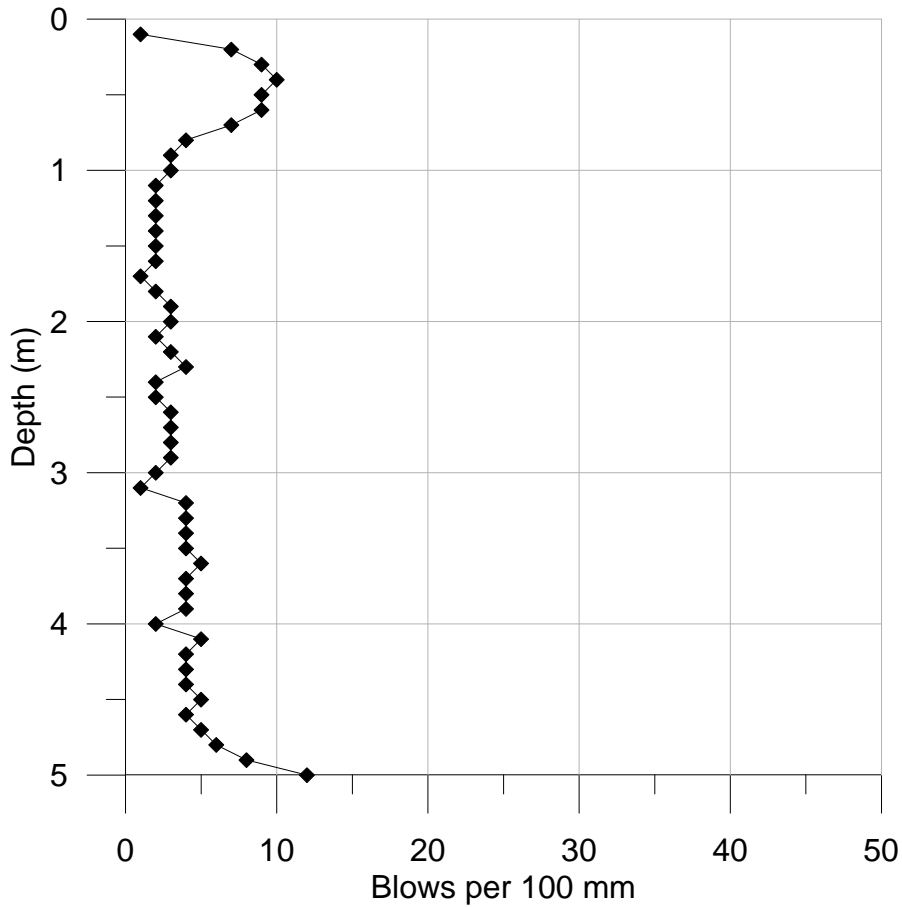
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
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TITLE: Wildcat Penetrometer Testing near BH-BGC10-22		
PROJECT No.	FIG No.	REV.
0792-006	G-24	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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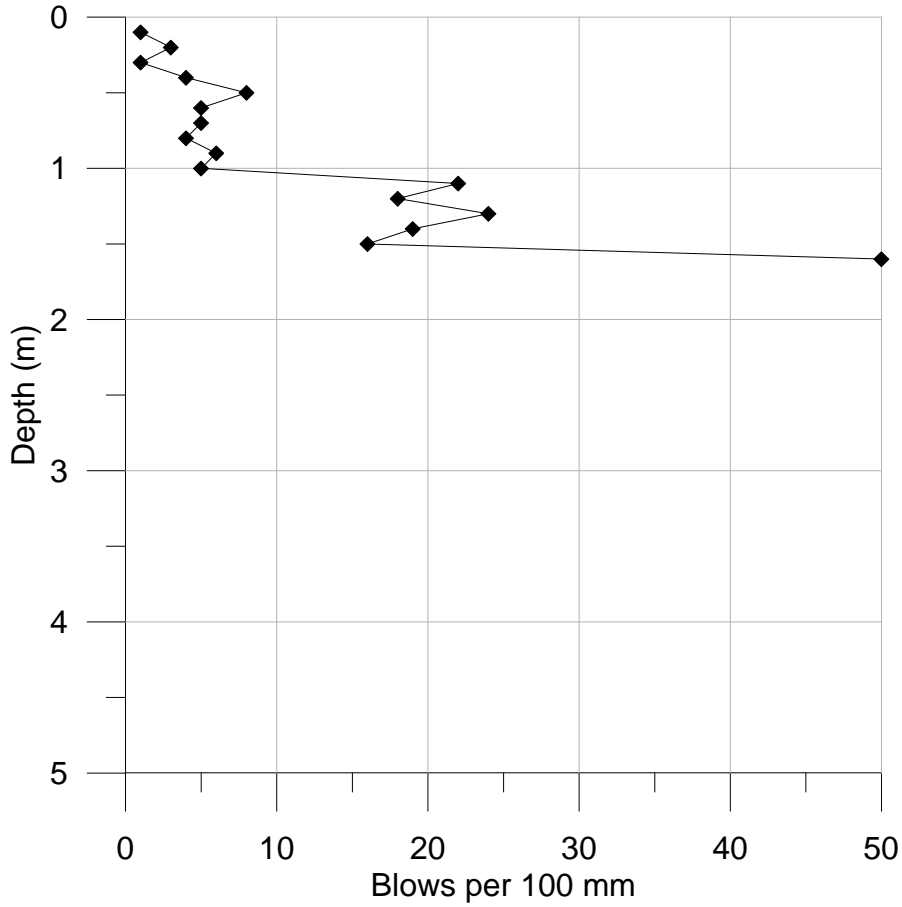
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

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TITLE: Wildcat Penetrometer Testing near BH-BGC10-24		
PROJECT No.	FIG No.	REV.
0792-006	G-25	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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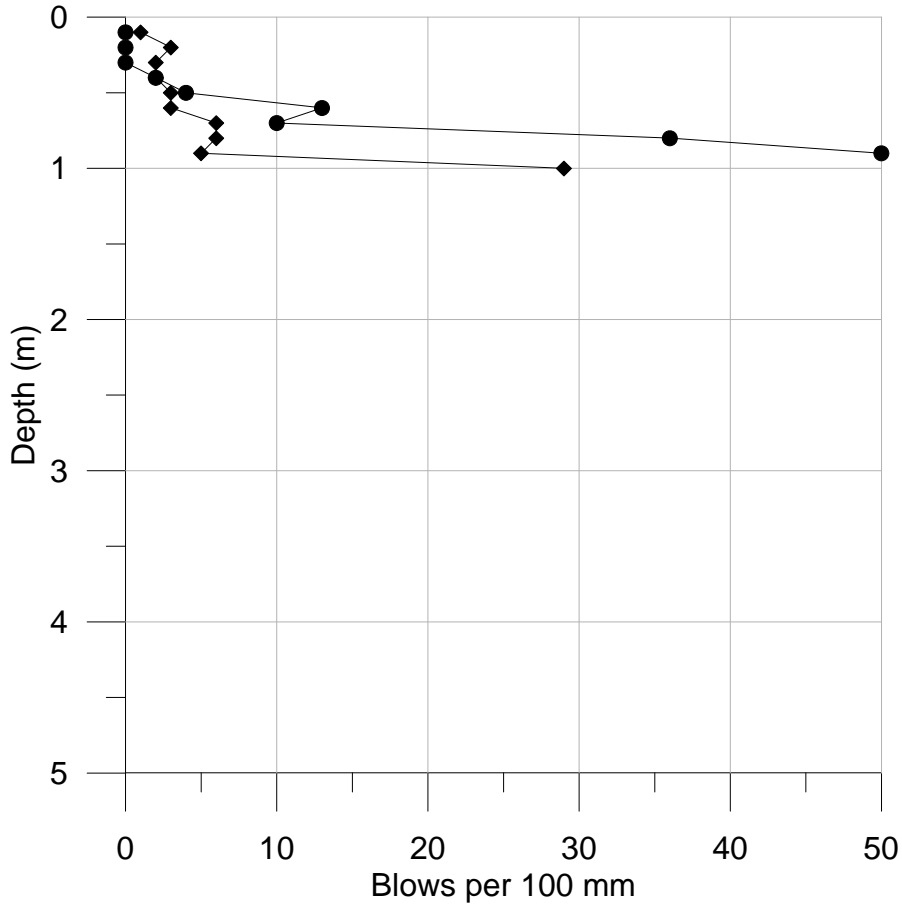
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CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: Wildcat Penetrometer Testing near BH-BGC11-53		
PROJECT No.	FIG No.	REV.
0792-006	G-26	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



LEGEND	
◆	BH-BGC11-55 (TEST A)
●	BH-BGC11-55 (TEST B)

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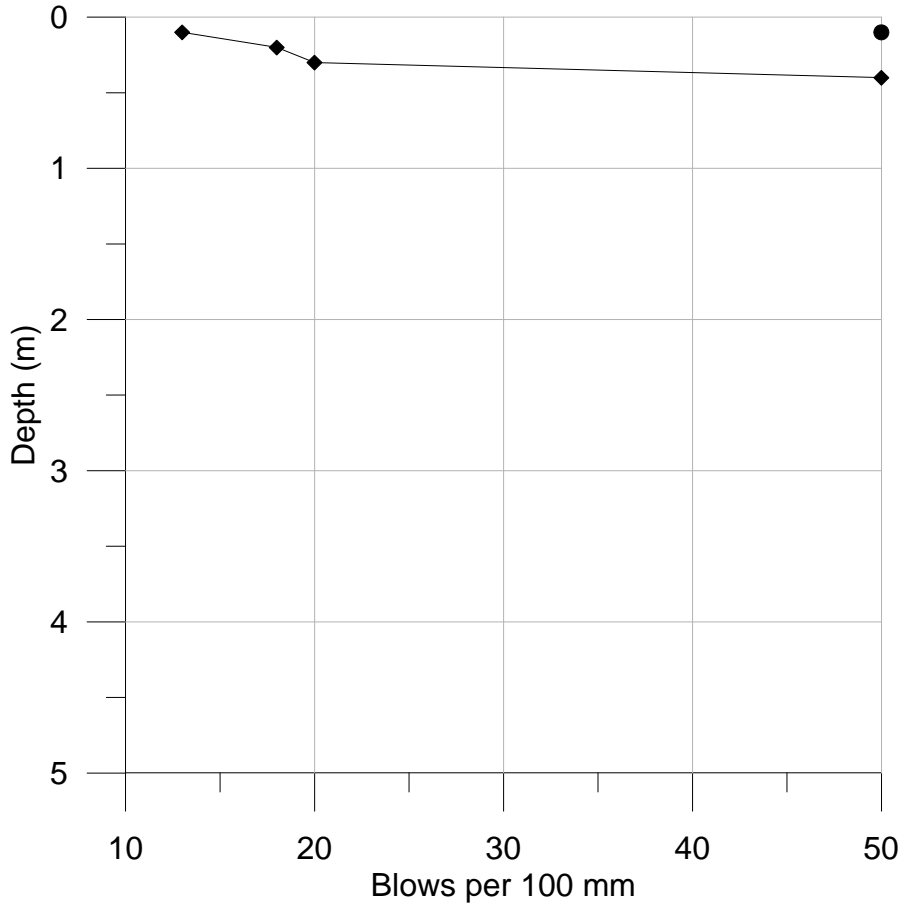
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CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: Wildcat Penetrometer Testing near BH-BGC11-55		
PROJECT No.	FIG No.	REV.
0792-006	G-27	0

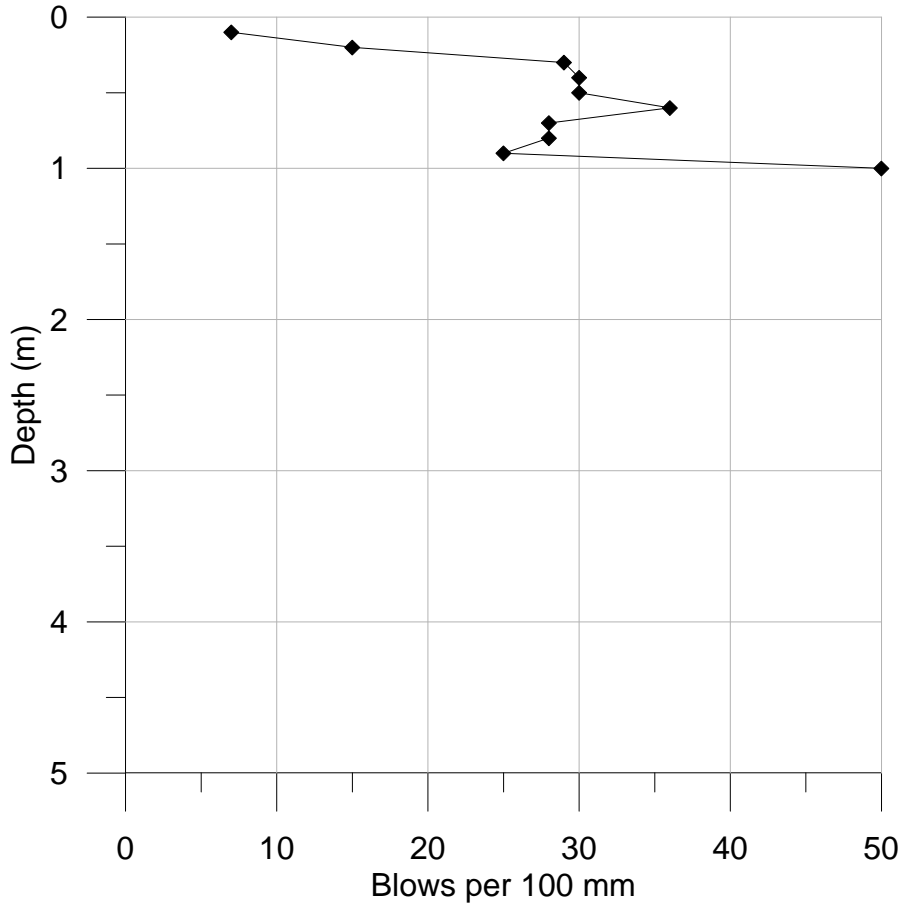
FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



LEGEND	
◆	TP-BGC11-103 (TEST A)
●	TP-BGC11-103 (TEST B)

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		DATE:	JAN 2012	CHECKED:	DW
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CLIENT:		PROJECT:			
VICTORIA GOLD CORP.		2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT			
		TITLE:			
		Wildcat Penetrometer Testing near TP-BGC11-103			
		PROJECT No.	0792-006	FIG No.	G-32
		REV.	0		

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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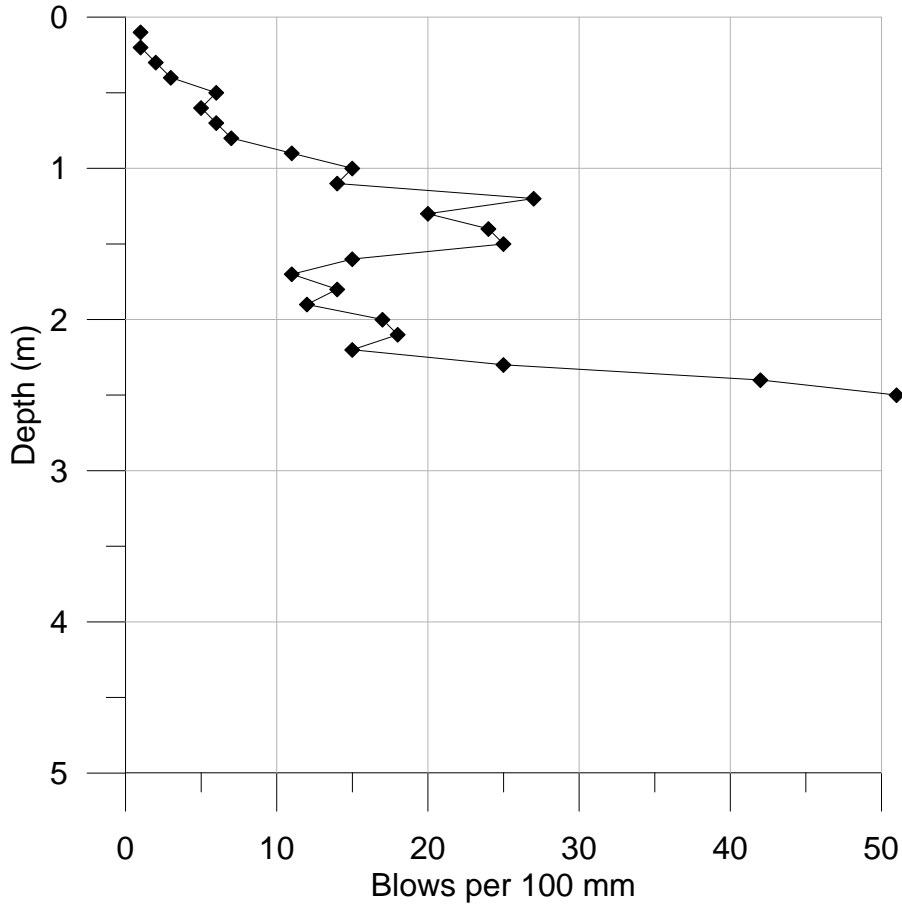
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TITLE: Wildcat Penetrometer Testing near TP-BGC11-105		
PROJECT No.	FIG No.	REV.
0792-006	G-33	0

J:\0863 Cerro Casale\0863-003 Plant\Site Reporting\REV A Plant Site Report\3. Appendices\1. Settlement Calculations\Settlement-scenarios.pdf

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



N:\BGC\Projects\0792_Victoria Gold\006 EG Infrastructure 2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix G Penetration Testing\Wildcat\grapher.plots

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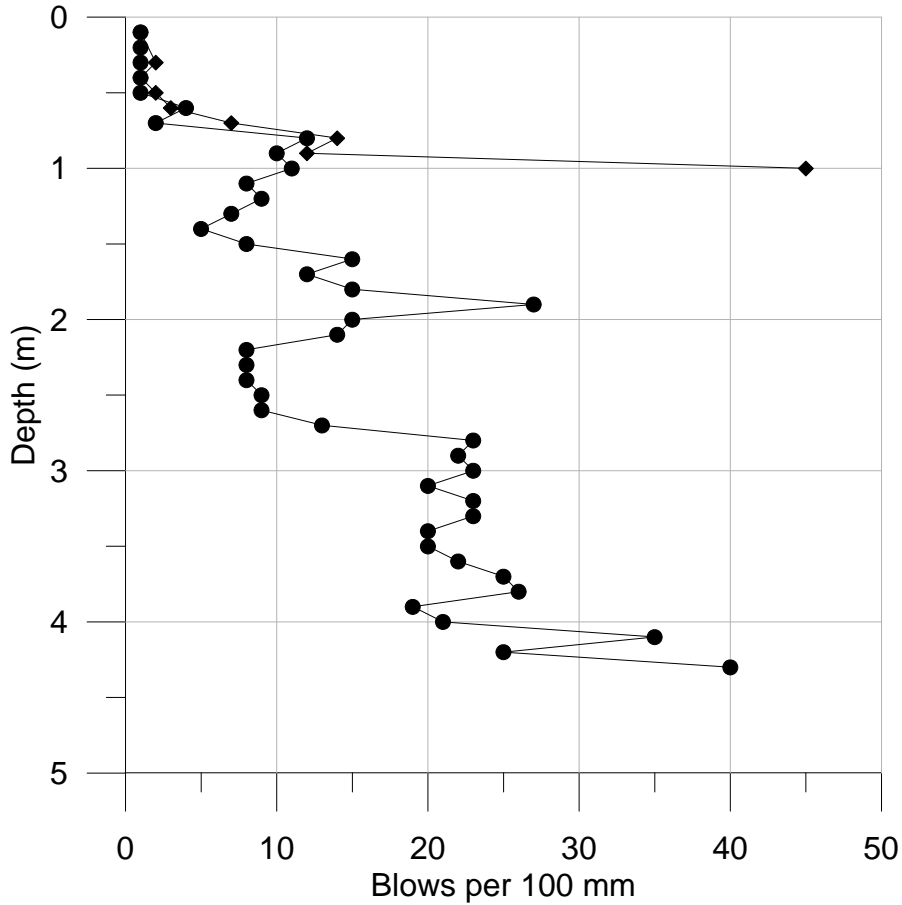
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PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

BGC ENGINEERING INC.
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TITLE: Wildcat Penetrometer Testing near TP-BGC11-62		
PROJECT No.	FIG No.	REV.
0792-006	G-28	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



LEGEND	
◆	TP-BGC11-69 (TEST A)
●	TP-BGC11-69 (TEST B)

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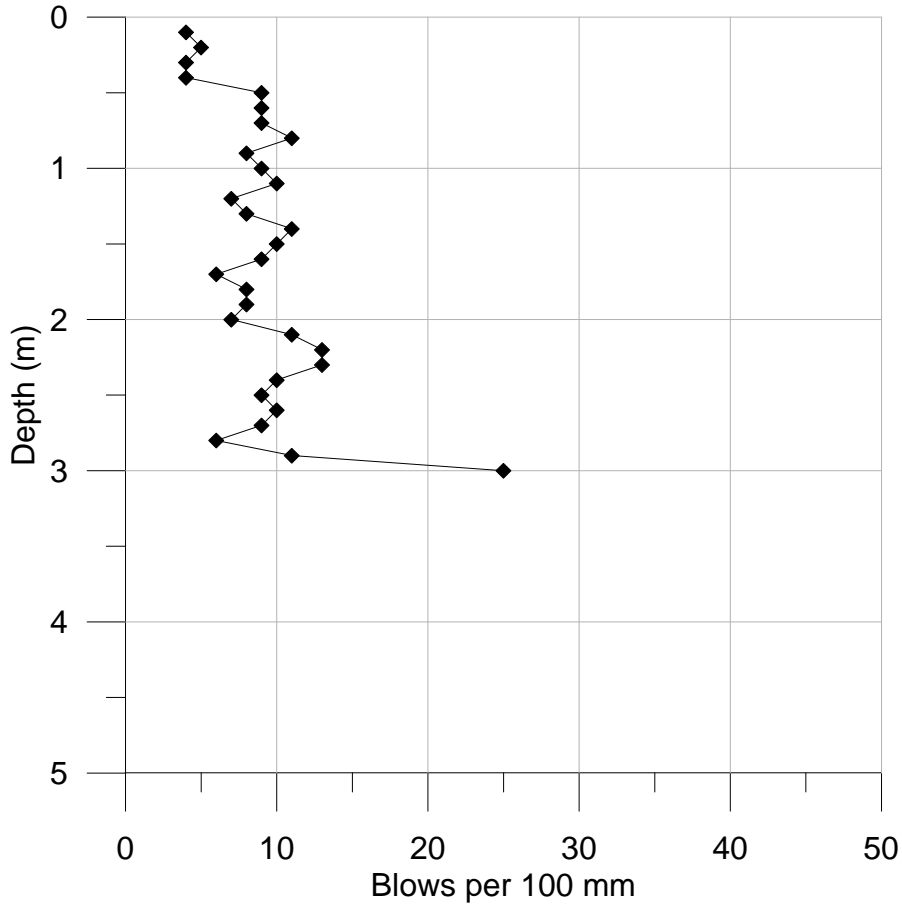
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TITLE: Wildcat Penetrometer Testing near TP-BGC11-69		
PROJECT No. 0792-006	FIG No. G-29	REV. 0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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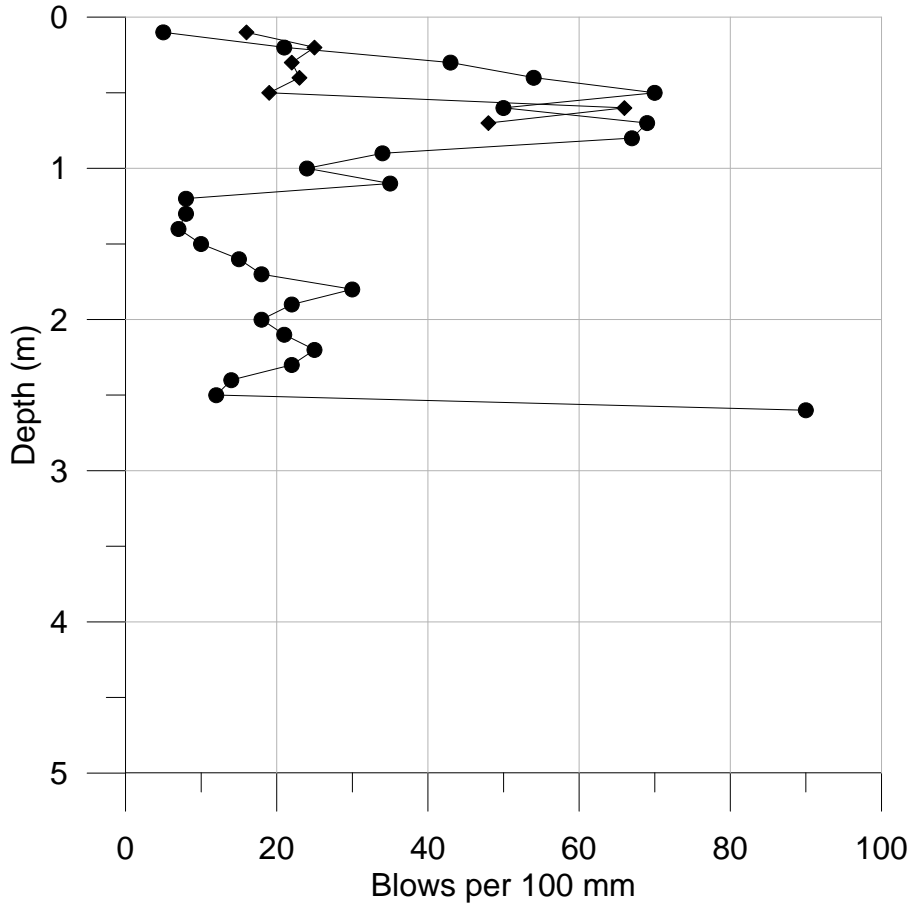
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PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: Wildcat Penetrometer Testing near TP-BGC11-90		
PROJECT No.	FIG No.	REV.
0792-006	G-30	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

Note:
Frozen ground present between 0.1 m to 2.0 m.



LEGEND	
◆	TP-BGC11-94 (TEST A)
●	TP-BGC11-94 (TEST B)

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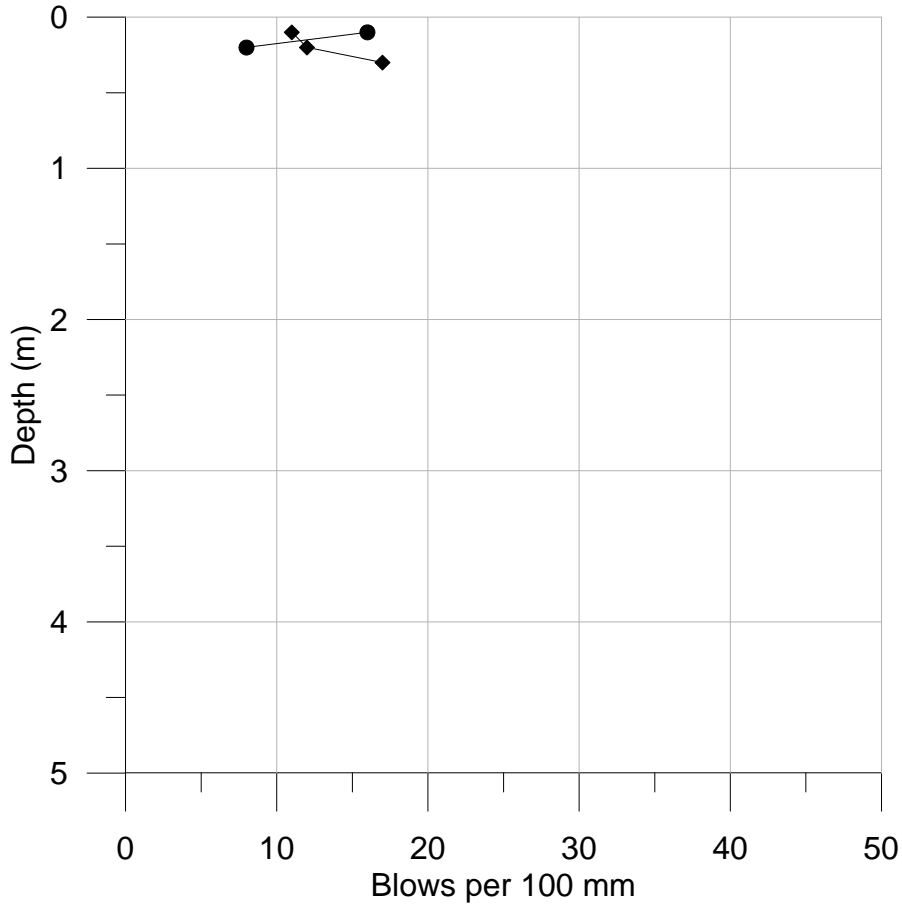
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PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: Wildcat Penetrometer Testing near TP-BGC11-94		
PROJECT No.	FIG No.	REV.
0792-006	G-31	0

N:\BGC\Projects\0792_Victoria Gold\006 EG Infrastructure 2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix G Penetration Testing\Wildcat\grapher.plots

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



LEGEND	
◆ — ◆ — ◆	Placer Tailings (PQ12-A)
● — ● — ●	Placer Tailings (PQ12-B)

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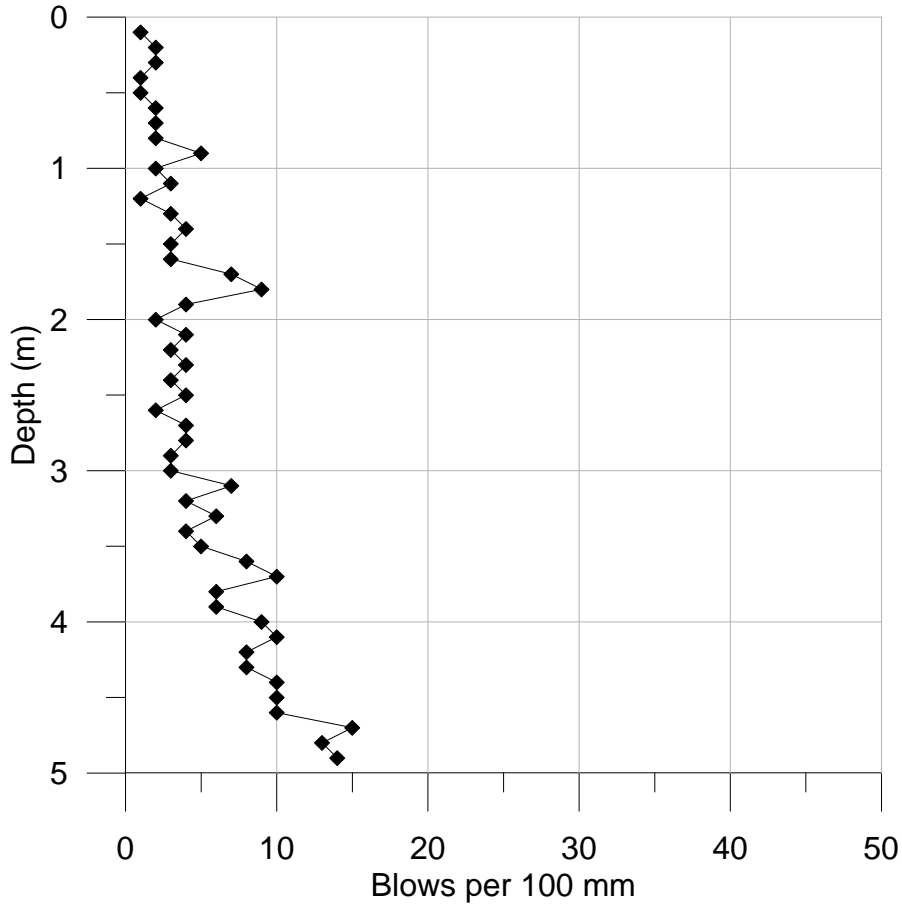
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TITLE: Wildcat Penetrometer Testing Placer Tailings (PQ12)		
PROJECT No.	FIG No.	REV.
0792-006	G-36	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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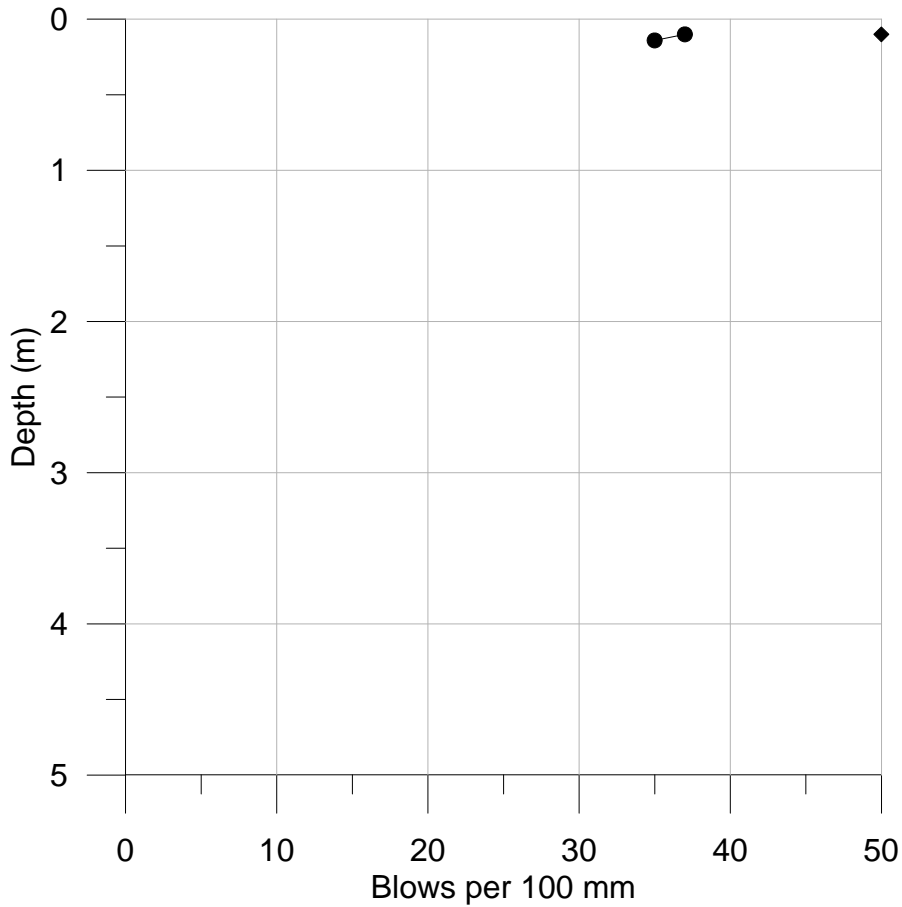
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TITLE: Wildcat Penetrometer Testing near Placer Tailings (PQ13)		
PROJECT No.	FIG No.	REV.
0792-006	G-37	0

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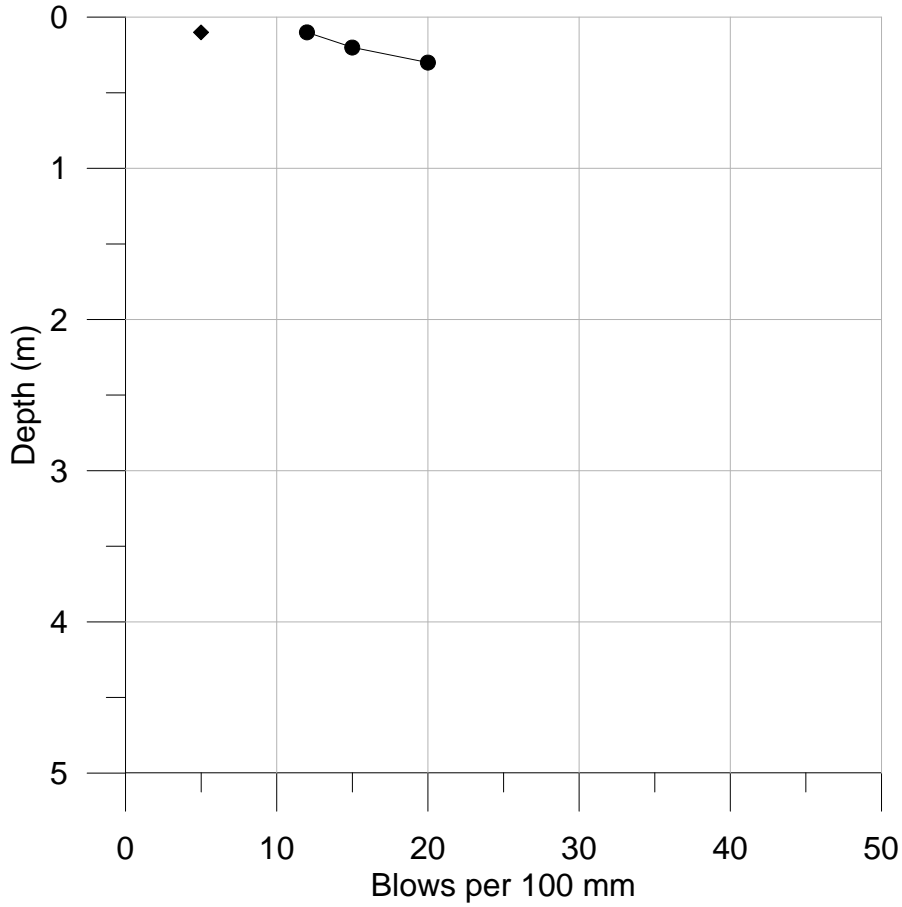
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LEGEND	
◆	Placer Tailings (PQ6-A)
●	Placer Tailings (PQ6-B)

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VICTORIA GOLD CORP.		2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT			
		TITLE:			
		Wildcat Penetrometer Testing Placer Tailings (PQ6)			
		PROJECT No.	0792-006	FIG No.	G-34
				REV.	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



LEGEND	
◆ — ◆ — ◆	Placer Tailings (PQ7-A)
● — ● — ●	Placer Tailings (PQ7-B)

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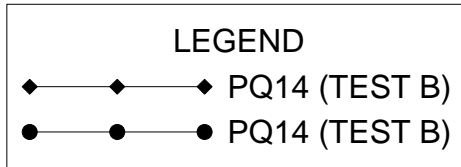
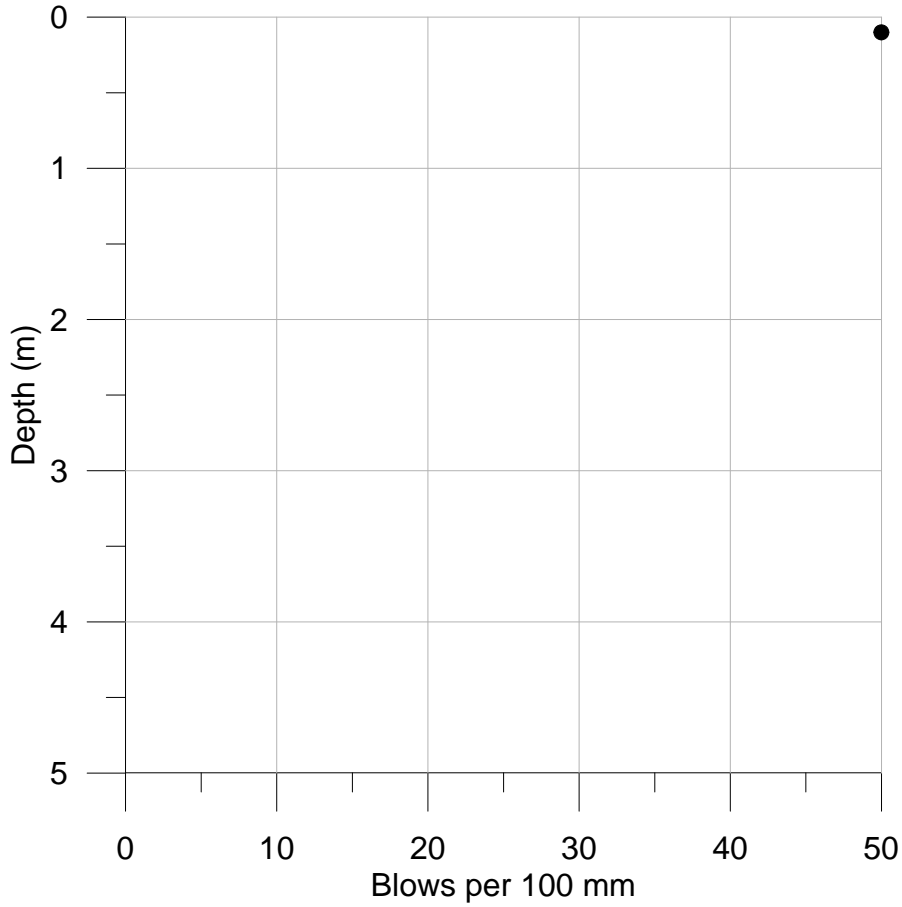
PROJECT: **2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT**

BGC ENGINEERING INC.

AN APPLIED EARTH SCIENCES COMPANY

TITLE: Wildcat Penetrometer Testing Placer Tailings (PQ7)		
PROJECT No.	FIG No.	REV.
0792-006	G-35	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



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CLIENT: VICTORIA GOLD CORP.

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TITLE: Wildcat Penetrometer Testing PQ14		
PROJECT No.	FIG No.	REV.
0792-006	G-38	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATIONS FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

APPENDIX H GEOPHYSICS

BGC ENGINEERING INC.
REPORT ON
SEISMIC REFRACTION AND
DOWNHOLE SEISMIC INVESTIGATION
PROPOSED MINE SITE FACILITIES
EAGLE GOLD PROJECT
YUKON

by

Alex Smith, M.Sc.

Russell Hillman, P.Eng.

September, 2011

PROJECT FGI-1216

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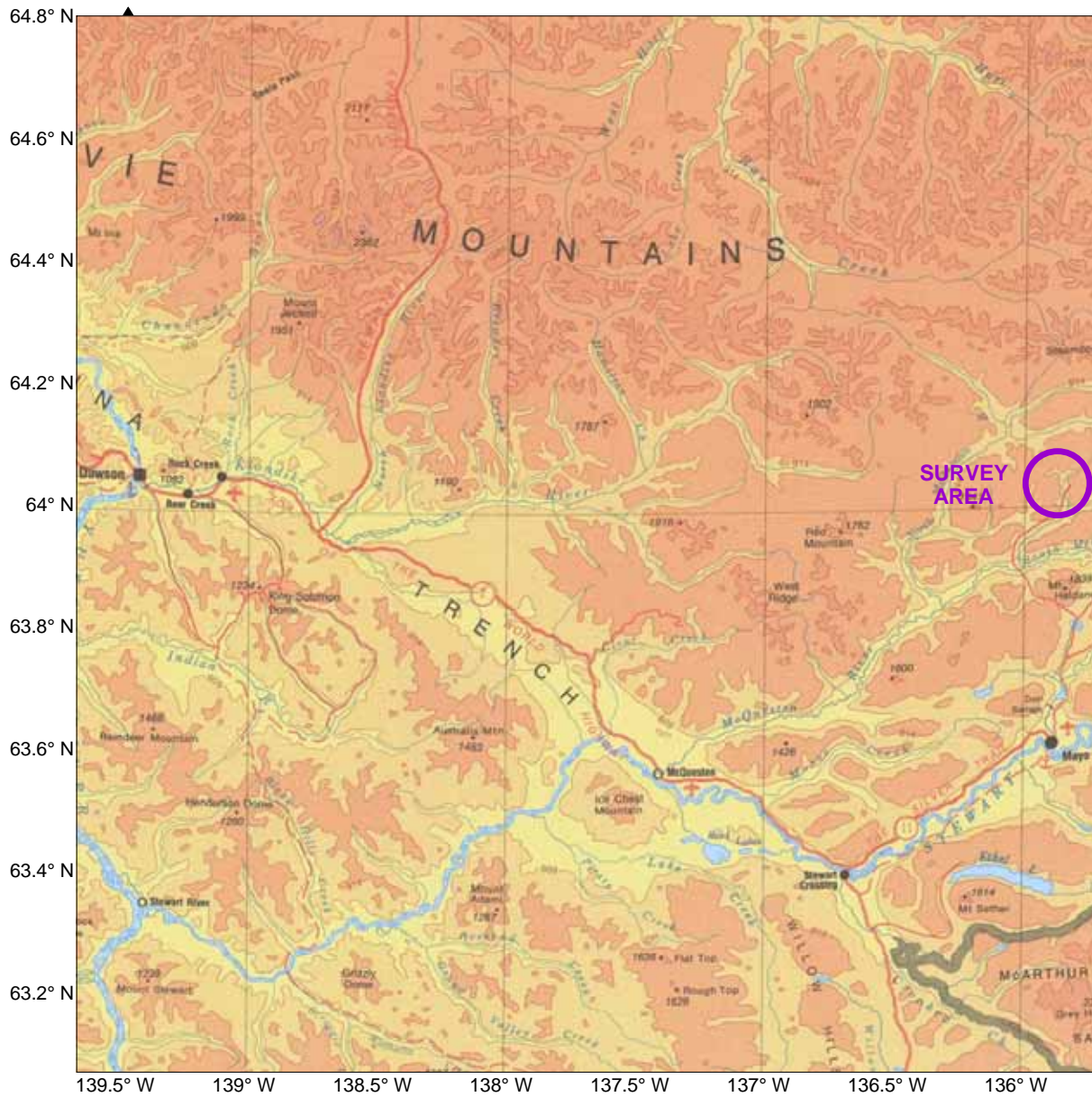
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1. INTRODUCTION

During the period August 24 to September 8, 2011, Frontier Geosciences Inc. carried out a seismic refraction and downhole compressional and shear wave investigation for BGC Engineering Inc. at Victoria Gold Corporation's Eagle Gold Project in the Tintina Gold Belt, central Yukon. The site is located approximately 50 kilometres north of the town of Mayo, at the intersection of Dublin Gulch and Haggart Creek. A Survey Location Plan of the area is shown at a scale of 1:1,250,000 in Figure 1.

The purpose of the seismic refraction and downhole surveys was to determine geological conditions at the proposed Heap Leach Embankment, Events Ponds and Plant Site Area. In addition, shear and compressional downhole surveys were conducted in three boreholes in the vicinity of the proposed Crusher site. A Site Plan of the survey area is presented at 1:10,000 scale in Figure 2 in the Appendix.

In all, 10 separate seismic traverses and 11 downhole compressional and shear wave surveys were completed at the proposed mine site. A total of approximately 5335 metres of detailed seismic refraction surveying was carried out in the investigation on 51 separate seismic spreads.



BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
SURVEY LOCATION PLAN		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	SCALE 1:1,250,000	FIG. 1

2. THE SEISMIC REFRACTION SURVEY METHOD

2.1 Equipment

The seismic refraction investigation was carried out using a Geometrics, Geode, 24 channel, signal enhancement seismograph and Mark Products Ltd., 48 Hz geophones. Geophone intervals along the multi-cored seismic cables were maintained at 5 metres in order to produce high resolution data on subsurface layering as well as to ensure adequate coverage of the basal layer. The zero delay or instantaneous blasting caps in the small explosive charges used for energy input, were detonated electrically with a Geometrics, HVB-1, high voltage, capacitor type blaster.

2.2 Survey Procedure

For each spread, the seismic cable was stretched out in a straight line and the geophones implanted. Six separate 'shots' were then initiated: one at either end of the geophone array, two at intermediate locations along the seismic cable, and one off each end of the line to ensure adequate coverage of the basal layer. The shots were detonated individually and arrival times for each geophone were recorded digitally in the seismograph. Data recorded during field surveying operations was generally of good to excellent quality.

Throughout the survey, notes were recorded regarding seismic line positions in relation to topographic and geological features, and survey stations in the area. Relative elevations on the seismic lines were recorded by chain and inclinometer with absolute elevations taken from a digital elevation model provided by BGC Engineering Inc.

2.3 Interpretive Method

The final interpretation of the seismic data was arrived at using the method of differences technique. This method utilises the time taken to travel to a geophone from shotpoints located to either side of the geophone. Using the total time, a small vertical time is computed which represents the time taken to travel from the refractor up to the ground surface. This time is then multiplied by the velocity of each overburden layer to obtain the thickness of each layer at that point.

3. THE DOWNHOLE SEISMIC SURVEY

3.1 Equipment

The downhole seismic survey was carried out with the Geometrics, Geode, 24 channel, signal enhancement seismograph which was also used for digital storage of the data. The receiver package employed was a Sercel GS14-L3 triaxial, downhole geophone package. The two horizontal geophones in the package are optimal for recording the down-going, horizontally polarised, shear wave generated at the ground surface. The package was held against the casing wall by a spring steel carrier which was acoustically decoupled from the suspending cable and the weight that draws the carrier down the drillhole.

The shear wave source was a large wooden beam approximately 2 metres in length which was secured against the ground for good shear wave transmission. Force was applied to both ends of the beam with a sledge hammer to provide both positive and negative wave directions. The compressional wave source was produced by applying the sledge hammer to a steel impact plate on the ground surface. In both the shear wave and the compressional wave surveys, an electric circuit was switched-closed with the impact of the sledgehammer, to trigger the record and provide accurate zero times.

3.2 Survey Procedure

Field procedure consisted of lowering the geophone package to the measuring point in the hole and ensuring firm coupling of the geophones to the drillhole wall. The holes were tested every 0.5 metres. The secured wood beam was then struck horizontally from one end with the sledge hammer to produce a shock wave rich in shear wave energy. The horizontal geophones in the downhole triaxial package are sensitive to horizontally polarised shear wave arrivals. After recording of the first impact, the source beam was struck horizontally from the opposite end. The shear wave recorded downhole from the second impact produced an opposite polarity to the initial impact. By recording impacts from each end at every depth location, the oppositely polarised shear waves were plotted to optimise the arrival times of the horizontal shear waves.

A compressional wave was also produced by striking the steel impact plate with the hammer. This produced a well defined compressional wave from the point of impact that penetrated the depth of the holes. This compressional wave was recorded by the vertical geophone in the triaxial package

3.3 Post-Processing Data Reduction

Initial analysis of the shear wave downhole data was conducted on site directly after data acquisition. Observations were made to ensure data quality and validity. Following data acquisition, the overlapping opposite direction blows for each depth station were plotted on a single baseline. This enabled recognition of the phase of the arrival from one depth station to the next.

Initial monitoring and analysis of the compressional wave downhole data was also conducted on site to confirm data quality and validity.

The picking of the first arrival times involved identification of the shear wave break on the basis of polarity, increase in amplitude and abrupt change in frequency. An evaluation of the location of the first break was made on the location of the departure from the baseline of oppositely polarised data. This was also facilitated by comparison with data recorded both above and below the data point and by knowledge that arrival times must increase with depth. The same procedure was followed for the picking of the first arrivals of the compressional wave with the exception of polarity change.

4. GEOPHYSICAL RESULTS

4.1 General

The majority of the 10 seismic profiles were conducted along the placer deposits in Dublin Gulch and across the Gulch at the proposed location of the Heap Leach Embankment. The results of the seismic interpretations for these lines are presented at 1:500 scale in Figures 3 through 20 of the Appendix. Compressional and shear wave velocity profiles for the surveyed boreholes within the proposed Plant site, Event Ponds, Heap Leach Facility and Crusher site are shown in Figures 21 through 31 in the Appendix.

4.2 Discussion

4.2.1 Heap Leach Embankment and Placer Tailings

A primary focus of the survey was to determine depths to bedrock and geological conditions within the previously deposited placer tailings and the proposed location of the Heap Leach Embankment.

Seismic lines SL-12, SL-2 and SL-33 were surveyed from west to east along the placer tailings deposits within Dublin Gulch. The seismic refraction data indicate the area is underlain by three or four velocity layers. In many areas, the deeper intermediate velocity layer is associated with a layer of highly weathered bedrock.

The surficial layer with compressional (P) wave velocities ranging from 305 m/s to 650 m/s, averages approximately 3.5 metres in thickness across the three traverses. This layer correlates to shallow shothole and drillhole intersections of cobbles, gravel, sand, silt and clay.

The surficial layer is underlain by one or two intermediate layers exhibiting significant variations in compressional wave velocities. Line SL-12 contains two intermediate layers with the shallower intermediate layer displaying (P-wave) velocities of 675 m/s to 1000 m/s. This narrow range in velocities is consistent with more compact silt, sand and silty sand. This shallow intermediate layer is underlain by a second intermediate layer with velocities varying from 1400 m/s to 1750 m/s. With an average thickness of 19.5 metres, the velocities suggest this layer is composed of very compact, saturated placer tailings. There exists however, the possibility that the base of this layer may be highly weathered bedrock.

The majority of line SL-2 is interpreted to be underlain by three velocity layers with the onset of a fourth layer at approximate station 420E. This intermediate layer with velocities from 700 m/s to 1700 m/s, correlates with drill log intersections of silt, sand, gravel, cobbles and boulders. Higher velocities are likely associated with localized zones of coarser or denser material. The deeper intermediate layer from 420E to the east end of the line, has a compressional wave velocity of approximately 1900 m/s. Borehole intersections of bedrock at a depth of 11.6 metres in DH-BGC09-DG-3, indicate this layer is highly weathered bedrock. Intersections of granodiorite boulders intermixed with gravel and cobbles immediately above the bedrock may account for the two metre discrepancy between the interpreted bedrock surface and the borehole intersection.

There is good agreement between the interpreted bedrock surface and the intersection of bedrock in borehole BH-BGC11-32. Downhole shear wave velocities for BH-BGC11-32 indicate the surficial materials have a shear wave velocity of 120 m/s. Velocities vary from 265 m/s to 480 m/s throughout the thick sequence of cobbles and boulders and increase to a value of 670 m/s at the onset of the highly weathered bedrock. Compressional wave velocities determined from the downhole survey are elevated when compared with values from seismic refraction analysis. This discrepancy may be associated with the downhole pressure wave preferentially travelling through the grout column.

Seismic line SL-33 was located between borehole DH-BGC09-DG-3 and a point north and east of borehole BH-BGC11-52. This traverse is underlain by four layers to station 560ENE, where the interpreted weathered bedrock layer pinches out against the basal, competent bedrock. The shallow intermediate layer exhibits compressional wave velocities ranging from 500 m/s to 1600 m/s. With an average thickness of 6.9 metres, the velocities generally increase to the east along the traverse. This correlates well with an increase in coarse materials observed in geotechnical investigations in the site area. The deeper intermediate layer is likely associated with the onset of weathered bedrock as observed in boreholes DH-BGC09-DG-3 and BH-BGC11-33. In the approximate station interval 300ENE to 440ENE, this layer may be associated with lower compressional wave sands and gravels. Black dashed lines on the sections indicate possible transitions from weathered bedrock to tailings material within this velocity layer. Basal layer velocities from 3075 m/s to 4240 m/s, denote the competent metasedimentary bedrock surface.

Shear wave velocity surveying of BH-BGC11-33 yielded values from 205 m/s to 510 m/s in the initial 8.5 metres of the borehole. There is a noticeable increase to 800 m/s at the onset of the weathered bedrock. Similarly, borehole BH-BGC11-34 exhibits shear wave velocities of

205 m/s to 510 m/s in the initial 15 metres of the hole. Below 15 metres, the shear wave velocity increases to 800 m/s. This velocity increase correlates reasonably well with the onset of weathered bedrock at a depth of 16.3 metres.

Seismic line SL-8 was situated between borehole BH-BGC11-34 to a point roughly 125 metres northeast of BH-BGC11-28. This traverse, with the exception of a segment from 290NE to 390NE, is believed to be underlain by four velocity layers. The surficial layer averages only 2.2 metres in thickness and exhibits compressional wave velocities of 305 m/s to 375 m/s. Consistent with loose soils, sands and gravels, this layer is underlain by a more compact layer of sands and gravels with velocities of 550 m/s to 1375 m/s. Borehole BH-BGC11-28 located approximately 11 metres off the line, indicates this layer may in places, be comprised of highly weathered bedrock. This shallower intermediate layer is underlain by a weathered bedrock layer with velocities ranging from 1350 m/s to 2450 m/s. The interpreted thicknesses of the weathered bedrock layer vary from 2 m to 28.7 metres, and average 14.6 metres. The basal layer with velocities from 3100 m/s to 4350 m/s is the interpreted competent bedrock surface.

Borehole BH-BGC11-28 has a calculated shear wave velocity of 210 m/s for the surficial 3.5 metres of moderately weathered bedrock. The velocity increases significantly to 880 m/s in the depth interval, 3.5 m to 17 metres. Velocities vary from 650 m/s to 1130 m/s below 17 metres depth, consistent with slightly to moderately weathered bedrock.

Seismic lines SL-1 and SL-3 cut across the placer tailings deposits in the western part of the survey area. Seismic line SL-1 has surficial layer velocities of 300 m/s to 445 m/s, which coincide with borehole intersections of silts and sands. This surficial layer is underlain by a thicker section of sands, gravels, cobbles and boulders exhibiting compressional wave velocities of 500 m/s to 1000 m/s. A deeper intermediate layer with velocities of 1500 to 1600 m/s coincides with the borehole intersection of sands, silts and gravels. This layer and the corresponding higher velocities is in good agreement with saturated conditions and the expected water table. The basal layer is interpreted to be competent bedrock, although there are two zones with abnormally low compressional wave velocities. From 45NNE to 110NNE and from 215NNE to 255NNE, compressional wave velocities varying from 2050 m/s to 2300 m/s were computed for the basal refractor. These velocities likely reflect either highly weathered bedrock or significant shear zones in the bedrock.

Borehole BH-BGC11-39 located approximately 2 metres off line SL-1 shows good agreement with the interpreted seismic section. Saturated overburden velocities are observed

at approximately 10 metres depth. Shear wave velocities obtained from the borehole survey are approximately 130 m/s in the surficial 3 metres of loose silts and sands. Velocities increase significantly to 325 m/s, at the onset of a layer of gravel, cobbles and boulders.

Seismic line SL-3 is interpreted to consist of three distinct layers, with compressional wave velocities for the surficial layer varying from 375 m/s to 420 m/s. Surficial layer thicknesses range from 0.1 metres to 8.1 metres, with the thinnest section located near the middle of the traverse. The surficial layer is underlain by a thick intermediate layer with P-wave velocities of 950 m/s to 1500 m/s. Higher velocities indicate the presence of coarser materials and possibly, increased water content. Basal velocities vary from 2125 m/s to 3630 m/s and are interpreted to be metasedimentary bedrock with varying degrees of weathering. The 2125 m/s velocity zone at the north end of the line may be indicative of either highly weathered bedrock or a shear zone in the bedrock

Parallel seismic lines SL-10 and SL-5 were oriented southeast to northwest, with line SL-5 located on the proposed heap embankment and line SL-10 running along the edge of the embankment up to the proposed Plant Site. Both lines are interpreted to be underlain by 3 or 4 velocity layers. Typically, the surficial layer is associated with loose sands and gravels with compressional wave velocities varying from 290 m/s to 550 m/s.

A thin layer of surficial material overlying weathered bedrock with compressional wave velocities of 2700 m/s to 2900 m/s, is present on the southern end of both traverses. The weathered bedrock in turn overlies more competent bedrock. The weathered bedrock layer is no longer discernible at approximately 100NW on lines SL-5 and SL-10, as indicated by the dashed black lines on the sections.

From station 110NW on line SL-10 and station 145NW on line SL-5, the interpreted sections consist of four layers. The shallow intermediate layer exhibits velocities ranging from 600 m/s to 1900 m/s. This layer is correlated in boreholes with gravel, and gravel with sand. This layer is underlain by a thicker intermediate layer with velocities of 1050 m/s to 2100 m/s. This intermediate layer is consistent with denser sands, gravels, cobbles and boulders as observed in BH-BGC11-34 on line SL-5, and highly weathered bedrock intersected in borehole BH-BGC11-33 on line SL-10. The deeper intermediate layer pinches out at the elevation low associated with the creek at 460NW on line SL-10B and 400NW on line SL-5B. Northwest of the creek, line SL-5 consists of 3 layers, with the intermediate layer varying in velocity from 750 m/s to 1900 m/s. Seismic line SL-10 has a thick intermediate layer with velocities ranging from 715 m/s to 1250 m/s from 460NW to the end of the

traverse. From station 635NW, this layer is underlain by a deeper intermediate layer with a velocity of 2050 m/s. This deeper layer is likely consistent with highly weathered bedrock.

Basal layer velocities on both lines vary from 1635 m/s to 4800 m/s, with low velocity zones observed at 495NW to 525NW and 665NW to 700NW on line SL-5, and 110NW to 120NW on line SL-10. The higher velocities are interpreted as competent metasedimentary bedrock. The three low velocity zones with values of 1635 m/s to 1900 m/s are believed to be indicative of highly weathered rock or shear zones.

Located approximately 5 metres off SL-5, borehole BH-BGC11-59 exhibits good agreement between the interpreted basal layer and the intersection of weathered bedrock. Shear wave velocities also show a noticeable increase from 290 m/s to 570 m/s at the onset of the bedrock. Inconsistent with some boreholes, the compressional wave velocities for BH-BGC11-59 compare reasonably well with the refraction results, with velocities of 2045 m/s in the frozen overburden and 3085 m/s in the slightly to moderately weathered bedrock.

The final section SL-7, is located at the eastern end of the survey area. Line SL-7 was oriented south to north and consists of 3 distinct velocity layers. The surficial layer varies in thickness from 0.3 to 4.5 metres, with an average thickness of 2.2 metres. Velocities ranging from 300 m/s to 380 m/s are consistent with loose, surficial, sands and gravels. The surficial layer is underlain by a thick layer of more compact sands, gravels and cobbles. Compressional wave velocities for this layer range from 650 m/s to 1150 m/s.

The interpreted basal layer is in good agreement with moderately weathered metasedimentary bedrock observed in borehole BH-BGC11-52. Depths to bedrock vary from 6.1 to 24.4 metres, with an average depth of 13.2 metres. Calculated basal layer velocities range from 2445 m/s to 4670 m/s. The lower velocity zone of 2445 m/s at the southern end of the line likely indicates highly to moderately weathered bedrock.

Downhole shear and compressional wave testing in borehole BH-BGC11-52 shows similar results to other boreholes, with low shear wave velocity of 115 m/s to 210 m/s in the shallow overburden materials. An increase in shear wave velocities at 4.5 metres depth to 475 m/s, coincides well with an increase in compressional wave velocities apparent on seismic line SL-7 at approximately 4 metres depth. Shear wave velocities increase again to 665 m/s at the onset of the metasedimentary rock..

4.2.2 Crusher Site Boreholes

Boreholes BH-BGC11-36, BH-BGC11-40B and BH-BGC11-62 are all located within the proposed crusher site. The boreholes all contain a thin section of overburden material overlying relatively shallow bedrock. Bedrock ranges from slightly to moderately weathered. Shear wave velocities typically exceed 800 m/s at or near, the surface of the bedrock. Shear wave testing in borehole BH-BGC11-36 resulted in one of the thickest, continuous shear wave zones in bedrock. The 1290 m/s zone is approximately 30 m thick and is underlain at a depth of about 40 m, by a 1745 m/s velocity zone that extends to the bottom of the hole.

4.2.3 Supplementary MASW Profiles.

The Multichannel analysis of surface wave (MASW) method was applied to surface seismic data within the vicinity of boreholes BH-BGC11-32, BH-BGC11-33, BH-BGC11-34 and BH-BGC11-52. The MASW profiles are presented at a vertical scale of 1:250 in Figures 32 to 37 of the Appendix. Each profile indicates the seismic line number and the distance along the traverse. Also shown at the lower left hand corner of the plots is the closest geotechnical borehole. The MASW results show general agreement with the downhole shear wave velocities calculated from borehole seismic logging.

5. LIMITATIONS

The depths to subsurface boundaries derived from seismic refraction surveys are generally accepted as accurate to within ten percent of the true depths to the boundaries. In some cases, unusual geological conditions may produce false or misleading data points with the result that computed depths to subsurface boundaries may be less accurate. In seismic refraction surveying difficulties with a 'hidden layer' or a velocity inversion may produce erroneous depths. The first condition is caused by the inability to detect the existence of a layer because of insufficient velocity contrasts or layer thicknesses. A velocity inversion exists when an underlying layer has a lower velocity than the layer directly above it. The interpreted depths shown on drawings are to the closest interface location, which may not be vertically below the measurement point if the refractor dip direction departs significantly from the survey line location.

In downhole seismic surveys, individual values taken at discrete depth points may precede or lag the anticipated arrival due to 'zero time', timing errors in the circuitry. These errors are generally quite small and average velocities taken over a broader zone are good representations of in situ shear wave and compressional wave velocities in a layer.

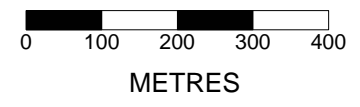
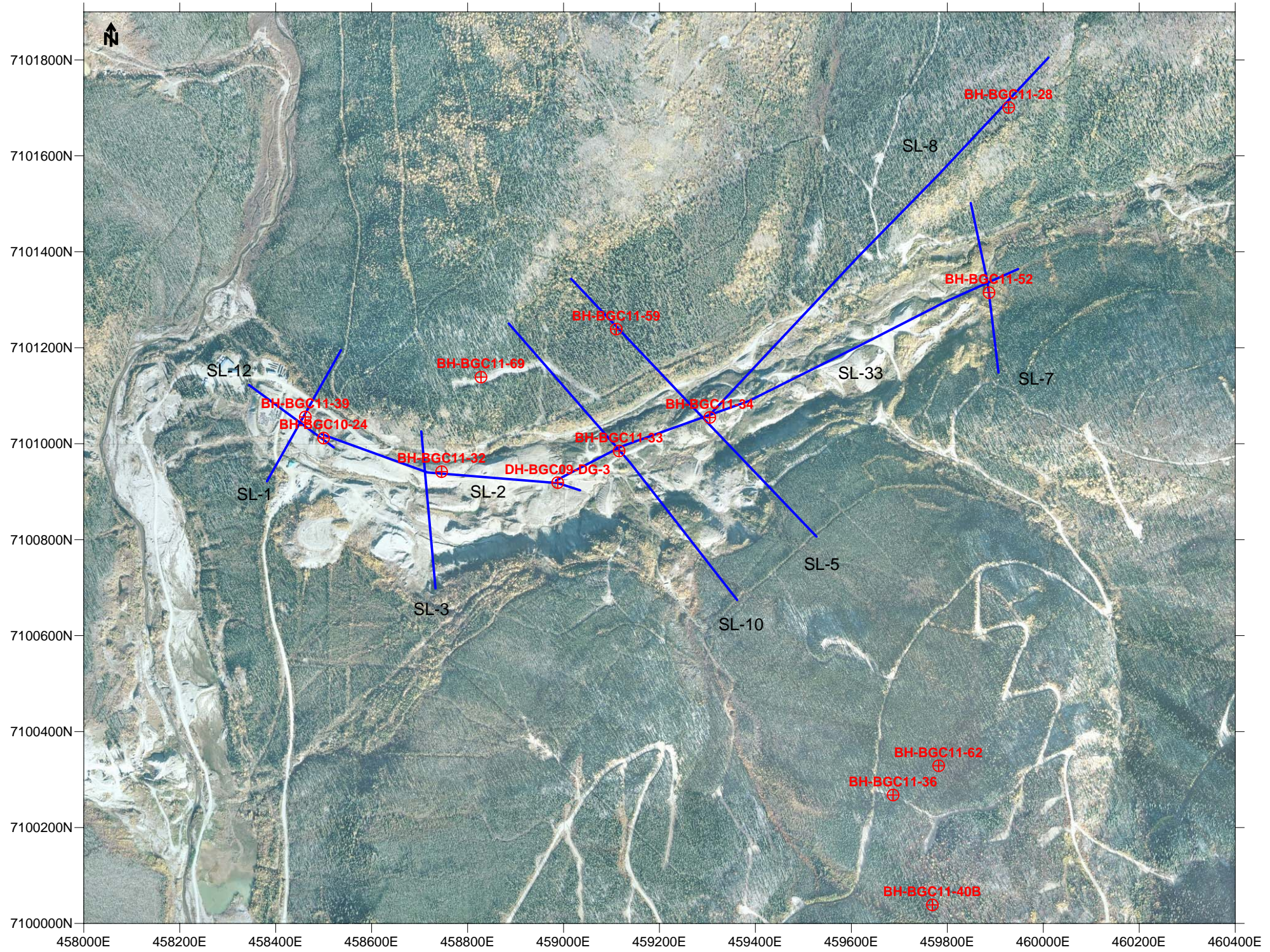
In this survey, some difficulty was encountered in downhole logging due to seismic energy travelling in the grout surrounding the plastic casing. This difficulty was generally overcome by careful picking of later seismic arrivals subsequent to the arrival of the grout wave.

The results are interpretive in nature and are considered to be a reasonably accurate representation of existing subsurface conditions within the limitations of the seismic refraction and downhole seismic methods.

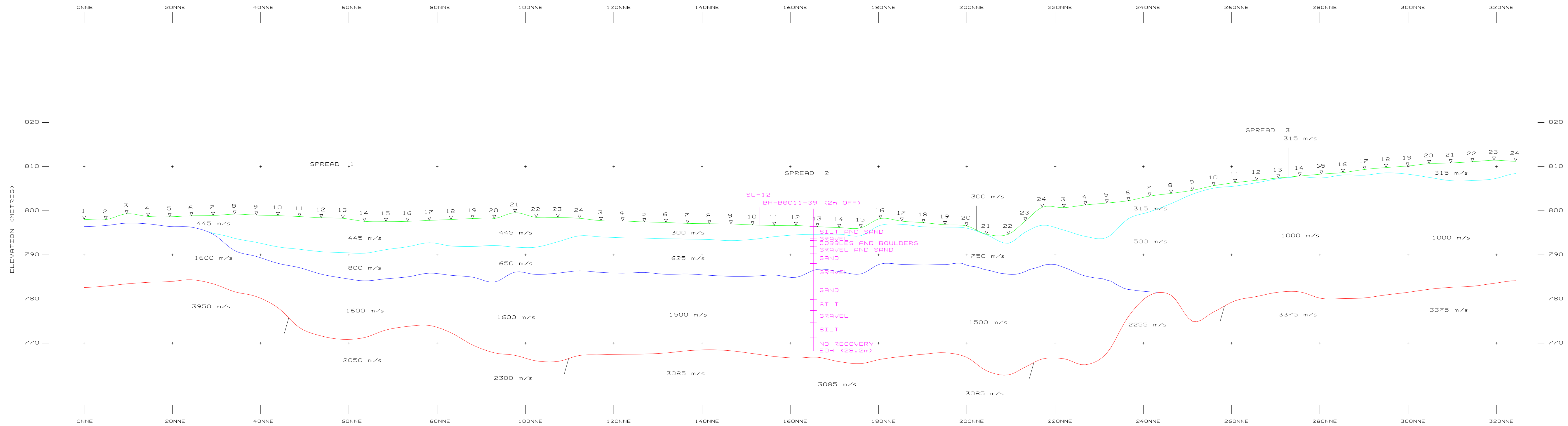
For: Frontier Geosciences Inc.

Alex Smith, M.Sc.

Russell Hillman, P.Eng.



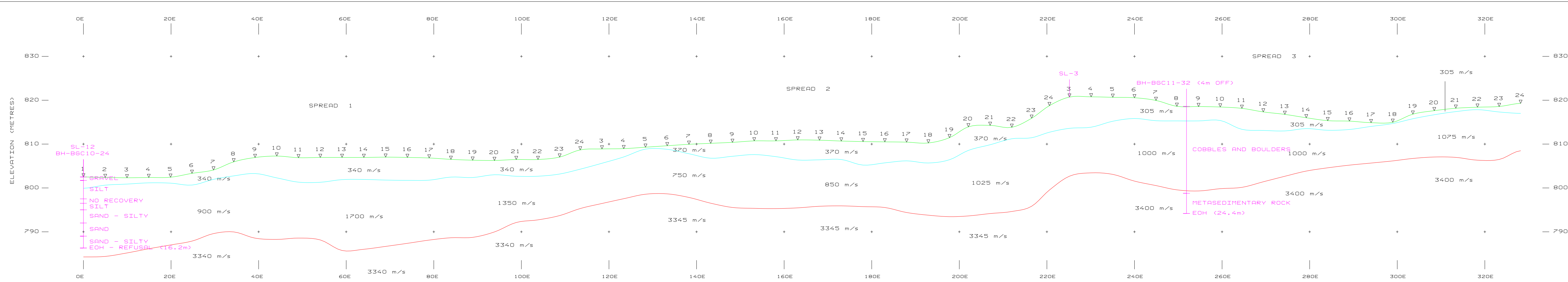
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
SITE PLAN		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT 2011	SCALE 1:10,000	FIG. 2



SEISMIC LINE SL-1

INSTRUMENT: GEOMETRICS GEODE

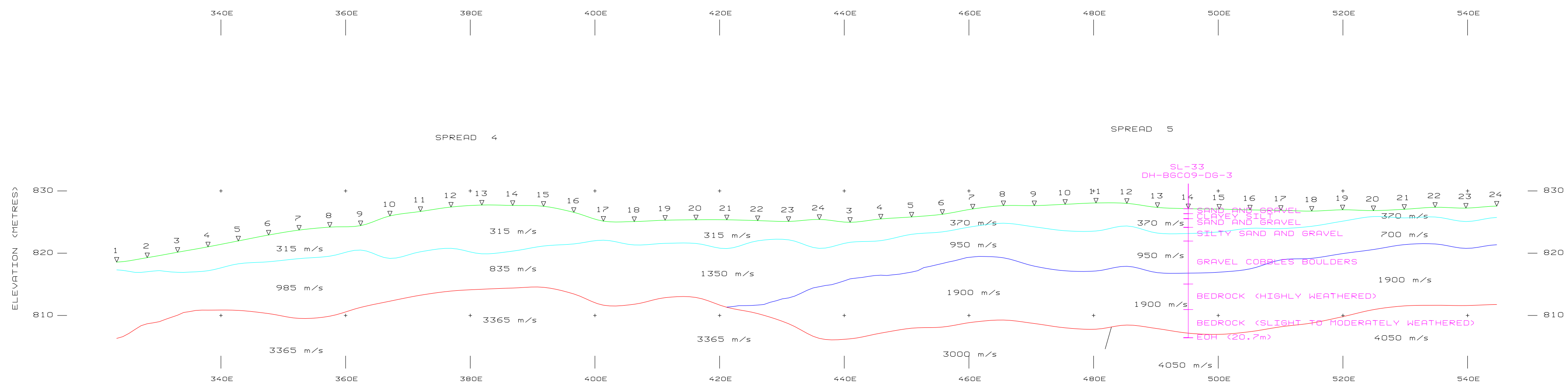
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-1		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 3



SEISMIC LINE SL-2A

INSTRUMENT: GEOMETRICS GEODE

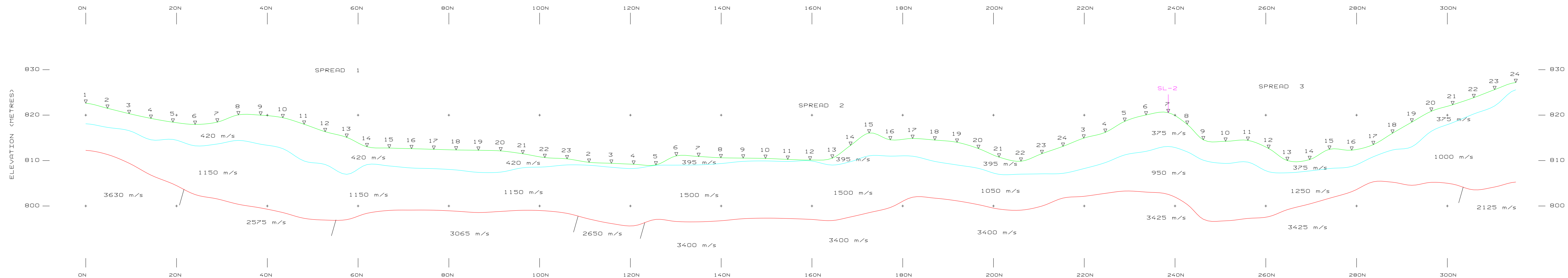
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-2A		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 4



SEISMIC LINE SL-2B

INSTRUMENT: GEOMETRICS GEODE

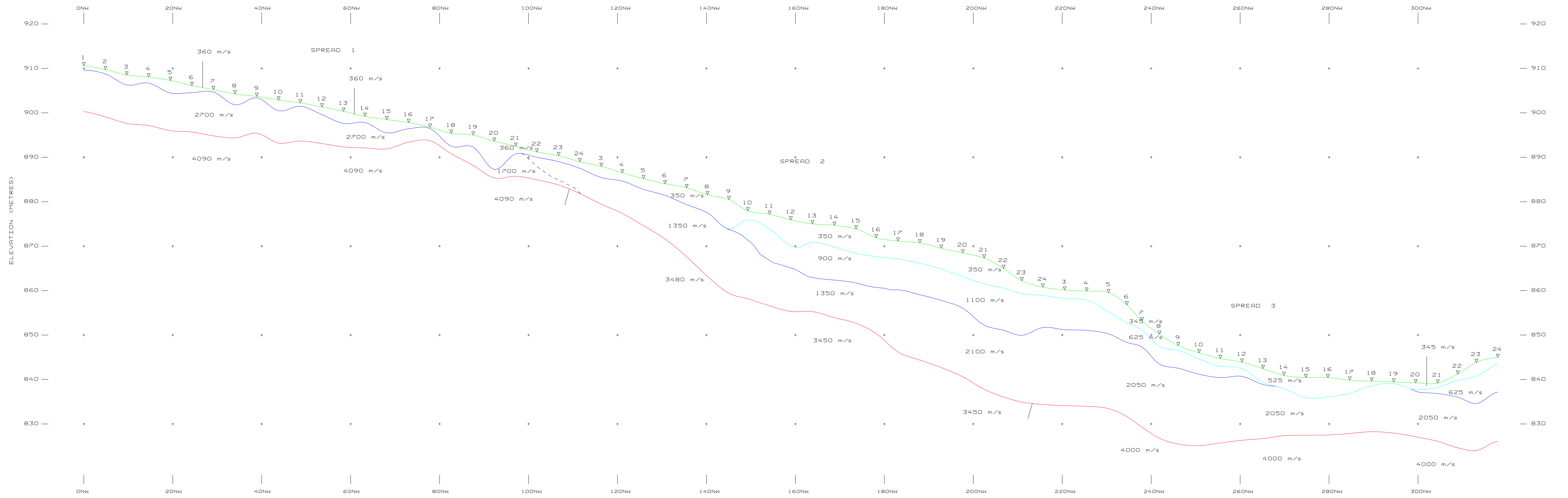
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-2B		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 5



SEISMIC LINE SL-3

INSTRUMENT: GEOMETRICS GEODE

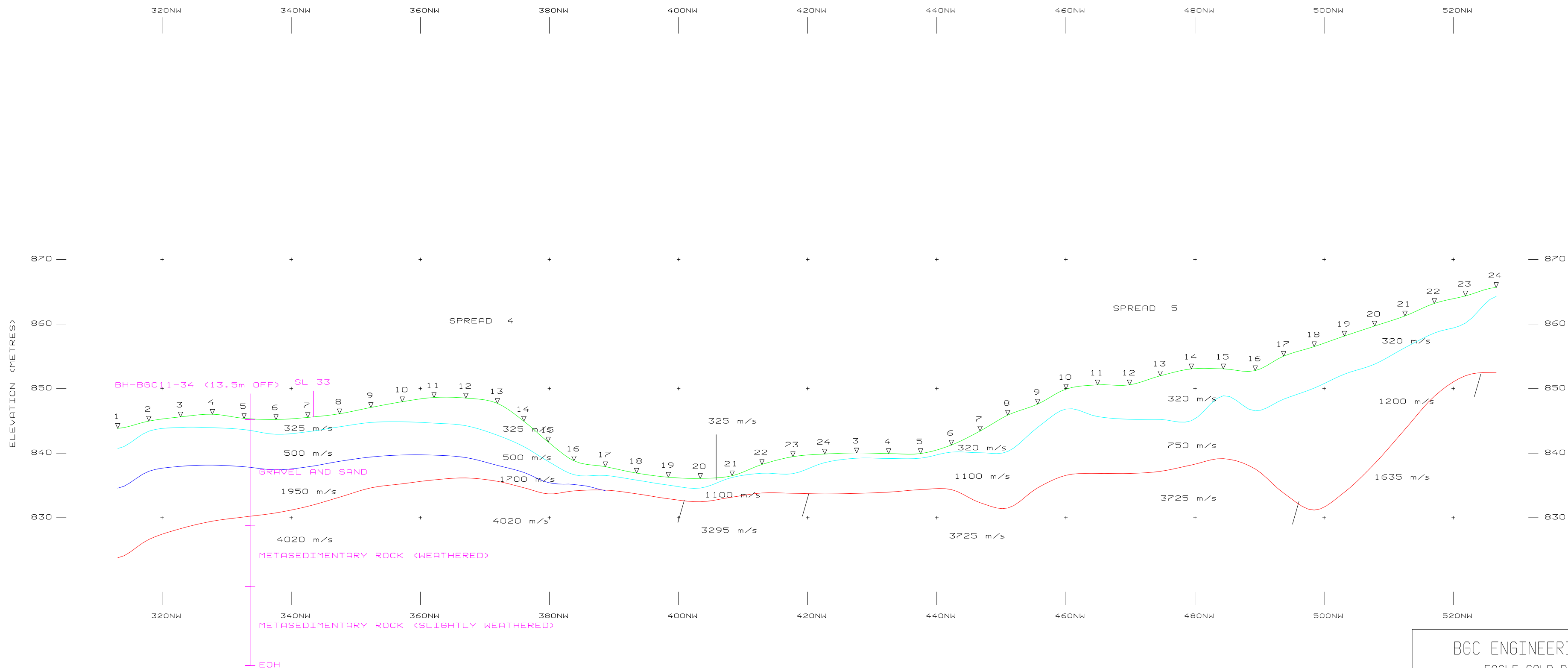
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-3		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 6



SEISMIC LINE SL-5A

INSTRUMENT: GEOMETRICS GEODE

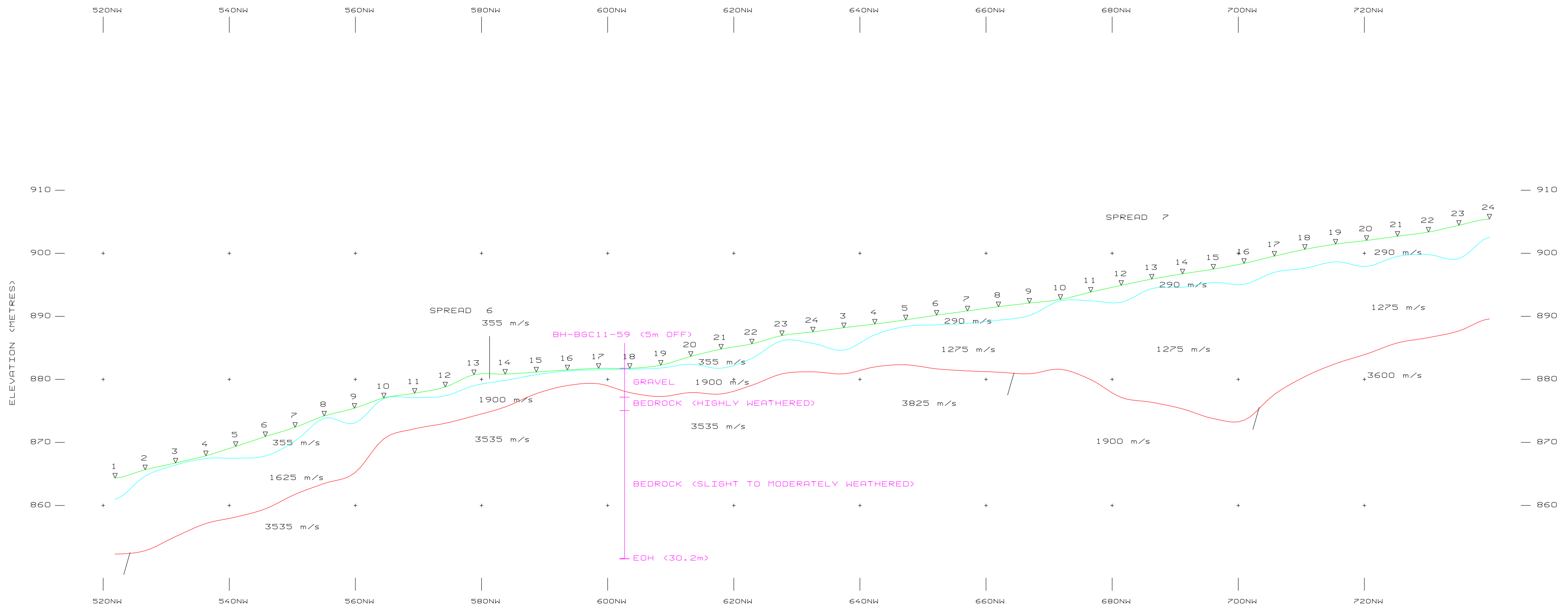
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SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-5A		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE: 1:500	FIG. 7



SEISMIC LINE SL-5B

INSTRUMENT: GEOMETRICS 6E0DE

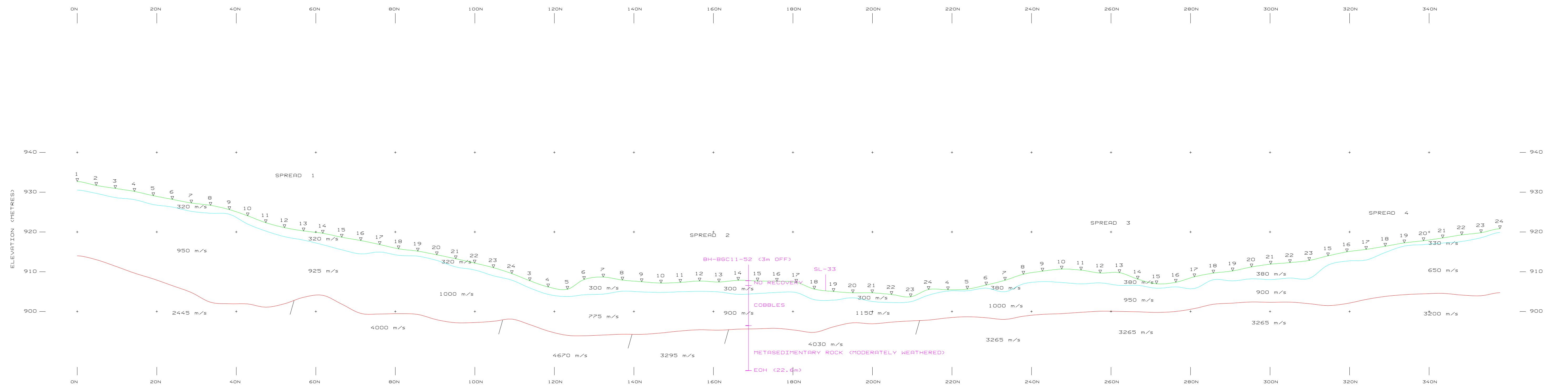
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-5B		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 8



SEISMIC LINE SL-5C

INSTRUMENT: GEOMETRICS 6E0DE

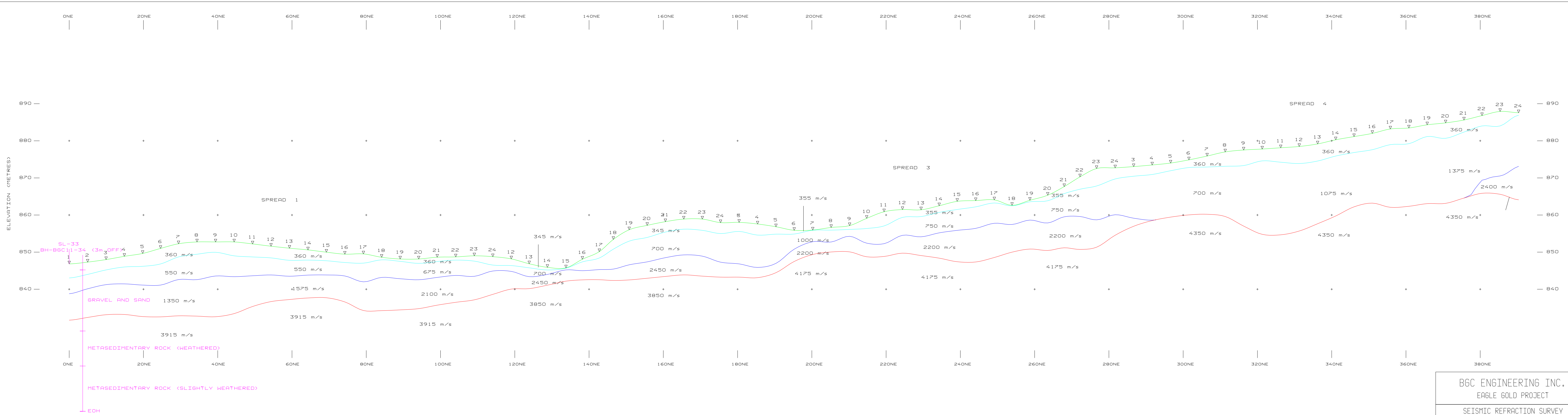
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SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-5C		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 9



SEISMIC LINE SL-7

INSTRUMENT: GEOMETRICS GEODE

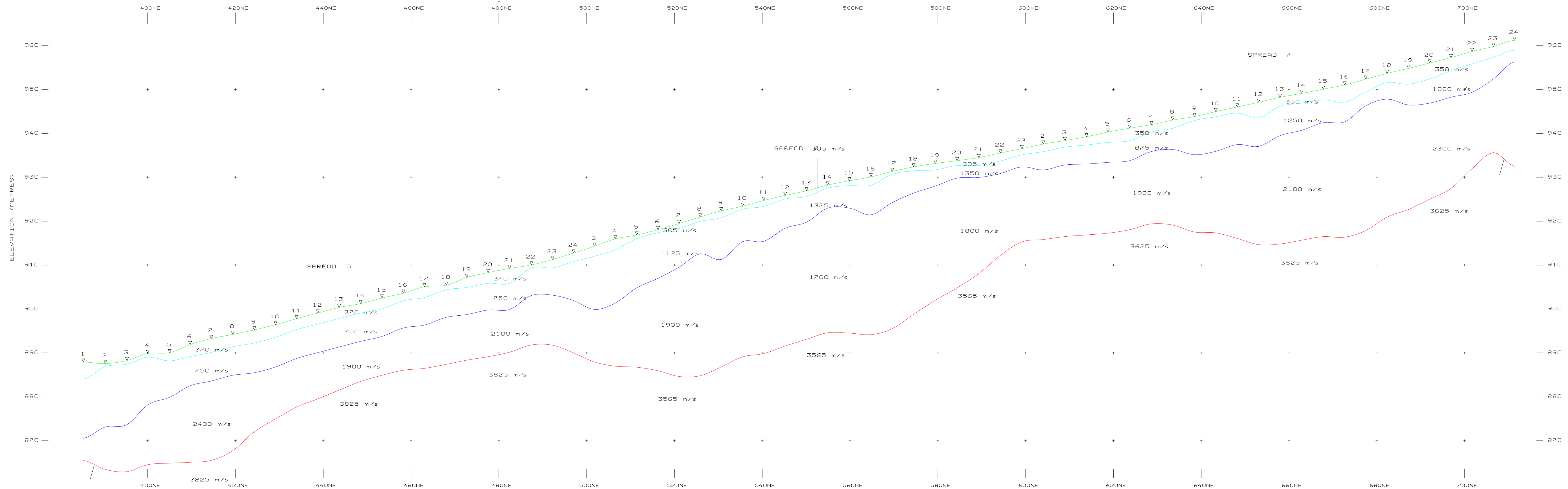
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-7		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 10



SEISMIC LINE SL-8A

INSTRUMENT: GEOMETRICS GEODE

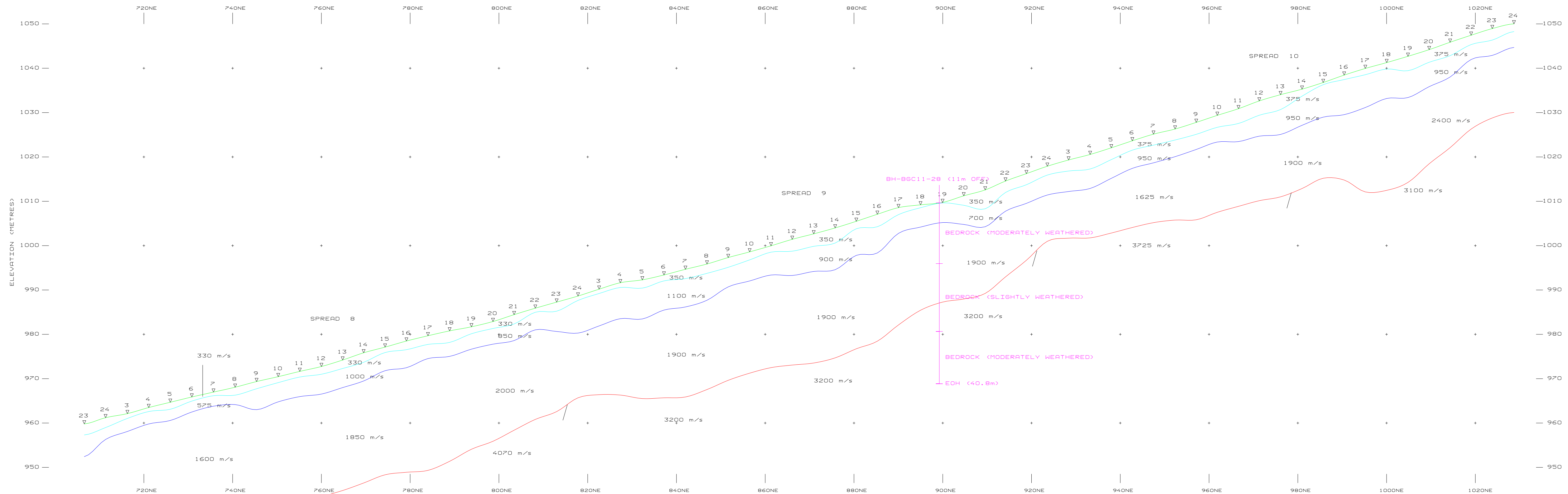
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-8A		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 11



SEISMIC LINE SL-8B

INSTRUMENT: GEOMETRICS GEODE

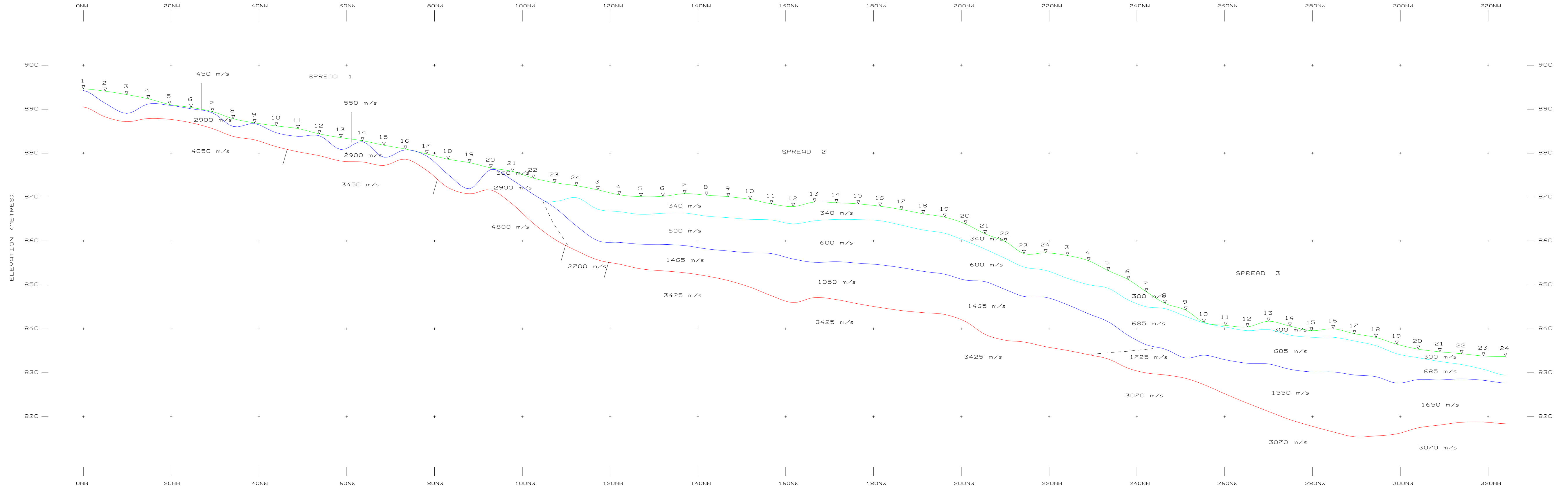
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-8B		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE: 1:500	FIG. 12



SEISMIC LINE SL-8C

INSTRUMENT: GEOMETRICS GEODE

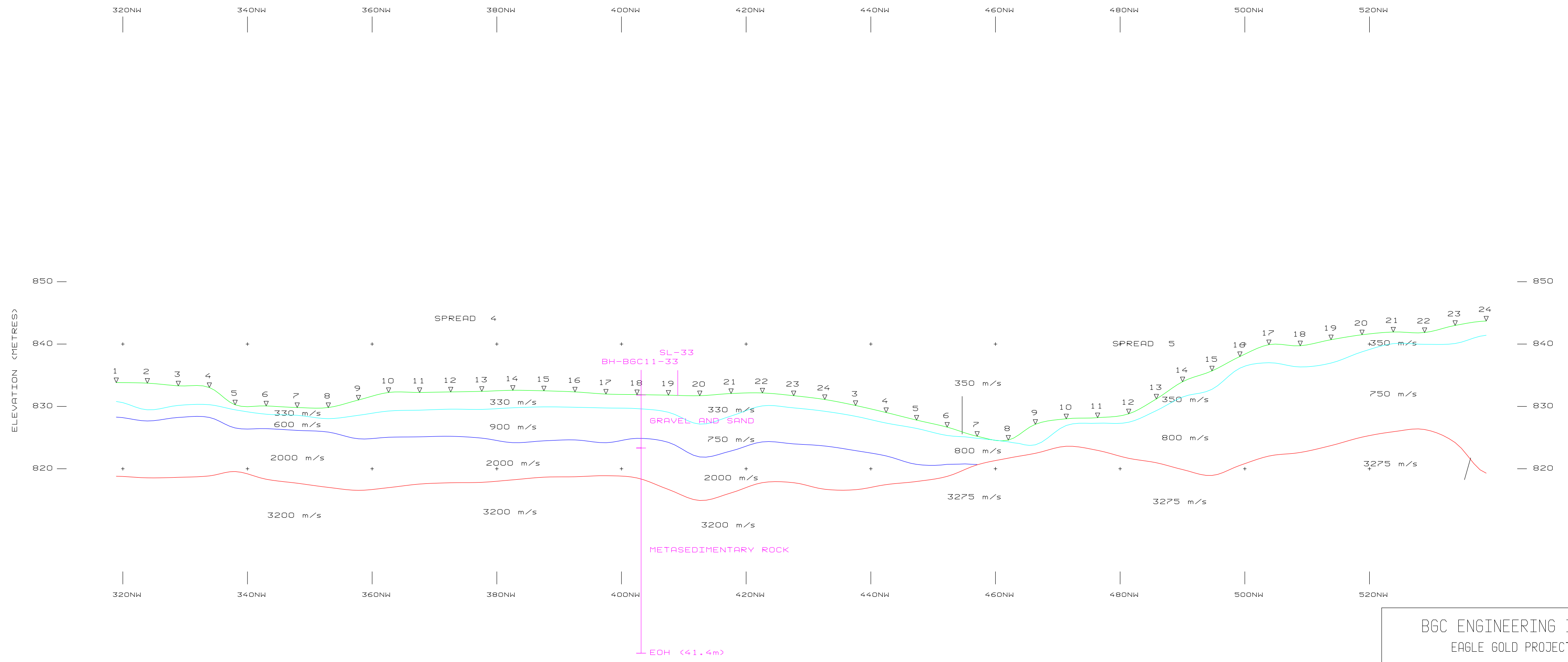
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-8C		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE: 1:500	FIG. 13



SEISMIC LINE SL-10A

INSTRUMENT: GEOMETRICS GEODE

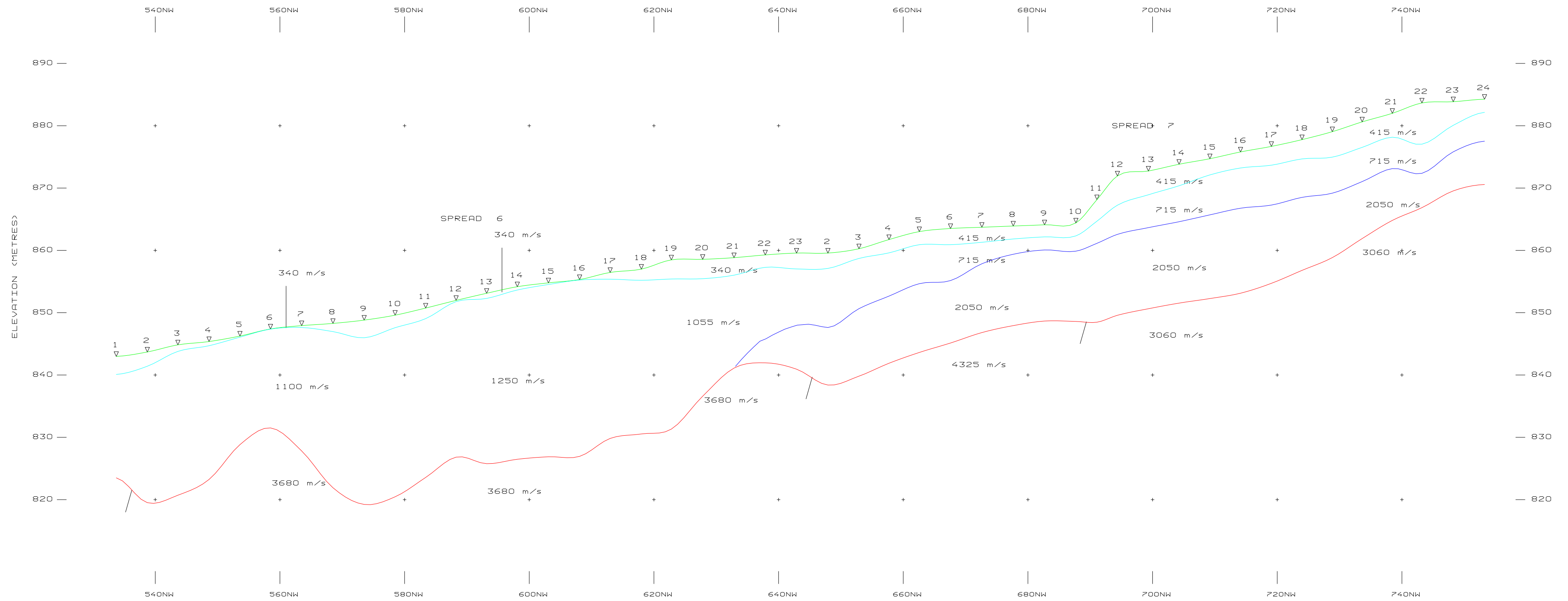
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-10A		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE: 1:500	FIG. 14



SEISMIC LINE SL-10B

INSTRUMENT: GEOMETRICS GEODE

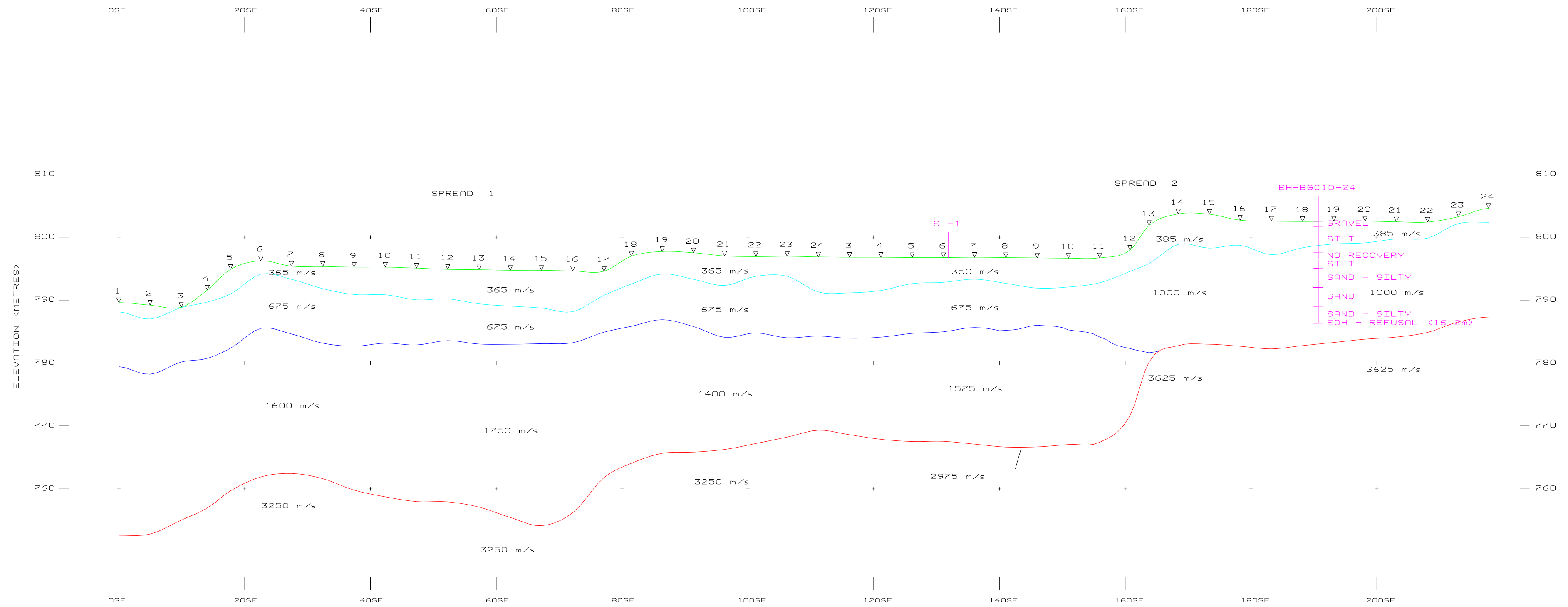
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-10B		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 15



SEISMIC LINE SL-10C

INSTRUMENT: GEOMETRICS GEODE

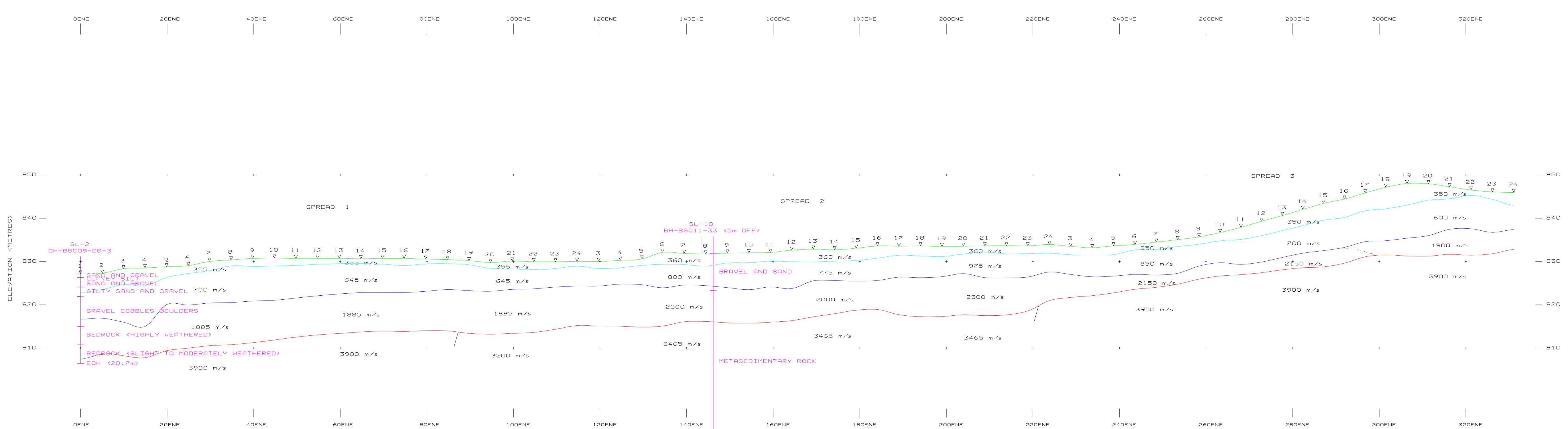
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-10C		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 16



SEISMIC LINE SL-12

INSTRUMENT: GEOMETRICS 6E0DE

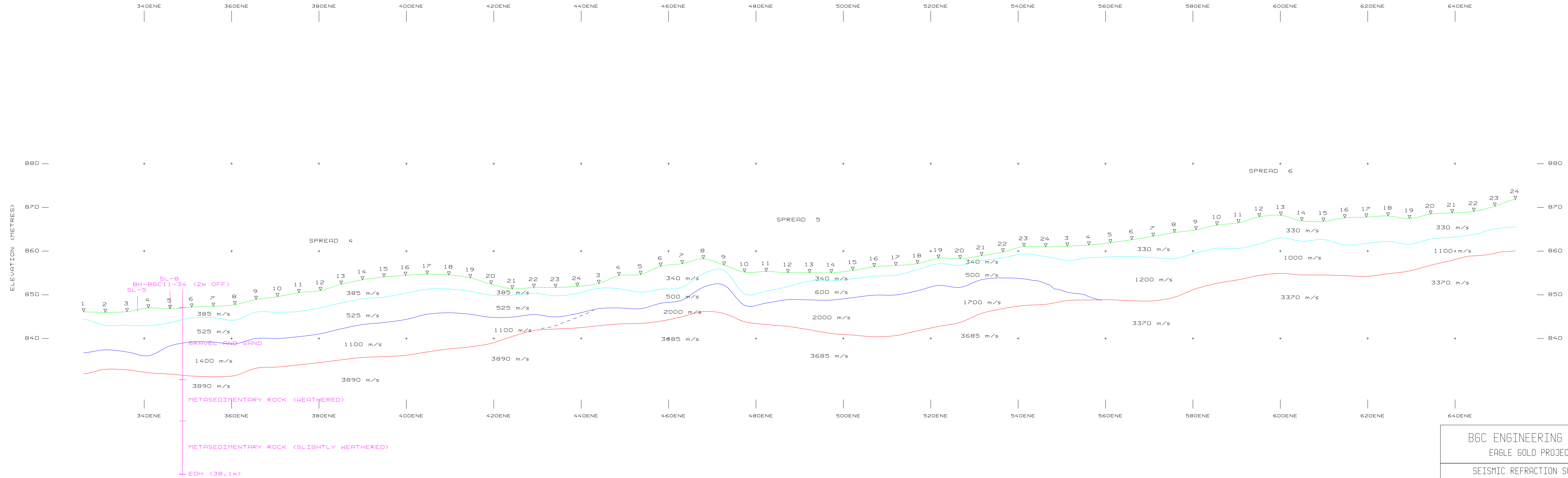
BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-12		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 17



SEISMIC LINE SL-33A

INSTRUMENT: GEOMETRICS GEODE

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-33A		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE 1:500	FIG. 18



SEISMIC LINE SL-33B

INSTRUMENT: GEOMETRICS GEODE

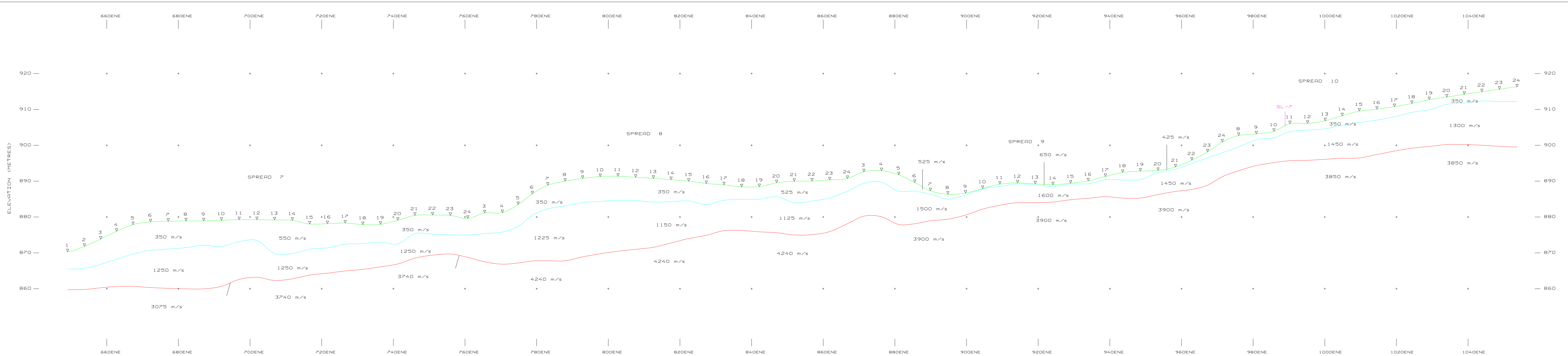
BGC ENGINEERING INC.
EAGLE GOLD PROJECT

SEISMIC REFRACTION SURVEY

INTERPRETED DEPTH SECTION SL-33B

FRONTIER GEOSCIENCES INC.

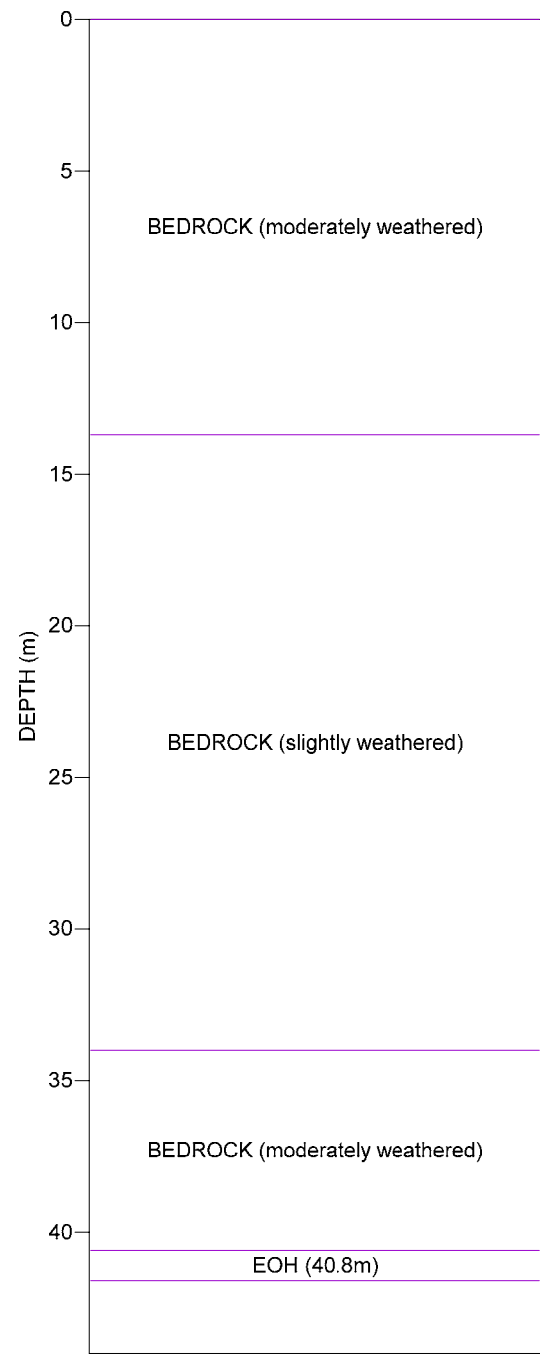
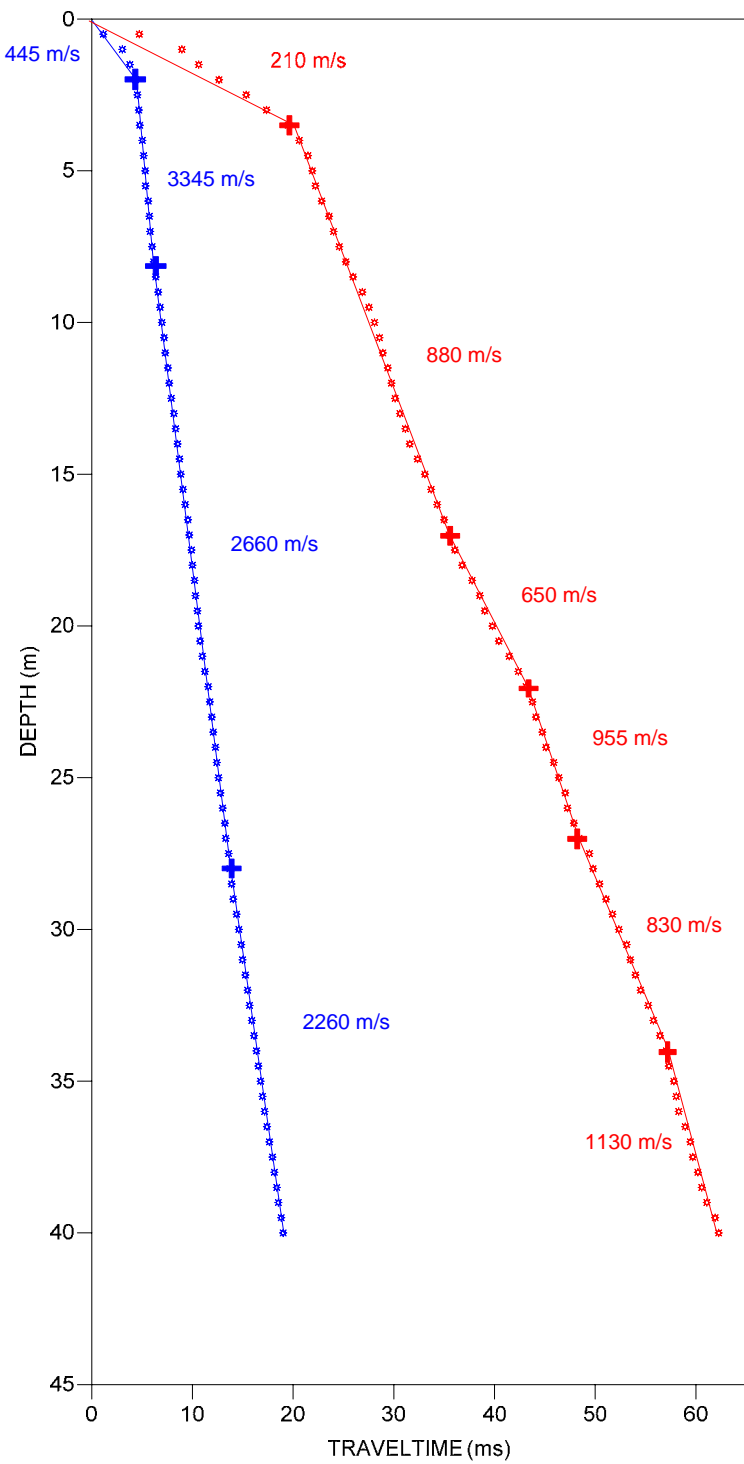
DATE: SEPTEMBER 2011 | SCALE 1:500 | FIG. 19



SEISMIC LINE SL-33C

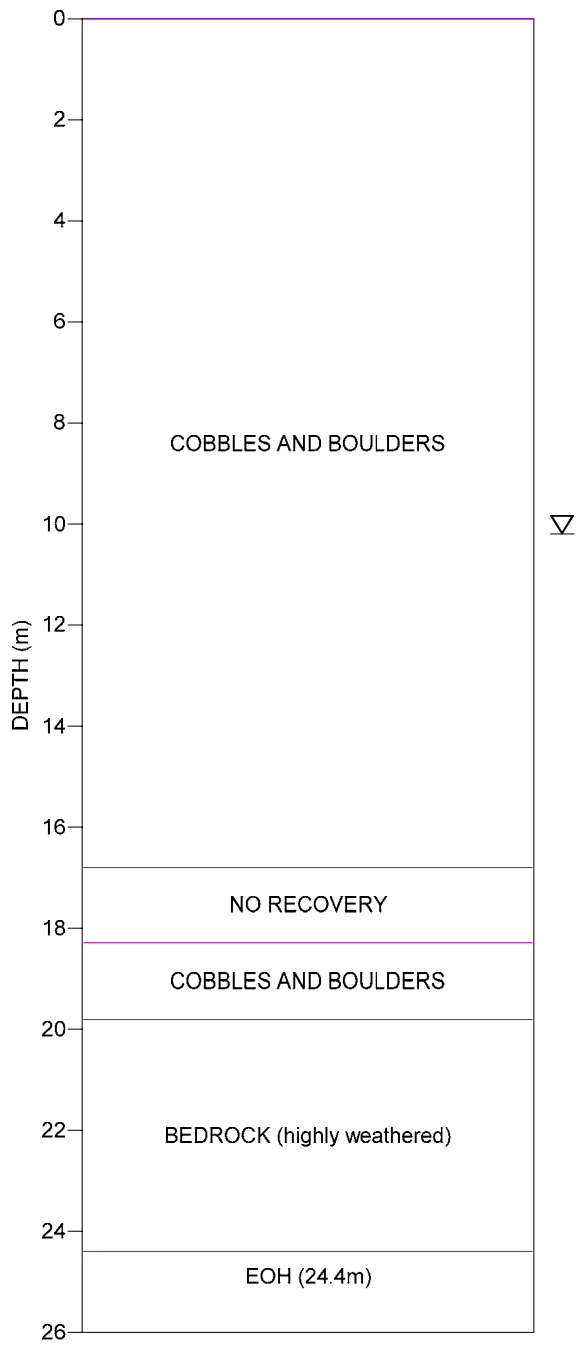
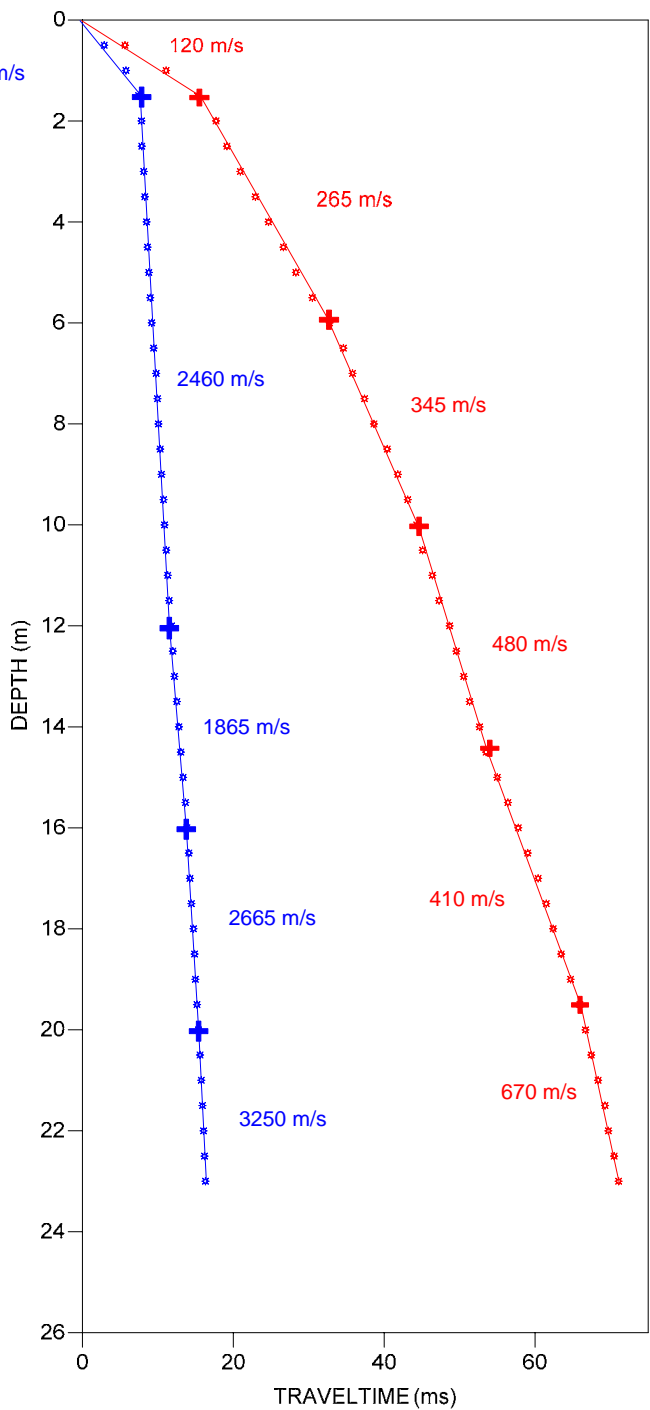
INSTRUMENT: GEOMETRICS GEODE

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
SEISMIC REFRACTION SURVEY		
INTERPRETED DEPTH SECTION SL-33C		
FRONTIER GEOSCIENCES INC.		
DATE: SEPTEMBER 2011	SCALE: 1:500	FIG. 20



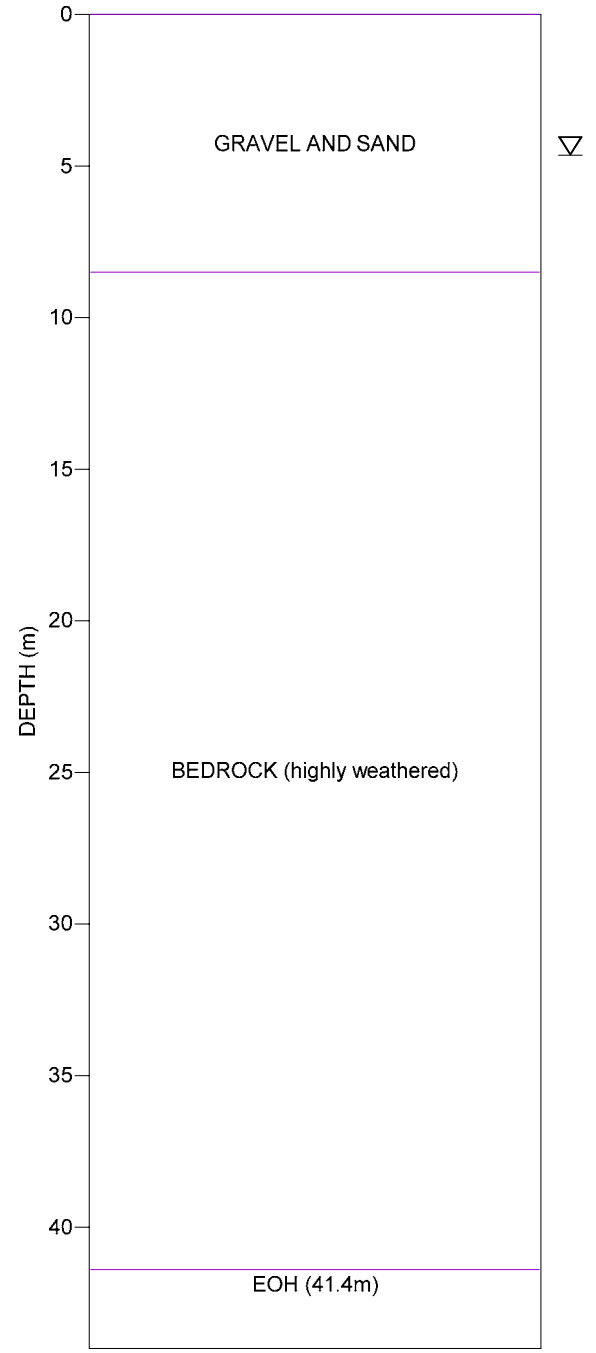
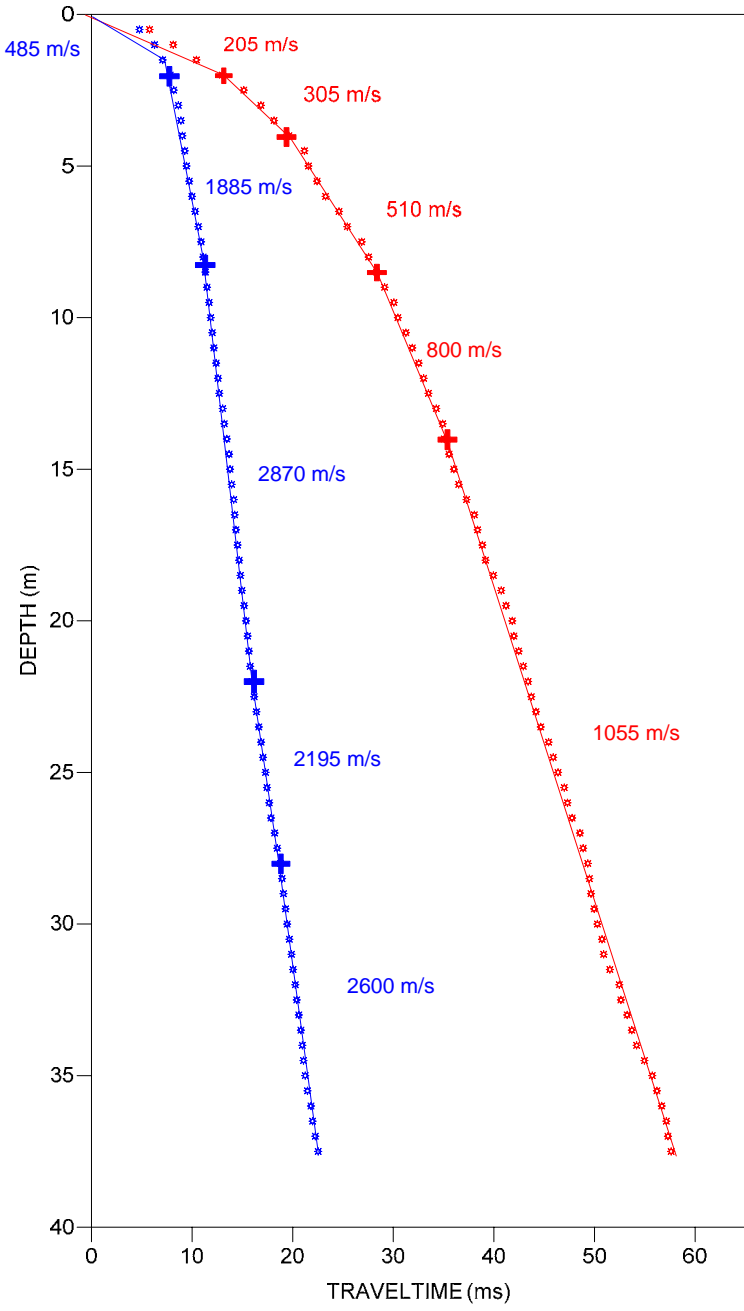
- LEGEND
- * * DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC.		
EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-28		
P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:250	FIG. 21



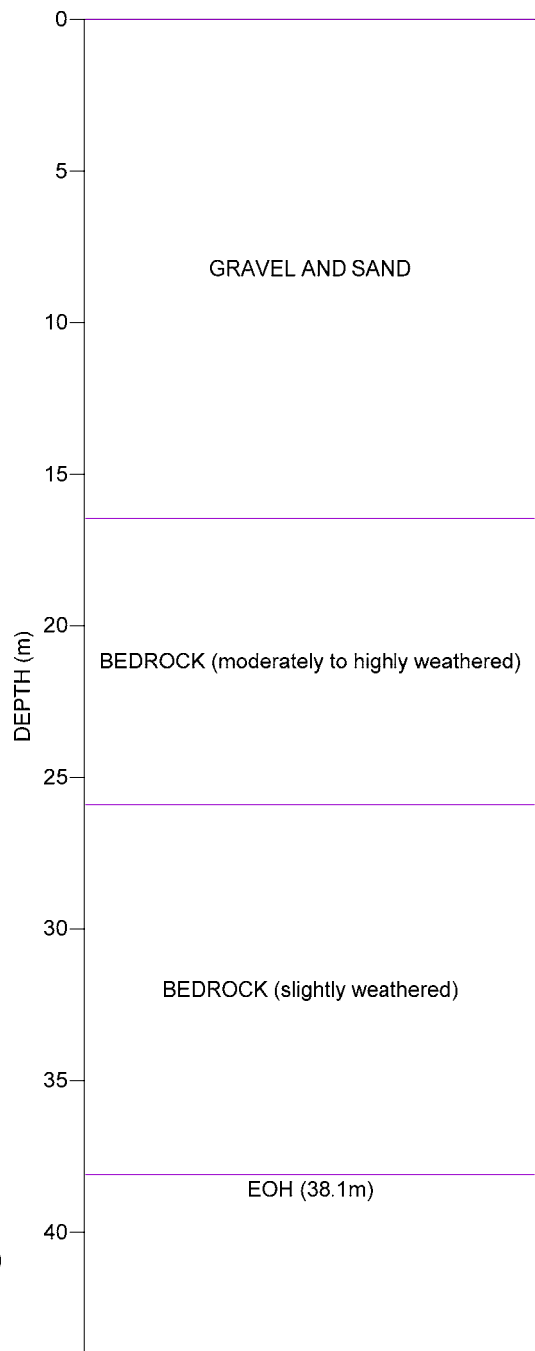
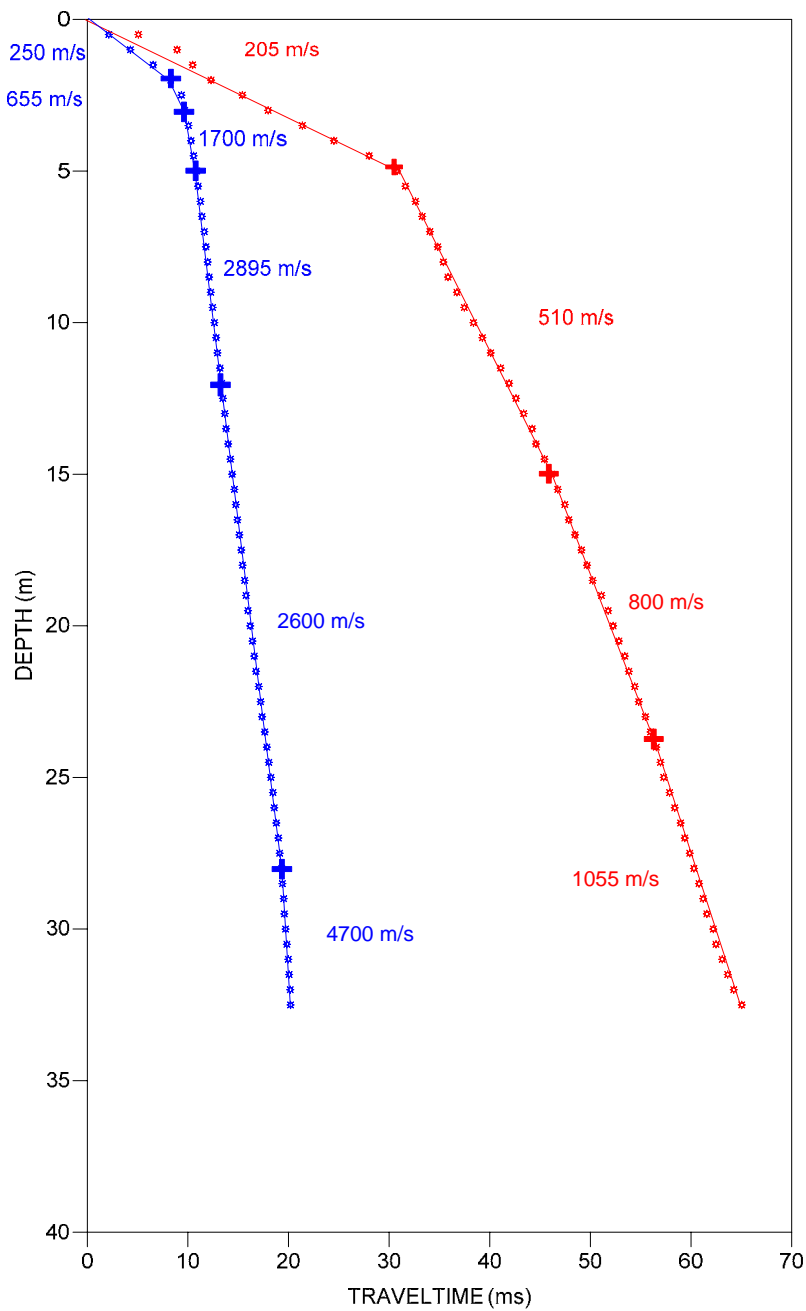
- LEGEND
- * * DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-32 P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:150	FIG. 22



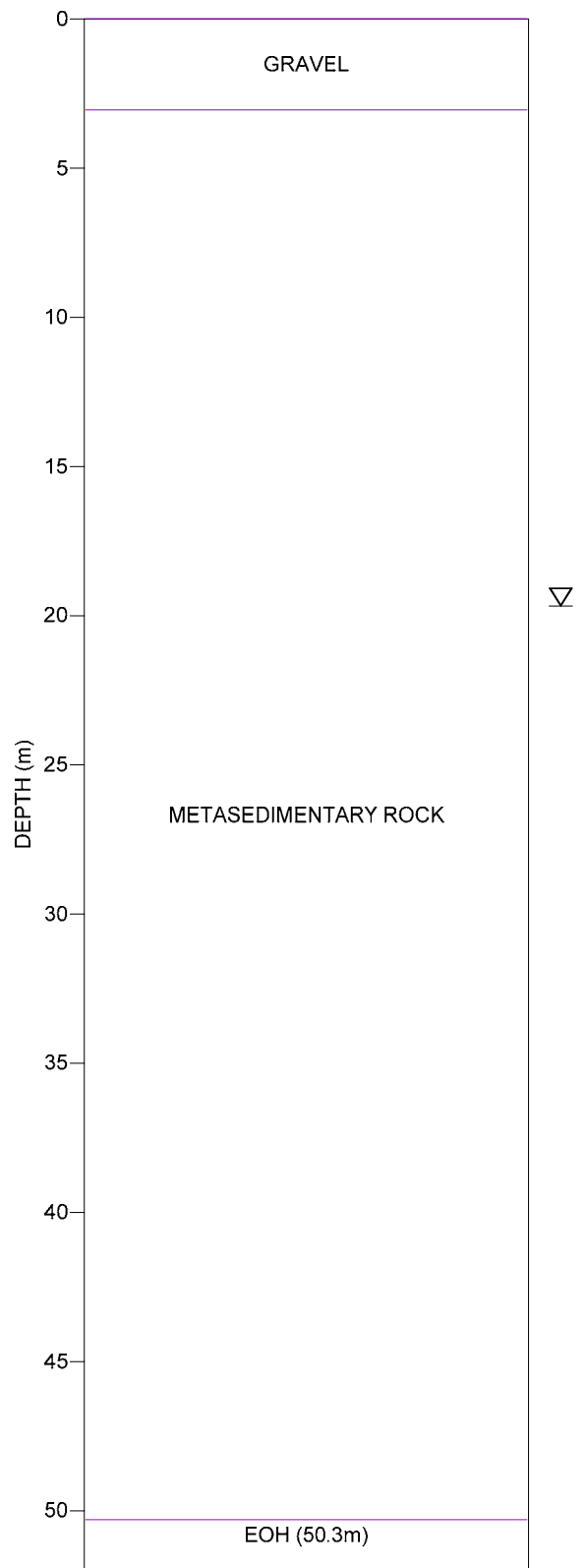
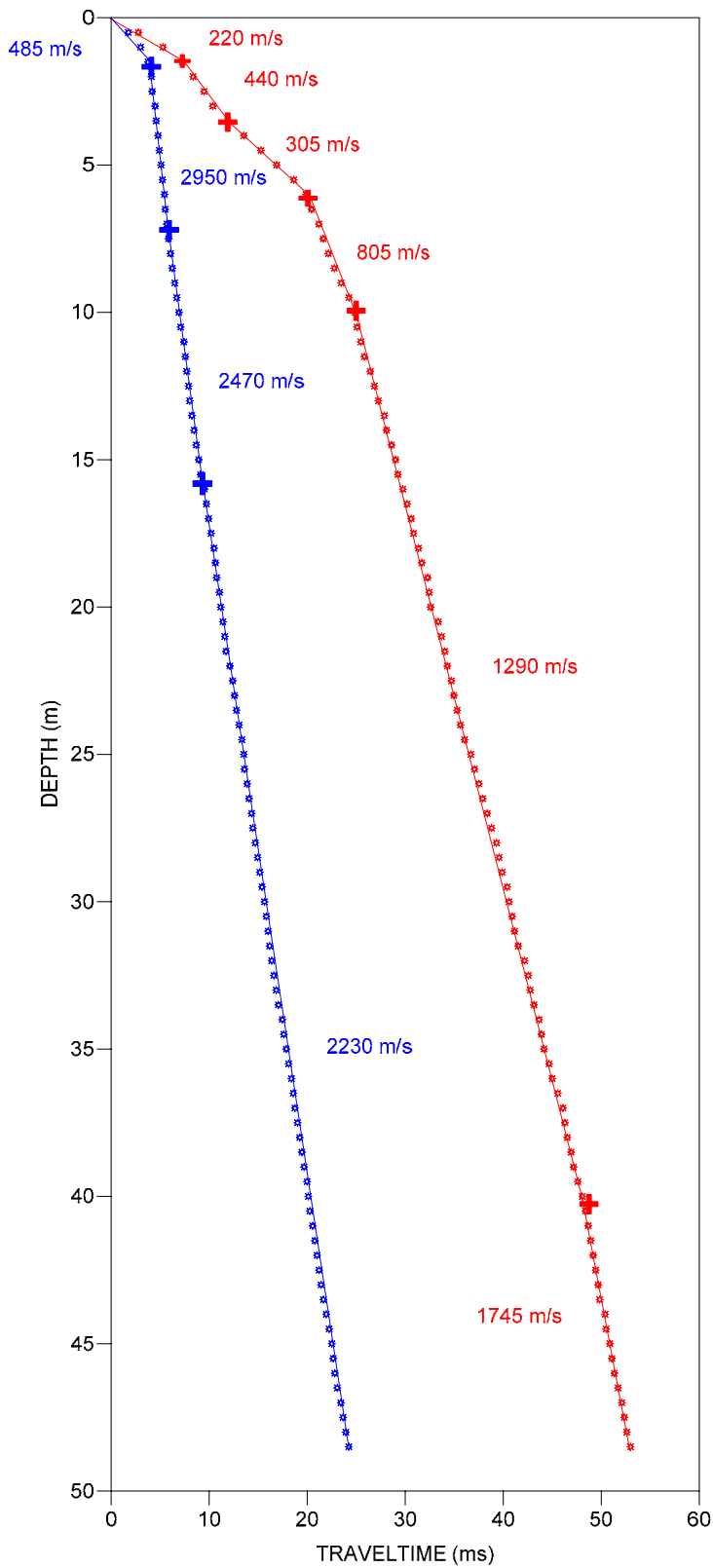
- LEGEND
- * * DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-33 P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:250	FIG. 23



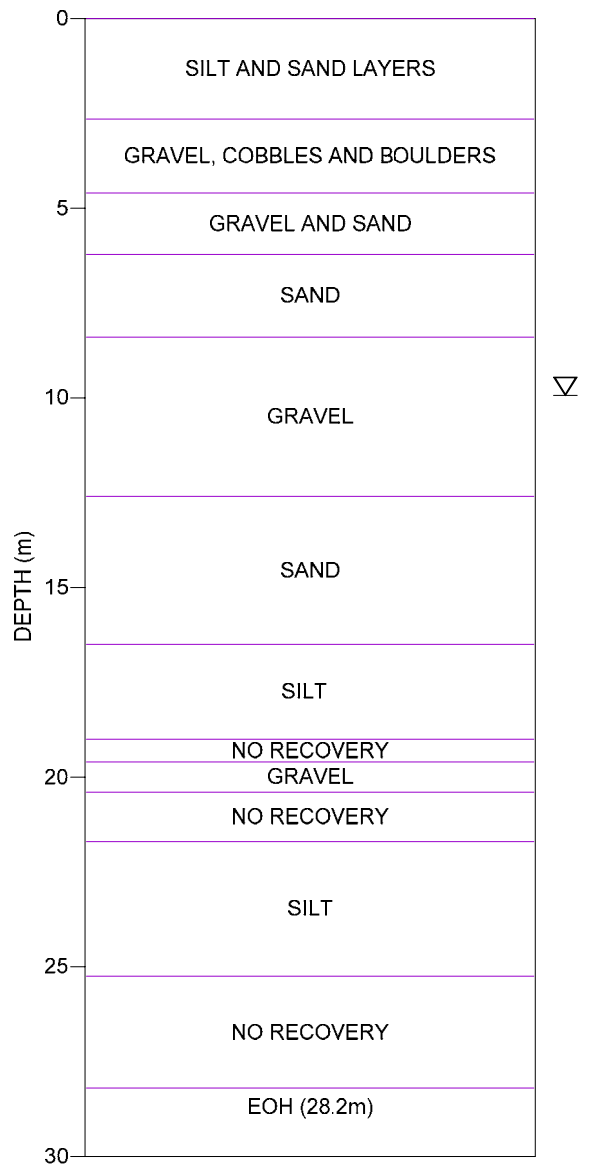
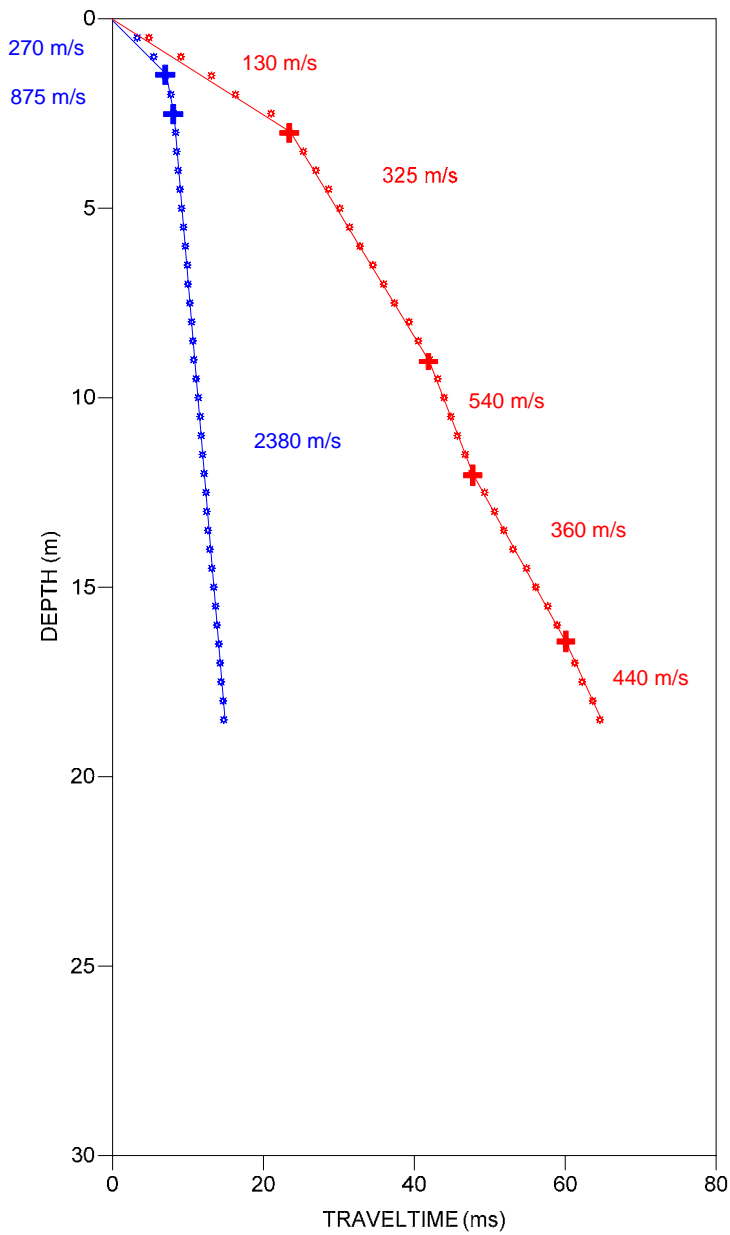
- LEGEND
- • DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-34 P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:250	FIG. 24



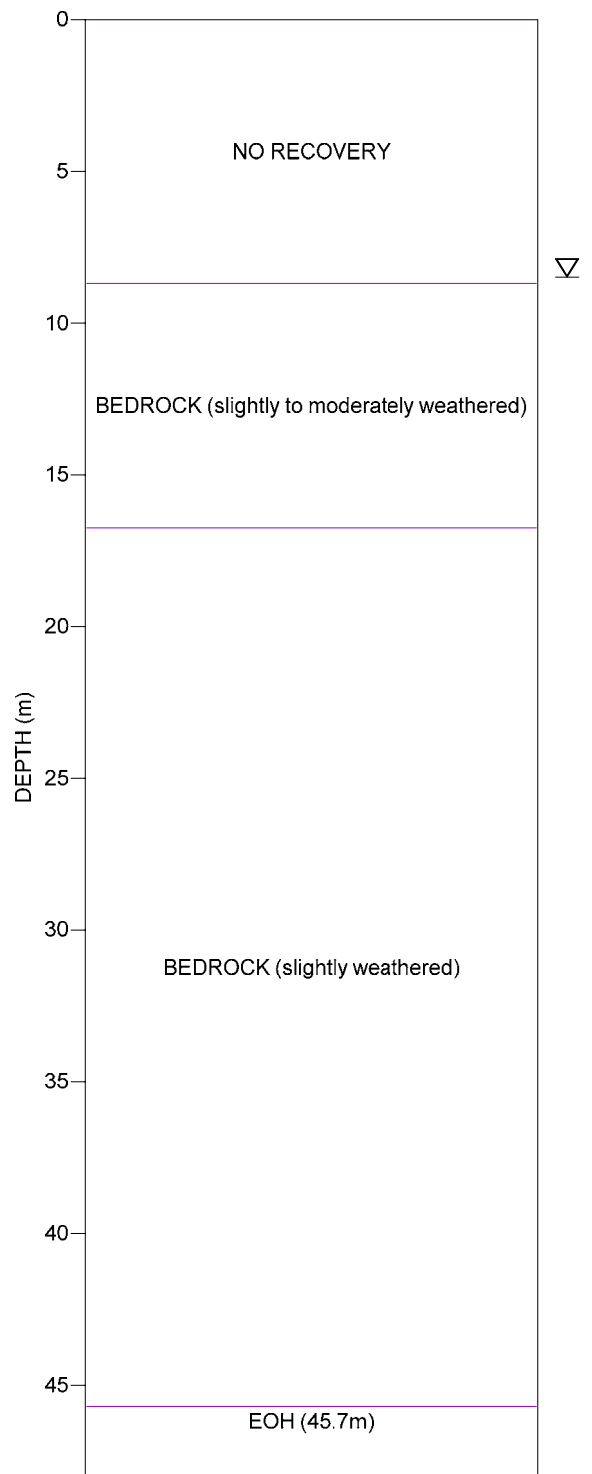
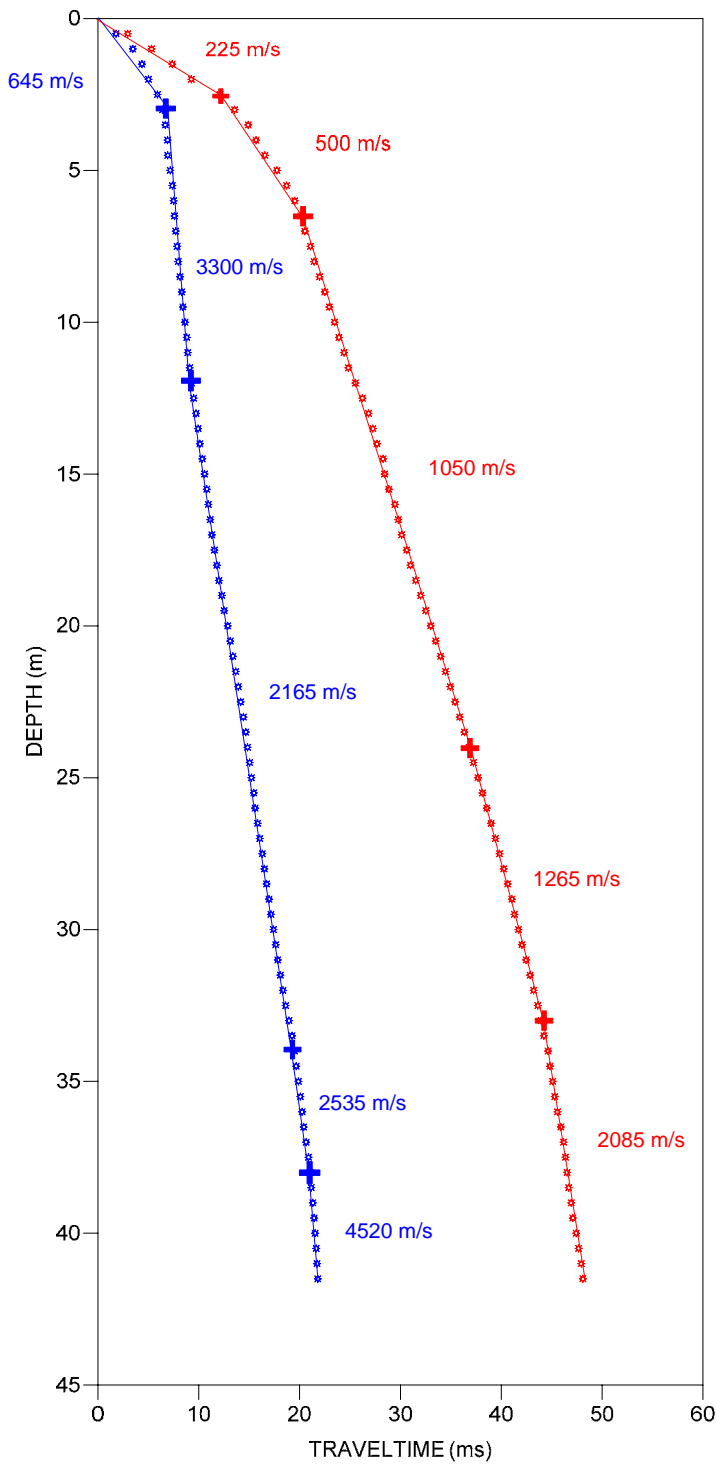
- LEGEND
- * * DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-36 P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:250	FIG. 25



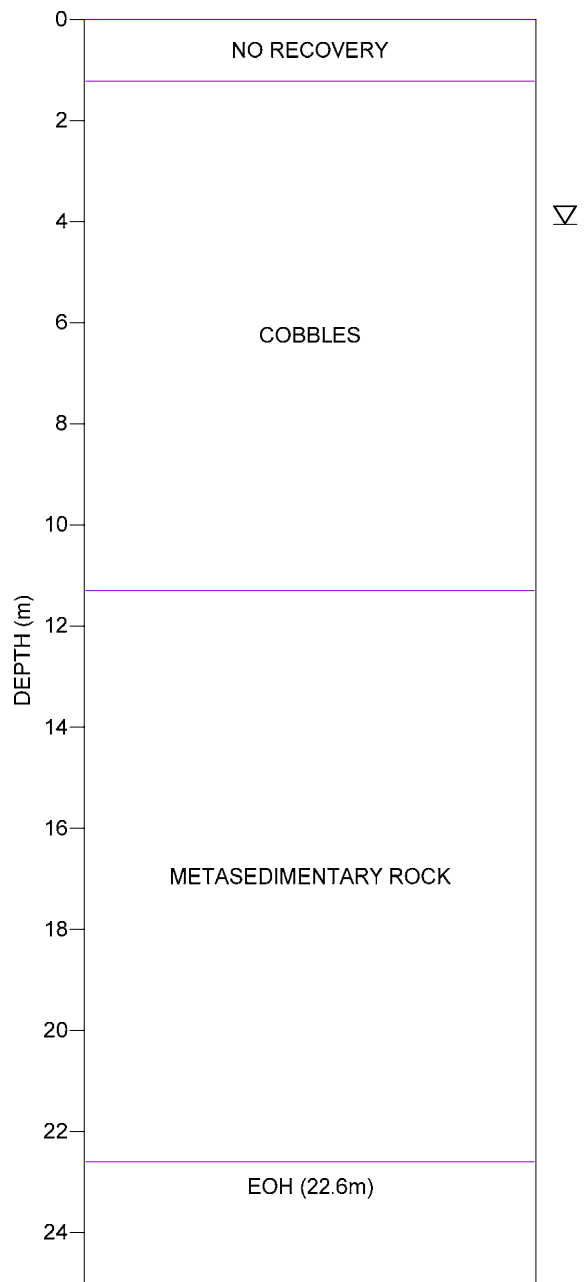
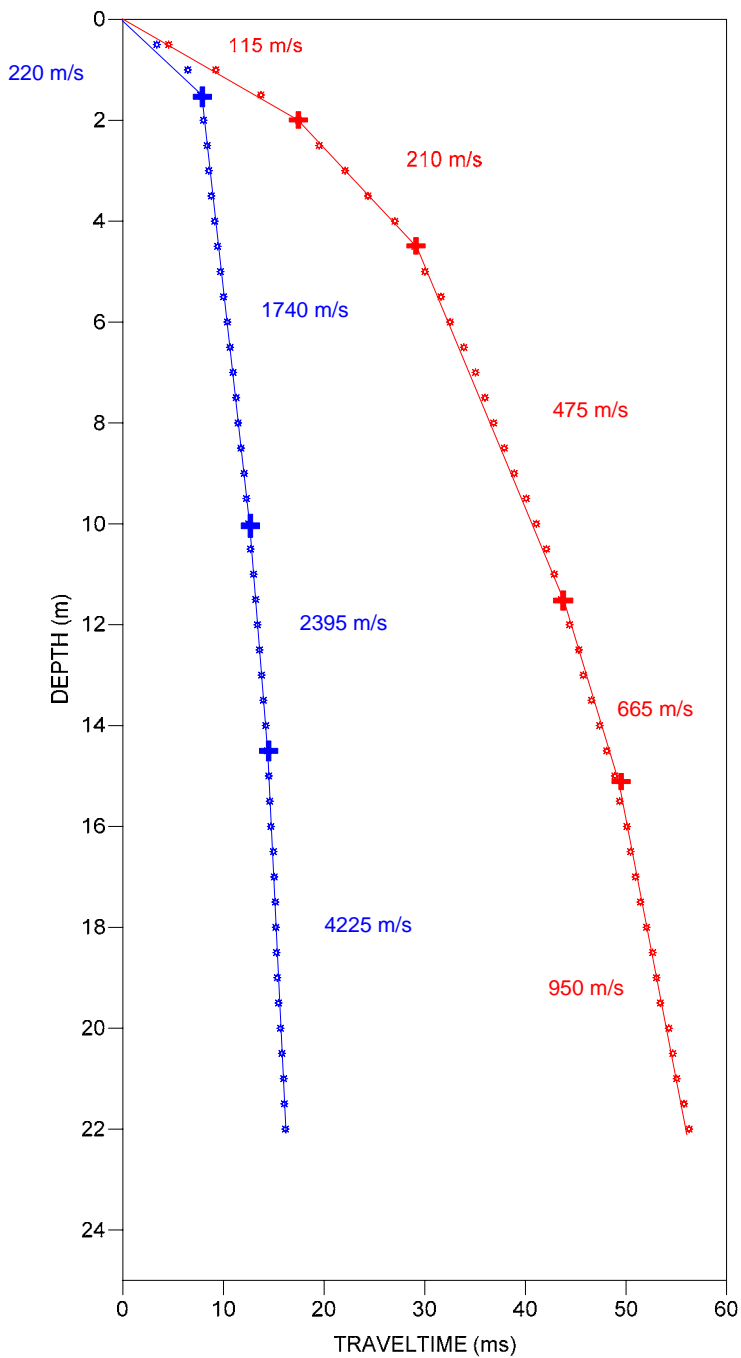
- LEGEND
- • DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC.		
EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-39		
P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:200	FIG. 26



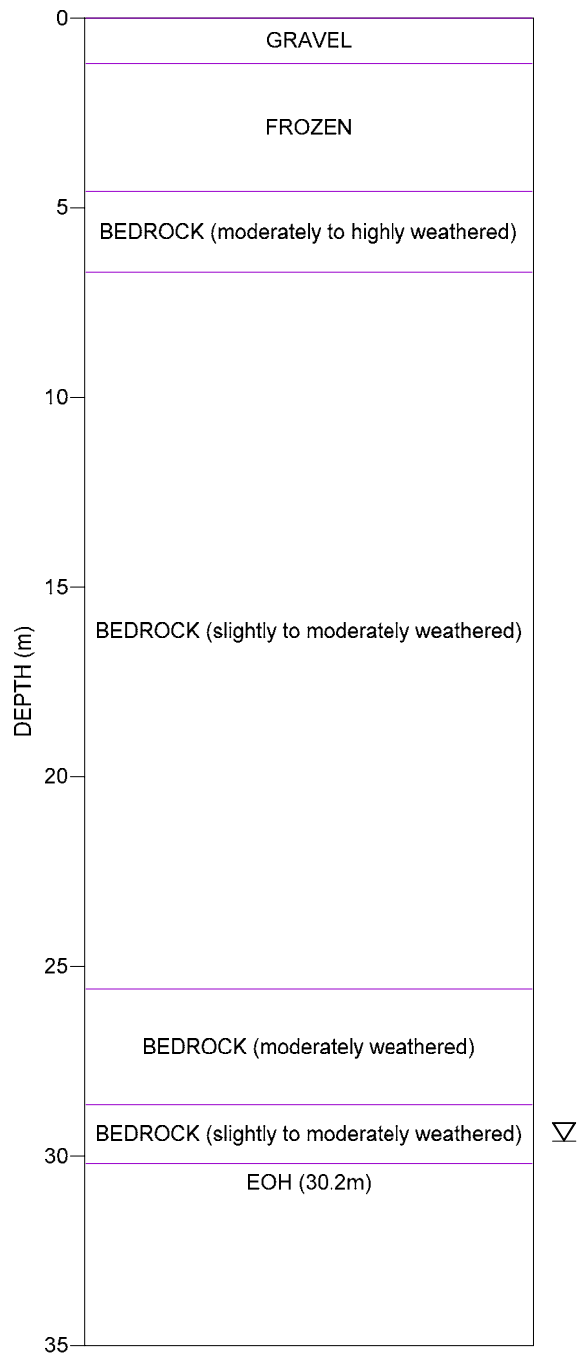
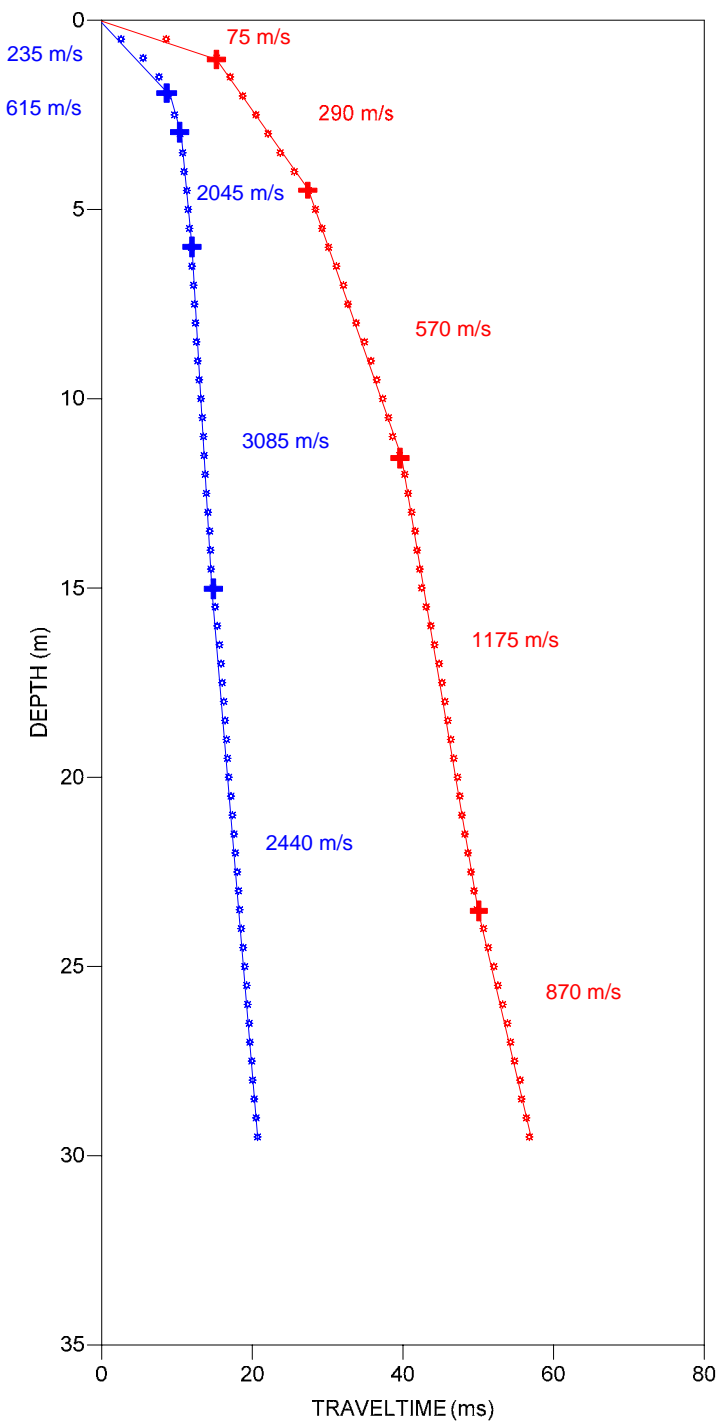
- LEGEND
- * * DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC.		
EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-40B		
P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:250	FIG. 27



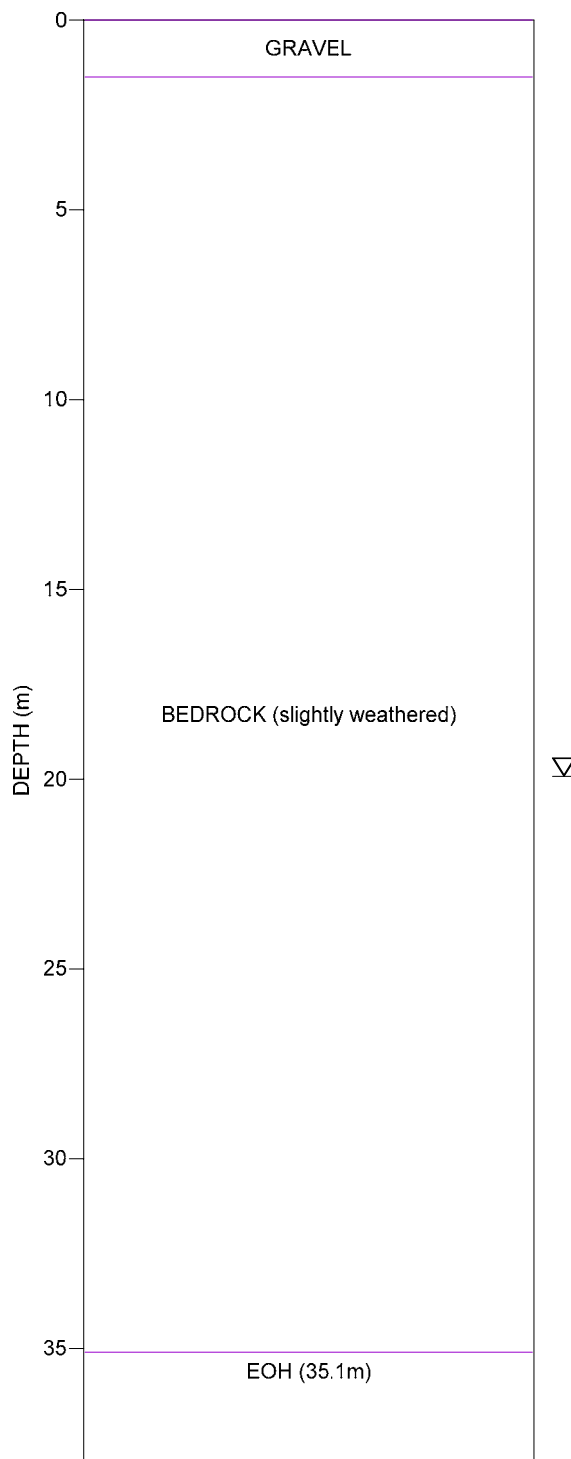
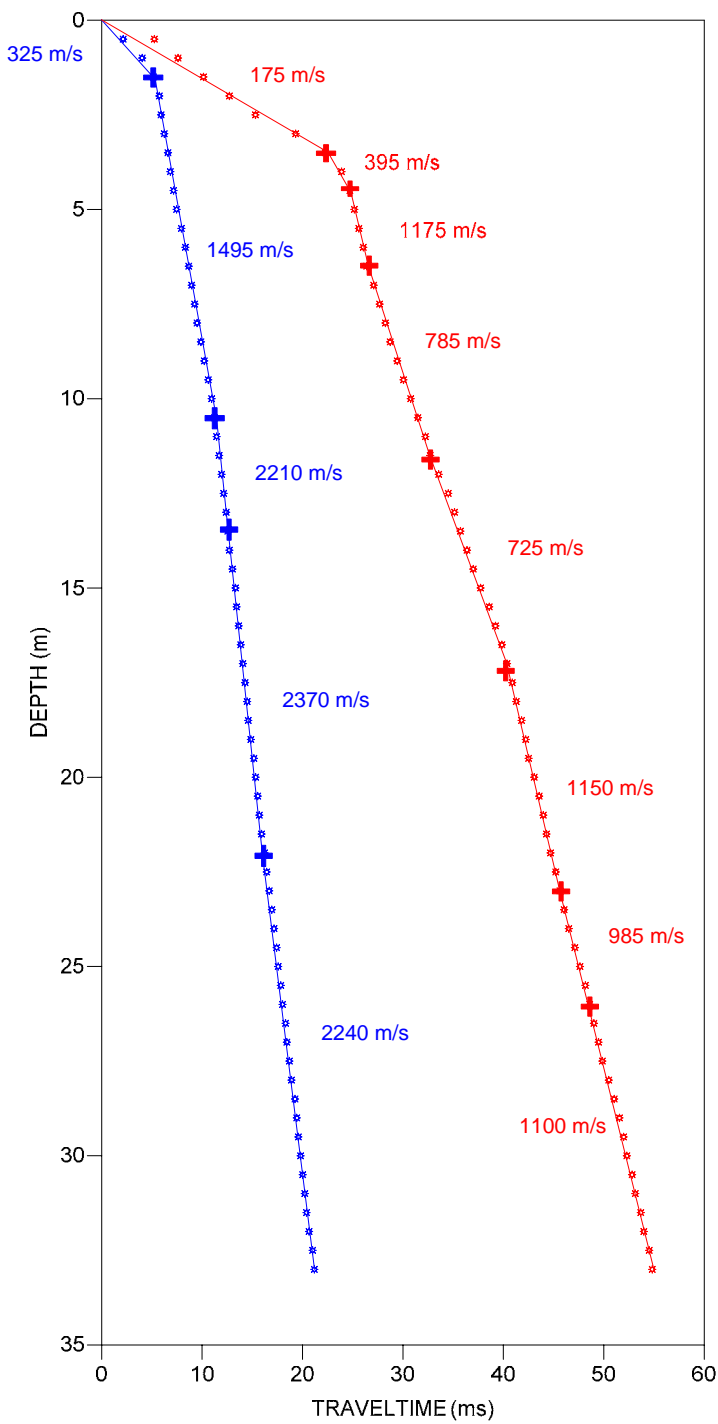
- LEGEND
- * * DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC.		
EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-52		
P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:150	FIG. 28



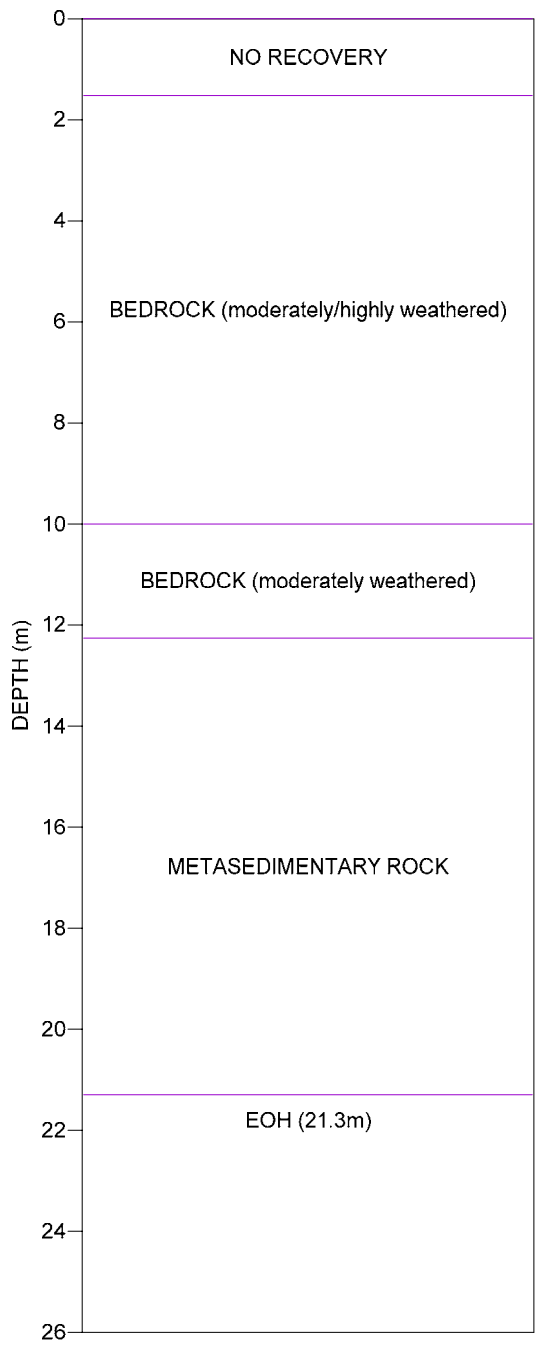
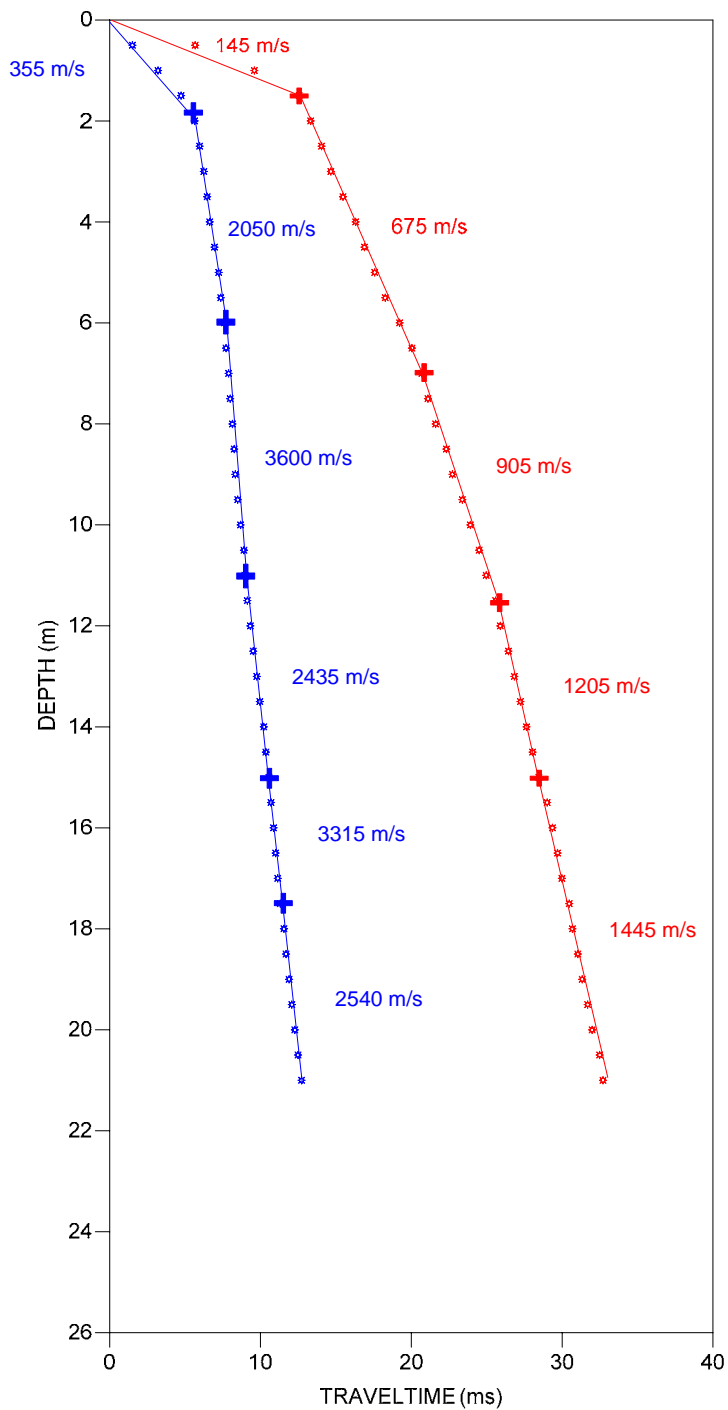
- LEGEND
- • DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-59 P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:200	FIG. 29



- LEGEND
- • DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

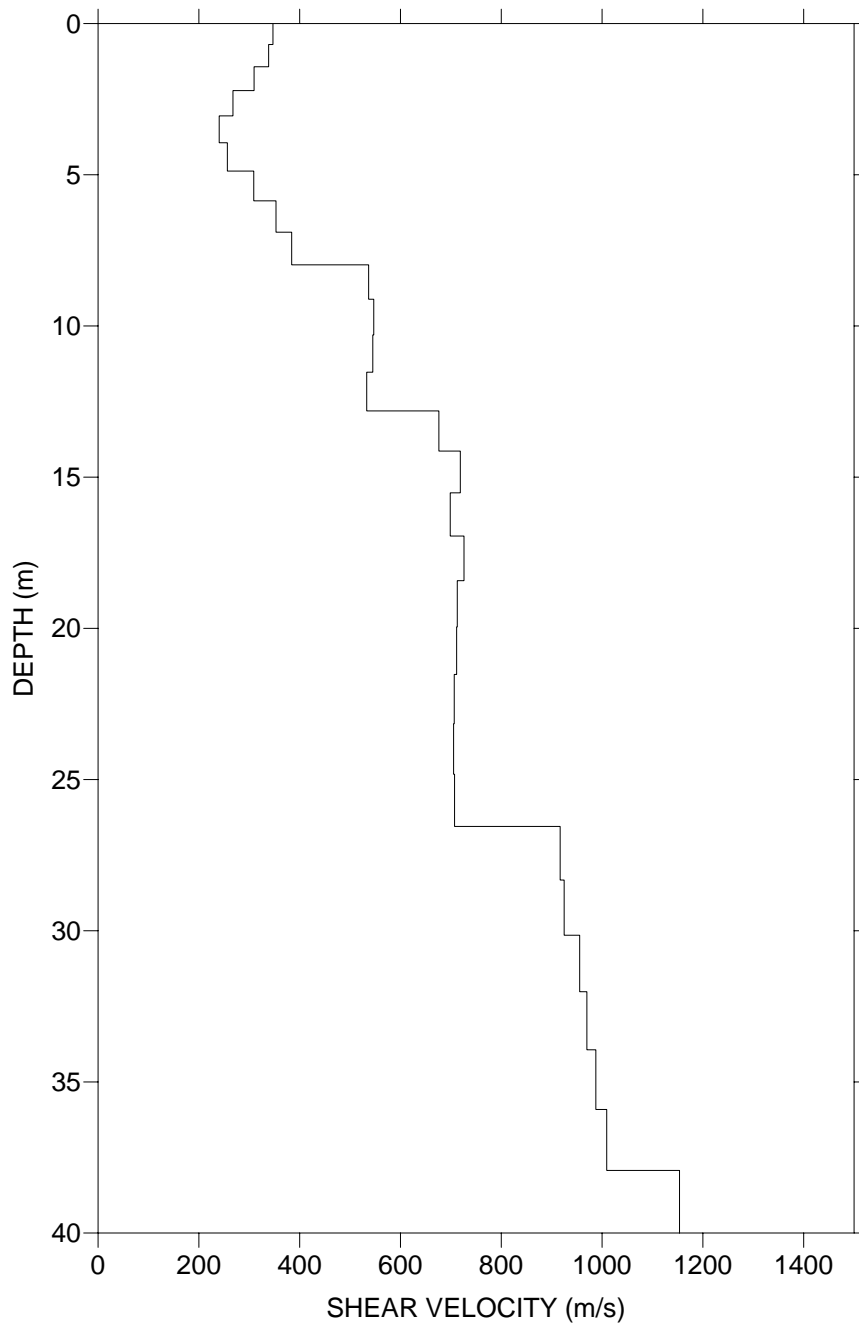
BGC ENGINEERING INC.		
EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-62		
P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:200	FIG. 30



- LEGEND
- * * DATA POINTS
 - + + VELOCITY CHANGE
 - P WAVE
 - S WAVE
 - LAYER BOUNDARIES

BGC ENGINEERING INC.		
EAGLE GOLD PROJECT		
DOWNHOLE SEISMIC SURVEY		
BOREHOLE BH11-69		
P AND S WAVE VELOCITIES		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT. 2011	VSCALE 1:150	FIG. 31

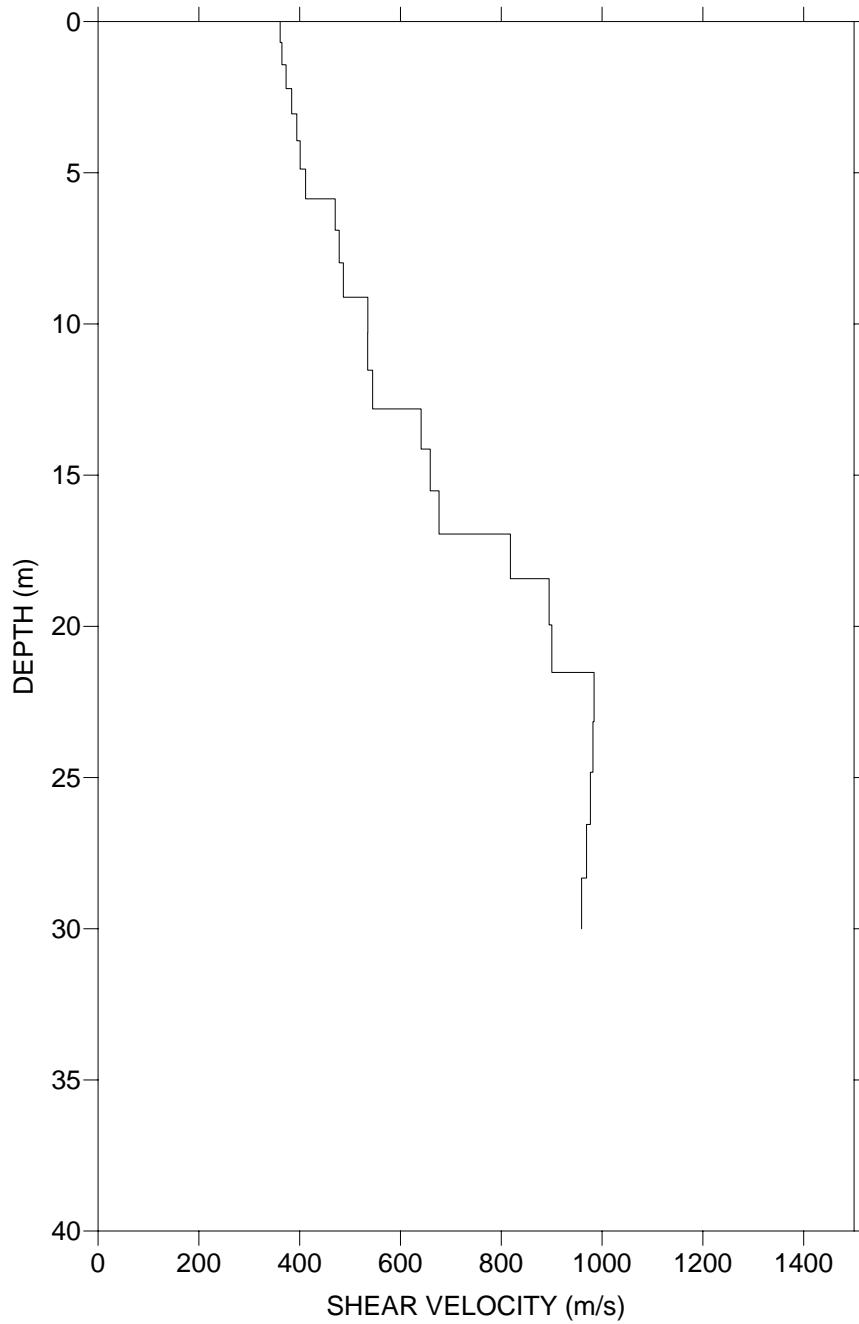
MASW PROFILE SL-2 STATION 300m



BH-BGC11-32

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
MASW PROFILE SL-2 STATION 300M		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT 2011	SCALE 1:250	FIG. 32

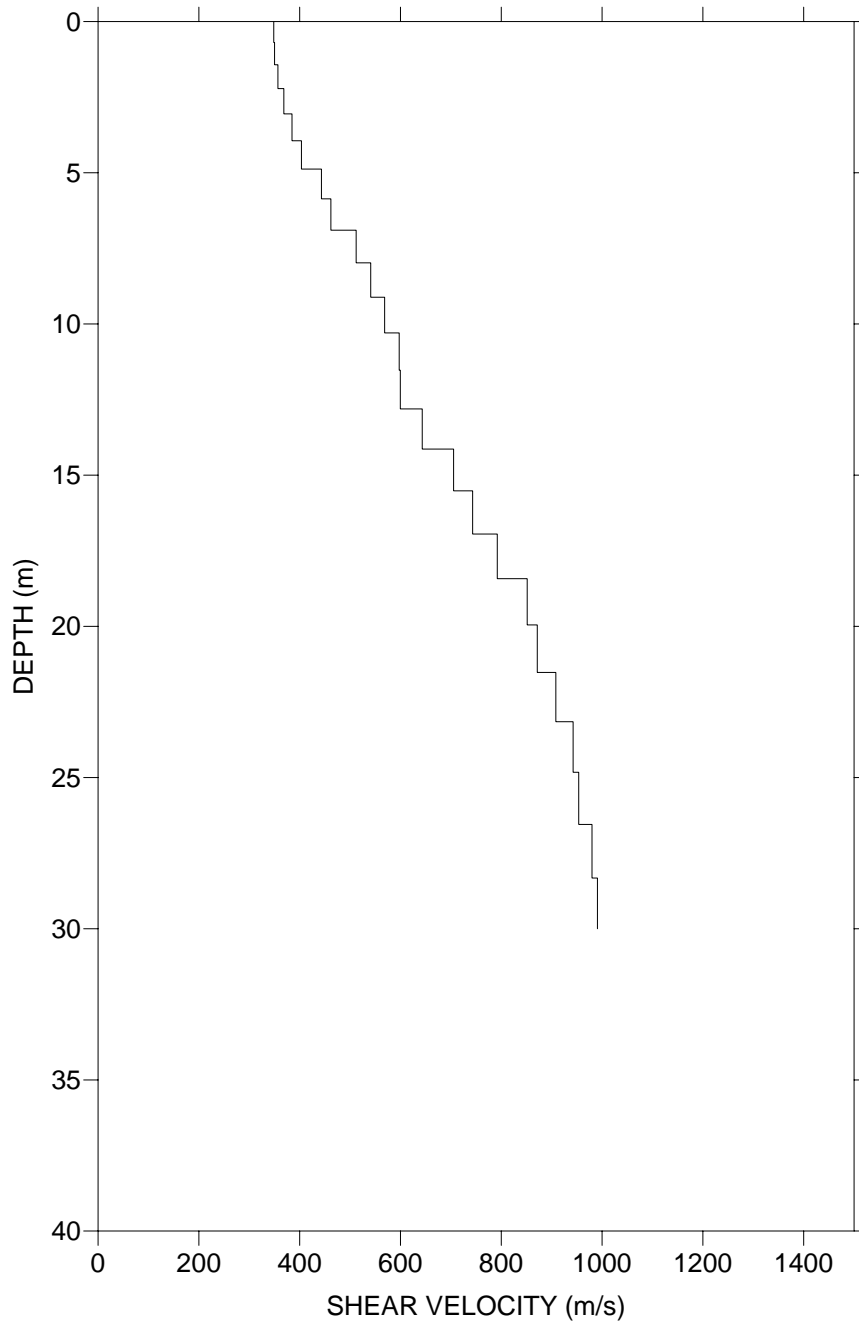
MASW PROFILE SL-7 STATION 155m



BH-BGC11-52

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
MASW PROFILE SL-7 STATION 155M		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT 2011	SCALE 1:250	FIG. 33

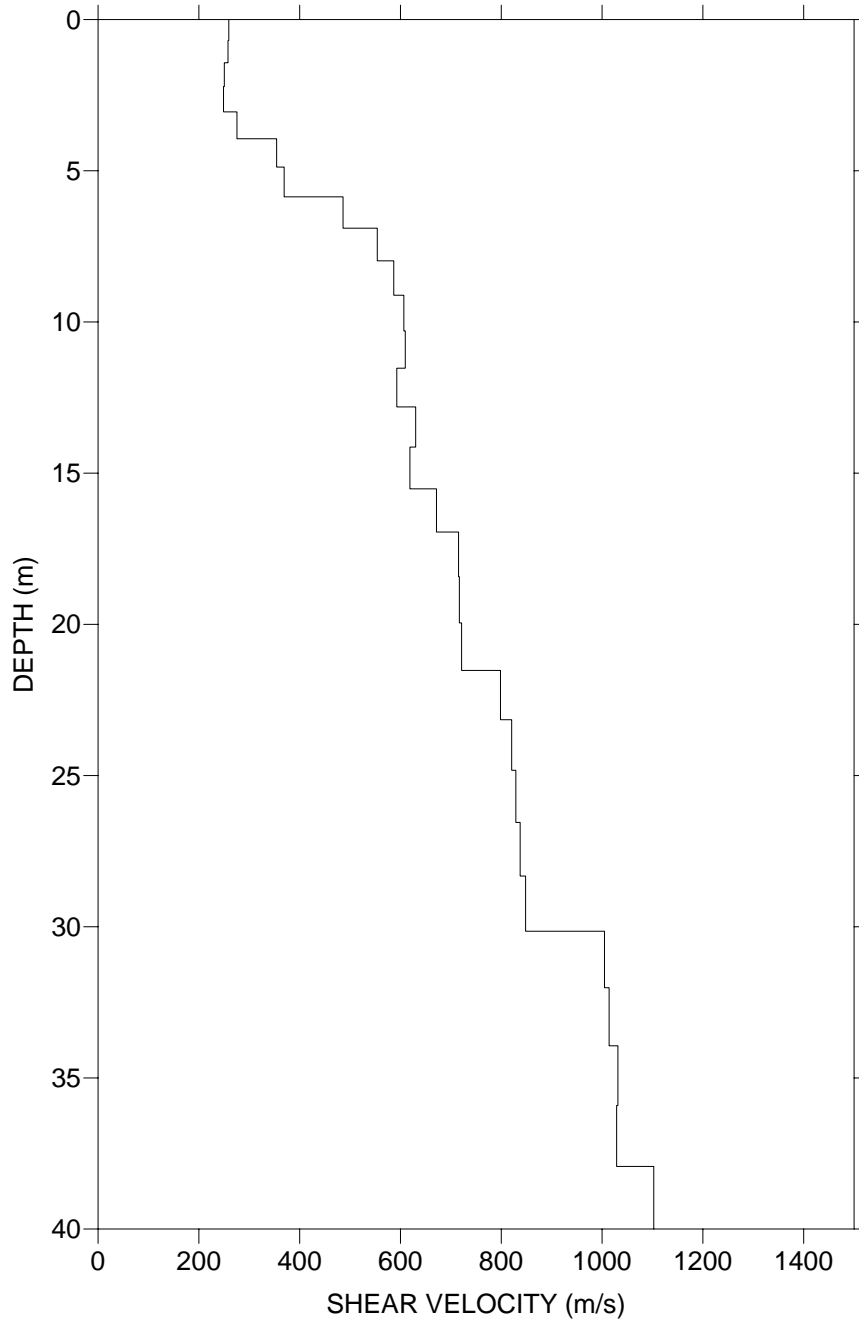
MASW PROFILE SL-7 STATION 185m



BH-BGC11-52

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
MASW PROFILE SL-7 STATION 185M		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT 2011	SCALE 1:250	FIG. 34

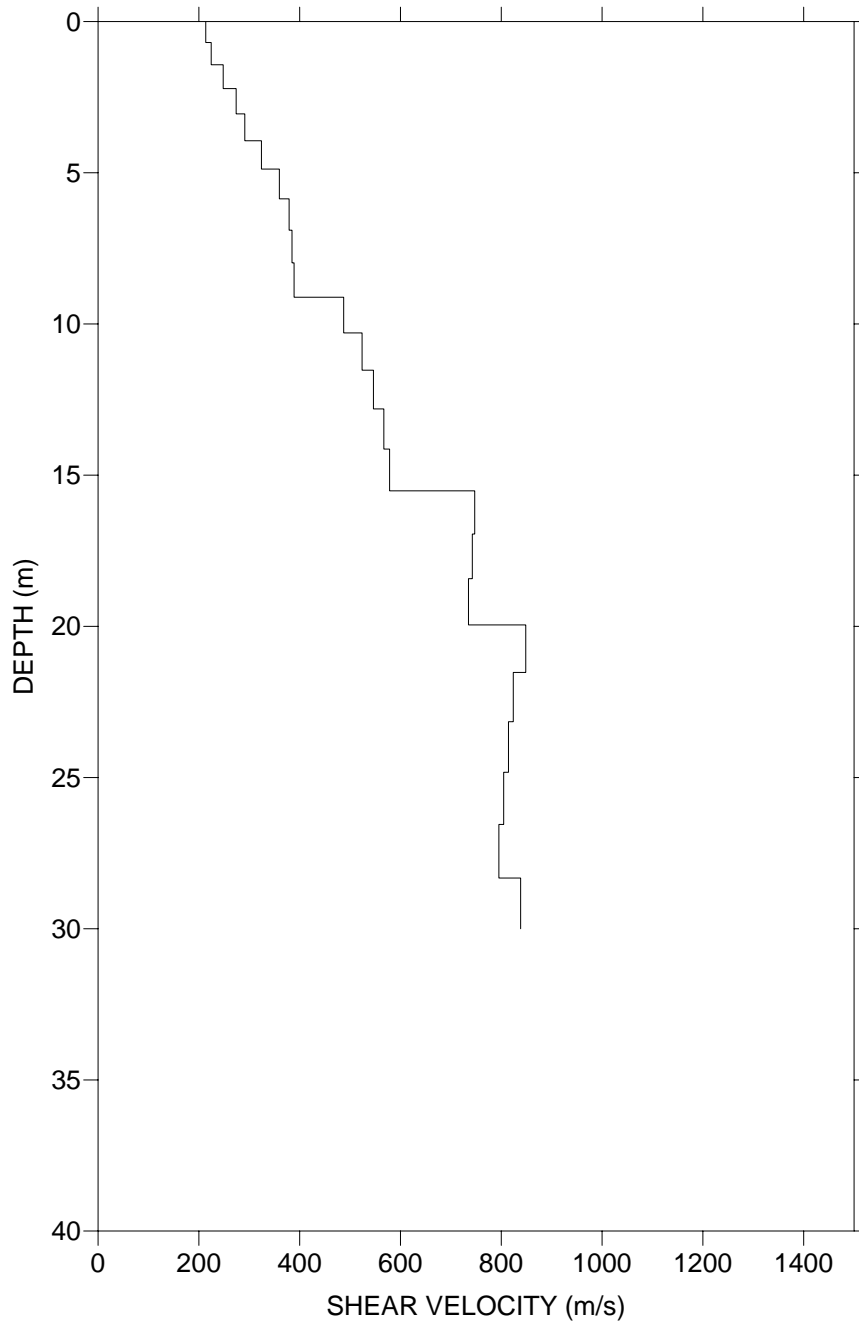
MASW PROFILE SL-33 STATION 165m



BH-BGC11-33

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
MASW PROFILE SL-33 STATION 165M		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT 2011	SCALE 1:250	FIG. 35

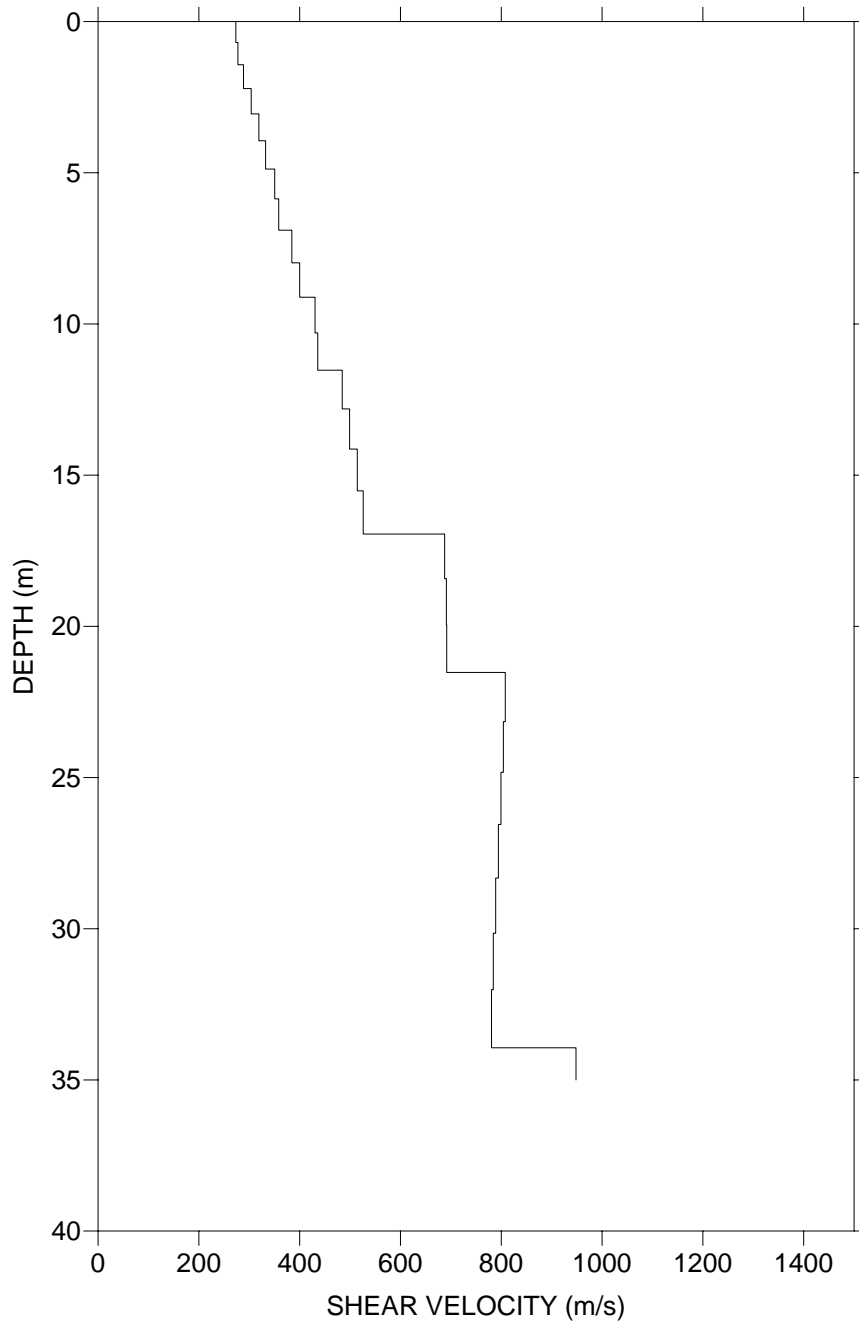
MASW PROFILE SL-33 STATION 360m



BH-BGC11-34

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
MASW PROFILE SL-33 STATION 360M		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT 2011	SCALE 1:250	FIG. 36

MASW PROFILE SL-33 STATION 380m



BH-BGC11-34

BGC ENGINEERING INC. EAGLE GOLD PROJECT		
GEOPHYSICAL SURVEY		
MASW PROFILE SL-33 STATION 380M		
FRONTIER GEOSCIENCES INC.		
DATE: SEPT 2011	SCALE 1:250	FIG. 37

APPENDIX I PLATE LOAD TESTS

1.0 INTRODUCTION

This Appendix documents the methods and results of plate load testing undertaken during the summer 2011 geotechnical investigations for mine site infrastructure.

Plate load tests were carried out by BGC personnel in August 2011 to assess the stiffness of exposed subgrade materials at or near selected proposed building pads. The aims of this testing program were to:

- Estimate the modulus of subgrade reaction of exposed subgrade materials.
- Estimate the yield stress pressure when possible.
- Test various diameters of plate to estimate the variation of material stiffness with bearing pressure and investigate the influence of plate size for extrapolation of results to full size footings.

2.0 PLATE LOAD TEST PROCESS DESCRIPTION

2.1. Test Locations

The tests were conducted in three excavated pits: two located within the proposed Plant Site footprint and one in the proximity of the proposed Secondary Crusher. The test areas were chosen for their proximity to the footprint of high bearing load structures and for their expected shallow occurrence of targeted rock formation. The pads were cautiously cut into undisturbed ground with an excavator under the supervision of BGC staff.

Tables I-1 and I-2 summarize the locations and characteristics of the tests performed. Refer to Drawing 07 for details of the layout of the plate load tests.

2.2. Installation

Specific test locations were identified in undisturbed representative subgrade and their surfaces were smoothed manually. For each location, the surface was cleaned of loose materials and a transition layer of clean coarse silica sand was placed at the test area to create a continuous and even contact between the base and the plates.

The plates were positioned parallel to the base of the rear differential of a D9H bulldozer that provided the approximately 48t reaction force for the test. A 25t hydraulic jack powered by a hand pump was used to apply the load to the ground and a 10,000 psi gauge was used to set and monitor the pressure increments. Refer to Photo 1 for a general arrangement of the test set up.

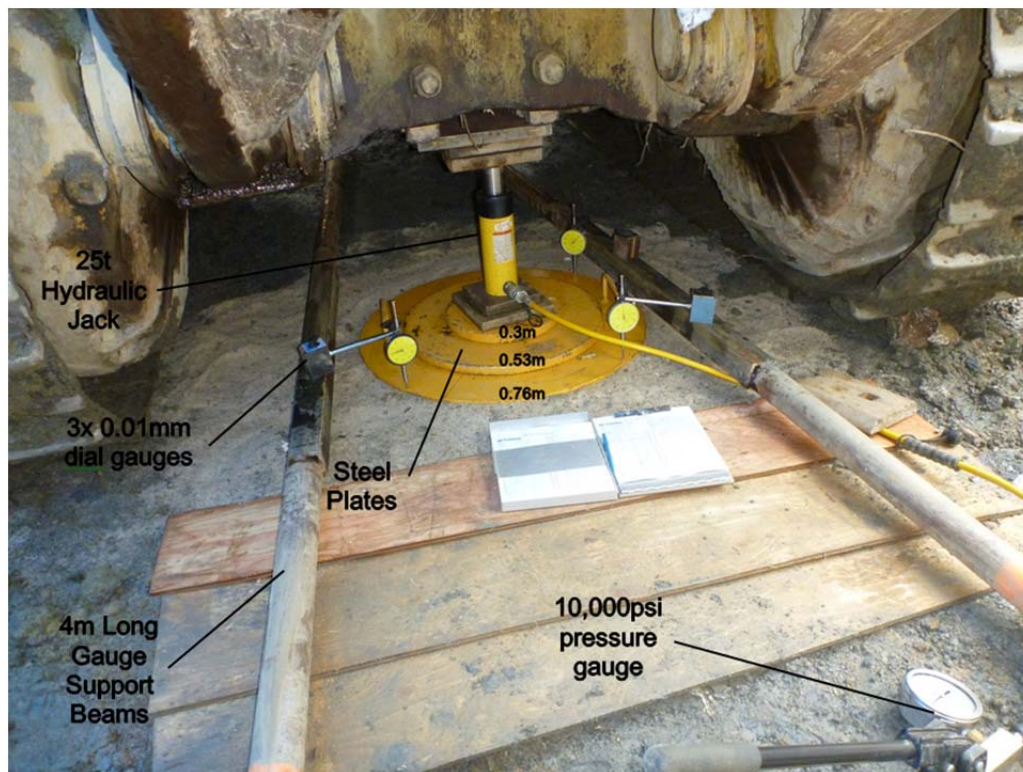


Photo 1. Details of the Instrumentation Set Up

2.3. Settlement monitoring and load increments

Vertical plate movements were measured to 0.01 mm resolution using three dial gauges positioned as shown in Photo 1.

The dial gauges were mounted on two 4m horizontal reference beams with supporting legs resting on the ground beyond the area of influence of the test. The load pattern was selected to obtain roughly 10 to 20 increments between the start and the maximum available pressure. The maximum pressure was limited by the capacity of the jack to 10,000 psi. After an intermediate load was reached, pressure was incrementally decreased and then re-increased, inducing an unload-reload cycle. Each load increment was maintained until the measured movements dropped to less than 0.01 mm/5 min.

2.4. Plate Sizes

Different plate sizes were tested at different locations to estimate variation of material stiffness with bearing pressure, and to investigate the influence of the load applied to different surface areas (plate diameters) on settlement and stiffness. Four different plate sizes were used in the plate load tests: 0.3 m, 0.53 m, 0.76 m and 1.2 m. All plates were 25mm thick and made of high grade steel.

2.5. Instruments Calibration

After completion of the field testing, the hydraulic gear was calibrated by an accredited laboratory (Acuren Group Inc, Richmond, BC). Loads estimated during testing and presented in this appendix have been corrected accordingly. The compression load calibration results are attached in the Calibration Report following the figures at the end of this appendix.

Table I-1. Summary of Plate Load Tests at the Plant Site

Location	Test #	Date	Approximate UTM Coordinates (NAD 84)		Depth (m) Below Grade	Plate Diameter (m)	Max. Pressure Applied (kPa)	Max. Gauge Deformation (mm)
			N (m)	E (m)				
TP-BGC11-103	PT-BGC11-01	10/08/11	7101124	458820	5.5	0.76	280	1.9
	PT-BGC11-02	11/08/11			5.0	0.76	390	2.8
	PT-BGC11-03	12/08/11			5.0	0.53	980	2.0
	PT-BGC11-04	13/08/11			5.0	0.3	3100	9.6
	PT-BGC11-05	14/08/11			5.5	0.53	780	4.7
TP-BGC11-105	PT-BGC11-06	15/08/11	7101190	458939	8.5	1.2	190	6.9
	PT-BGC11-07	16/08/11			8.5	0.76	480	14.4
	PT-BGC11-08	17/08/11			8.5	0.76	480	12.9
	PT-BGC11-09	22/08/11			8.5	0.53	490	14.2

Table I-2. Summary of Plate Load Test at the Secondary Crusher Area

Location	Test #	Date	UTM Coordinates (NAD 84)		Depth (m) Below Grade	Plate Diameter (m)	Max. Pressure Applied (kPa)	Max. Gauge Deformation (mm)
			N (m)	E (m)				
Between OC-BGC11-47 and BH-BGC11-36	PT-BGC11-10	23/08/11	7100285	459722	3.2	0.53	880	4.1
	PT-BGC11-11	24/08/11			3.2	0.3	2790	5.0
	PT-BGC11-12	25/08/11	7100288	459722	3.2	0.53	960	4.8
	PT-BGC11-13	26/08/11	7100291	459722	3.8	0.3	2160	14.7
	PT-BGC11-14	27/08/11	7100295	459722	4.2	0.53	980	2.1

3.0 DATA RESULTS AND ANALYSIS

The applied plate pressures and recorded movements for each dial gauge were plotted to produce plate load test curves.

The vertical subgrade reaction moduli for the virgin curve, k_v , and for the rebound curve, $k_{v(u-r)}$, were determined for every plate load test curve. From the load/settlement relationship, the vertical subgrade reaction modulus for the virgin curve is calculated as $k_v = dP/d\delta$, derivative of the tangent line that was most reflective of the ground behavior post immediate settlement (>60 KPa) and prior to occurrence of yield behavior (when applicable). The vertical subgrade reaction modulus for the rebound curve is calculated as $k_{v(u-r)} = dP_{(u-r)}/d\delta$, derivative of the tangent to the most representative section of the reload section of the unload/reload loop, before the virgin loading gradient of the load/settlement relationship is recovered. Figures I-1 to I-14 illustrates the plate load test curves for each individual test.

Results and test details are summarized in Table I-3 and Table I-4 for the plant site area and the secondary crusher area, respectively.

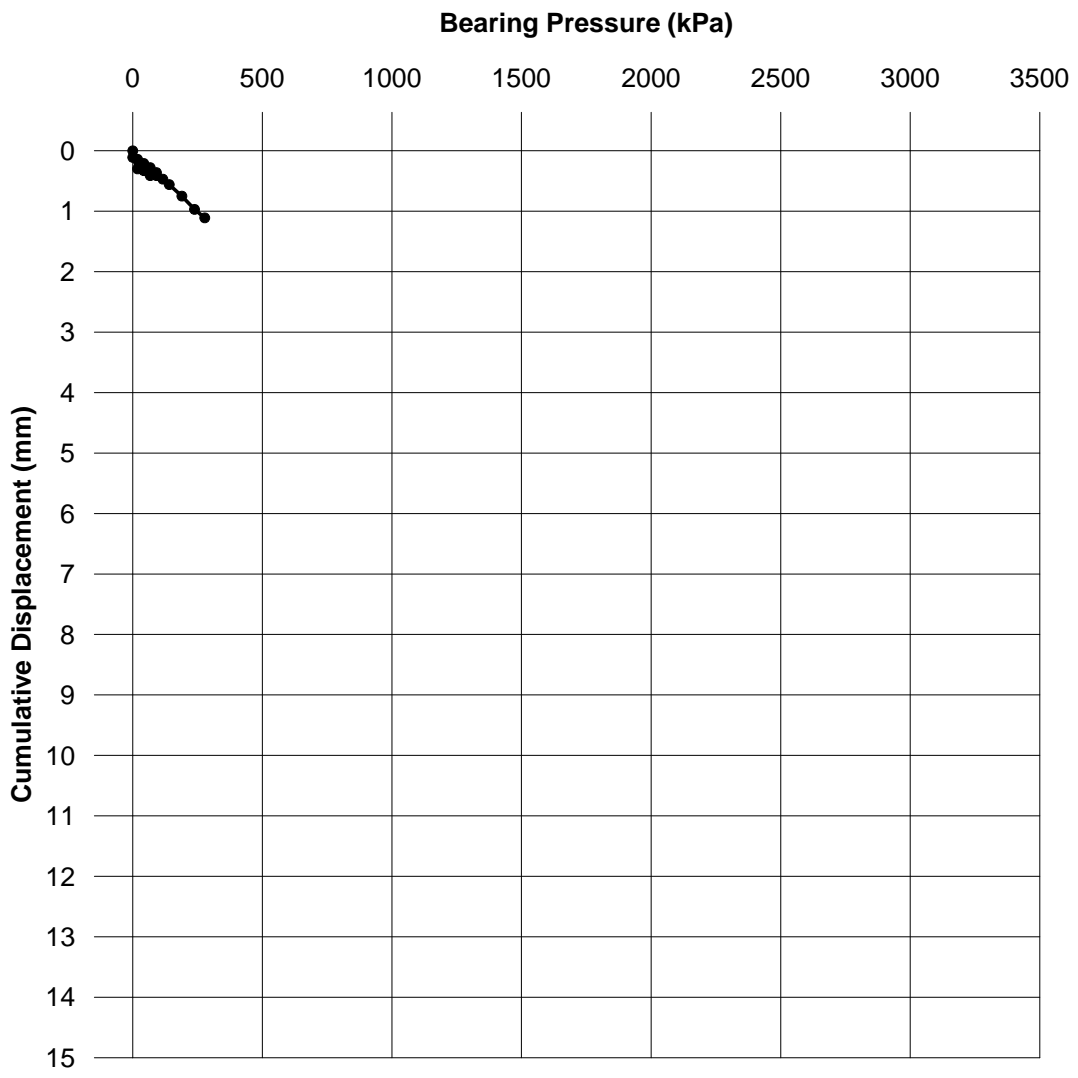
Table I-3. Summary of Subgrade Reaction Modulus from Plate Load Tests at the Plant Site

Location	Test #	Depth Below Grade (m)	Plate Diameter (m)	Vertical Subgrade Reaction Modulus (kPa/mm ³)	
				Virgin Curve, k_v	Rebound Curve, $k_{v(u-r)}$
TP-BGC11-103	PT-BGC11-01	5.5	0.76	130	220
TP-BGC11-103	PT-BGC11-02	5	0.76	140	310
TP-BGC11-103	PT-BGC11-03	5	0.53	130	290
TP-BGC11-103	PT-BGC11-04	5	0.3	540	1170
TP-BGC11-103	PT-BGC11-05	5.5	0.53	130	250
TP-BGC11-105	PT-BGC11-06	8.52	1.2	30	60
TP-BGC11-105	PT-BGC11-07	8.52	0.76	40	80
TP-BGC11-105	PT-BGC11-08	8.52	0.76	50	120
TP-BGC11-105	PT-BGC11-09	8.52	0.53	40	120

Table I-4. Summary of Subgrade Reaction Modulus from Plate Load Tests at the Secondary Crusher

Location	Test #	Depth Below Grade (m)	Plate Diameter (m)	Vertical Subgrade Reaction Modulus (kPa/mm ³)	
				Virgin Curve, kv	Rebound Curve, kv(u-r)
Between OC-BGC11-47 and BH-BGC11-36	PT-BGC11-10	3.2	0.53	190	600
	PT-BGC11-11	3.2	0.3	640	1600
	PT-BGC11-12	3.2	0.53	200	500
	PT-BGC11-13	3.8	0.3	220	1000
	PT-BGC11-14	4.2	0.53	400	1040

FIGURES



NOTES:
 Test: PT-BGC11-01
 Depth: 5.5 m
 Plate Diameter: 0.76 cm

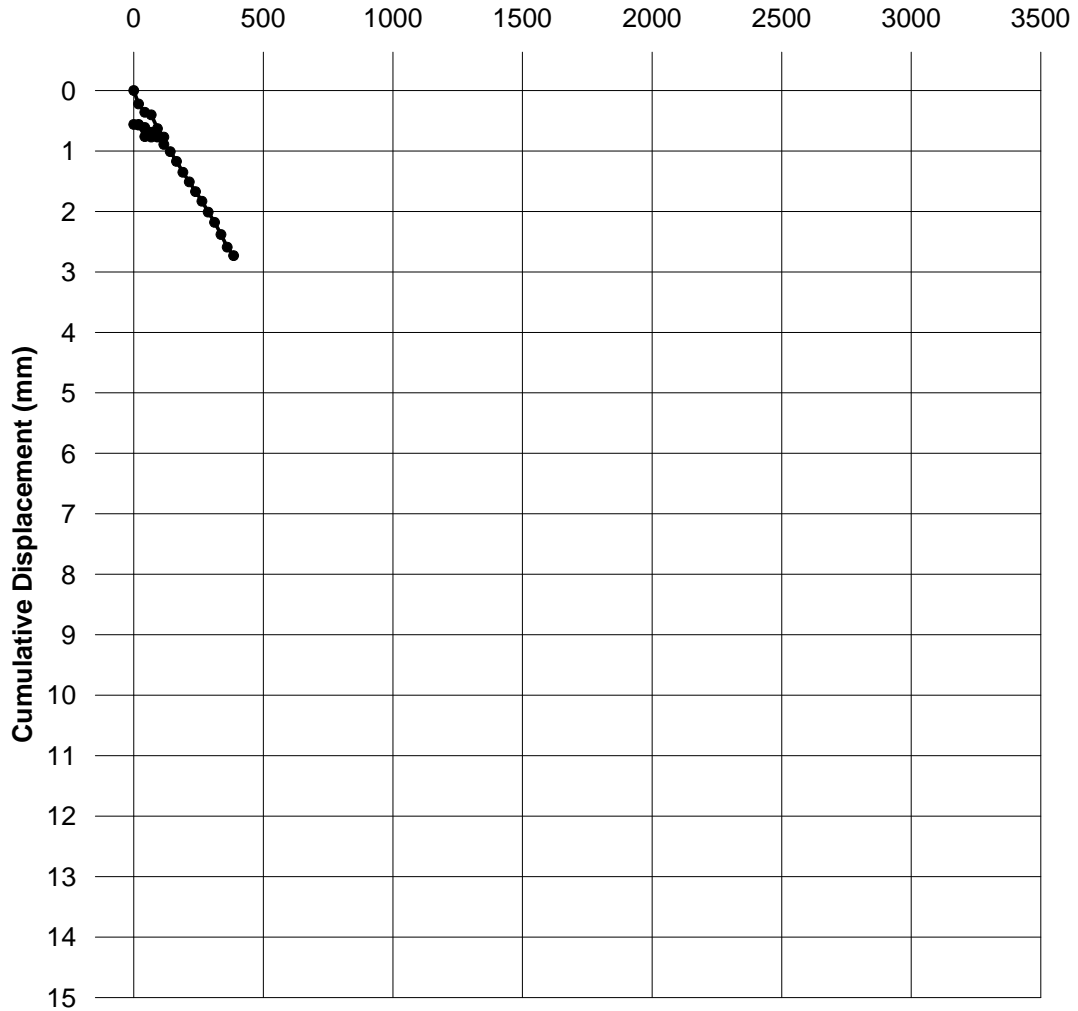
Subgrade Reaction Modulus Summary:
 Virgin Curve: $k_v = 130 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 220 \text{ kPa/mm}$

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CLIENT: VICTORIA GOLD CORP.	PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT		
	TITLE: PLATE TEST RESULTS PT-BGC11-01		
	PROJECT No. 0792-006	FIG No. I-1	REV. 0

N:\BGC\Projects\0792_Victoria Gold\006 EG Infrastructure\2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix I Plate Load Testing\Figures\Grapher plots for report

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.

Bearing Pressure (kPa)



NOTES:

Test: PT-BGC11-02
 Depth: 5 m
 Plate Diameter: 76 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 140 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 310 \text{ kPa/mm}$

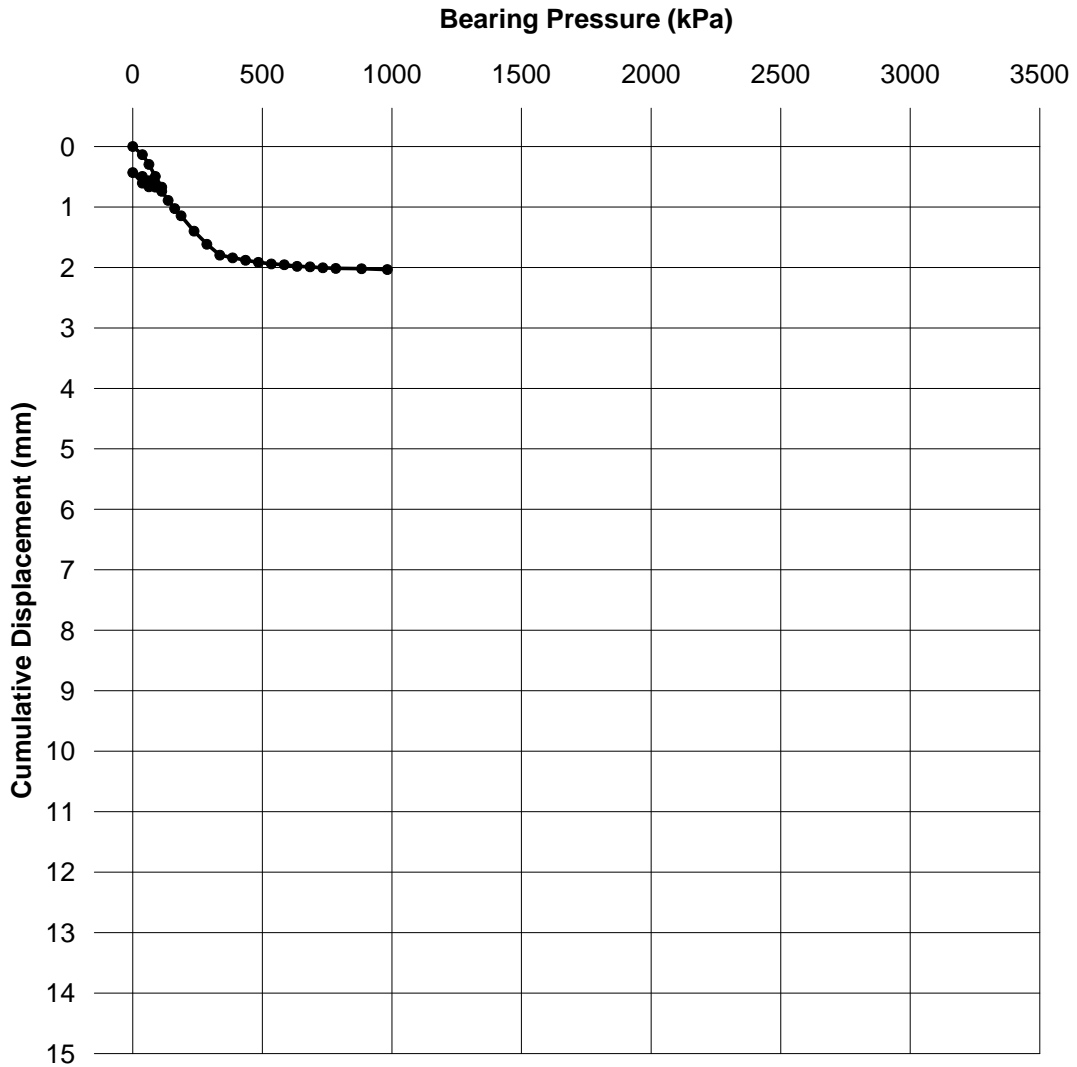
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DATE: JANUARY 2012	CHECKED: PQ
DRAWN: JND	APPROVED: PQ

CLIENT: VICTORIA GOLD CORP.

PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-02		
PROJECT No. 0792-006	FIG No. I-2	REV. 0



NOTES:

Test: PT-BGC11-03
 Depth: 5 m
 Plate Diameter: 53.3 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 130 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 290 \text{ kPa/mm}$

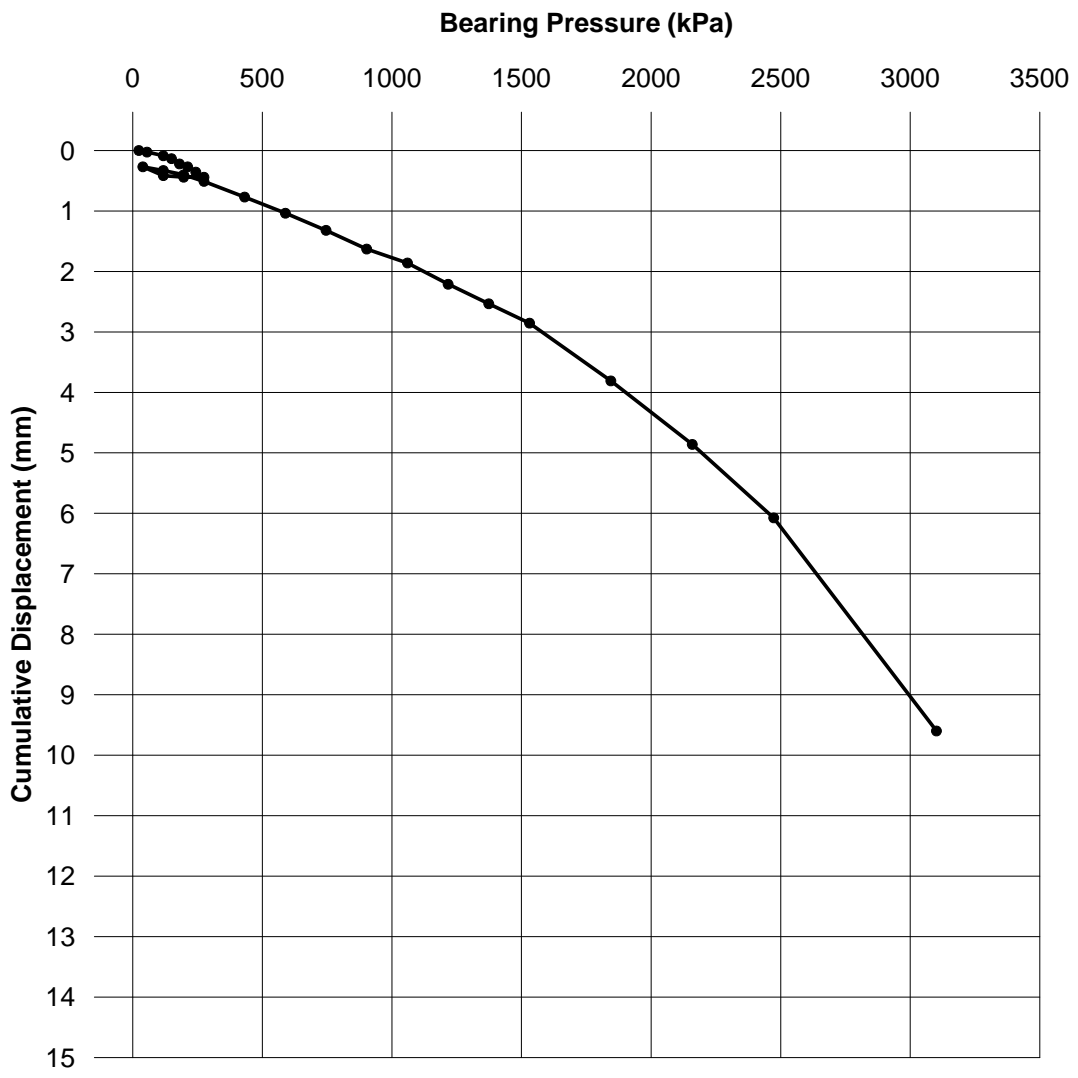
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CLIENT: VICTORIA GOLD CORP.

PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-03		
PROJECT No. 0792-006	FIG No. I-3	REV. 0



NOTES:
 Test: PT-BGC11-04
 Depth: 5 m
 Plate Diameter: 30 cm

Subgrade Reaction Modulus Summary:
 Virgin Curve: $k_v = 540 \text{ kPa/mm}$
 Reboud Curve $k_{v(u-r)} = 1170 \text{ kPa/mm}$

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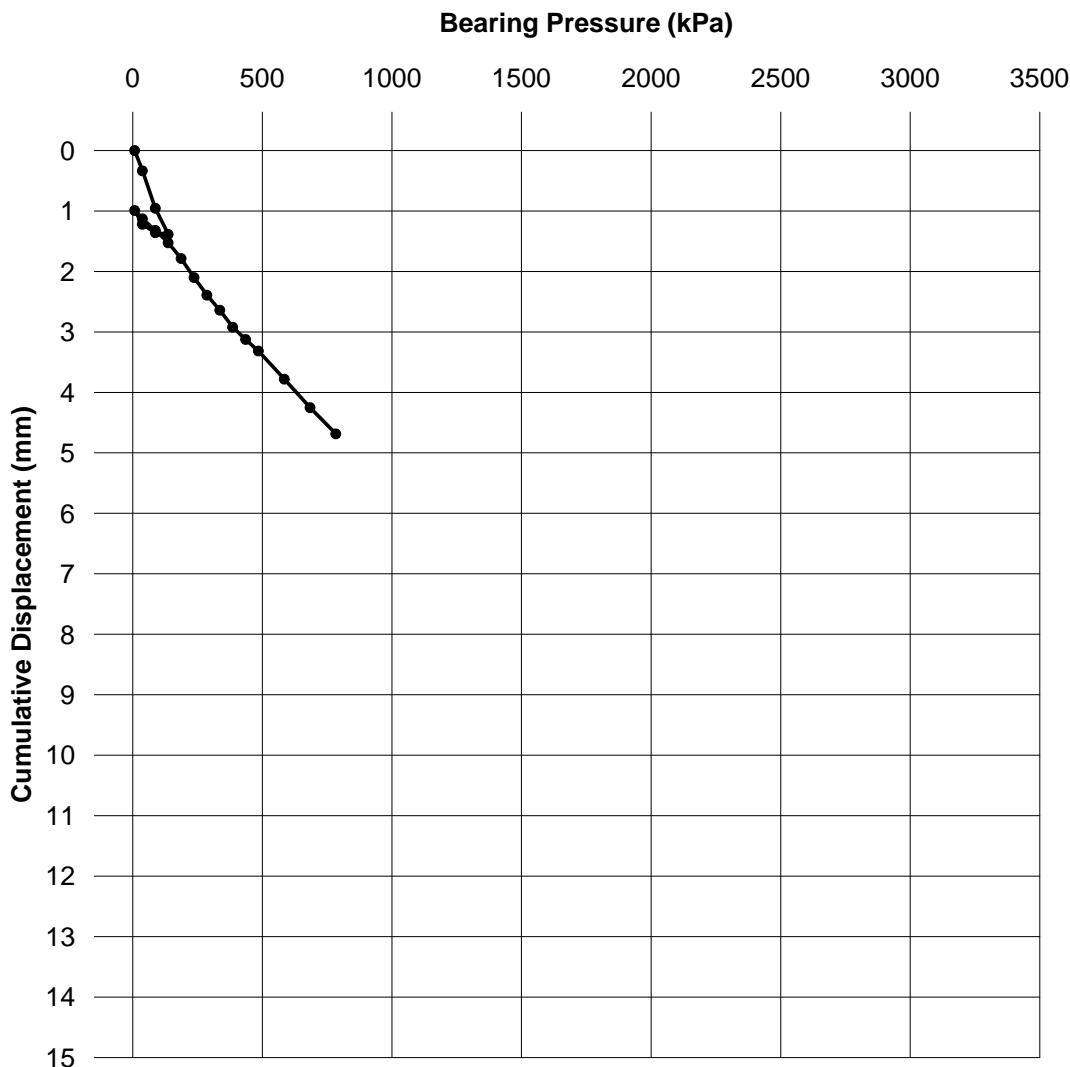
CLIENT: VICTORIA GOLD CORP.

PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-04		
PROJECT No.	FIG No.	REV.
0792-006	I-4	0

N:\BGC\Projects\0792_Victoria Gold\006 EG Infrastructure 2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix I Plate Load Testing\Figures\Grapher plots for report

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



NOTES:
 Test: PT-BGC11-05
 Depth: 5.5 m
 Plate Diameter: 53.3 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 130 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 250 \text{ kPa/mm}$

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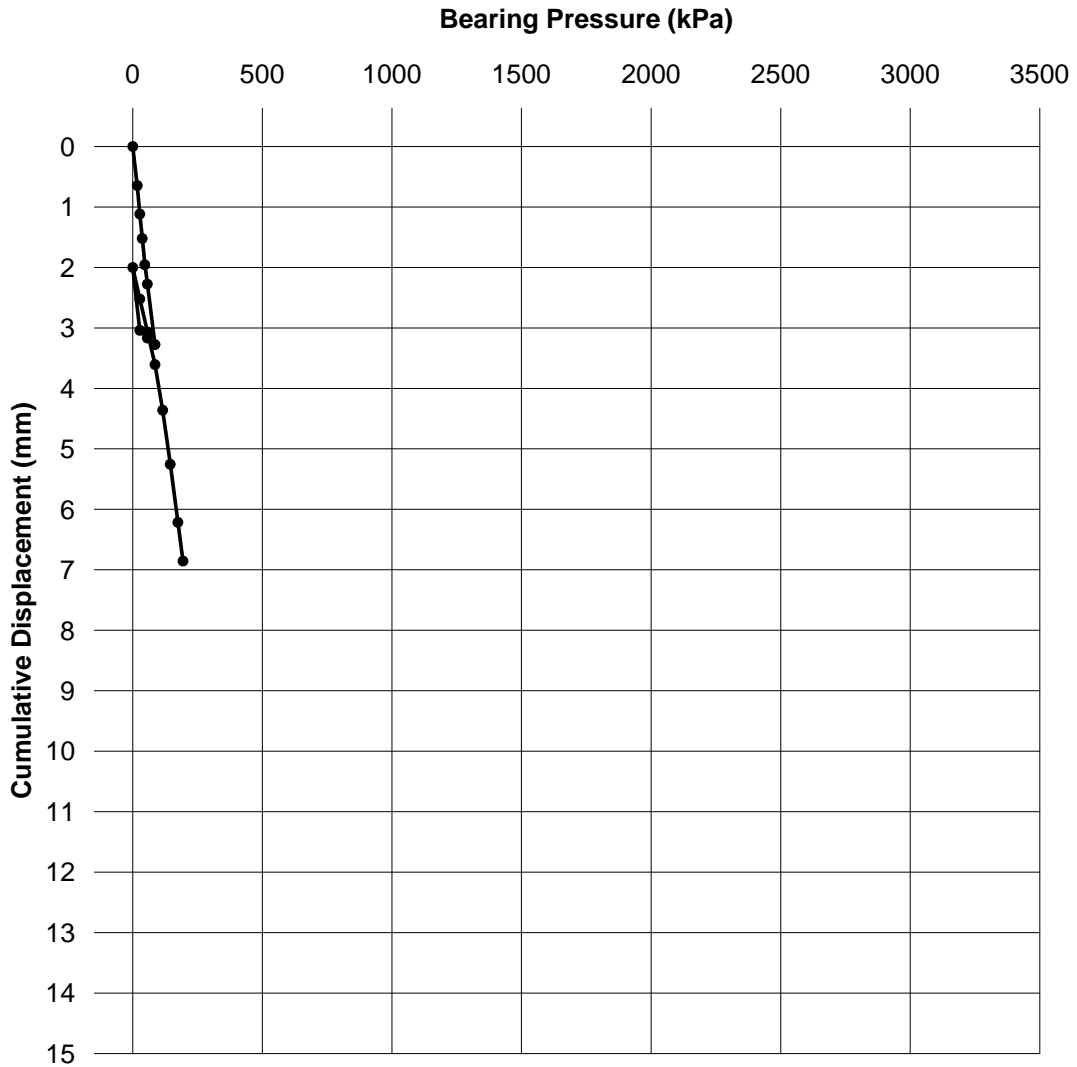
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CLIENT: VICTORIA GOLD CORP.

PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-05		
PROJECT No.	FIG No.	REV.
0792-006	I-5	0

N:\BGC\Projects\0792_Victoria Gold\006 EG Infrastructure 2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix I\Plate Load Testing\Figures\Grapher plots for report



NOTES:

Test: PT-BGC11-06
 Depth: 8.52 m
 Plate Diameter: 120 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 30 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 60 \text{ kPa/mm}$

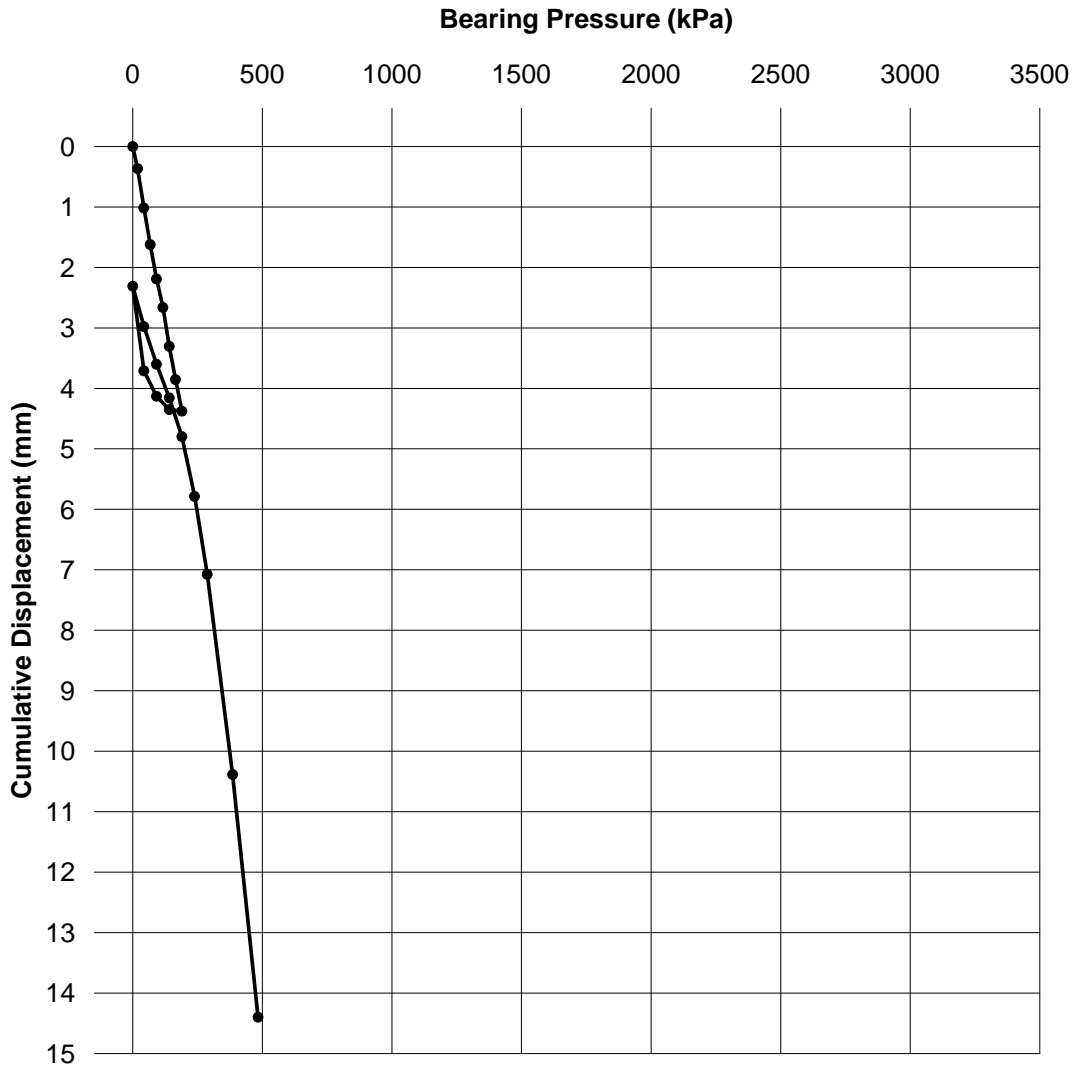
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TITLE: PLATE TEST RESULTS PT-BGC11-06		
PROJECT No.	FIG No.	REV.
0792-006	I-6	0



NOTES:

Test: PT-BGC11-07
 Depth: 8.52 m
 Plate Diameter: 76 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 40 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 80 \text{ kPa/mm}$

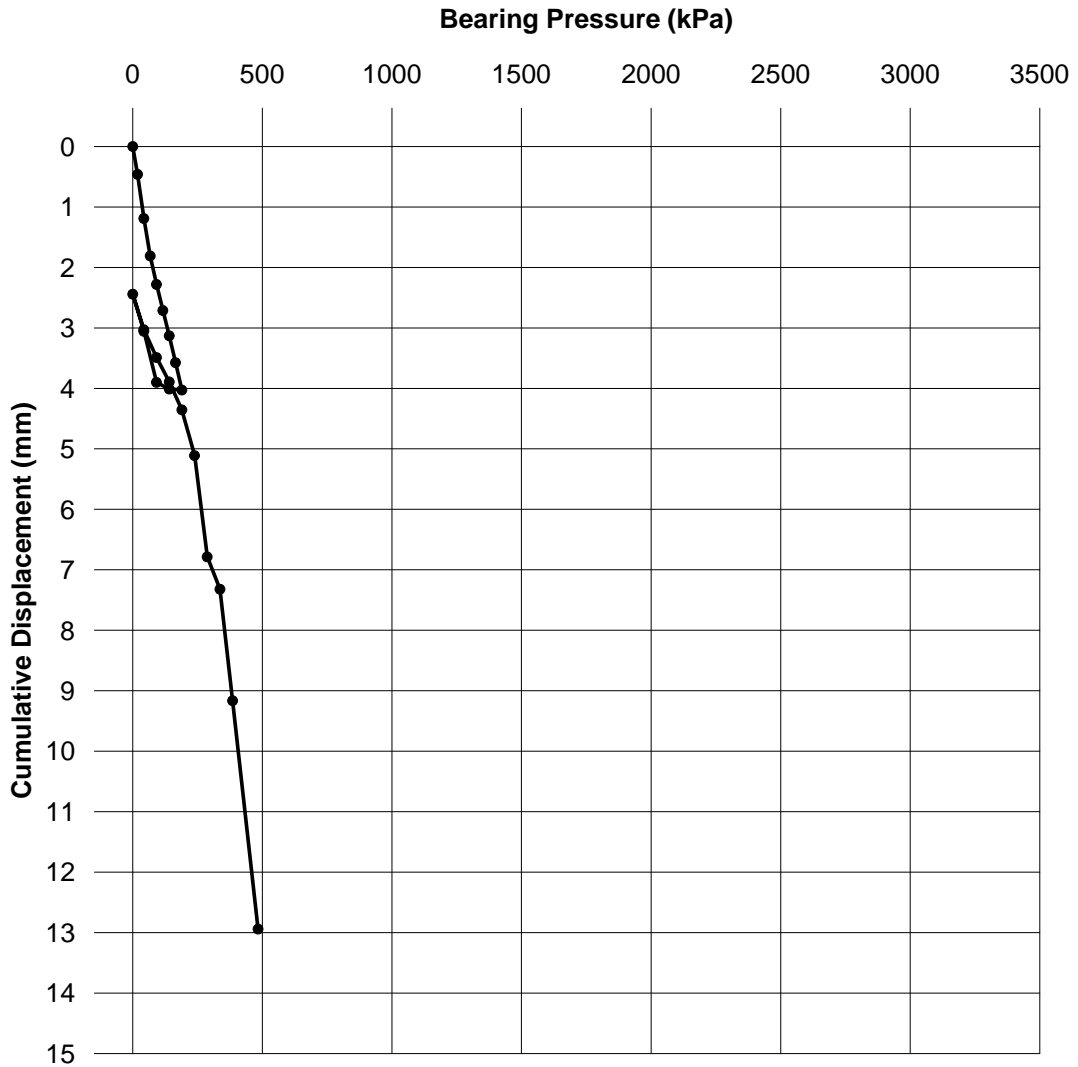
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TITLE: PLATE TEST RESULTS PT-BGC11-07		
PROJECT No. 0792-006	FIG No. I-7	REV. 0



NOTES:

Test: PT-BGC11-08
 Depth: 8.52 m
 Plate Diameter: 76 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 50 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 120 \text{ kPa/mm}$

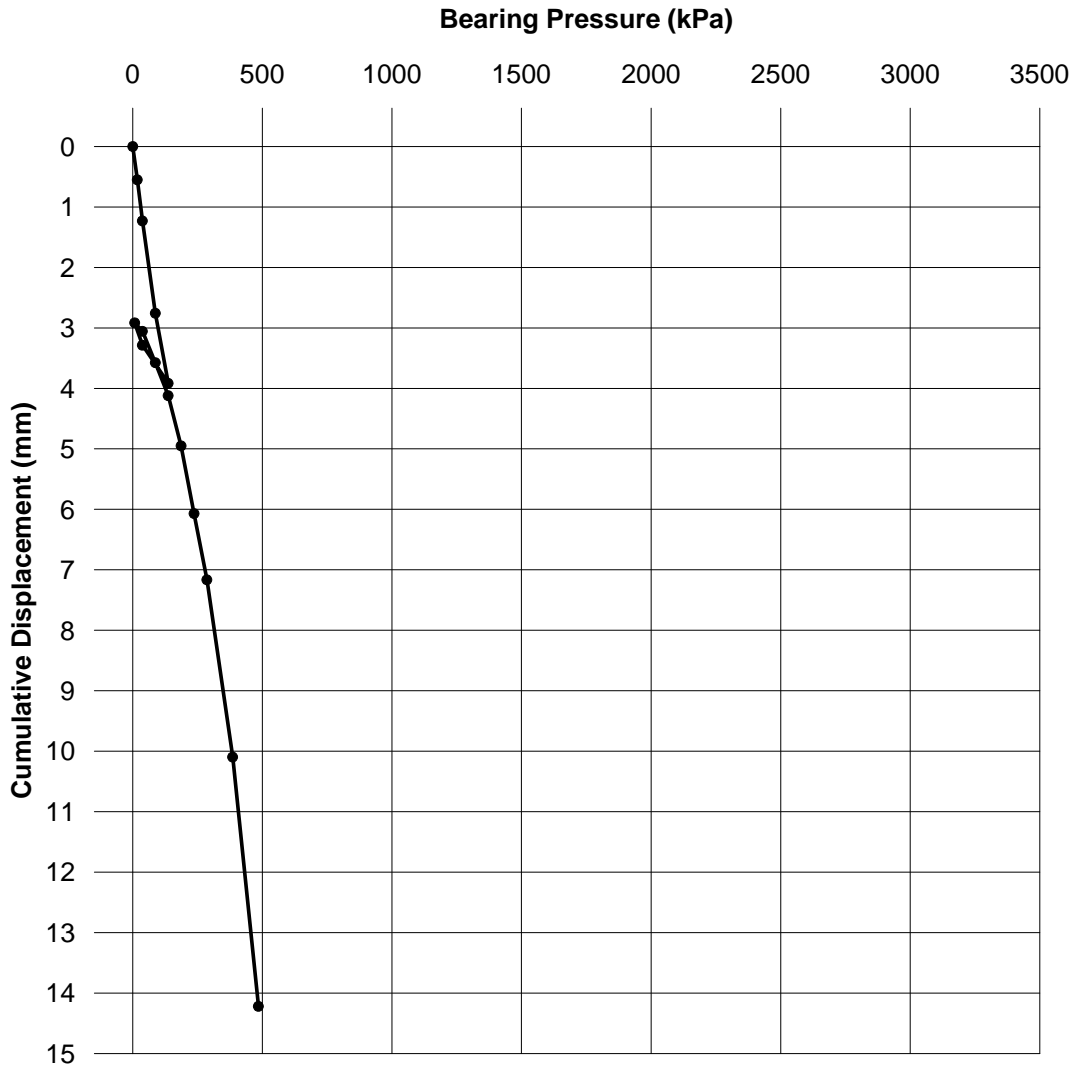
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TITLE: PLATE TEST RESULTS PT-BGC11-08		
PROJECT No. 0792-006	FIG No. I-8	REV. 0



NOTES:

Test: PT-BGC11-09
 Depth: 8.52 m
 Plate Diameter: 53.3 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 40 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 120 \text{ kPa/mm}$

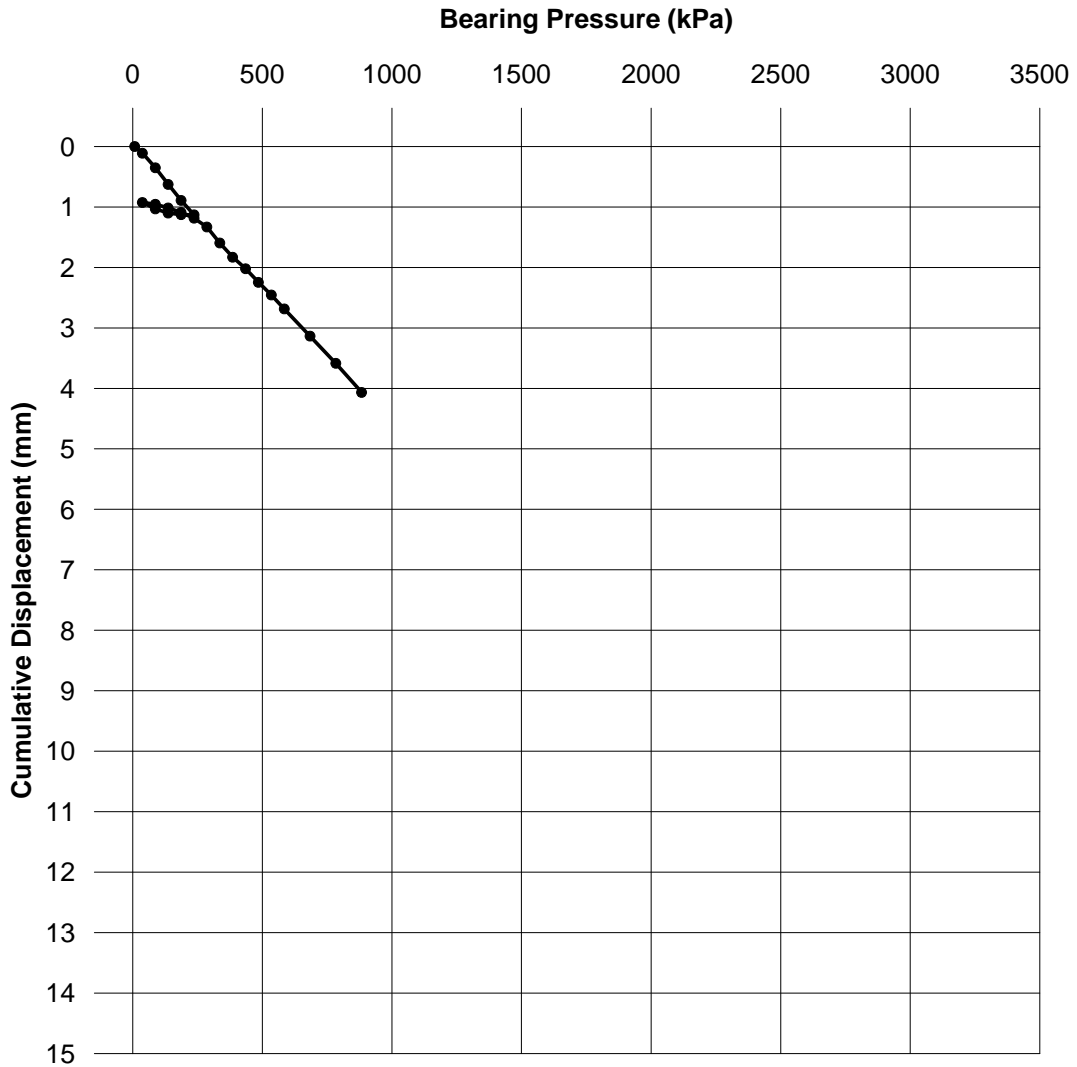
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PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-09		
PROJECT No.	FIG No.	REV.
0792-006	I-9	0



NOTES:

Test: PT-BGC11-10
 Depth: 3.2 m
 Plate Diameter: 53.3 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 190 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 600 \text{ kPa/mm}$

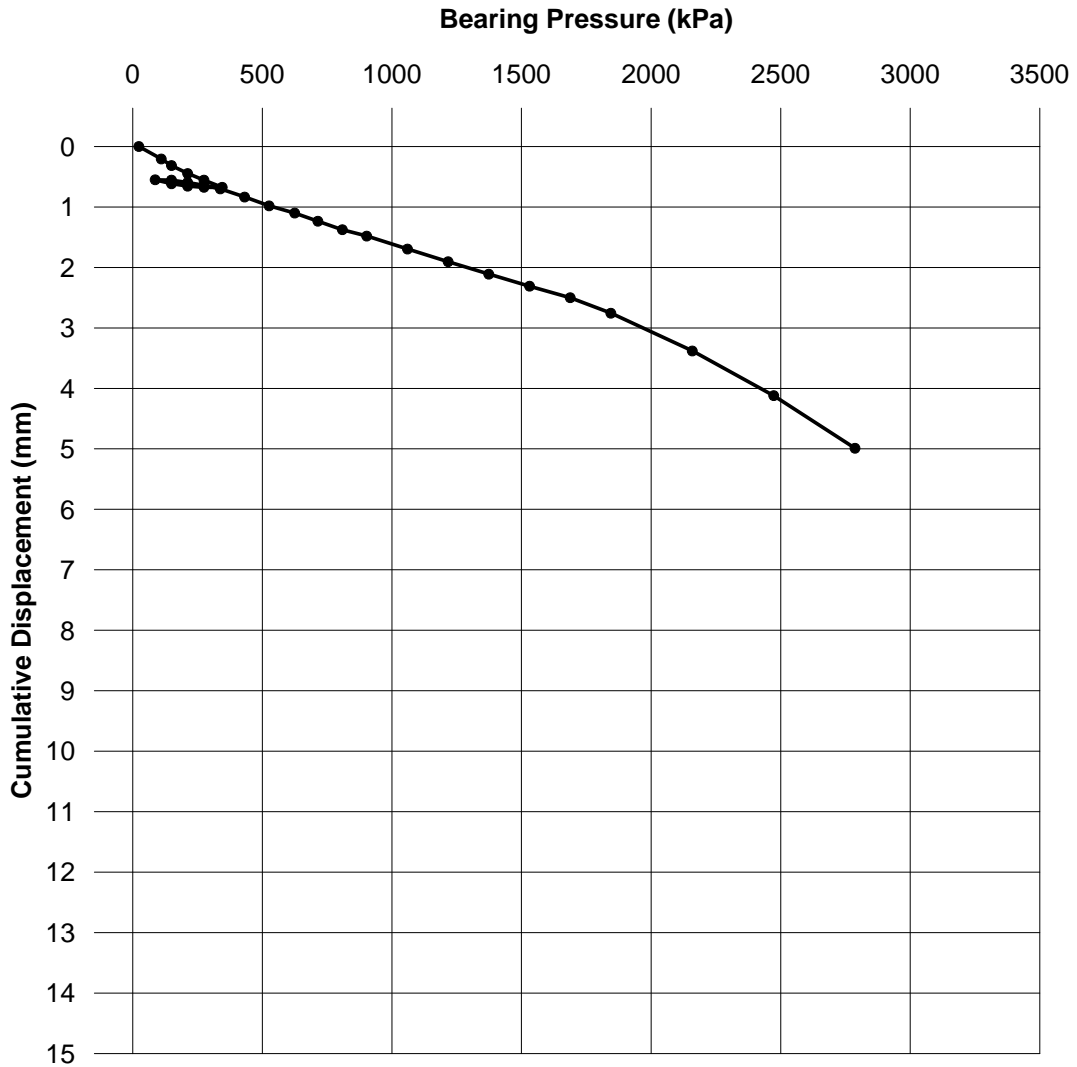
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PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-10		
PROJECT No. 0792-006	FIG No. I-10	REV. 0



NOTES:

Test: PT-BGC11-11
 Depth: 3.2 m
 Plate Diameter: 30 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 642 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 1600 \text{ kPa/mm}$

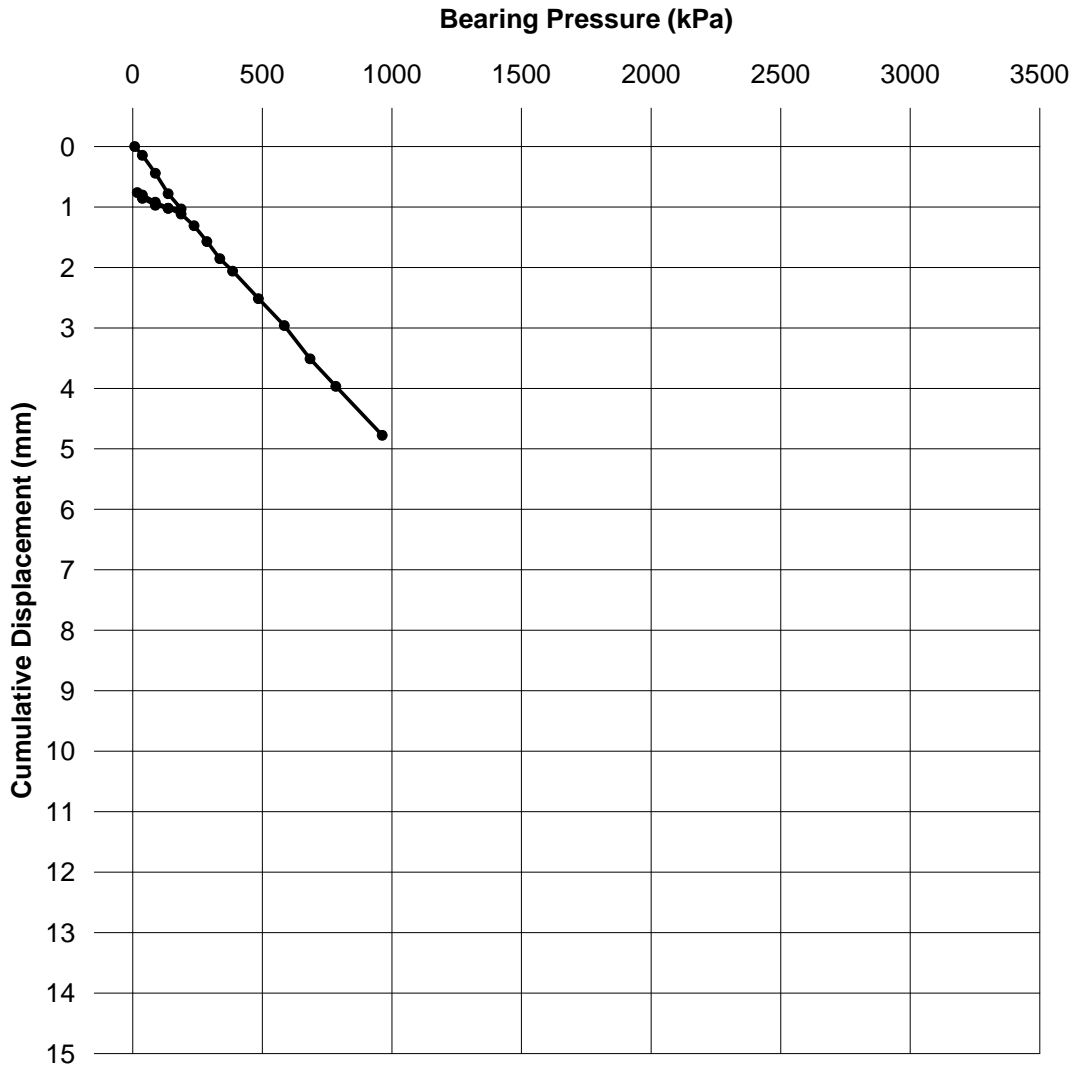
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TITLE: PLATE TEST RESULTS PT-BGC11-11		
PROJECT No.	FIG No.	REV.
0792-006	I-11	0



NOTES:

Test: PT-BGC11-12
 Depth: 3.2 m
 Plate Diameter: 53.3 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 200 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 500 \text{ kPa/mm}$

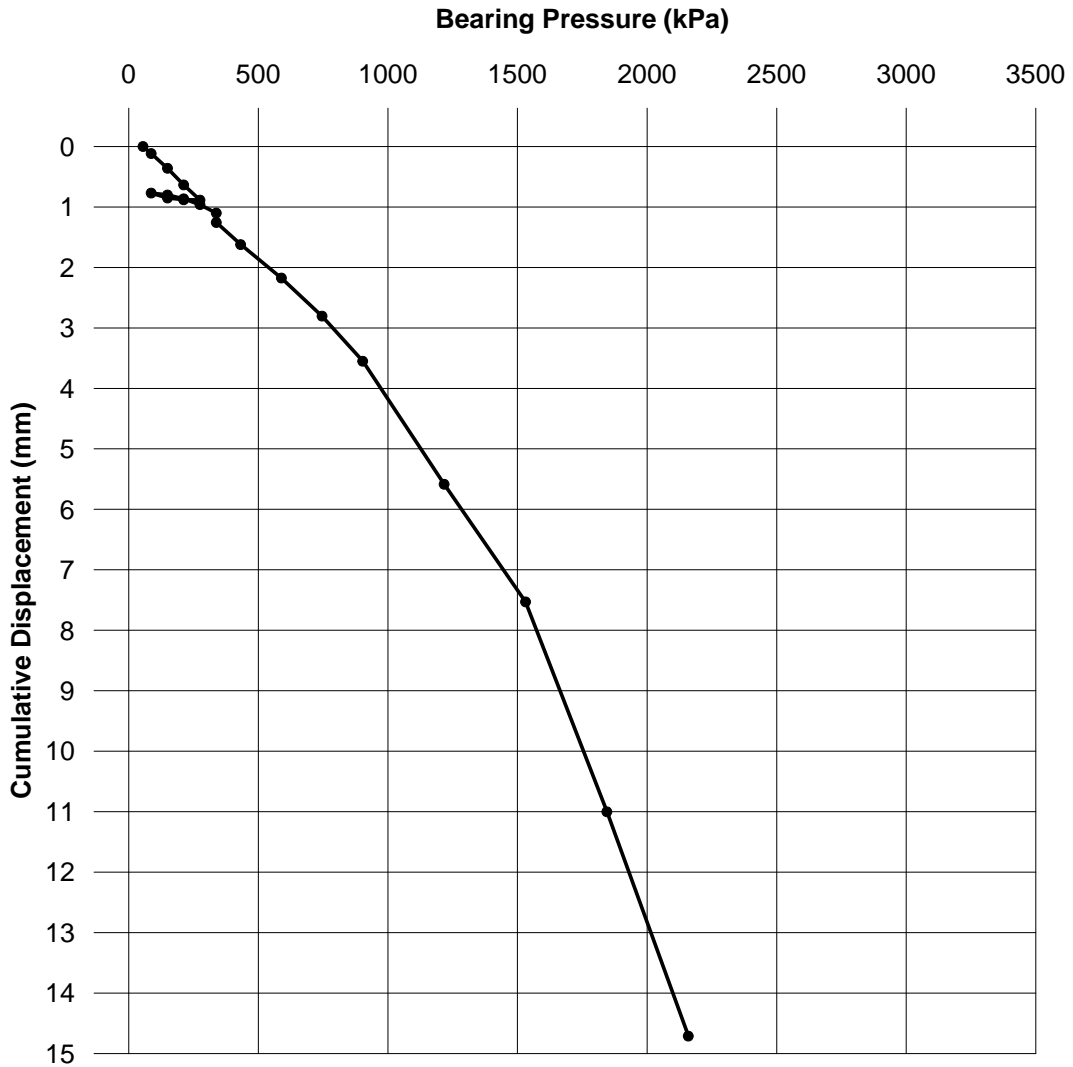
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PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-12		
PROJECT No.	FIG No.	REV.
0792-006	I-12	0



NOTES:

Test: PT-BGC11-13
 Depth: 3.8 m
 Plate Diameter: 30 cm


Subgrade Reaction Modulus Summary:

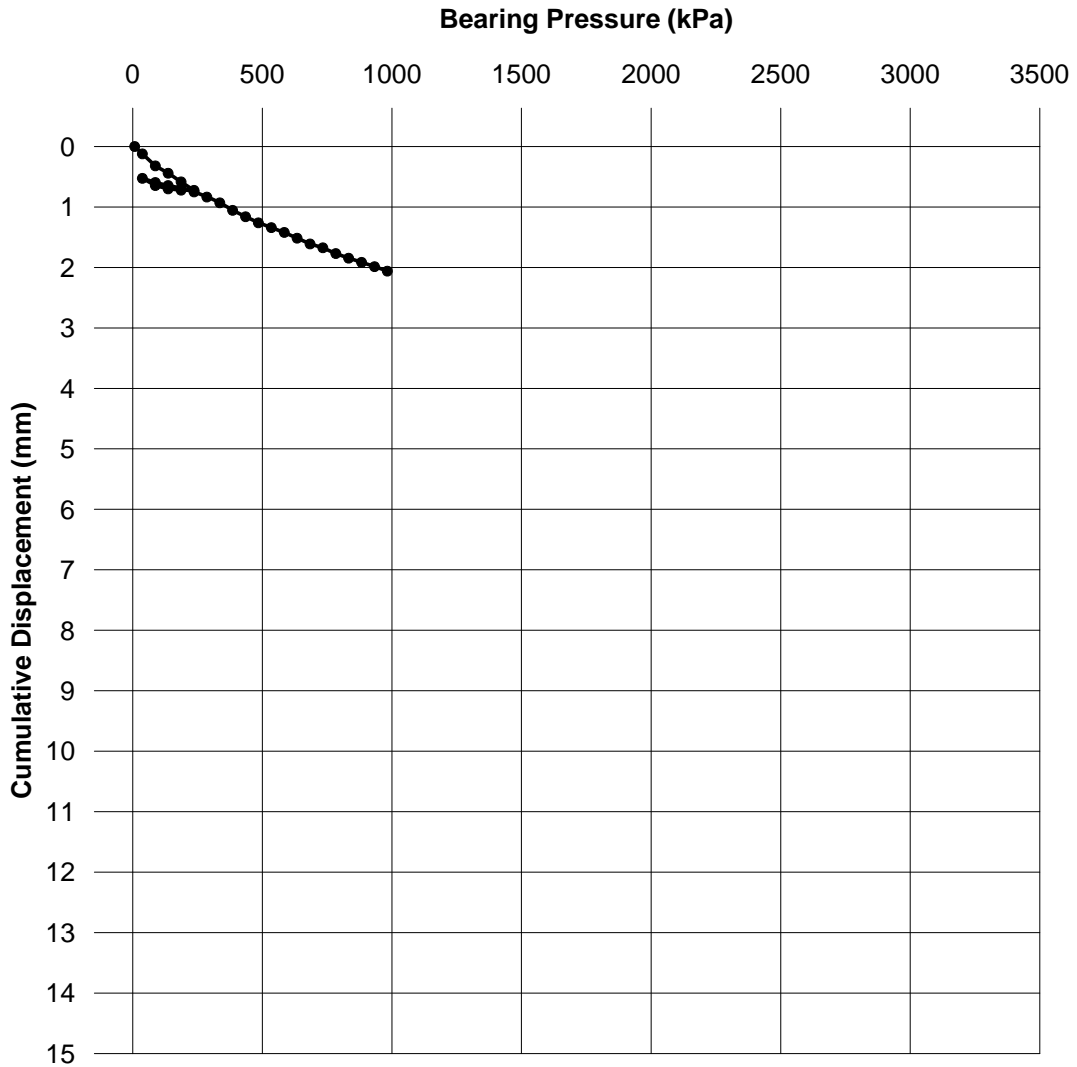
Virgin Curve: $k_v = 220 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 1000 \text{ kPa/mm}$

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CLIENT: VICTORIA GOLD CORP.	PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT
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 BGC ENGINEERING INC. AN APPLIED EARTH SCIENCES COMPANY	TITLE: PLATE TEST RESULTS PT-BGC11-13		
	PROJECT No. 0792-006	FIG No. I-13	REV. 0



NOTES:

Test: PT-BGC11-14
 Depth: 4.2 m
 Plate Diameter: 53.3 cm

Subgrade Reaction Modulus Summary:

Virgin Curve: $k_v = 400 \text{ kPa/mm}$
 Rebound Curve $k_{v(u-r)} = 1040 \text{ kPa/mm}$

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CLIENT: VICTORIA GOLD CORP.

PROJECT: EAGLE GOLD PROJECT 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: PLATE TEST RESULTS PT-BGC11-14		
PROJECT No.	FIG No.	REV.
0792-006	I-14	0

CALIBRATION REPORT



Acuren Group Inc.
 12271 Horseshoe Way
 Richmond, BC, Canada V7A 4V4
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Phone: 604.275.3800
 Fax: 604.274.7235



NDT, Inspection and Materials Engineering
a Rockwood Company

CALIBRATION REPORT

BGC ENGINEERING INC.
 SUITE 500 - 1045 HOWE STREET
 VANCOUVER BC
 V6Z 2A9
 Attention: LUC TOUSSAINT

Date: September 21, 2011
 File No.: 6057329
 P.O. No.: VISA
 Report No.: 1

Description: 25 Ton ENERPAC Ram for Compression Load Calibration
 Model: RC256

Calibration Date: September 21, 2011


Testing Machine ID: SATEC Universal Testing Machine, Serial 120HVL-1164

Calibration Data

Gauge Reading (psi)	Run #1 (lbf)	Run #2 (lbf)	Test Machine Reading (Average Load) (lbf)
100	0	0	0
1000	4393	4426	4410
2000	9489	9459	9474
3000	14246	14220	14233
4000	19262	19015	19139
5000	24160	24213	24187
6000	29126	29045	29086
7000	34284	34172	34228
8000	39274	39348	39311
9000	44477	44418	44448

* Test machine calibrated to ASTM E4 and CSA A23.2-9C specifications

Reviewed by: 
 Alex Wong, ASCT

Reviewed by: 
 Bill Johnson, ASCT

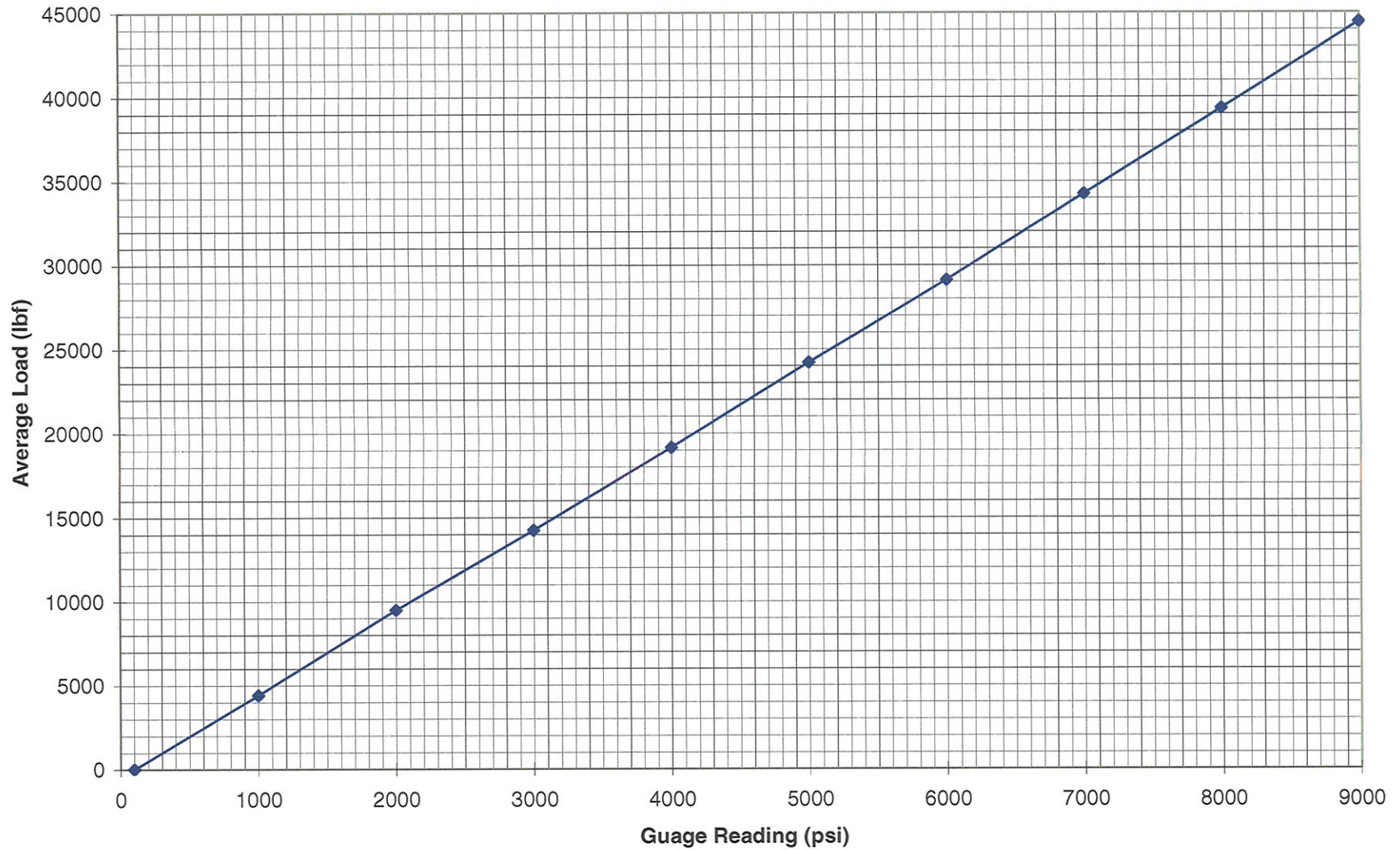
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APPENDIX J THERMISTORS

THERMISTORS

1.0 INTRODUCTION

Thermistors were installed in eight new boreholes to measure ground temperatures and determine the presence of frozen ground and thickness of the active layer. In addition, four thermistors had been previously installed in boreholes in 2009 and 2010. This appendix describes the thermistor work and presents initial temperature data obtained from these instruments in summer 2011.

2.0 THERMISTOR INSTALLATIONS

Thermistors installed in 2011 were manufactured by M-squared Instruments of Cochrane, Alberta. Thermistors were installed in 2" diameter schedule 80 PVC pipe. The annulus between the thermistor and the PVC was not backfilled, but the top of the pipe was sealed to prevent air circulation. The annulus between the PVC pipe and the borehole wall was backfilled with grout in diamond drill holes and with bentonite chips in auger holes. Selected installed thermistors were subsequently moved to different holes to balance the distribution of subsurface temperature measurements at appropriate areas of the site where the data are expected to be of most interest. Three 25 m long thermistors with eight thermocouple beads and four 10 m long thermistors with eight thermocouple beads were installed in 2011. Thermistors installed in 2009 and 2010 were manufactured by EBA Engineering Consultants Ltd. The thermistor installations are summarized in Table J.1.

Table J.1 Thermistor Installation Details

Hole	Facility/Area	Lithology	Date Installed	Hole Depth (m)	Length of Thermistor (m)	Installation details	Thermistor Cable ID #	Number of sensors	Manufacturer	Model	Comments
DH-BGC09-STU-3	Lower Stuttle Gulch	Sand and gravel to 6.1, then silt with, clay sand and gravel	24/08/2009	31.09	10 (plus 2m lead)		EBA#2194	6	EBA Engineering Consultants Ltd		
DH-BGC09-STU-4	Lower Stuttle Gulch	Silty sand and gravel to 7.5m, then gravelly clay (till) to 18.29m	24/08/2009	18.29m	10 (plus 2m lead)		EBA#2193	6	EBA Engineering Consultants Ltd		
DH-BGC09-AG3	Lower Ann Gulch	Sand and gravel to 2.8, silty sand to 7.6m, slightly to moderately weathered quartzite to 13.7	25/08/2009	13.7	10 (plus 2m lead)		EBA#2192	1	EBA Engineering Consultants Ltd		One bead working at 10m
BH-BGC10-07	Previously proposed location of tertiary crusher, south side of Dublin Gulch between Stuttle Gulch and Eagle Pup	Sand Gravel, low recovery to 18.0 (colluvium? Completely weathered rock?)	17/08/2010	29.95	10 (plus 2m lead)	50mm diameter sched 80 PVC casing to 10.1m backfilled with sand, bentonite plug at the top.	EBA#2256	6	EBA Engineering Consultants Ltd	W12101148	
BH-BGC11-42	In ice-rich colluvium located in the Eagle Pup WRSA area	Sand (SW), some silt, some gravel, frozen Vx with 1.3m thick pure ice encountered	25/08/2011	28.19	25 (plus 1.5m lead)	26.9m long 2" threaded sched 40 PVC casing, outer backfilled with bentonite chips, no infill of casing around cable. Steel monument at collar	26.5-2	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	
BH-BGC11-63	In ice-rich colluvium in the Eagle Pup WRSA area	Silt (ML), frozen Vx (assumed Colluvium)	25/08/2011	26.7	25 (plus 1.5m lead)	26.7m long 2" threaded sched 40 PVC casing, outer backfilled with bentonite chips, no infill of casing around cable. Steel monument at collar	26.5-3	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	Vibrating wire piezometer also installed in the hole
BH-BGC11-47	Silt borrow area	Silt (ML) with occasional sand bed, trace fine gravel	10/08/2011	16	10 (plus 1.5m lead)	14.6m long 2" threaded sched 40 PVC casing, outer backfilled with bentonite chips, no infill of casing around cable. Steel monument at collar	11.5-1	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	Thermistor was moved from this hole to BH-BGC11-58 on Aug 28th
BH-BGC11-44	Silt borrow area	Silt (ML and MH) with occasional silt and gravel layer, potentially frozen to bottom of hole	10/08/2011	14.68	10 (plus 1.5m lead)	12.4m long 2" threaded sched 40 PVC casing, outer backfilled with bentonite chips, no infill of casing around cable. Steel monument at collar	11.5-3	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	Thermistor got damaged on Aug 28 2011
BH-BGC11-49	Silt borrow area	Silt (ML) with occasional sand bed, trace fine gravel	10/08/2011	16	10 (plus 1.5m lead)	14.95m long 2" threaded sched 40 PVC casing, outer backfilled with bentonite chips, no infill of casing around cable. Steel monument at collar	11.5-4	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	

Hole	Facility/Area	Lithology	Date Installed	Hole Depth (m)	Length of Thermistor (m)	Installation details	Thermistor Cable ID #	Number of sensors	Manufacturer	Model	Comments
BH-BGC11-57	Truck shop area	Silt (ML), some sand, trace to some gravel, frozen to 6.5-7.0m?	26/08/2011	12.1	10 (plus 1.5m lead)	Standpipe piezo 2" casing screening from 6.6 to 9.9m	11.5-2	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	Thermistor relocated to this hole from BH-BGC11-51
BH-BGC11-58	Truck shop area	Silt (ML), some sand, trace to some gravel, frozen to Nbn and Vx to 4.9m?	29/08/2011	10.8	10 (plus 1.5m lead)	10.8m long 2" threaded sched 40 PVC casing, outer backfilled with bentonite chips, no infill of casing around cable. Steel monument at collar	11.5-1	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	Thermistor relocated to this hole from BH-BGC11-47
BH-BGC11-51	Silt borrow area	Interbedded silts and sand, frozen from approx 6.0 to 17.0m	25/08/2011	25.15	25 (plus 1.5m lead)	24.39m long 2" threaded sched 40 PVC casing, outer backfilled with bentonite chips, no infill of casing around cable. Steel monument at collar	26.5-1	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	
BH-BGC11-51			10/08/2011	25.15	10 (plus 1.5m lead)		11.5-2	8	M-Squared Instruments, Cochrane, AB	Termination: MS3106E20-29P Thermistor Type: YSI44007	Thermistor was removed from this hole on Aug 25th and transferred to BH-BGC11-57

3.0 THERMISTOR READINGS

Thermistors were read on six different days in July and August. In July, thermistors installed in 2009 and 2010 were read. In August, the new thermistors were installed and read in new boreholes. Selected 2009 thermistors were also read in August.

Thermistor readings are taken using a handheld ohmmeter and a converter box used to control which thermistor bead is being read. Readings are recorded as resistance in kilohms (k Ω). Resistance readings are converted to temperatures using a polynomial function for thermistors manufactured by EBA and a table of conversions for thermistors manufactured by M-squared instruments.

Thermistor readings are tabulated in Table J.2 and shown graphically in Figures J-1 through J-13.

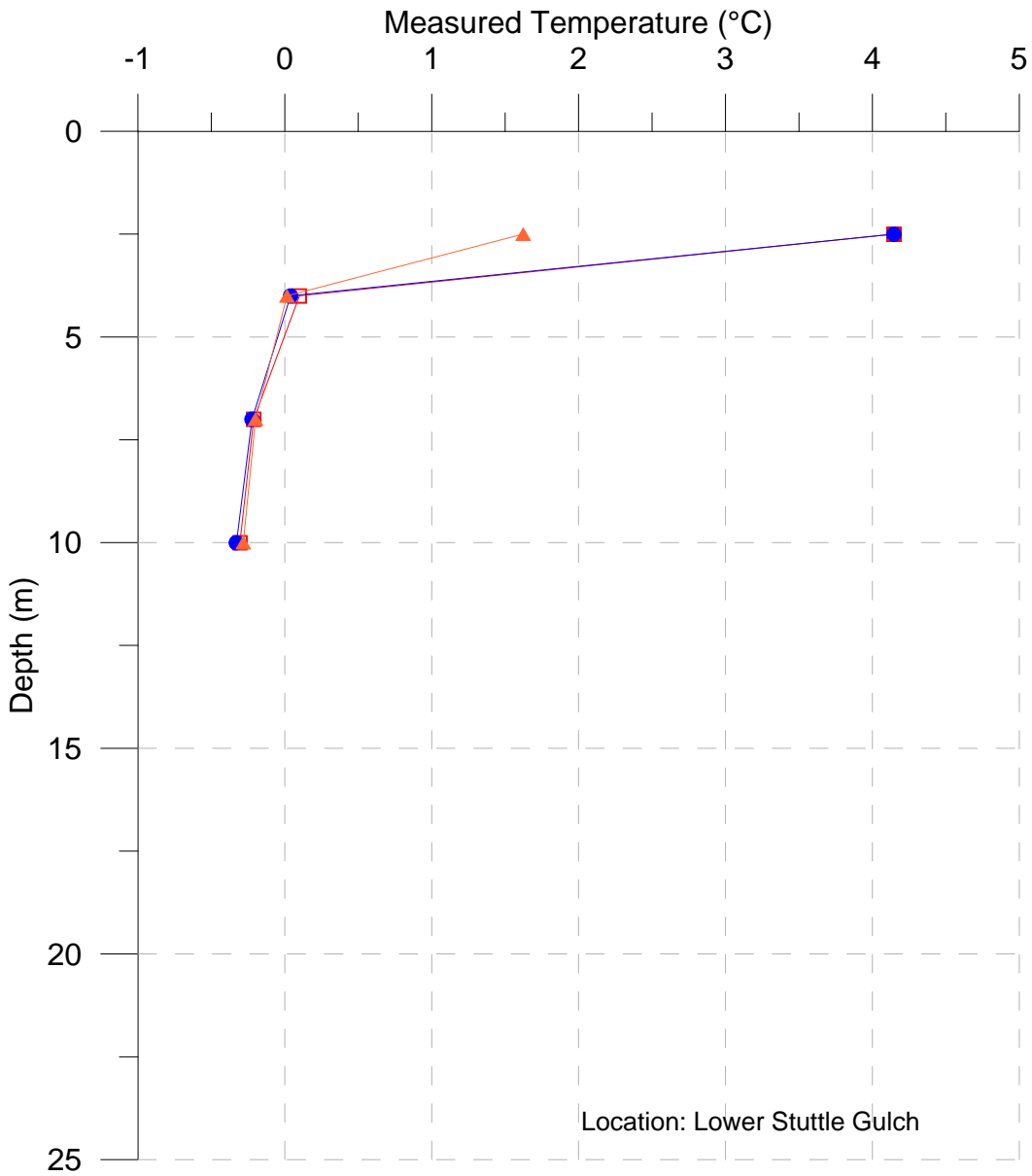
Table J.2 Thermistor Readings

Hole (Cable ID#)	Bead number	Depth of Bead BGL (m)	Date installed	07/11/2011		08/11/2011		08/25/2011		08/26/2011		08/29/2011		08/30/2011	
				Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)
DH-BGC09- STU-3 (EBA#2194)	1	0.5	2009	9.21	11.6			10.12	9.6			10.62	8.6		
	2	1.5		11.71	6.6			11.42	7.1			11.5	7.0		
	3	2.5		15.03	1.6			13.24	4.1			13.24	4.1		
	4	4		16.31	0.0			16.29	0.0			16.24	0.1		
	5	7		16.49	-0.2			16.51	-0.2			16.5	-0.2		
	6	10		16.56	-0.3			16.6	-0.3			16.58	-0.3		
DH-BGC09- STU-4 (EBA#2193)	1	0.5	2009	9.5	10.9			10.5	8.9			11.02	7.9		
	2	1.5		14.1	2.9			12.64	5.1			12.92	4.6		
	3	2.5		16.2	0.1			15.12	1.5			15.15	1.5		
	4	4		16.42	-0.1			16.33	0.0			16.4	-0.1		
	5	7		16.48	-0.2			16.48	-0.2			16.48	-0.2		
	6	10		16.52	-0.2			16.51	-0.2			16.52	-0.2		
DH-BGC09- AG-3 (EBA#2192)	1	10	2009	15.8	0.6										
BH-BGC10- 07 (EBA#2256)	1	0.5	17/08/2010	8.89	12.3							9.86	10.2	9.98	9.9
	2	1.5		10.89	8.1							10.15	9.6	10.18	9.5
	3	2.5		12.83	4.8							10.78	8.3	10.78	8.3
	4	4		15.09	1.5							12.26	5.7	12.28	5.7
	5	7		16.18	0.2							15.27	1.3	15.26	1.3
	6	10		16.32	0.0							16.3	0.0	16.29	0.0
BH-BGC11- 42 (26.5-2)	1	0.30	25/08/2011									11.02	7.9	11.13	7.7
	2	0.60										12.34	5.6	12.42	5.4
	3	1.20										13.55	3.7	13.63	3.6
	4	2.00										15.47	1.1	15.41	1.1
	5	5.00										16.55	-0.3	16.47	-0.2
	6	10.00										16.56	-0.3	16.46	-0.2
	7	15.00										16.67	-0.4	16.56	-0.3
	8	25.00										16.58	-0.3	16.45	-0.1

Hole	Bead number	Depth of Bead BGL (m)	Date installed	07/11/2011		08/11/2011		08/25/2011		08/26/2011		08/29/2011		08/30/2011		
				Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	
BH-BGC11-63 (26.5-3)	1	0.30	25/08/2011									12.48	5.4	12.98	4.6	
	2	0.60											14.76	2.0	14.92	1.8
	3	1.20											16.5	-0.2	16.5	-0.2
	4	2.00											16.53	-0.2	16.52	-0.2
	5	5.00											16.46	-0.2	16.46	-0.2
	6	10.00											16.54	-0.2	16.54	-0.2
	7	15.00											16.56	-0.3	16.56	-0.3
	8	25.00											16.35	0.0	16.35	0.0
BH-BGC11-47 (11.5-1)	1	0.15	10/08/2011			10.71	8.5					12.06	6.0	Removed		
	2	0.50				12.2	5.8					12.54	5.3			
	3	1.00				15.19	1.4					14.61	2.2			
	4	1.50				16.12	0.3					16.21	0.1			
	5	2.50				15.95	0.5					16.08	0.3			
	6	4.00				16.19	0.2					16.26	0.1			
	7	7.00				16.27	0.1					16.36	0.0			
	8	10.00				16.31	0.0					16.42	-0.1			
BH-BGC11-44 (11.5-3)	1	0.15	10/08/2011			9.08	11.9					11.44	7.1	10.76	8.4	
	2	0.50				10.51	8.9					11.24	7.5	11.49	7.0	
	3	1.00				11.36	7.3					11.56	6.9	11.63	6.8	
	4	1.50				12.73	5.0					12.54	5.3	12.53	5.3	
	5	2.50				14.64	2.2					14.12	2.9	14.09	2.9	
	6	4.00				15.87	0.6					15.54	1.0	15.54	1.0	
	7	7.00				16.32	0.0					16.35	0.0	16.35	0.0	
	8	10.00				16.37	0.0					16.41	-0.1	16.41	-0.1	
BH-BGC11-49 (11.5-4)	1	0.15	10/08/2011			8.62	13.0									
	2	0.50				11.1	7.7									
	3	1.00				13.34	4.0									
	4	1.50				15.5	1.0									
	5	2.50				16.26	0.1									
	6	4.00				16.34	0.0									
	7	7.00				16.29	0.0									
	8	10.00				16.31	0.0									

Hole	Bead number	Depth of Bead BGL (m)	Date installed	07/11/2011		08/11/2011		08/25/2011		08/26/2011		08/29/2011		08/30/2011		
				Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	Field reading (KΩ)	Corrected Temperature (°C)	
BH-BGC11-57 (11.5-2)	1	0.15	26/08/2011									10.14	9.6	11.43	7.1	
	2	0.50											13.15	4.3	13.48	3.8
	3	1.00											15.16	1.5	15.29	1.3
	4	1.50											16.42	-0.1	16.44	-0.1
	5	2.50											16.44	-0.1	16.45	-0.1
	6	4.00											16.5	-0.2	16.52	-0.2
	7	7.00											16.48	-0.2	16.48	-0.2
	8	10.00											16.41	-0.1	16.41	-0.1
BH-BGC11-58 (11.5-1)	1	0.15	29/08/2011											12.46	5.4	
	2	0.50													15.14	1.5
	3	1.00													16.42	-0.1
	4	1.50													16.61	-0.3
	5	2.50													16.75	-0.5
	6	4.00													16.91	-0.7
	7	7.00													16.91	-0.7
	8	10.00													16.84	-0.6
BH-BGC11-51 (26.5-1)	1	0.30	25/08/2011							7.4	16.3	6.35	19.6	7.37	16.4	
	2	0.60									8.78	12.6	9.4	11.2	9.39	11.2
	3	1.20									9.4	11.2	9.57	10.8	9.73	10.5
	4	2.00									9.8	10.3	9.87	10.2	9.9	10.1
	5	5.00									14.3	2.6	14.26	2.7	14.24	2.7
	6	10.00									16.26	0.1	16.25	0.1	16.26	0.1
	7	15.00									16.39	-0.1	16.39	-0.1	16.4	-0.1
	8	25.00									16.45	-0.1	16.45	-0.1	16.45	-0.1
BH-BGC11-51 (11.5-2)	1	0.15	10/08/2011			7.91	14.8	8.24	14.0	Removed						
	2	0.50				8.58	13.1	9.31	11.4							
	3	1.00				8.82	12.5	9.54	10.9							
	4	1.50				9.28	11.5	9.77	10.4							
	5	2.50				11.07	7.8	11.03	7.9							
	6	4.00				13.74	3.4	13.62	3.6							
	7	7.00				16.63	-0.4	15.8	0.6							
	8	10.00				16.18	0.2	16.31	0.0							

FIGURES



Location: Lower Stuttle Gulch

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 19/08/09
 - 3) Thermistor brand: EBA, ID#: EBA#2194, installed on 24/08/09

DH-BGC09-STU-3

- 29/08/2011
- 25/08/2011
- ▲— 07/07/2011

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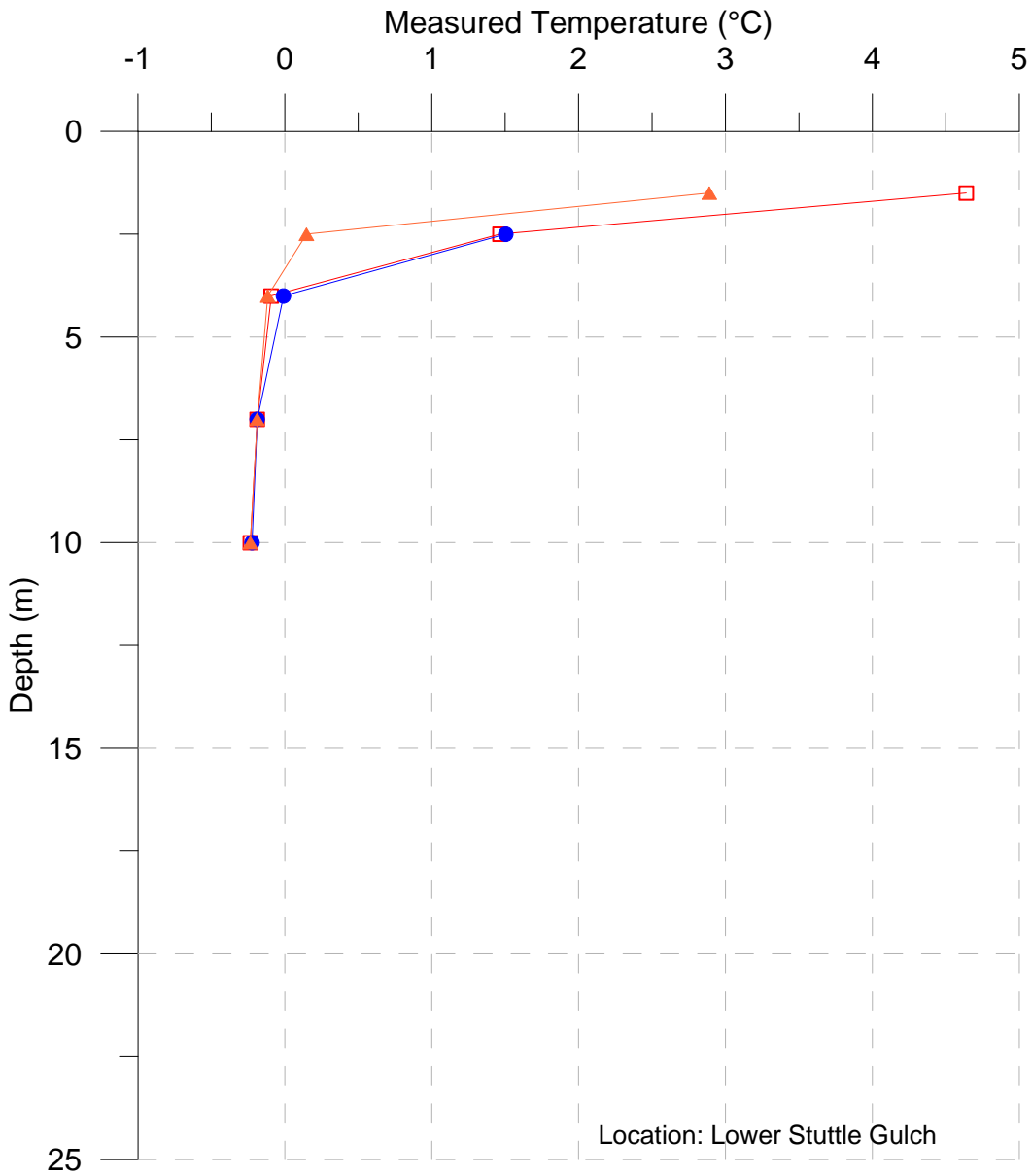
PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

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TITLE: MEASURED GROUND TEMPERATURE PROFILES, DH-BGC09-STU-3		
PROJECT No.	FIG No.	REV.
0792-006	J-1	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Lower Stuttle Gulch

Notes:

- 1) Measured temperatures warmer than 5°C not shown for clarity
- 2) Drillhole completed on 21/08/09
- 3) Thermistor brand: EBA, ID#: EBA#2193, installed on 24/08/09

DH-BGC09-STU-4

- ▲— 07/07/2011
- 25/08/2011
- 29/08/2011

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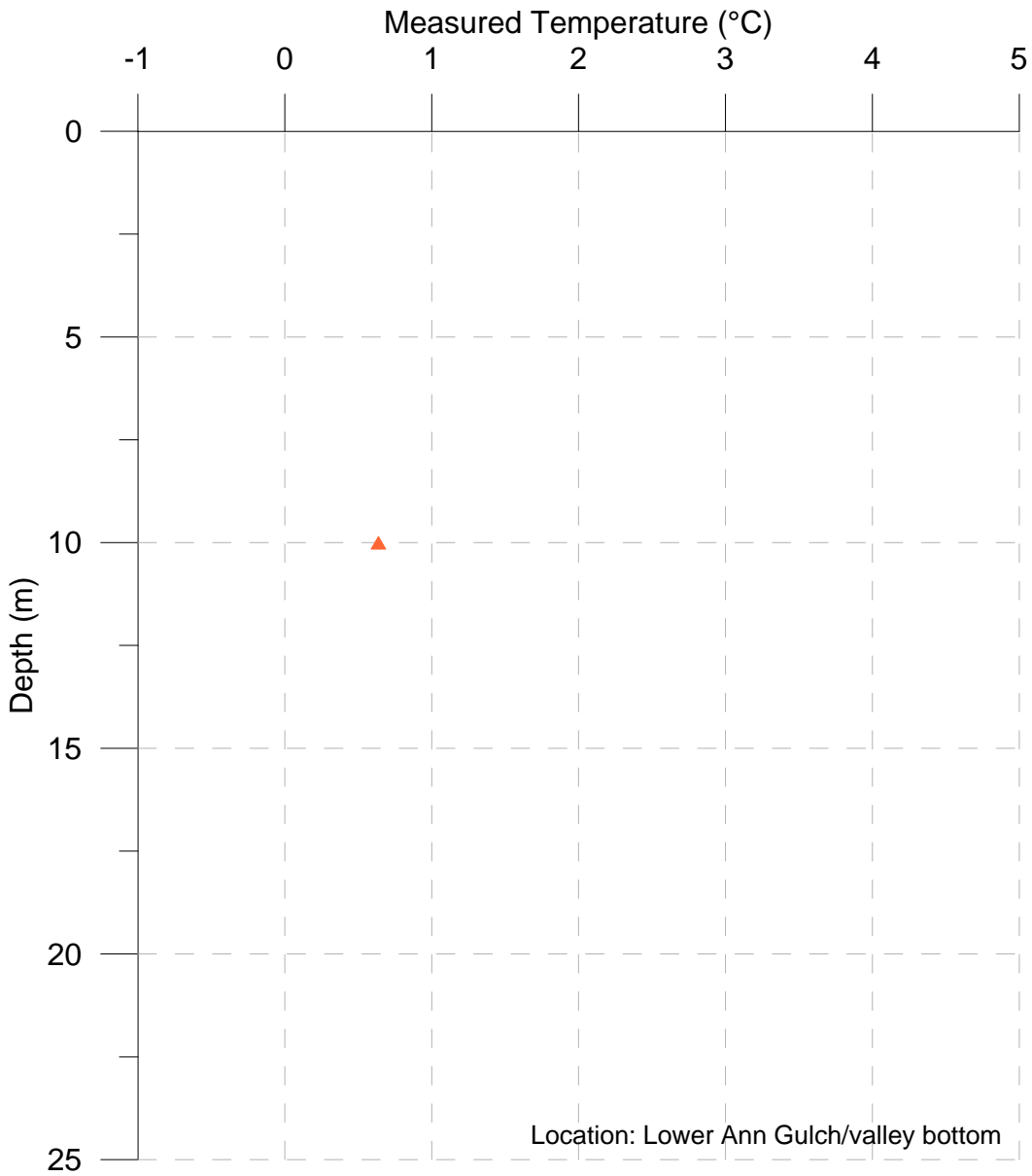
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TITLE: MEASURED GROUND TEMPERATURE PROFILE, DH-BGC09-STU-4		
PROJECT No.	FIG No.	REV.
0792-006	J-2	0

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Lower Ann Gulch/valley bottom

Notes:

- 1) Measured temperatures warmer than 5°C not shown for clarity
- 2) Drillhole completed on 23/08/09
- 3) Thermistor brand: EBA, ID#: EBA#2192, installed on 25/08/09
- 4) Thermistor consisted of a single thermocouple bead at 10 m depth

DH-BGC09-AG-3

07/11/2011

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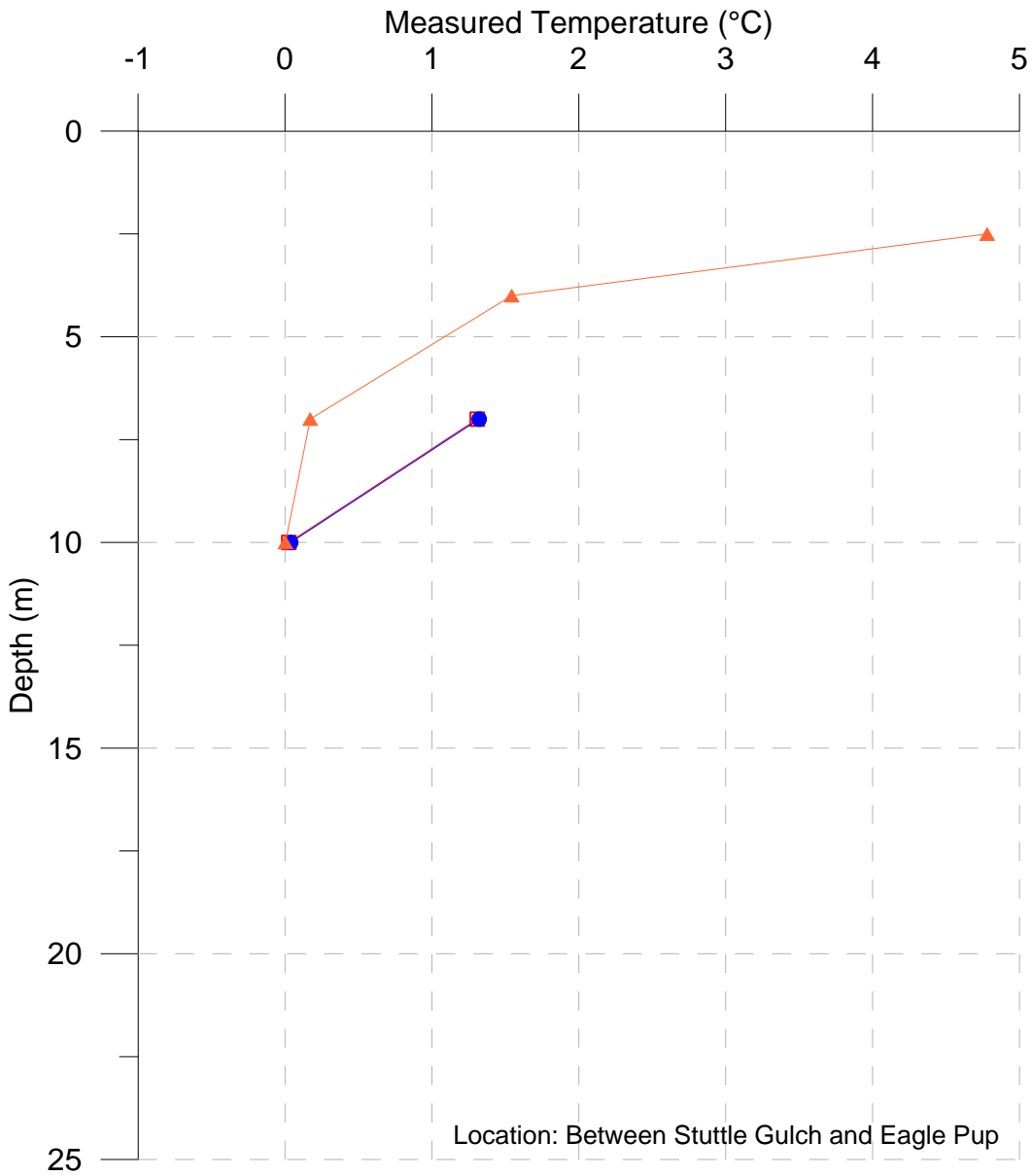
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, DH-BGC09-AG-3		
PROJECT No.	FIG No.	REV.
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N:\BGC\Projects\0792_Victoria Gold\006 EG Infrastructure_2011\06 Reporting\01 Data Report\03 Working Appendices\Appendix J Thermistors

FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Between Stuttle Gulch and Eagle Pup

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 17/08/10
 - 3) Thermistor brand: EBA, Model: W12101148, ID#:EBA#2256 installed on 17/08/10

BH-BGC10-07

- ▲ 07/11/2011
- 29/08/2011
- 30/08/2011

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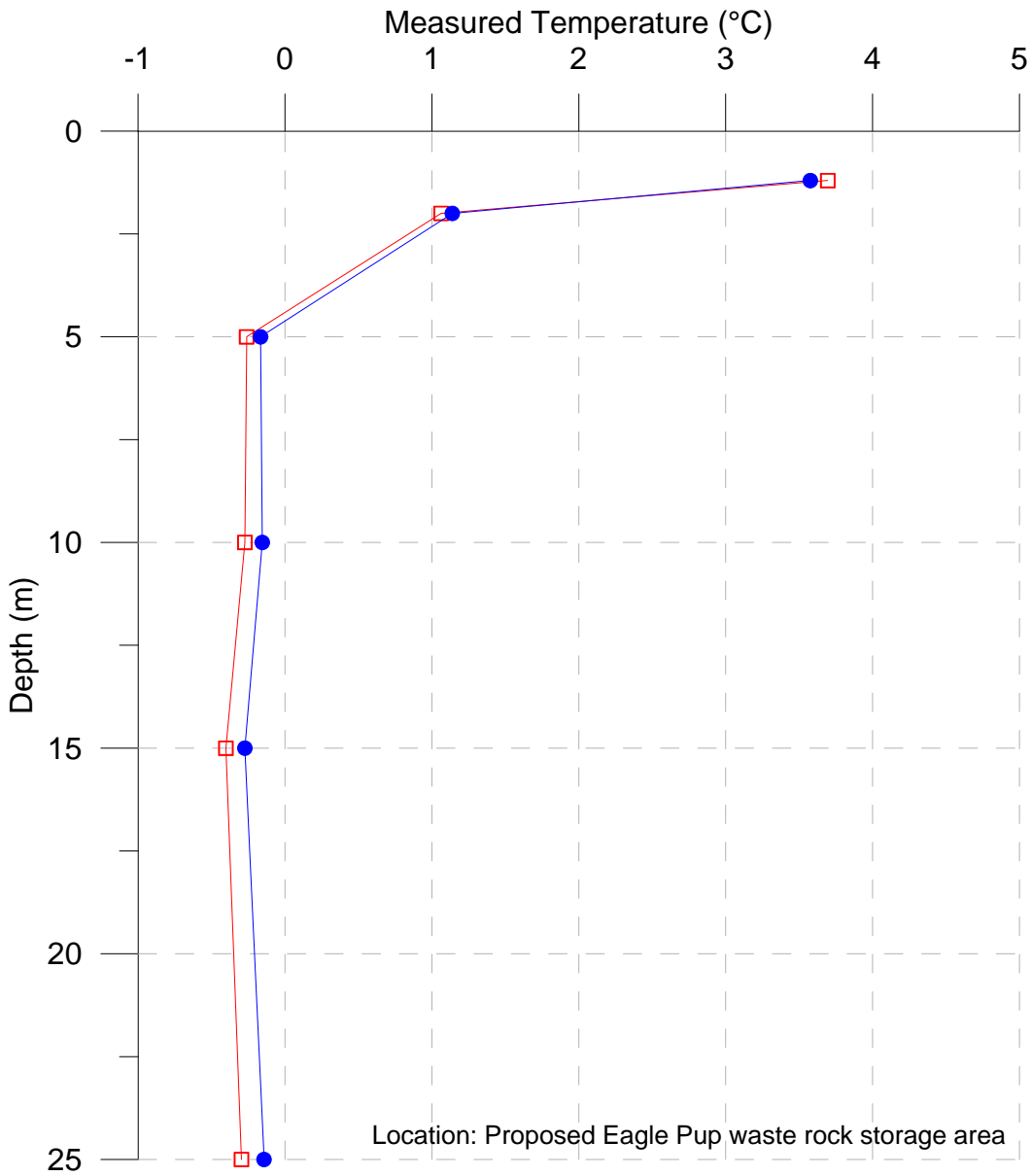
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC10-07		
PROJECT No.	FIG No.	REV.
0792-006	J-4	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed Eagle Pup waste rock storage area

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 01/08/11
 - 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#: 26.5-2, installed on 25/08/11

BH-BGC11-42

—□— 29/08/2011

—●— 30/08/2011

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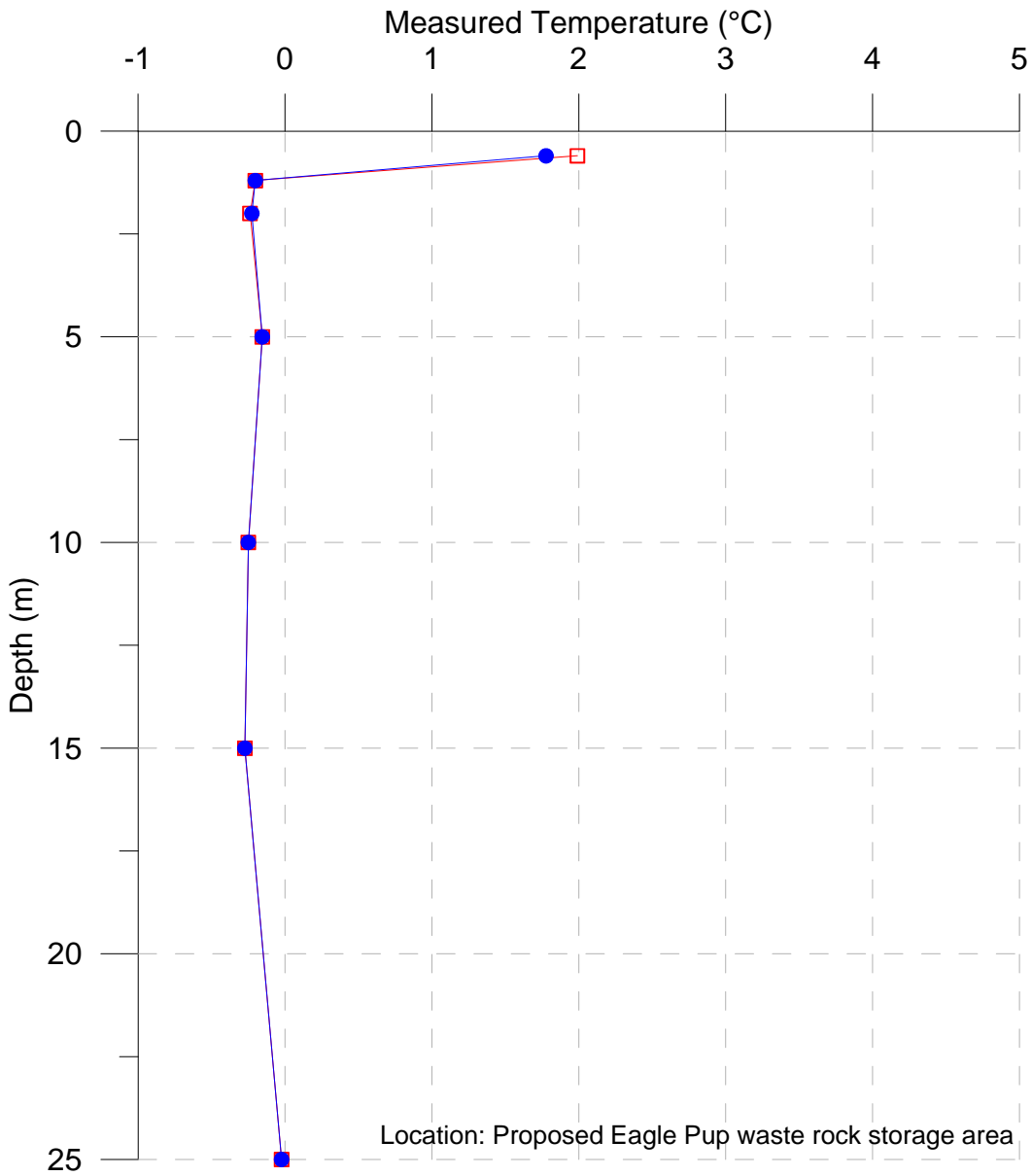
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PROJECT No.	FIG No.	REV.
0792-006	J-5	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed Eagle Pup waste rock storage area

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 13/08/11
 - 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#: 26.5-3, installed on 25/08/11
 - 4) Vibrating wire piezometer also installed in the drillehole casing

BH-BGC11-63

- □ — 29/08/2011
- ● — 30/08/2011

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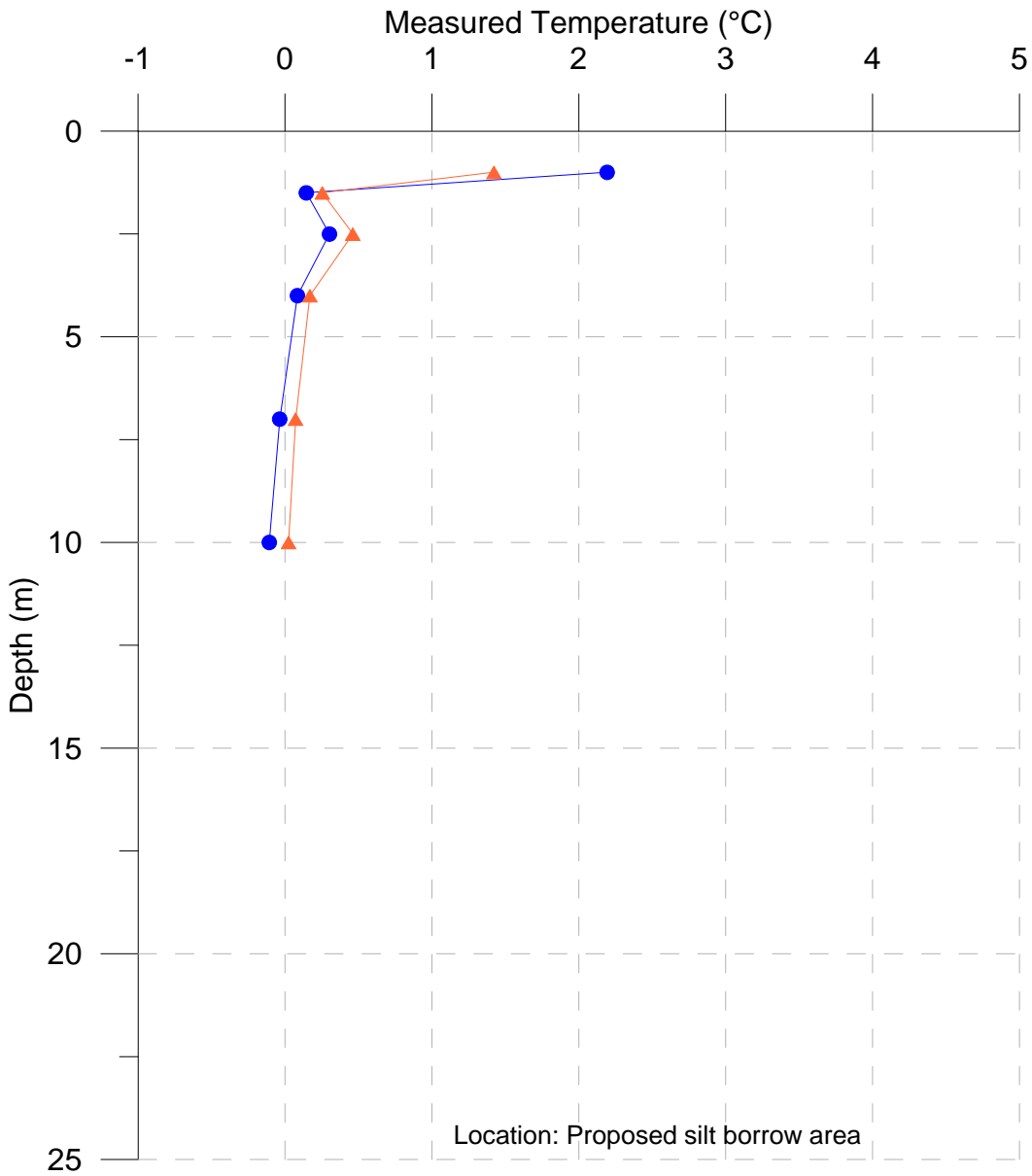
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC11-63		
PROJECT No.	FIG No.	REV.
0792-006	J-6	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed silt borrow area

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 04/08/11
 - 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#: 11.5-1, installed on 10/08/11
 - 4) Thermistor moved to BH-BGC11-58 on 28/08/11

BH-BGC11-47
 ▲ 11/08/2011
 ● 29/08/2011

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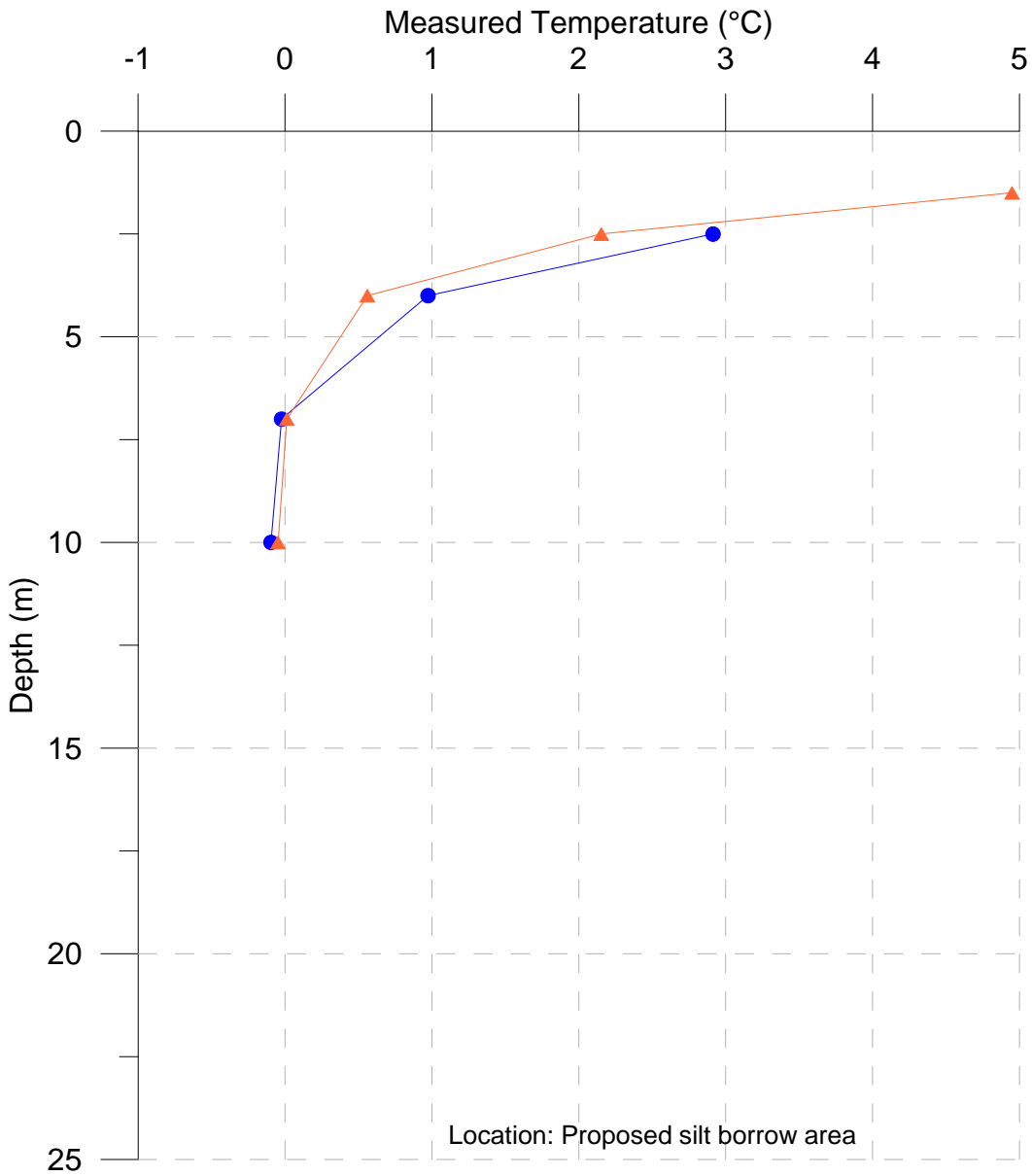
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC11-47		
PROJECT No.	FIG No.	REV.
0792-006	J-7	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed silt borrow area

BH-BGC11-44

- ▲ 11/08/2011
- 29/08/2011
- 30/08/2011

Notes:

- 1) Measured temperatures warmer than 5°C not shown for clarity
- 2) Drillhole completed on 03/08/11
- 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#:11.5-3, installed on 10/08/11
- 4) Instrument got damaged and removed on 28/08/11

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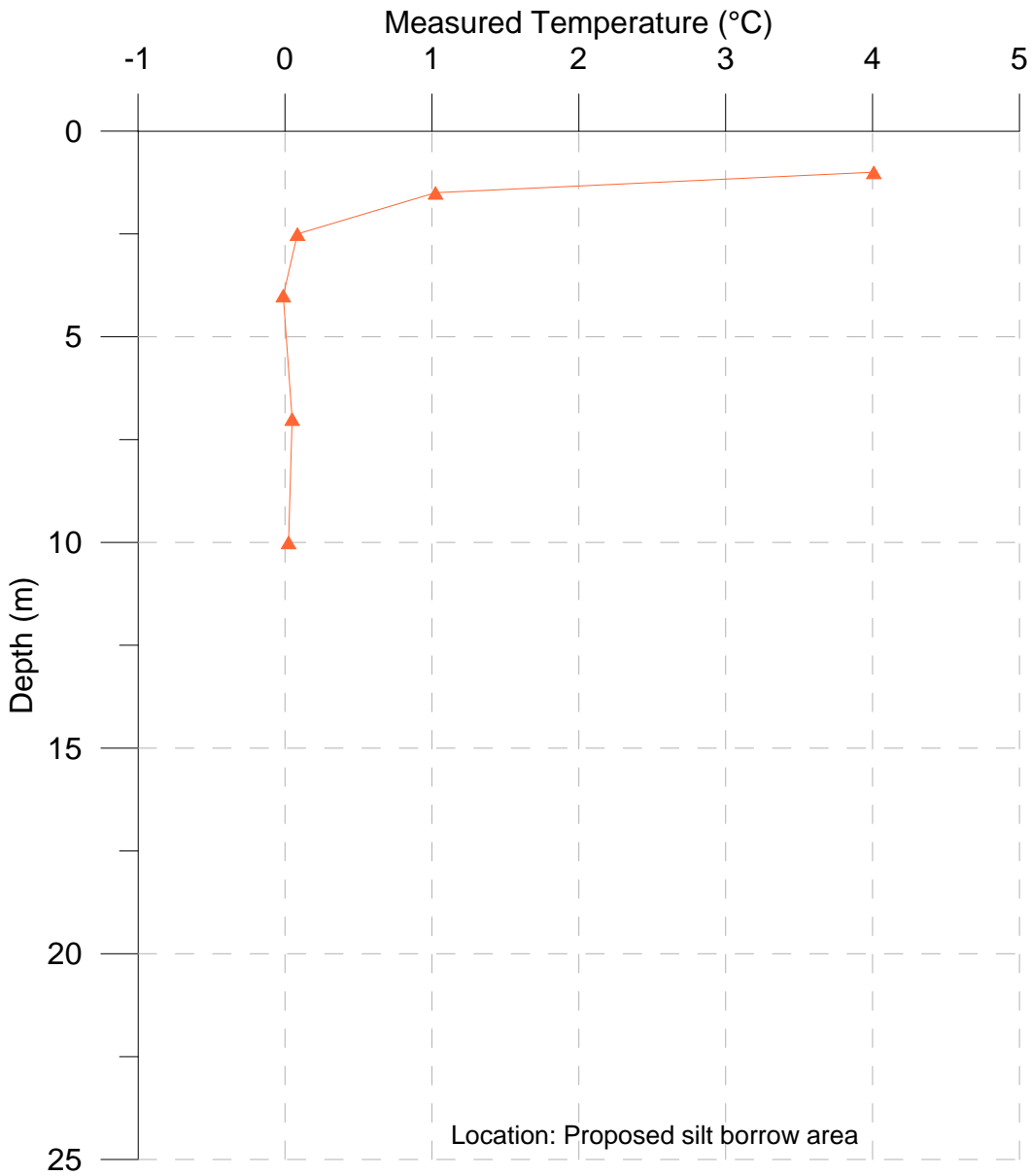
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC11-44		
PROJECT No.	FIG No.	REV.
0792-006	J-8	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed silt borrow area

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 05/08/11
 - 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#:11.5-4 installed on 10/08/11

BH-BGC11-49

11/08/2011

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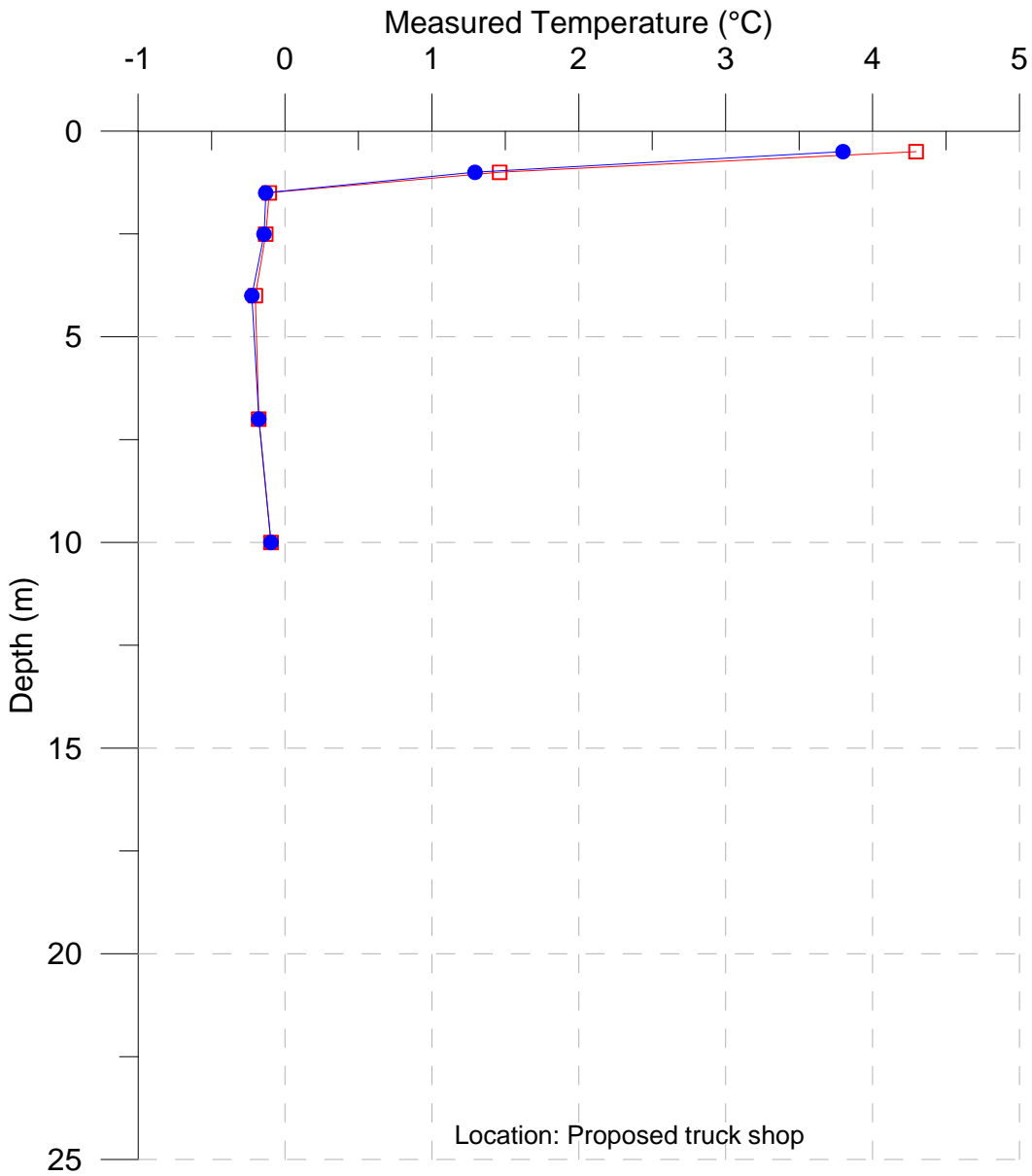
CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC11-49		
PROJECT No.	FIG No.	REV.
0792-006	J-9	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed truck shop

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 10/08/11
 - 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#:11.5-2, installed on 26/08/11

BH-BGC11-57

—□— 29/08/2011

—●— 30/08/2011

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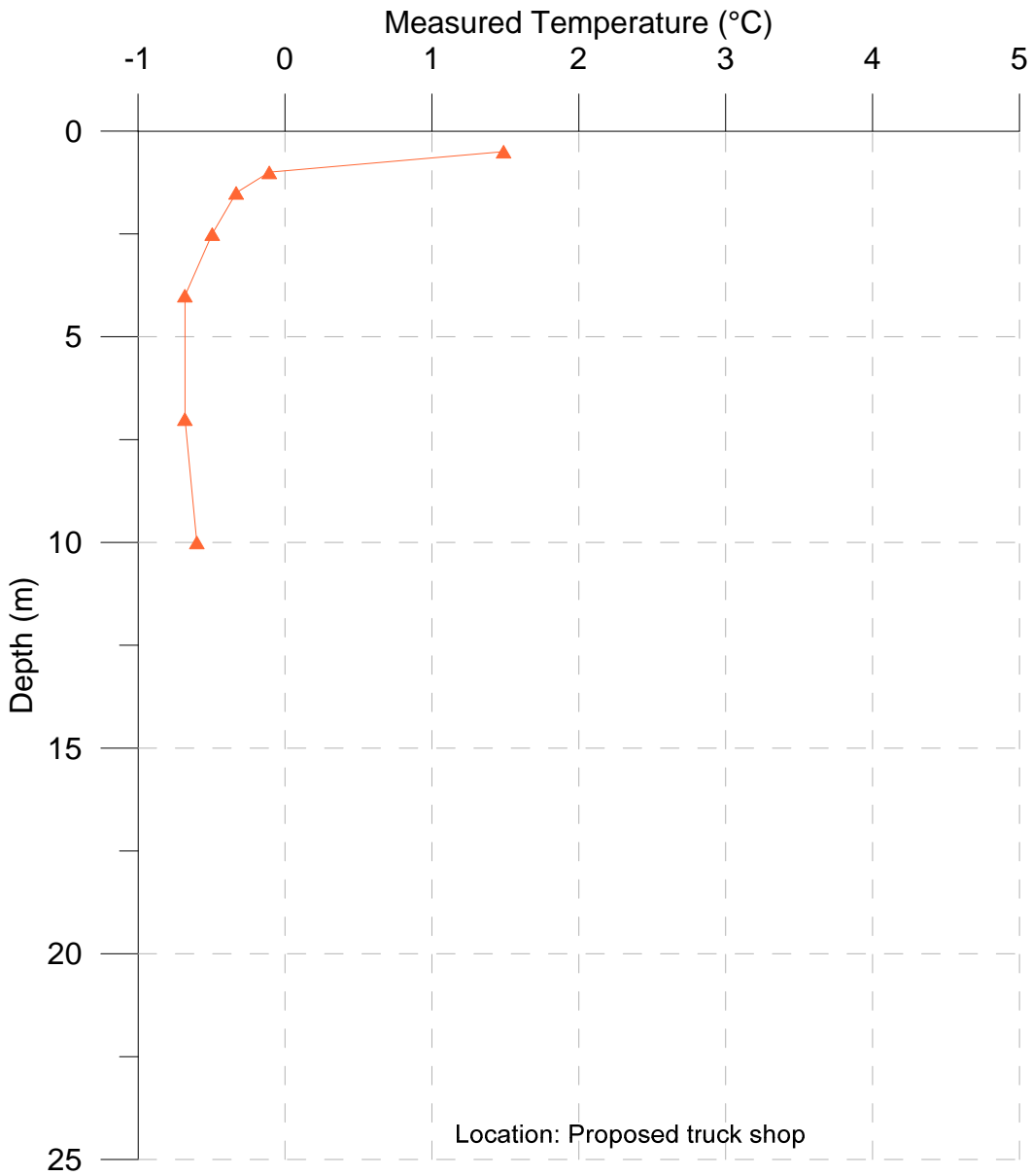
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TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC11-57		
PROJECT No.	FIG No.	REV.
0792-006	J-10	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed truck shop

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 10/08/11
 - 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#:11.5-1, installed on 29/08/11

BH-BGC11-58

30/08/2011

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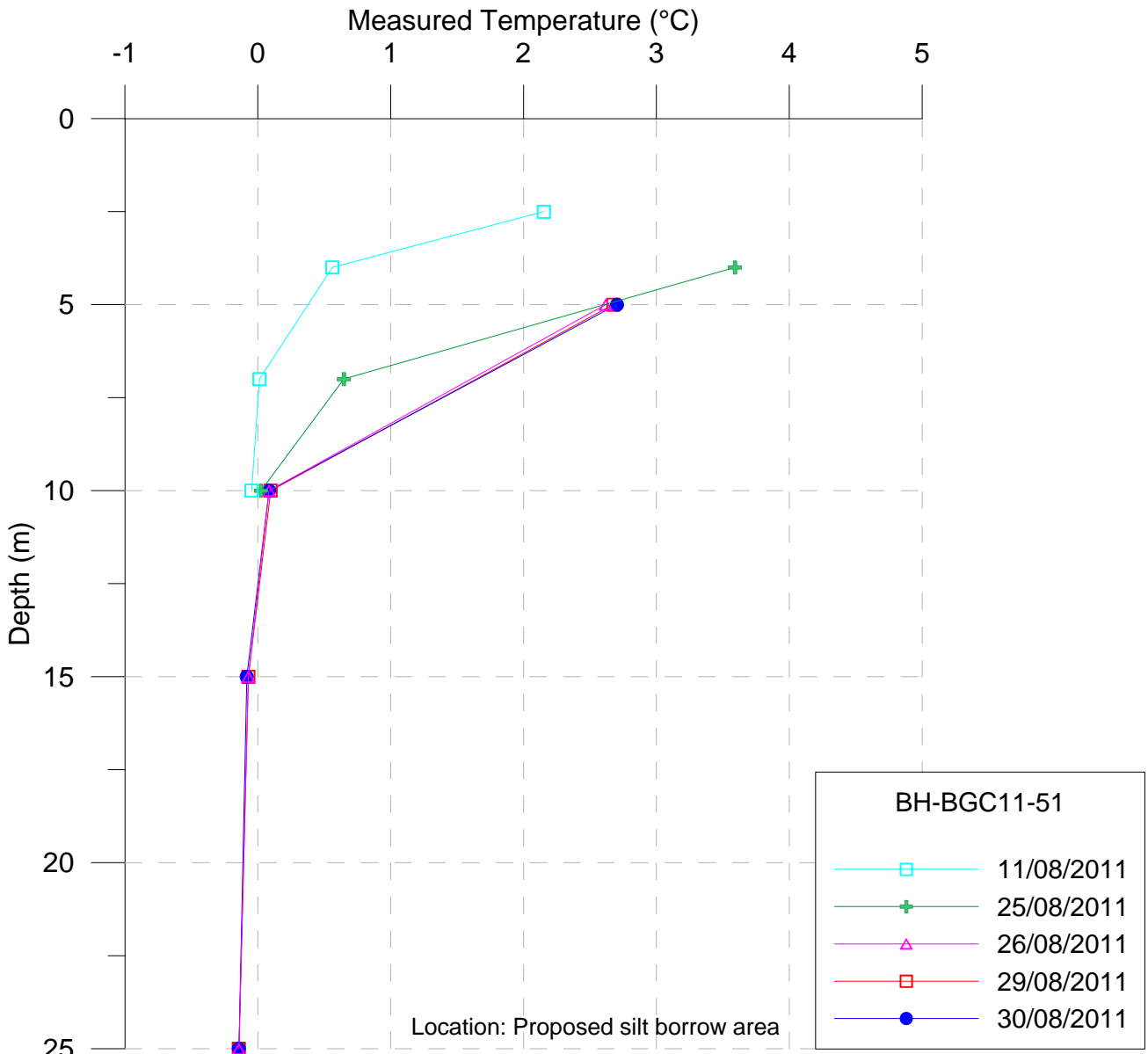
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PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC11-58		
PROJECT No.	FIG No.	REV.
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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.



Location: Proposed silt borrow area

BH-BGC11-51	
—□—	11/08/2011
—+—	25/08/2011
—△—	26/08/2011
—□—	29/08/2011
—●—	30/08/2011

- Notes:**
- 1) Measured temperatures warmer than 5°C not shown for clarity
 - 2) Drillhole completed on 06/08/11.
 - 3) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#: 11.5-2, installed on 10/08/11 was removed on 25/08/11
 - 4) Thermistor brand: M², Termination: MS3106E20-29P, Type: YSI44007, ID#: 26.5-2, installed on 25/08/11

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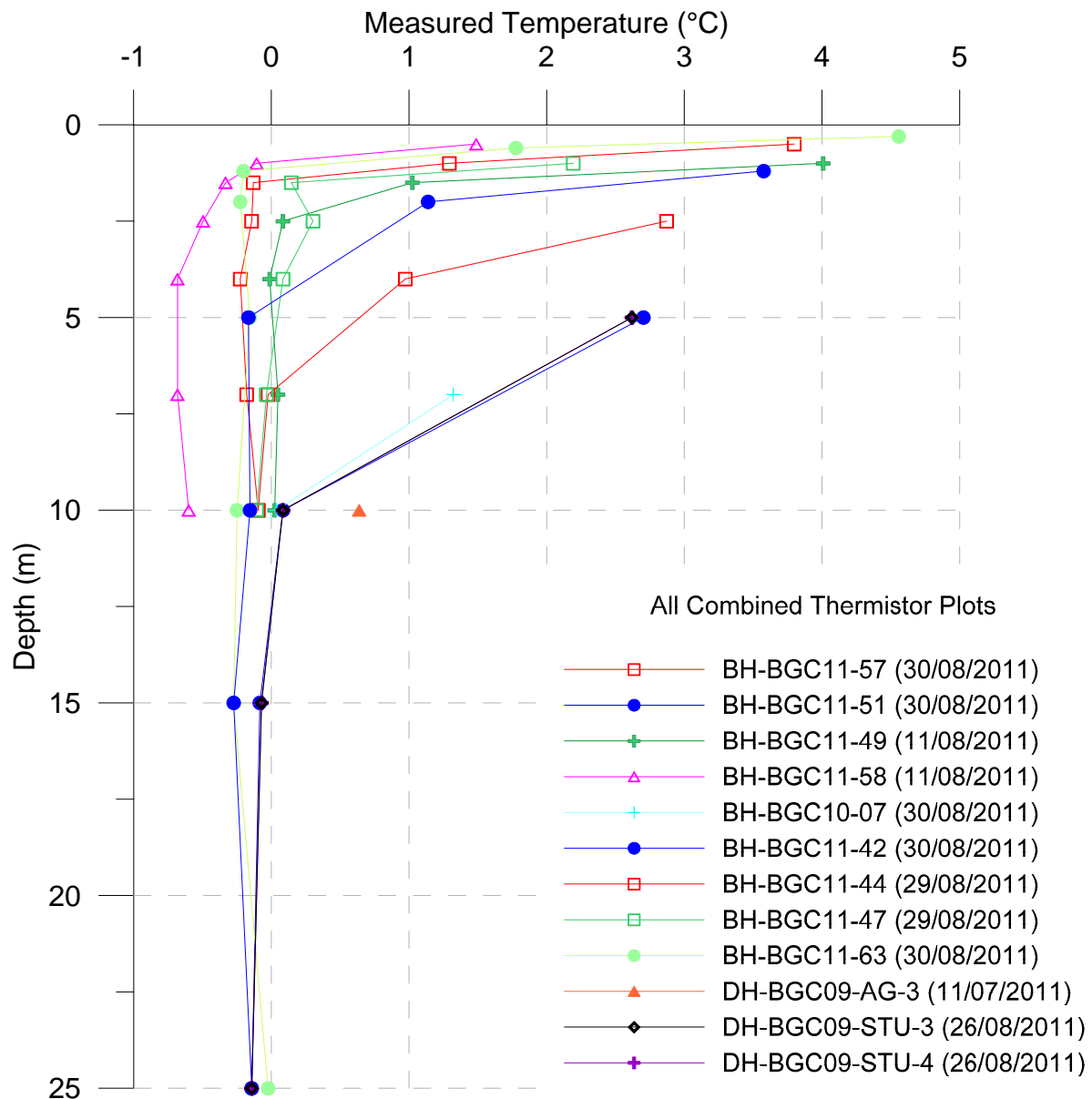
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PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES, BH-BGC11-51		
PROJECT No.	FIG No.	REV.
0792-006	J-12	0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT", DATED JAN 2012.



Note:
1) Temperatures warmer than 5°C not shown for clarity

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CLIENT: VICTORIA GOLD CORP.

PROJECT: 2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: MEASURED GROUND TEMPERATURE PROFILES ALL INSTRUMENTS END OF AUGUST 2011		
PROJECT No. 0792-006	FIG No. J-13	REV. 0

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FIGURE TO BE READ WITH BGC REPORT TITLED "2011 GEOTECHNICAL SITE INVESTIGATION FOR MINE SITE INFRASTRUCTRE FACTUAL DATA REPORT", DATED JAN 2012.

APPENDIX K GROUNDWATER INVESTIGATIONS

GROUNDWATER

1.0 INTRODUCTION

Groundwater observations were made as one component of the 2011 site investigation program. The work consisted of measuring static water levels in standpipe piezometers and obtaining readings of piezometric head from vibrating wire piezometers. The program also included slug testing in selected 2011 BGC standpipes to estimate hydraulic conductivity of selected geological units. Finally, observations were made of surface seepage locations throughout the site as and when encountered during the summer site investigation.

BGC completed a separate program of groundwater investigation for pit depressurization, and for groundwater supply. Those programs will be reported under separate cover.

2.0 PIEZOMETER INSTALLATIONS

A total of 22 standpipe piezometers were installed in auger and diamond drill holes during the 2011 geotechnical site investigation for mine site infrastructure. Two vibrating wire piezometers were installed; one in an auger hole and the other in a diamond drill hole. Piezometer installation details are summarized in Table K-1 and Table K-2, and illustrated graphically on the borehole logs in Appendix D. Piezometer locations are shown on Drawing 09.

Standpipe piezometers typically consisted of 2 inch diameter schedule 80 PVC with 3 m (10 ft) slotted screens. One piezometer with a 6 m (20 ft) screen was installed. Piezometers were either installed at the bottom of the hole or, the hole was backfilled with bentonite to the desired elevation prior to placing the piezometer. Sand packs in the screened zone of the piezometers used medium silica sand. For piezometers installed in diamond drill holes, a bentonite seal of typically 0.5 m to 2 m thick was installed above the sand pack, and the remainder of the hole grouted to surface using a cement-bentonite grout. For piezometers installed in auger holes, the hole was backfilled with bentonite above the sand pack, with the exception of BH-BGC11-55 where a bentonite seal was installed above the sand pack and the hole was backfilled with grout to allow for downhole geophysics.

The vibrating wire piezometer installed in the diamond drill hole (BH-BGC11-54) was taped to a PVC tremmie tube, lowered down the hole and grouted to surface using the sacrificial PVC tremmie. The grout was a cement-bentonite grout mix. The vibrating wire piezometer installed in the auger hole (BH-BGC11-63) was taped to 2 inch solid PVC pipe to be used for a thermistor installation. There was running water at the bottom of BH-BGC11-63 and sandy material from above the bottom of the hole had collapsed into the bottom of the hole. The vibrating wire was pushed through the sloughed material and bentonite chips were used to backfill the hole to surface.

Table K-1 Standpipe Piezometer Installation Details

Hole	Northing (m)	Eastings (m)	Elevation (masl)	PVC Diameter (mm)	Bottom of Sand Pack (mbgs)	Bottom of Screen (mbgs)	Top of Screen (mbgs)	Top of Sand Pack (mbgs)	Height of Stickup (m)
BH-BGC11-26	7102088.1	460205.8	1139.6	51	19.8	18.3	12.2	9.5	0.14
BH-BGC11-29	7101802.1	459977.8	1045.3	51	20.24	20	17	16.2	0.79
BH-BGC11-30	7101567.8	460122.0	952.0	51	32.2	30.7	27.7	27.32	0.85
BH-BGC11-32	7100934.4	458745.2	818.7	51	23.6	23.1	20.1	19.5	0.81
BH-BGC11-33	7100983.6	459115.7	833.0	51	41.4	40.01	37.01	36.3	0.56
BH-BGC11-34	7101053.5	459308.5	848.3	51	34.4	33.7	30.7	30.1	0.82
BH-BGC11-35	7100440.3	459695.7	986.3	51	49	48.3	45.3	44.9	0.97
BH-BGC11-36	7100274.7	459699.9	1002.5	51	50.29	48.8	45.8	45.2	0.93
BH-BGC11-37	7100180.5	459762.8	1034.4	51	42.6	42	39	38.5	0.21
BH-BGC11-38	7100416.0	459820.0	1013.0	51	49.1	48.6	45.6	44.3	1.35
BH-BGC11-39	7101056	458462	804	51	19.1	18.59	15.55	14.6	0.68
BH-BGC11-40B	7100038.8	459767.0	1049.8	51	45.36	43.97	40.97	40.57	0.94
BH-BGC11-45	7099109.5	460556.2	1354.4	51	20.73	19.89	16.89	16.28	0.92
BH-BGC11-46	7099004.0	460262.7	1246.1	51	20.12	17.53	14.53	13.37	1.15
BH-BGC11-48	7099118.1	459936.0	1140.0	51	32.92	32.32	29.27	28.61	0.97
BH-BGC11-52	7101316.4	459891.2	909.0	51	22.56	21.8	18.8	18.3	1
BH-BGC11-55	7100918.4	459440.6	881.0	51	13.1	12.65	9.65	8.5	0.92
BH-BGC11-57	7099978.2	458798.6	859.2	51	10	9.9	6.9	6.1	0.97
BH-BGC11-59	7101239.4	459113.1	883.7	51	30.18	29.08	26.08	25.5	0.93
BH-BGC11-60	7100041.6	458795.6	859.2	51	8.85	8.6	5.6	4.9	0.78
BH-BGC11-62	7100334.1	459785.9	1017.9	51	35.05	34.2	31.2	30.32	0.71
BH-BGC11-64	7100262.9	460182.4	1044.1	51	48.77	46.69	43.6	43.2	1.13

- Holes surveyed by Underhill in August and September 2011 using a differential GPS, with the exception of BH-BGC11-39, which was surveyed by BGC staff using a handheld GPS.
- Coordinates are in UTM NAD 83, Zone 8N.
- Depths are recorded in meters below ground surface (mbgs).

Table K-2 Vibrating Wire Piezometer Installation Details

Hole	Northing (m)	Easting (m)	Elevation (masl)	Serial Number	Manufacturer	Tip Northing (m)	Tip Easting (m)	Tip Elevation (masl)	Tip Depth (mbgs)
BH-BGC11-54	7101245.6	458886.2	883.8	VW18102	RST Instruments	7101245.6	458886.2	846	39.7
BH-BGC11-63	7100114.1	460302.9	1100.7	VW18103	RST Instruments	7100114.1	460302.9	1076	26.3

1. Holes surveyed by Underhill in August and September 2011 using a differential GPS.
2. Coordinates are in UTM NAD 83, Zone 8N.
3. Depths are recorded in meters below ground surface (mbgs).

3.0 WATER LEVEL OBSERVATIONS

Water level measurements in standpipe piezometers and vibrating wire piezometers were made in two separate campaigns. Water levels in select standpipes installed by others in previous years were measured on July 3rd and July 4th, 2011. Water levels in standpipes and vibrating wires installed by BGC in 2011 along with select standpipes installed by others were read between August 25th and August 29th, 2011. Table K-3 summarizes the observed ground water depths.

Table K-3 Groundwater Level Observations

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Date Collected	Depth (mbgs)	Piezometer Type
BH-BGC11-26	7102088.1	460205.8	1139.6	27-Aug-11	16.4	Standpipe
BH-BGC11-29	7101802.1	459977.8	1045.3	27-Aug-11	7.8	Standpipe
BH-BGC11-30	7101567.8	460122.0	952.0	27-Aug-11	16.3	Standpipe
BH-BGC11-32	7100934.4	458745.2	818.7	24-Aug-11	10.8	Standpipe
BH-BGC11-33	7100983.6	459115.7	833.0	26-Aug-11	4.2	Standpipe
BH-BGC11-34	7101053.5	459308.5	848.3	24-Aug-11	8.3	Standpipe
BH-BGC11-35	7100440.3	459695.7	986.3	27-Aug-11	24.6	Standpipe
BH-BGC11-36	7100274.7	459699.9	1002.5	27-Aug-11	19.9	Standpipe
BH-BGC11-38	7100416.0	459820.0	1013.0	25-Aug-11	26.3	Standpipe
BH-BGC11-39	7101056	458462	804	24-Aug-11	9.8	Standpipe
BH-BGC11-40B	7100038.8	459767.0	1049.8	25-Aug-11	8.5	Standpipe
BH-BGC11-45	7099109.5	460556.2	1354.4	27-Aug-11	8.4	Standpipe
BH-BGC11-46	7099004.0	460262.7	1246.1	27-Aug-11	15.3	Standpipe
BH-BGC11-48	7099118.1	459936.0	1140.0	27-Aug-11	31.3	Standpipe
BH-BGC11-52	7101316.4	459891.2	909.0	25-Aug-11	4.1	Standpipe
BH-BGC11-54	7101245.6	458886.2	883.8	29-Aug-11	11.8	Vibrating Wire
BH-BGC11-55	7100918.4	459440.6	881.0	27-Aug-11	dry	Standpipe
BH-BGC11-57	7099978.2	458798.6	859.2	27-Aug-11	dry	Standpipe
BH-BGC11-59	7101239.4	459113.1	883.7	27-Aug-11	29.4	Standpipe
BH-BGC11-60	7100041.6	458795.6	859.2	27-Aug-11	dry	Standpipe
BH-BGC11-62	7100334.1	459785.9	1017.9	27-Aug-11	20.2	Standpipe
BH-BGC11-63	7100114.1	460302.9	1100.7	29-Aug-11	18.1	Vibrating Wire
GT96-26	7101825	462617	n/a	3-Jul-11	30.1	Standpipe
GT96-35	7099081	460238	n/a	27-Aug-11	3.4	Standpipe
MW09-AG1	7101949	459425	1017	25-Aug-11	15.7	Standpipe
MW09-AG2	7101981	459785	1009	27-Aug-11	12.7	Standpipe
MW09-DG1	7101009	459325	840	4-Jul-11	1.8	Standpipe
MW09-DG1	7101009	459325	840	27-Aug-11	2.3	Standpipe
MW09-DG2	7100877	458992	824	4-Jul-11	0.9	Standpipe

Hole ID	Northing (m)	Easting (m)	Elevation (masl)	Date Collected	Depth (mbgs)	Piezometer Type
MW09-DG4	7101111	458284	787	4-Jul-11	5.6	Standpipe
MW09-DG5	7100607	458397	810	27-Aug-11	10.7	Standpipe
MW09-OG2	7100599	462221	1332	3-Jul-11	2.2	Standpipe
MW09-OG3	7101552	461223	1065	3-Jul-11	0.4	Standpipe
MW09-STU1	7100648	459771	967	4-Jul-11	14.3	Standpipe
MW09-STU1	7100648	459771	967	27-Aug-11	14.4	Standpipe
MW09-STU2	7100799	459212	857	4-Jul-11	2.1	Standpipe
MW10-DG06	7101120	459552	859	3-Jul-11	2.2	Standpipe
MW10-OBS1	7101100	458443	796	4-Jul-11	7.2	Standpipe
MW10-OBS2	7101116	458405	793	4-Jul-11	5.3	Standpipe
MW96-01	7100965	463763	1398	3-Jul-11	23.1	Standpipe
MW96-02	7101047	463679	1394	3-Jul-11	8.1	Standpipe
MW96-03a	7101135	463597	1384	3-Jul-11	1.9	Standpipe
MW96-04	7101229	463508	1373	3-Jul-11	4.6	Standpipe
MW96-05	7101301	463429	1363	3-Jul-11	2.7	Standpipe
MW96-06a	7101636	463913	1405	3-Jul-11	2.6	Standpipe
MW96-07b	7101670	463596	n/a	3-Jul-11	0.0	Standpipe
MW96-08	7101458	463252	1339	3-Jul-11	4.9	Standpipe
MW96-09a	7101244	463076	1361	3-Jul-11	4.5	Standpipe
MW96-09b	7101252	463073	1361	3-Jul-11	6.7	Standpipe
MW96-10a	7101121	462939	1367	3-Jul-11	2.2	Standpipe
MW96-10b	7101132	462936	1368	3-Jul-11	2.0	Standpipe
MW96-13a	7100925	460003	984	3-Jul-11	5.3	Standpipe
MW96-13b	7100925	460003	984	3-Jul-11	14.9	Standpipe
MW96-14a	7100609	460145	976	3-Jul-11	dry	Standpipe
MW96-14a	7100609	460145	976	27-Aug-11	dry	Standpipe
MW96-14b	7100609	460145	976	27-Aug-11	3.6	Standpipe
MW96-14b	7100609	460145	976	3-Jul-11	3.6	Standpipe
MW96-15a	7101037	459679	943	4-Jul-11	0.8	Standpipe
MW96-15a	7101037	459679	943	27-Aug-11	1.5	Standpipe
MW96-26	7101516	462873	1316	3-Jul-11	6.5	Standpipe
MW96-27	7101562	462876	1308	3-Jul-11	9.3	Standpipe

1. Holes surveyed in August and September 2011 using a differential GPS, with the exception of BH-BGC11-39, which was surveyed by BGC staff using a handheld GPS.
2. Coordinates in UTM NAD 83, Zone 8N.
3. Depths are recorded in meters below ground surface (mbgs).
4. Linear calibration used for vibrating wire piezometers.

4.0 SLUG TESTS

Five single well response tests (slug tests) were conducted in the holes BH-BGC11-32, -33, -34, -39 and -52, all of which are located in the Dublin Gulch valley bottom. Three iterations of a 0.9 L rising head slug test were performed for each well, however in one of the iterations in borehole BH-BGC11-39 problems occurred with the data logger and the results were not analyzed. The water level response was induced by removing a single bailer (0.9 L) full of water. The water level response was recorded with Solinst's 3001 Levellogger Gold submersible pressure transducers and data loggers, as well as manually, for an in-field QA/QC. The data loggers were set to record water level recovery versus time on a reading per second or reading per half-second interval.

The testing results were reviewed and interpreted using the Hvorslev method of analysis as implemented in AQTESOLVE Pro Version 4.50.002 to calculate hydraulic conductivity. Hvorslev (1951) developed a semi-analytical method for the analysis of an overdamped slug test in a fully or partially penetrating well in a homogeneous, anisotropic confined aquifer. The Hvorslev method employs a quasi-steady-state model that ignores elastic storage in the aquifer. Well response data and calculated hydraulic conductivity results are summarized in Table K-4. Detailed calculation results are provided in the figures at the end of this Appendix.

5.0 SURFACE SEEPAGE OBSERVATIONS

When observed, locations of surface seepage were recorded. The location was noted and approximate seepage rate was estimated where possible. The list of surface seepage locations in Table K-5 is not an exhaustive list of surface seepage locations at the site, but rather a list of surface seepage locations observed by BGC staff on an opportunity basis throughout the course of the site investigation program. Seeps were typically observed while marking bore hole or test pit locations, investigating access to bore hole or test pit locations or while completing test pits and mapping outcrops. Photos of selected surface seepage observations are included below.

Table K-4 Summary of hydraulic conductivities

Hole ID	Elevation (masl)	Formation/Unit	Top and Bottom of sand pack (mbgs)	Average conductivity, k (m/sec)
BH-BGC11-32	819	Placer Tailings and Weathered Bedrock ¹	19.5 – 23.6	2.5 x 10 ⁻⁶
BH-BGC11-33	833	Weathered Bedrock	36.3 – 41.4	1.1 x 10 ⁻⁷
BH-BGC11-34	848	Weathered Bedrock	30.1 – 34.4	2.6 x 10 ⁻⁶
BH-BGC11-39	804	Placer Tailings	14.6 – 18.6	8.1 x 10 ⁻⁵
BH-BGC11-52	909	Weathered Bedrock	18.3 – 22.6	9.7 x 10 ⁻⁶

Note:

1) Screened pipe was installed over two units, therefore the conductivity (k) cannot be representative of any one unit.

Table K-5 Surface Seepage Observations

Seep Number	Northing (m)	Easting (m)	Elevation (masl)	Date	Approximate flow rate (L/min)	Photo taken	Trending (°)	Comments
1	7100826	460028	941	10-Jul-11	-	Y	-	Moderate seep on side of the Eagle Pup road
2	7098974	459969	1144	11-Jul-11	-	-	-	
3	7100583	460169	982	11-Jul-11	-	-	-	
4	7099527	459241	899	16-Jul-11	4.5	Y	40	Seeping from two locations 20 m apart beneath topsoil
5	7099502	459276	907	16-Jul-11	13.6-18.2	Y	-	40 m east of seep #4, seeping from under moss
6	7099327	459584	986	16-Jul-11	13.6-18.2	Y	28	Seep through 2 m bank, 6 m wide
7	7100205	459144	936	22-Jul-11	0.5	-	-	Very minor seep at side of road beside TP; a second small seep ~10 m to the North
8	7099601	460594	1334	27-Jul-11	-	-	-	2 very small seeps ~10 m apart on an old drill pad
9	7099623	460571	1330	27-Jul-11	-	-	-	Small seep on road below previous seeps
10	7099978	460937	1320	27-Jul-11	0.5	-	-	Very small seep at old drill pad at top of Stewart Gulch,
11	7101520	461754	1209	27-Jul-11	0.25	-	-	Second small seep ~25m North (~.75L/min)
12	7100836	460035	942	23-Jul-11	< 1	-	-	2-3 coalescing seeps along side of road
13	7100379	460384	1069	23-Jul-11	< 5	-	-	Moderate seep beside drill pad
14	7100358	460356	1067	23-Jul-11	> 5-10	-	-	Large seep along old access road
15	7100283	460307	1067	23-Jul-11	< 1	-	-	Small seep along access road





Seep Number	Northing (m)	Easting (m)	Elevation (masl)	Date	Approximate flow rate (L/min)	Photo taken	Trending (°)	Comments
16	7100259	460295	1068	23-Jul-11	< 5	-	-	Moderate seep
17	7100230	460277	1068	23-Jul-11	< 100	-	-	Very small creek
18	7100234	460688	1204	23-Jul-11	1-5	-	-	Moderate seep along road cut
19	7100144	460664	1207	23-Jul-11	< 1	-	-	Small seep along road cut
20	7100120	460652	1208	23-Jul-11	< 1	-	-	Small seep along road cut
21	7100094	460633	1208	23-Jul-11	< 0.5	-	-	Very small seep along road cut
22	7100078	460624	1208	23-Jul-11	< 1	-	-	Very small seep at old drill pad ct
23	7099047	459965	1125	29-Jul-11	-	-	-	
24	7099116	460250	1264	31-Jul-11	-	-	-	Large seep along existing access road
25	7098868	460034	1162	29-Jul-11	-	-	-	
26	7099061	459007	922	17-Aug-11	-	-	-	
27	7101806	461903	1166	12-Jul-11	-	-	-	Logged at test pit TP-BGC11-79
28	7100144	460540	1158	27-Aug-11	-	Y	-	Coordinates are for test pit TP-BGC11-144. Several seeps noted along access trail to the test pit.

1. Location for BGC holes from handheld GPS measurements.
2. Coordinates in UTM NAD 83, Zone 8N.



Seep 1

Seep 4

	
<p>Seep 5</p>	<p>Seep 6 – View A</p>
	
<p>Seep 6 – View B</p>	<p>Seep 28 – Seep found at TP-BGC11-144</p>

6.0 REFERENCES

Hvorslev, M.J., 1951. Time Lag and Soil Permeability in Ground-Water Observations, Bull. No. 36, Waterways Exper. Sta. Corps of Engrs, U.S. Army, Vicksburg, Mississippi, pp. 1-50.

FIGURES

BH-BGC11-32 Test 1

Prepared By:

BGC Engineering

Prepared For:

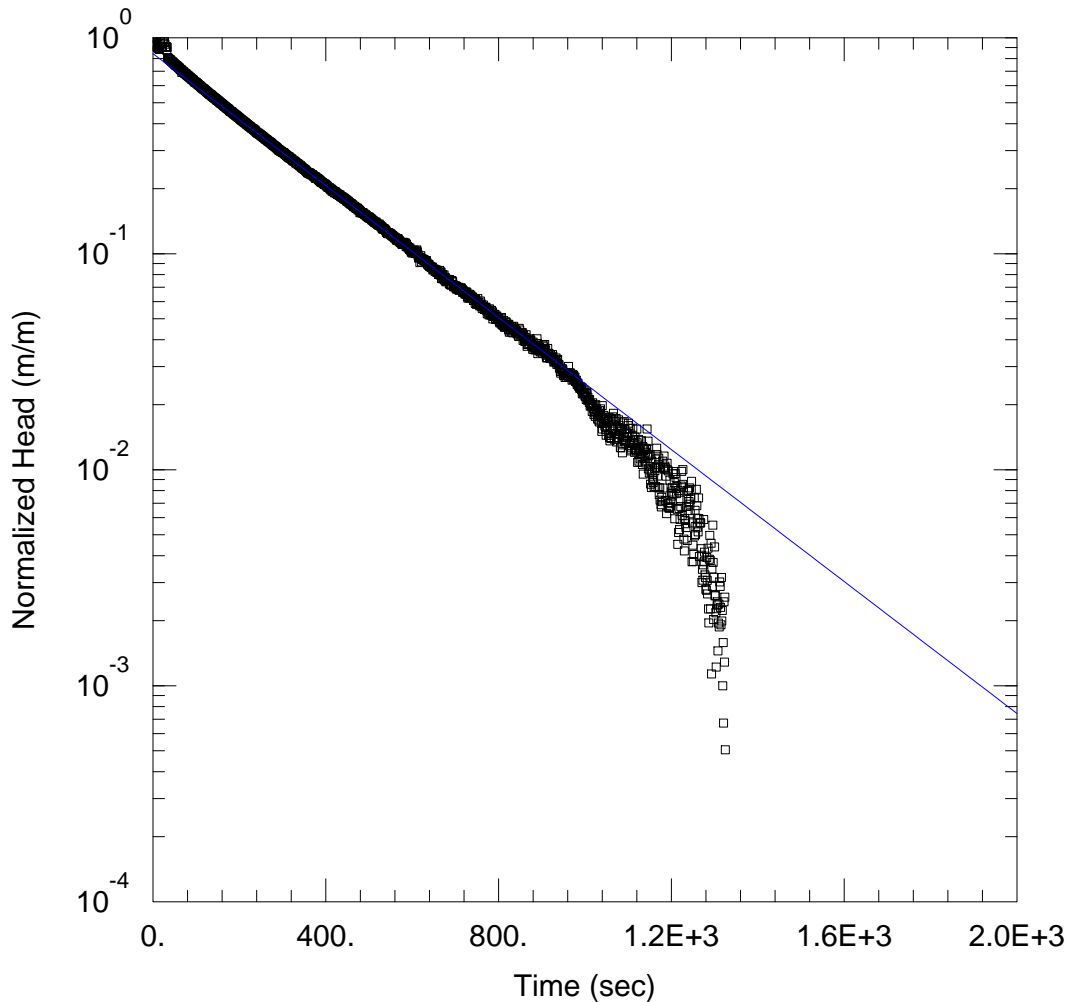
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-32 Test 1.aqt
Date: 10/18/11 Time: 11:36:23

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 1.564×10^{-6} m/sec $y_0 =$ 0.3743 m

AQUIFER DATA

Saturated Thickness: 3.5 m Anisotropy Ratio (K_z/K_r): 1.

WELL DATA (BH-BGC11-32)

Initial Displacement: 0.4436 m
Static Water Column Height: 12.29 m
Total Well Penetration Depth: 3.3 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-32 Test 2

Prepared By:

BGC Engineering

Prepared For:

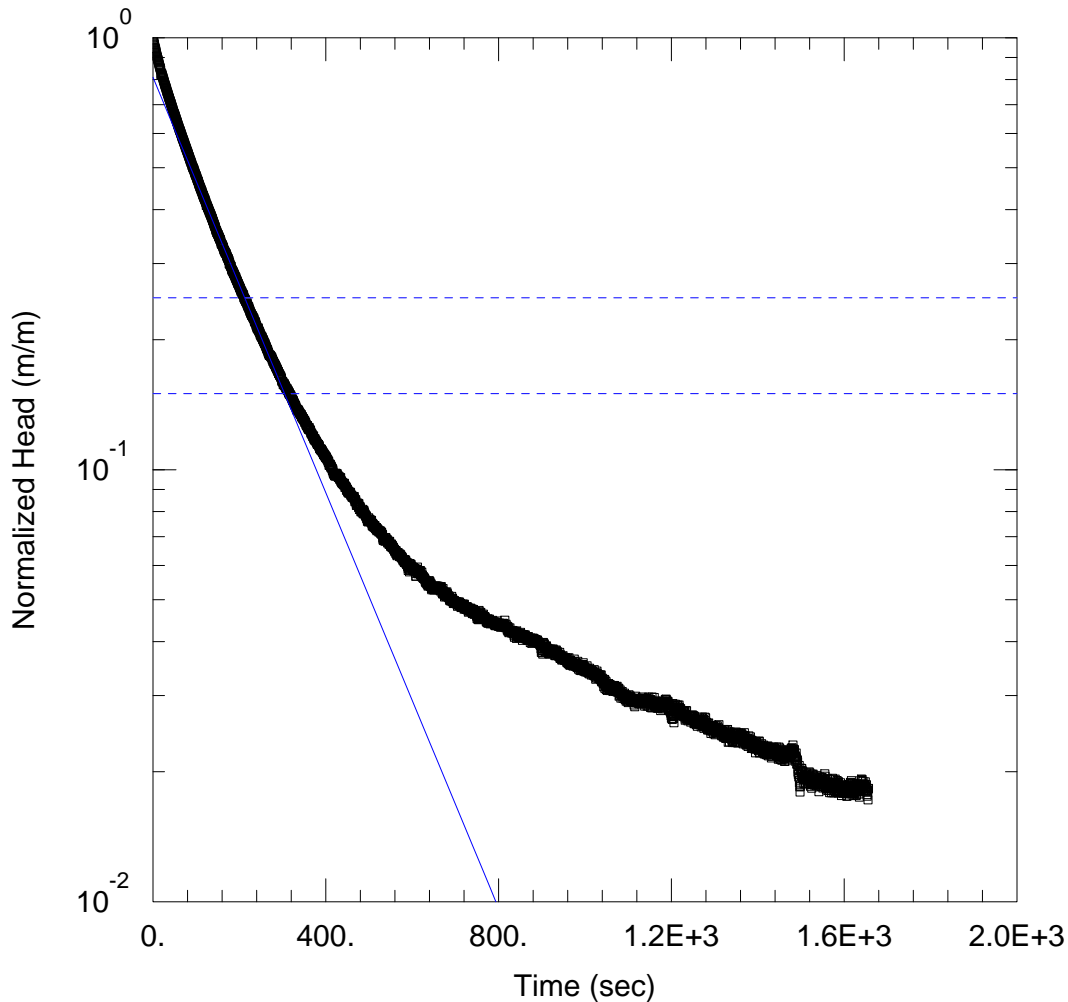
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-32 Test 2.aqt
Date: 10/18/11 Time: 11:39:58

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 2.873E-6 m/sec y0 = 0.4243 m

AQUIFER DATA

Saturated Thickness: 3.3 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-32)

Initial Displacement: 0.5241 m
Static Water Column Height: 12.31 m
Total Well Penetration Depth: 3.3 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-32 Test 3

Prepared By:

BGC Engineering

Prepared For:

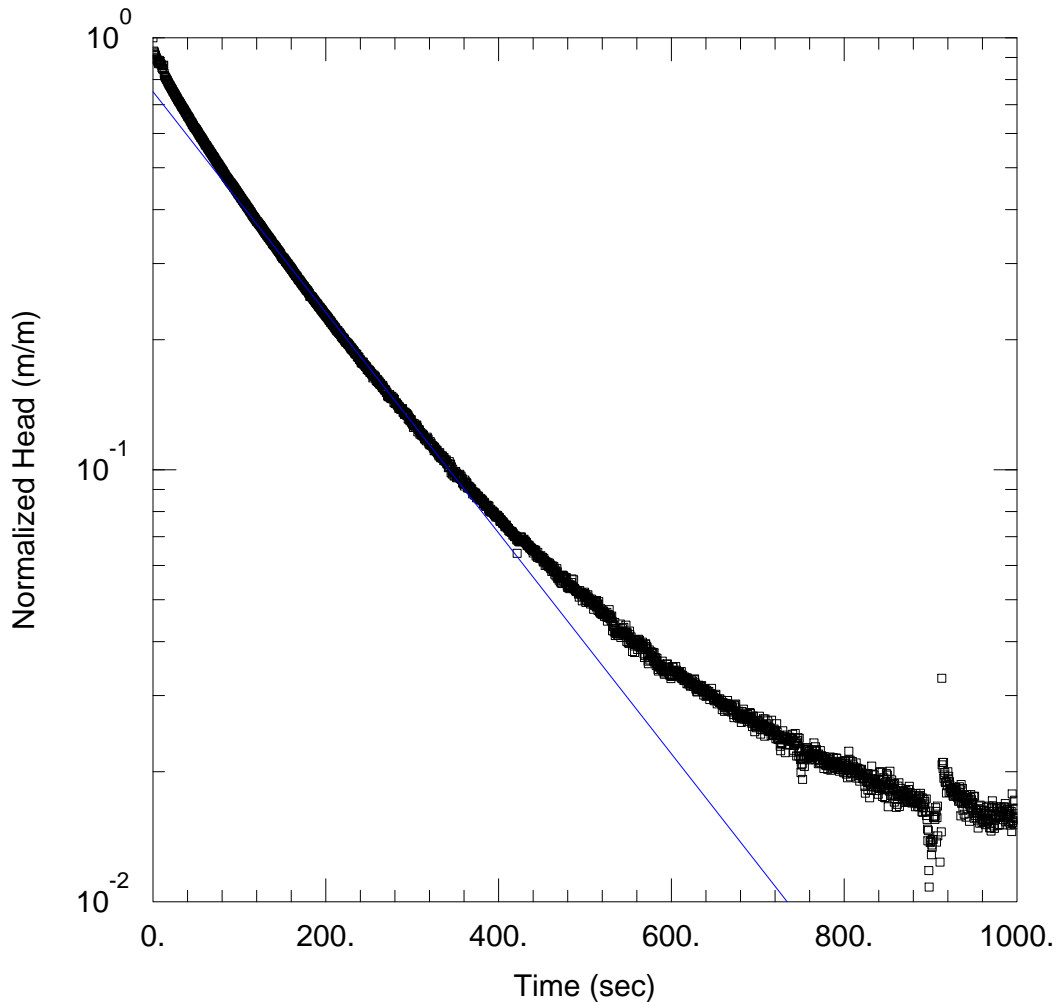
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-32 Test 3.aqt
Date: 10/18/11 Time: 11:42:04

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 3.054E-6 m/sec y0 = 0.3856 m

AQUIFER DATA

Saturated Thickness: 3.3 m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-32)

Initial Displacement: 0.514 m
Static Water Column Height: 12.31 m
Total Well Penetration Depth: 3.3 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-33 Test 1

Prepared By:

BGC Engineering

Prepared For:

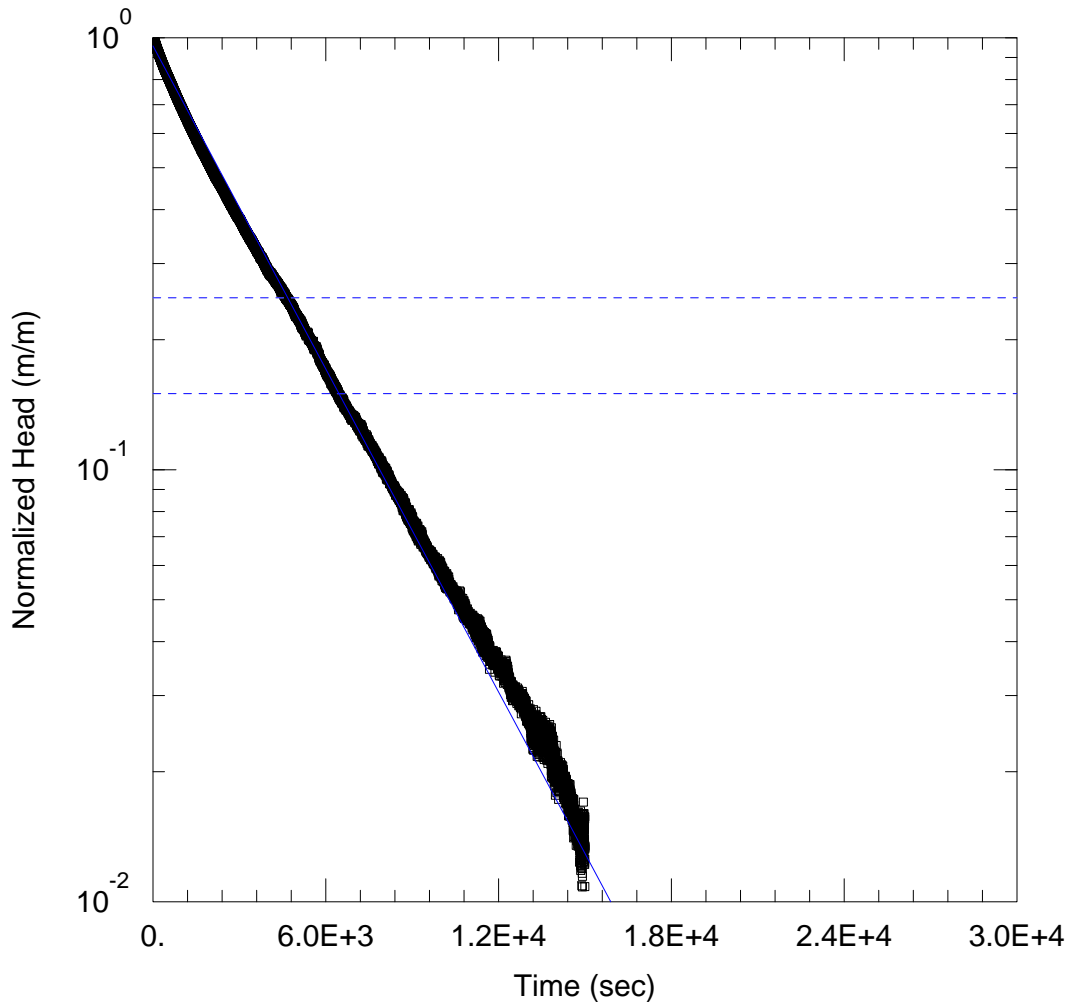
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-33 Test 1.aqt
Date: 10/18/11 Time: 13:19:19

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 1.276E-7 m/sec y0 = 0.4767 m

AQUIFER DATA

Saturated Thickness: 29.6 Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-33)

Initial Displacement: 0.498 m
Static Water Column Height: 33.98 m
Total Well Penetration Depth: 28.95 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-33 Test 2

Prepared By:

BGC Engineering

Prepared For:

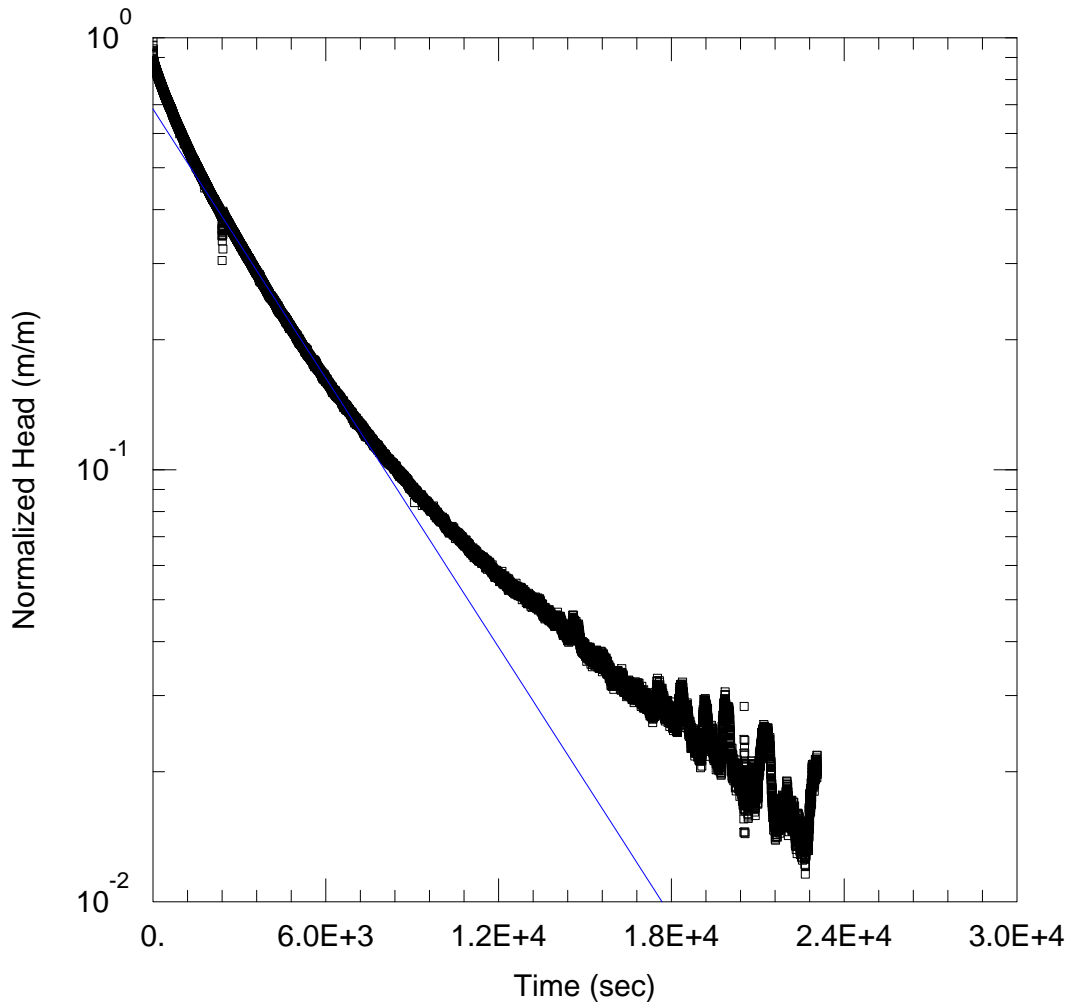
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-33 Test 2.aqt
Date: 10/18/11 Time: 11:55:48

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 1.063E-7 m/sec y0 = 0.3466 m

AQUIFER DATA

Saturated Thickness: 29.6 Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-33)

Initial Displacement: 0.507 m
Static Water Column Height: 34. m
Total Well Penetration Depth: 28.95 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

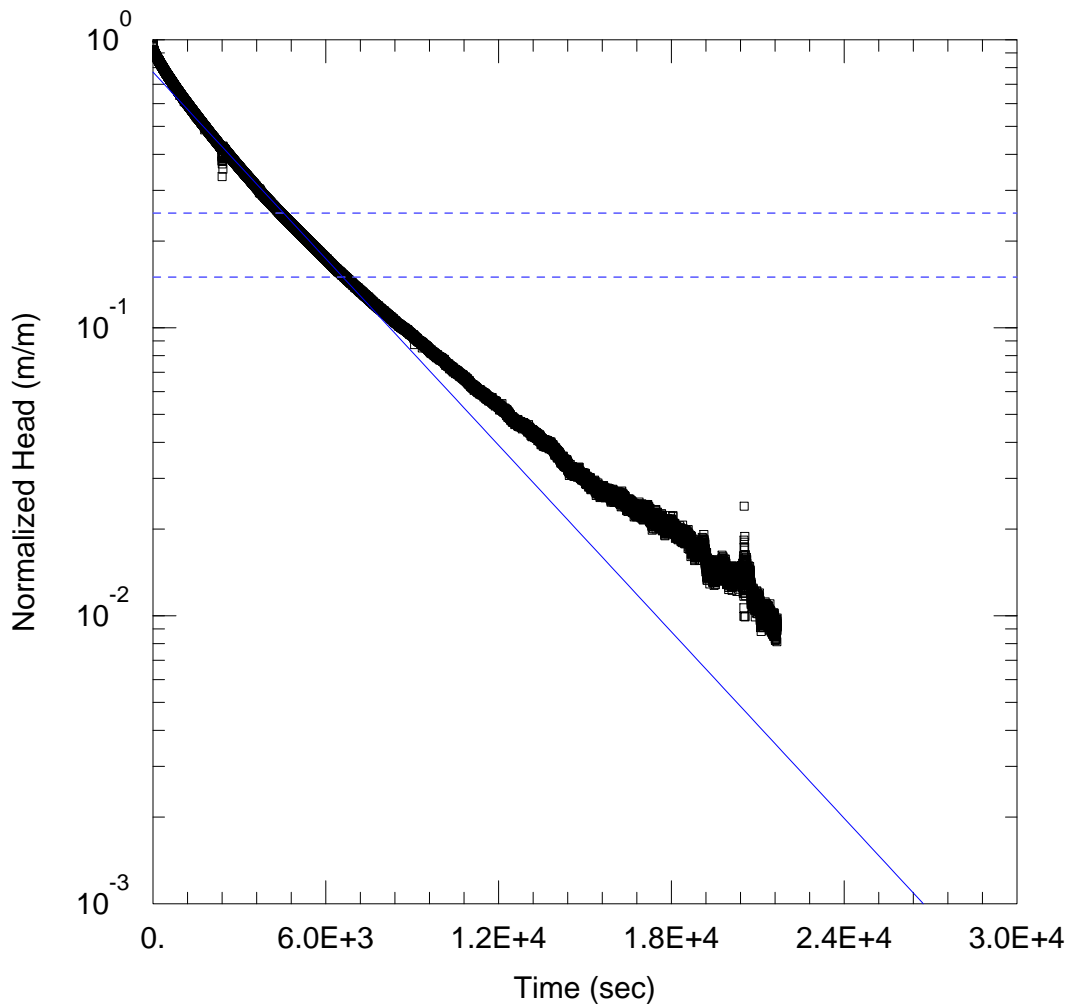
BH-BGC11-33 Test 3

Prepared By:
BGC Engineering

Prepared For:
Victoria Gold Corp.

Project:
0792006

Location:
Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-33 Test 3.aqt
 Date: 10/18/11 Time: 11:58:19

SOLUTION

Aquifer Model: Confined
 Solution Method: Hvorslev
 $K = 1.105E-7$ m/sec $y_0 = 0.3905$ m

AQUIFER DATA

Saturated Thickness: 29.6 Anisotropy Ratio (K_z/K_r): 1.

WELL DATA (BH-BGC11-33)

Initial Displacement: 0.506 m
 Static Water Column Height: 34. m
 Total Well Penetration Depth: 28.95 m
 Screen Length: 3. m
 Casing Radius: 0.0254 m
 Well Radius: 0.048 m

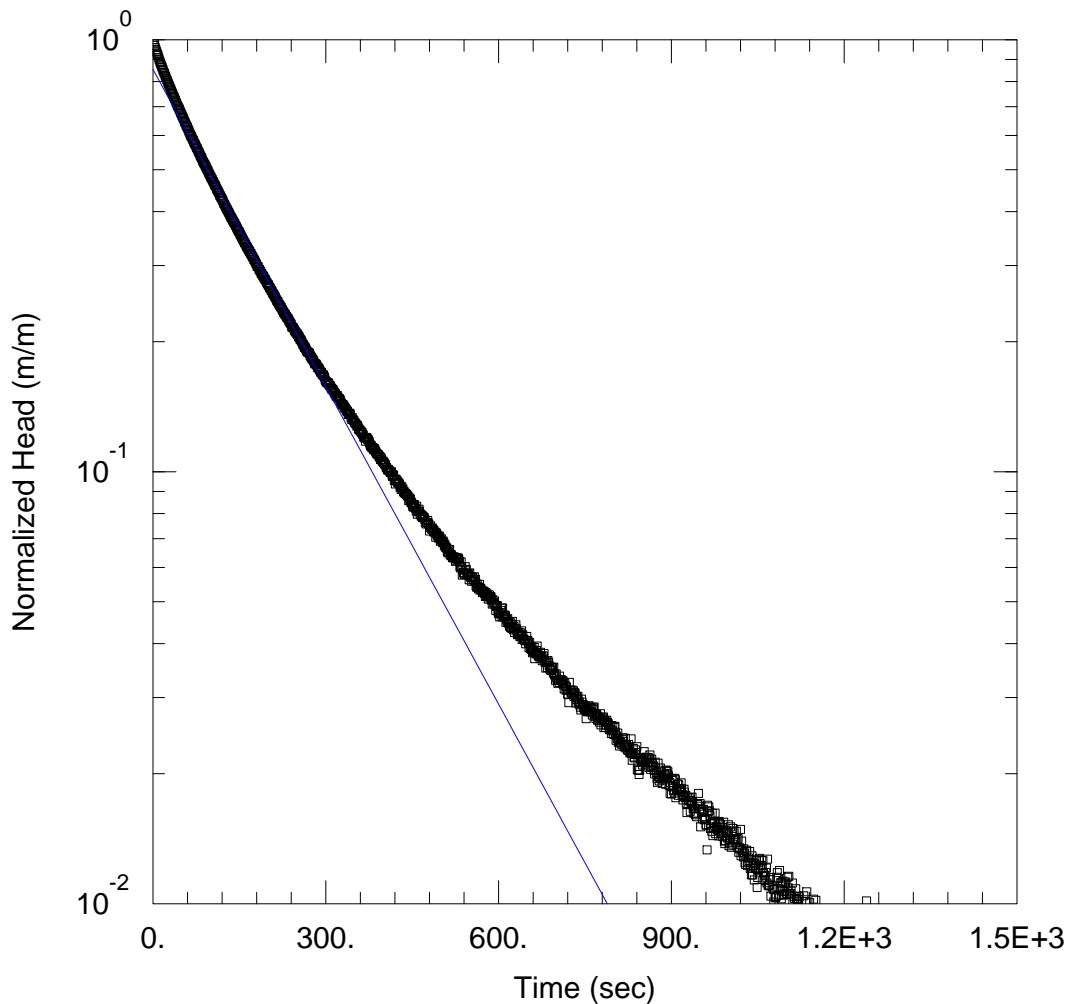
BH-BGC11-34 Test 1

Prepared By:
BGC Engineering

Prepared For:
Victoria Gold Corp.

Project:
0792006

Location:
Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-34 Test 1.aqt
Date: 10/18/11 Time: 12:58:00

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 2.507E-6 m/sec y0 = 0.455 m

AQUIFER DATA

Saturated Thickness: 16.3 Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-34)

Initial Displacement: 0.533 m
Static Water Column Height: 24.48 m
Total Well Penetration Depth: 15.75 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-34 Test 2

Prepared By:

BGC Engineering

Prepared For:

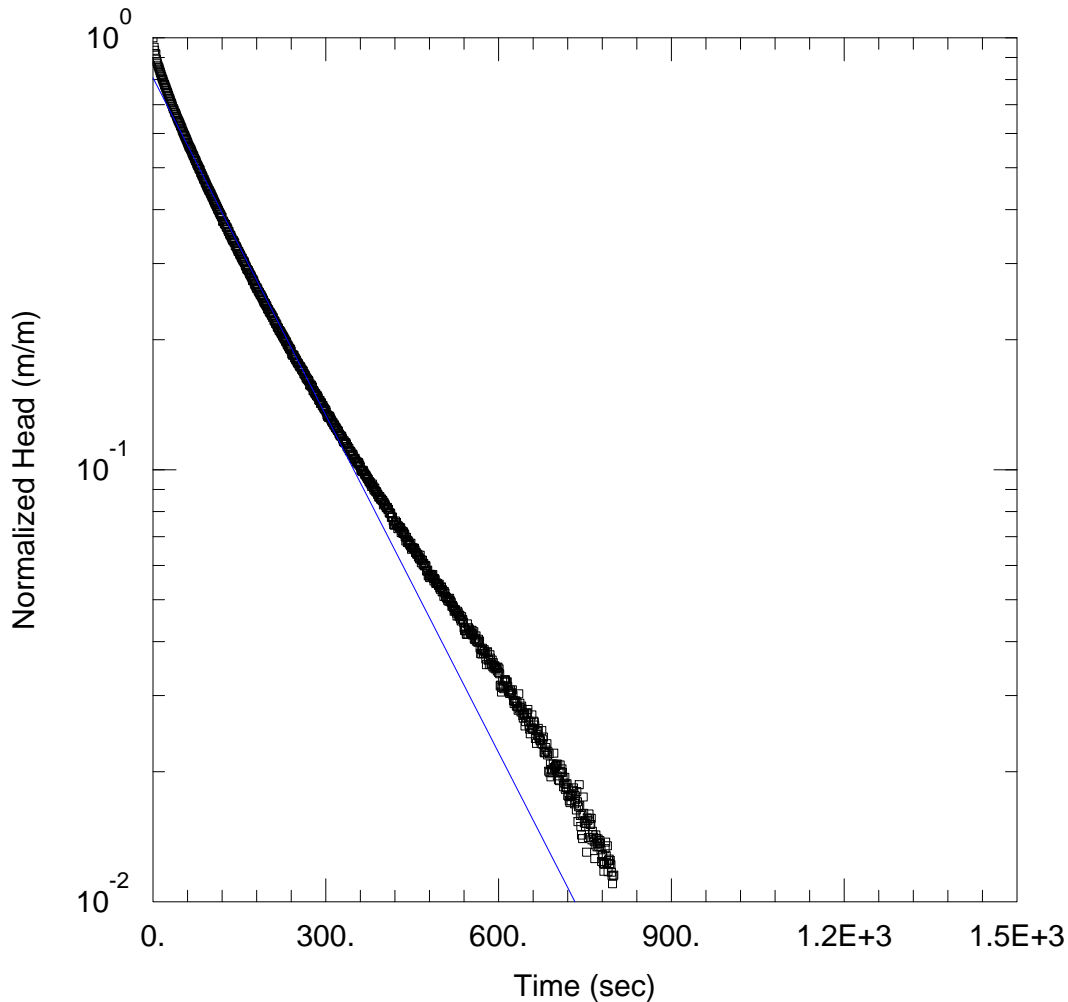
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-34 Test 2.aqt
Date: 10/18/11 Time: 12:59:56

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = $2.666E-6$ m/sec $y_0 =$ 0.4499 m

AQUIFER DATA

Saturated Thickness: 16.3 Anisotropy Ratio (K_z/K_r): 1.

WELL DATA (BH-BGC11-34)

Initial Displacement: 0.557 m
Static Water Column Height: 24.48 m
Total Well Penetration Depth: 15.75 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-34 Test 3

Prepared By:

BGC Engineering

Prepared For:

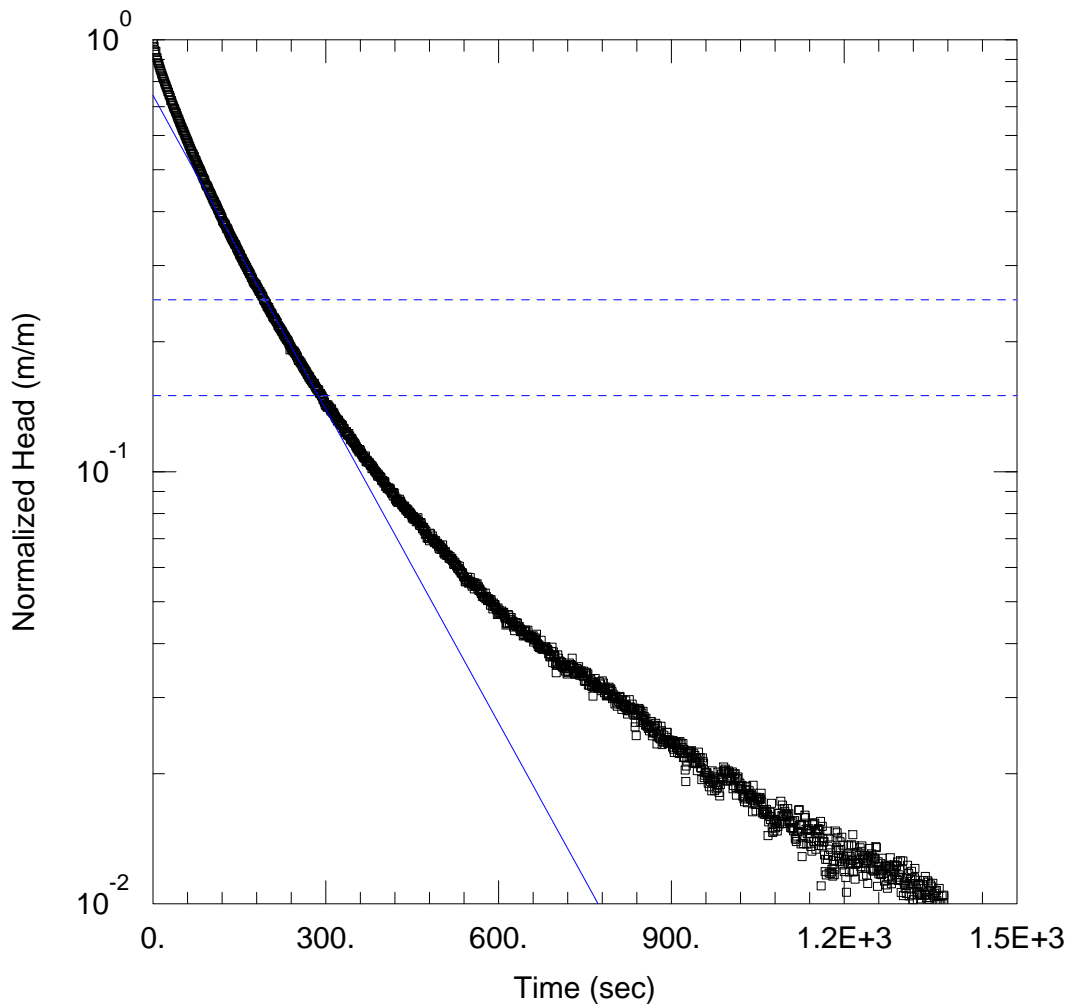
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-34 Test 3.aqt
Date: 10/18/11 Time: 13:16:59

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 2.479E-6 m/sec y0 = 0.3887 m

AQUIFER DATA

Saturated Thickness: 16.3 Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-34)

Initial Displacement: 0.523 m
Static Water Column Height: 24.49 m
Total Well Penetration Depth: 15.75 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

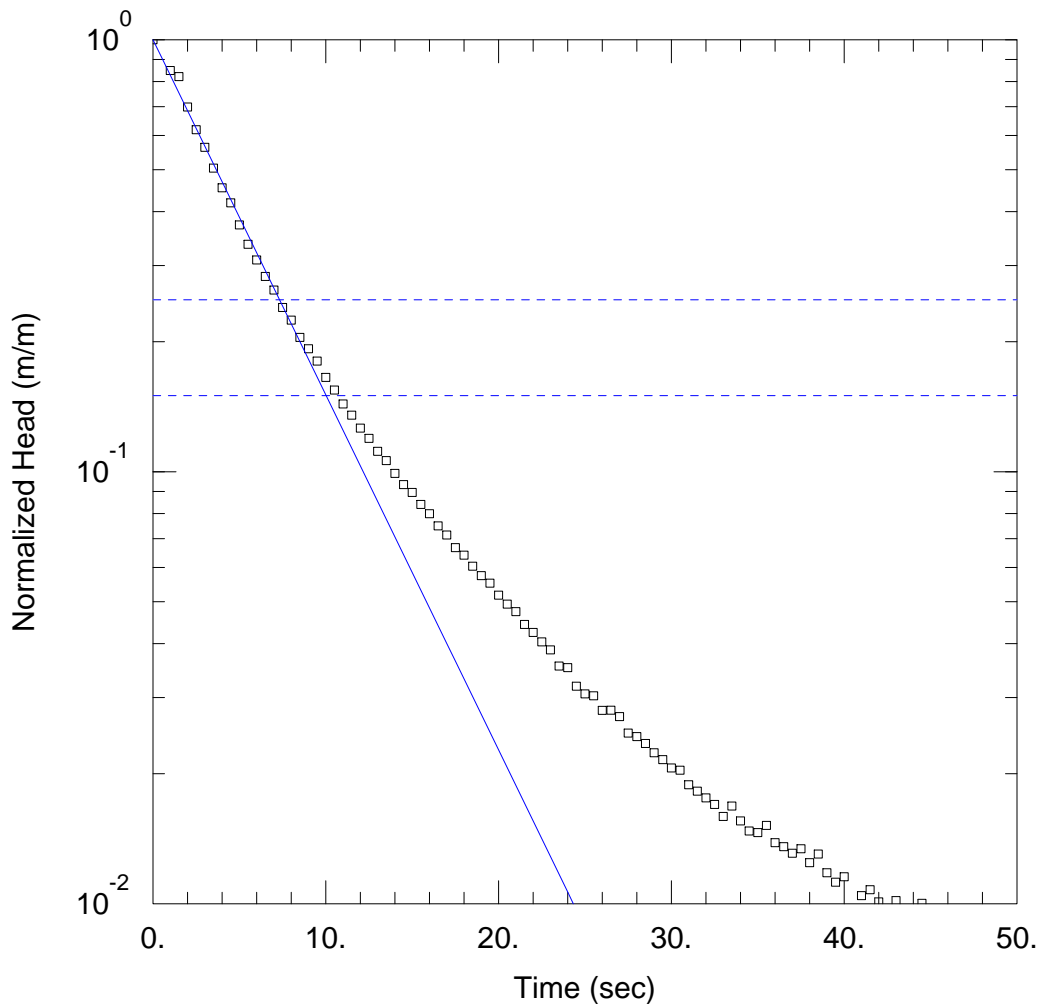
BH-BGC11-39 Test 1

Prepared By:
BGC Engineering

Prepared For:
Victoria Gold Corp.

Project:
0792006

Location:
Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-39 Test 1_twc_CaseA.aqt
 Date: 10/18/11 Time: 14:17:36

AQUIFER DATA

Saturated Thickness: 8.84 Anisotropy Ratio (Kz/Kr): 1.

SOLUTION

Aquifer Model: Unconfined
 Solution Method: Hvorslev
 K = 0.0001074 m/sec y0 = 0.4792 m

WELL DATA (BH-BGC11-39)

Initial Displacement: 0.479 m
 Static Water Column Height: 9.73 m
 Total Well Penetration Depth: 6.74 m
 Screen Length: 1.8 m
 Casing Radius: 0.0254 m
 Well Radius: 0.076 m

BH-BGC11-39 Test 2

Prepared By:

BGC Engineering

Prepared For:

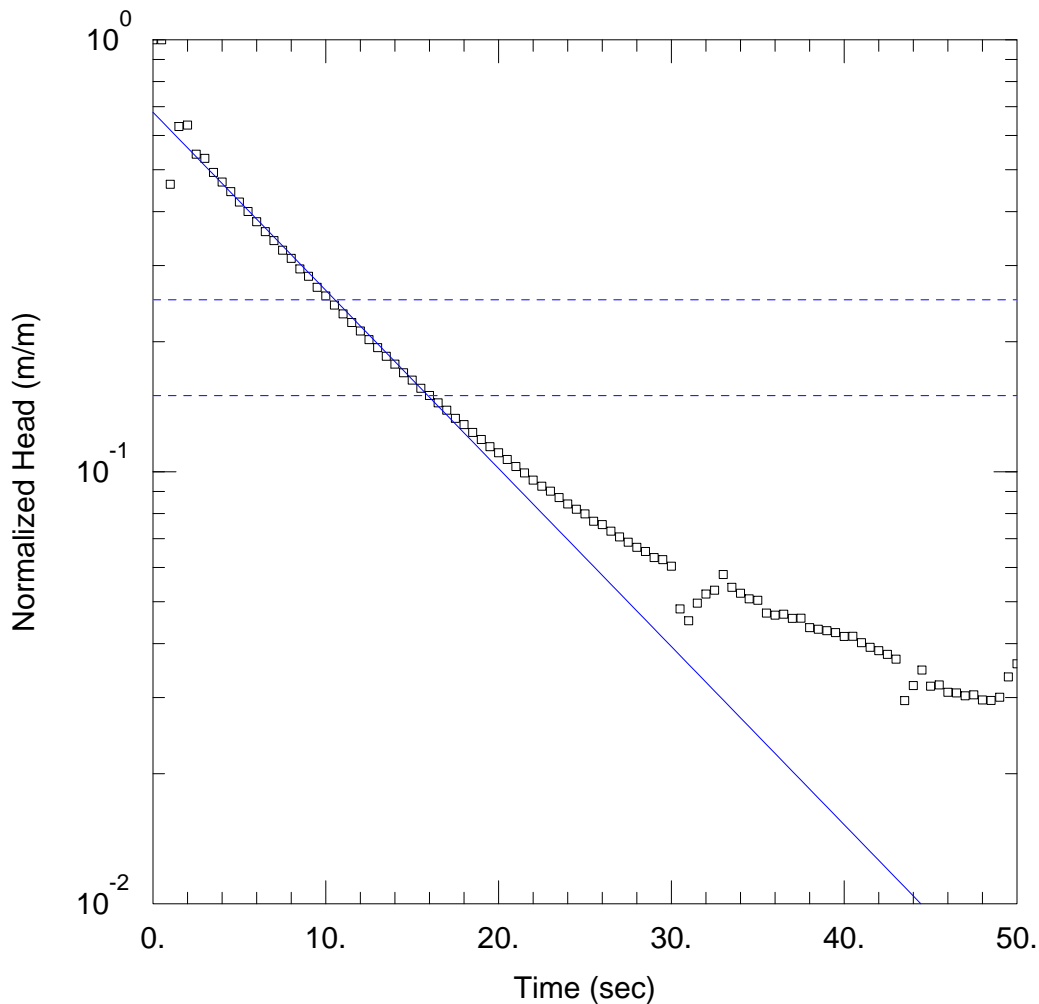
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-39 Test 2twc_CaseA.aqt

Date: 10/18/11

Time: 13:38:47

AQUIFER DATA

Saturated Thickness: 8.82 Anisotropy Ratio (K_z/K_r): 1.

SOLUTION

Aquifer Model: Unconfined

Solution Method: Hvorslev

$K = 5.387E-5$ m/sec $y_0 = 0.4056$ m

WELL DATA (BH-BGC11-39)

Initial Displacement: 0.597 m

Static Water Column Height: 9.78 m

Total Well Penetration Depth: 6.72 m

Screen Length: 1.8 m

Casing Radius: 0.0254 m

Well Radius: 0.076 m

BH-BGC11-52 Test 1

Prepared By:

BGC Engineering

Prepared For:

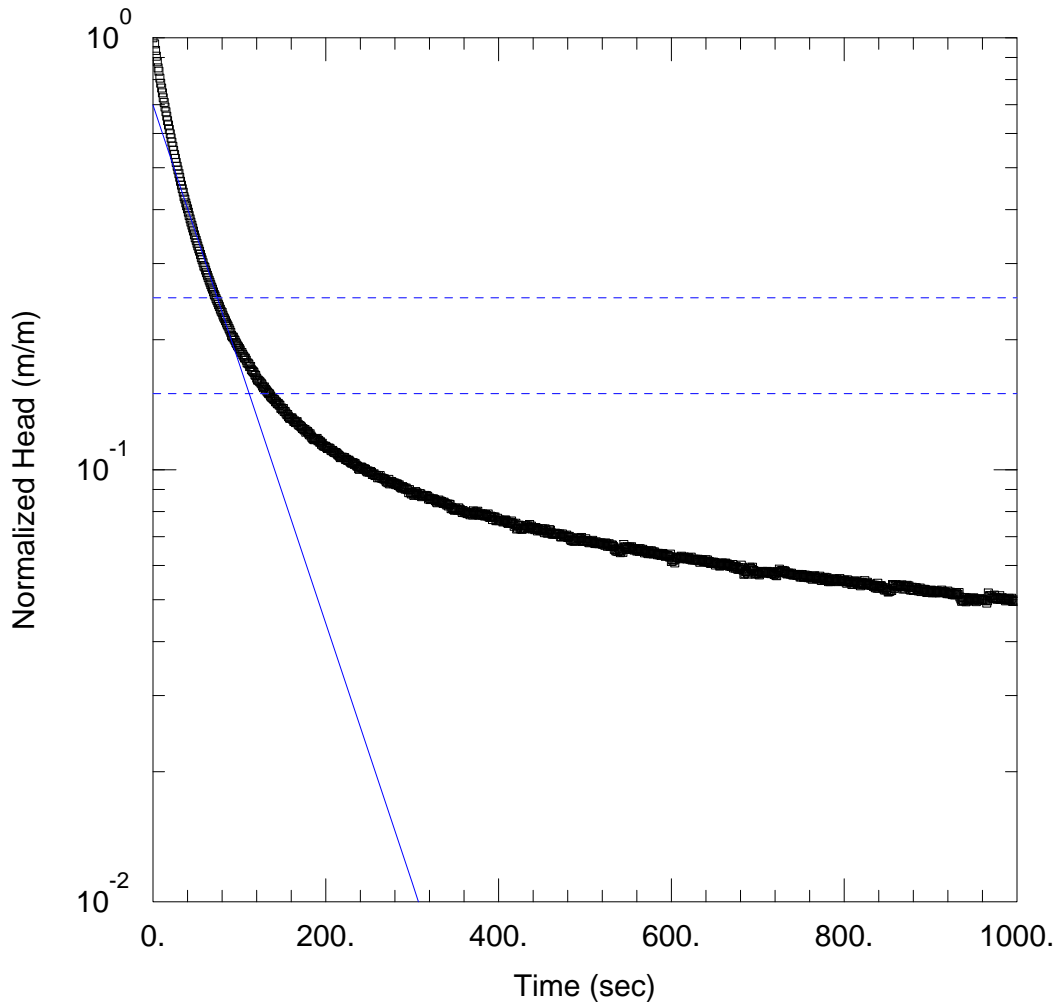
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-52 Test 1.aqt
Date: 10/18/11 Time: 14:06:37

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 7.174E-6 m/sec y0 = 0.376 m

AQUIFER DATA

Saturated Thickness: 10.5 Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-52)

Initial Displacement: 0.538 m
Static Water Column Height: 17.72 m
Total Well Penetration Depth: 10.5 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-52 Test 2

Prepared By:

BGC Engineering

Prepared For:

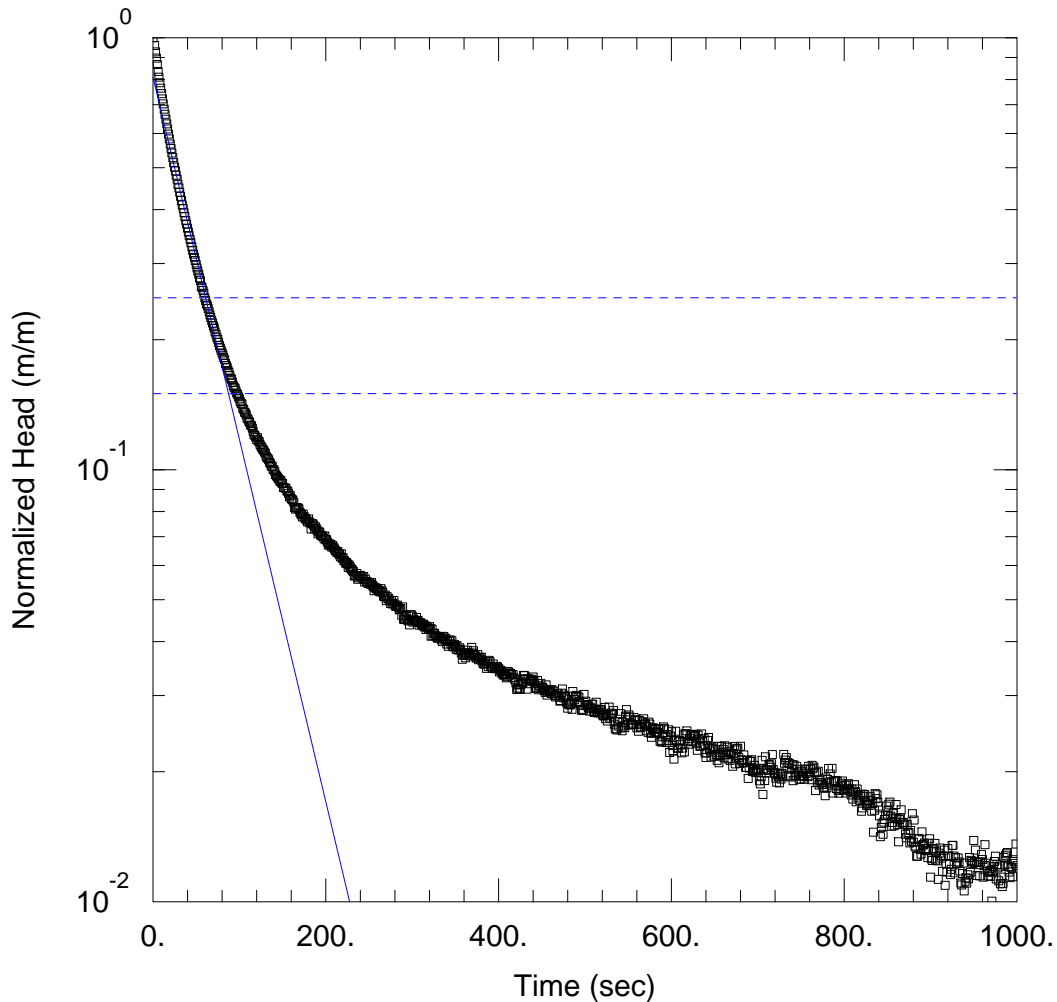
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-52 Test 2.aqt
Date: 10/18/11 Time: 14:09:38

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 1.003E-5 m/sec $y_0 =$ 0.4369 m

AQUIFER DATA

Saturated Thickness: 10.5 Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-52)

Initial Displacement: 0.539 m
Static Water Column Height: 17.67 m
Total Well Penetration Depth: 10.5 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

BH-BGC11-52 Test 3

Prepared By:

BGC Engineering

Prepared For:

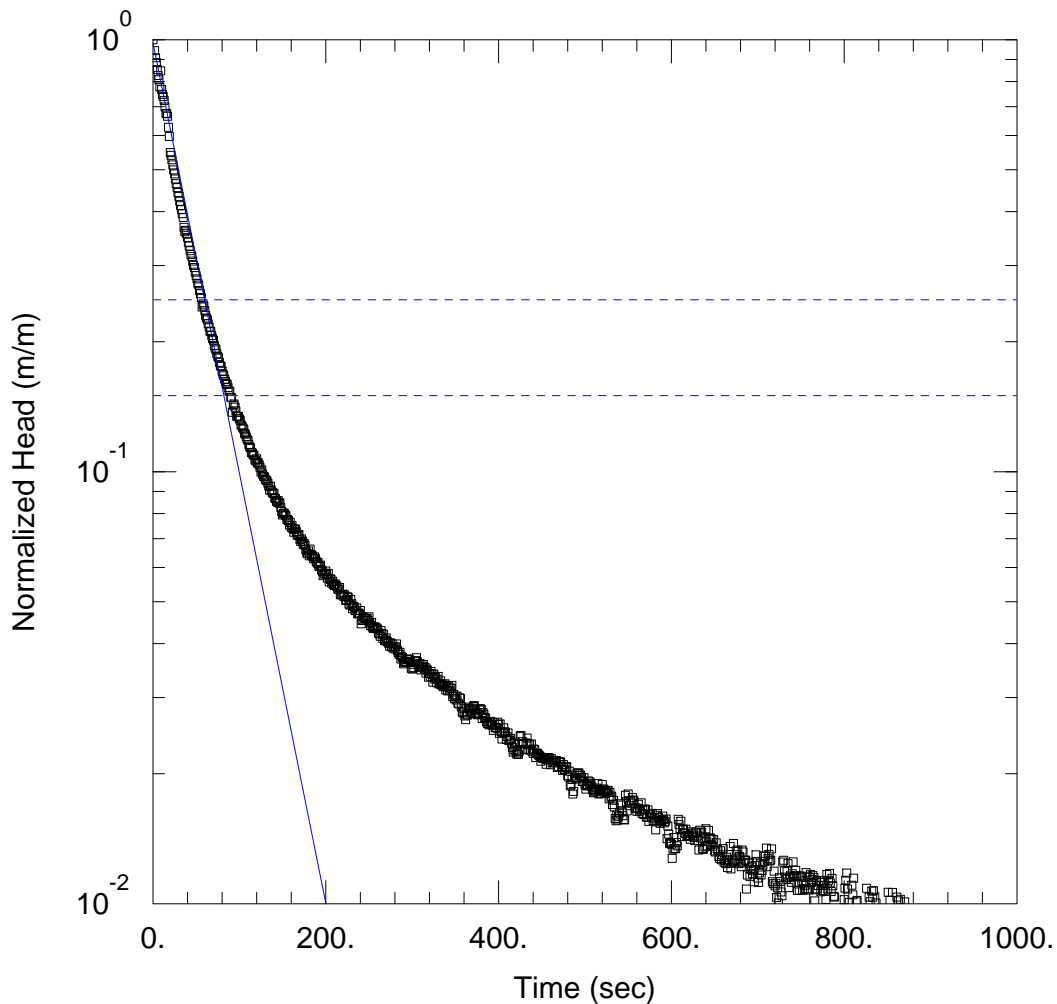
Victoria Gold Corp.

Project:

0792006

Location:

Dublin Gulch, YT



Data Set: N:\...\BH-BGC11-52 Test 3.aqt
Date: 10/18/11 Time: 14:12:10

SOLUTION

Aquifer Model: Confined
Solution Method: Hvorslev
K = 1.19E-5 m/sec $y_0 =$ 0.5338 m

AQUIFER DATA

Saturated Thickness: 10.5 Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (BH-BGC11-52)

Initial Displacement: 0.543 m
Static Water Column Height: 17.66 m
Total Well Penetration Depth: 10.5 m
Screen Length: 3. m
Casing Radius: 0.0254 m
Well Radius: 0.048 m

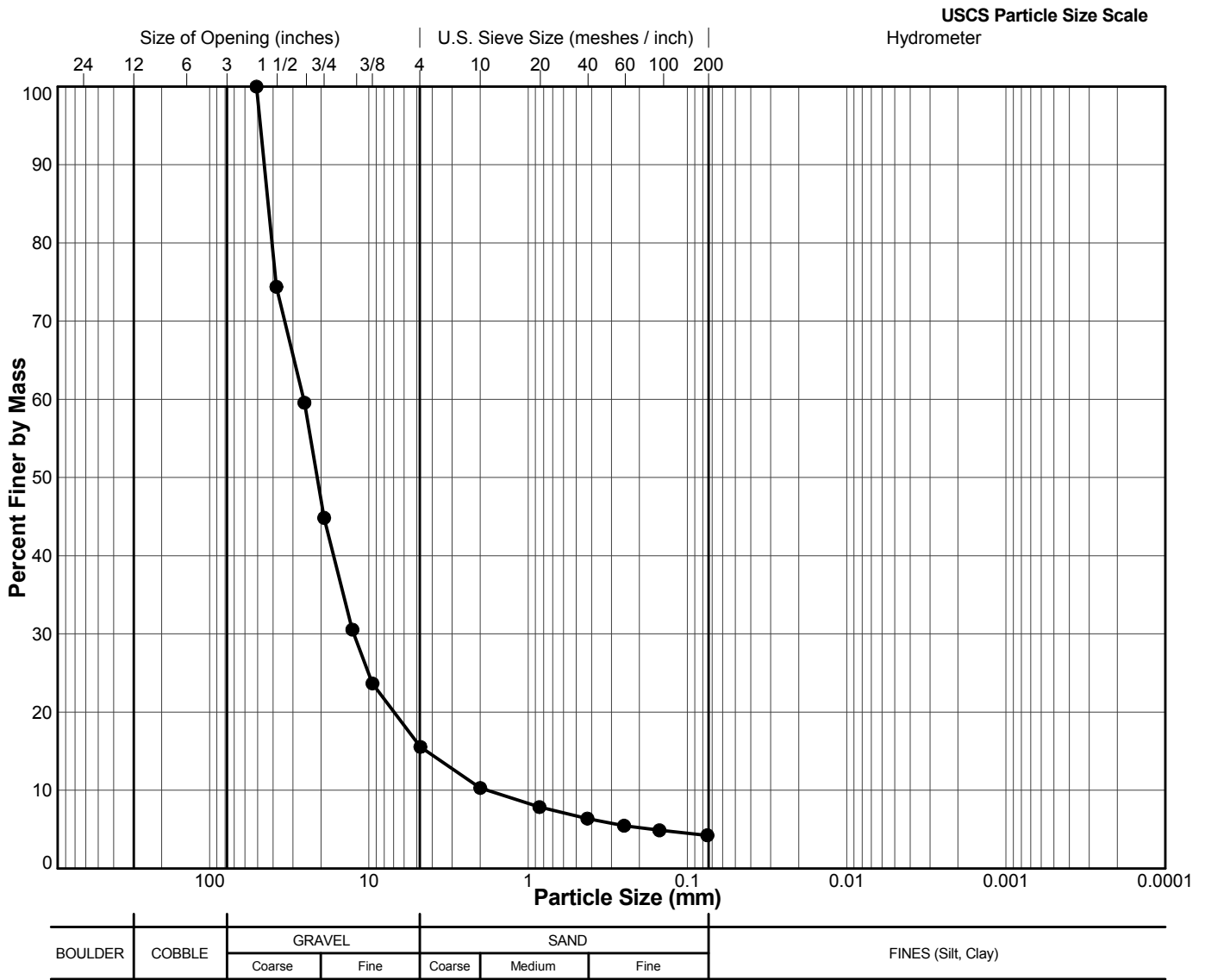
APPENDIX L LABORATORY REPORTS

PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-50
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.80 to 3.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



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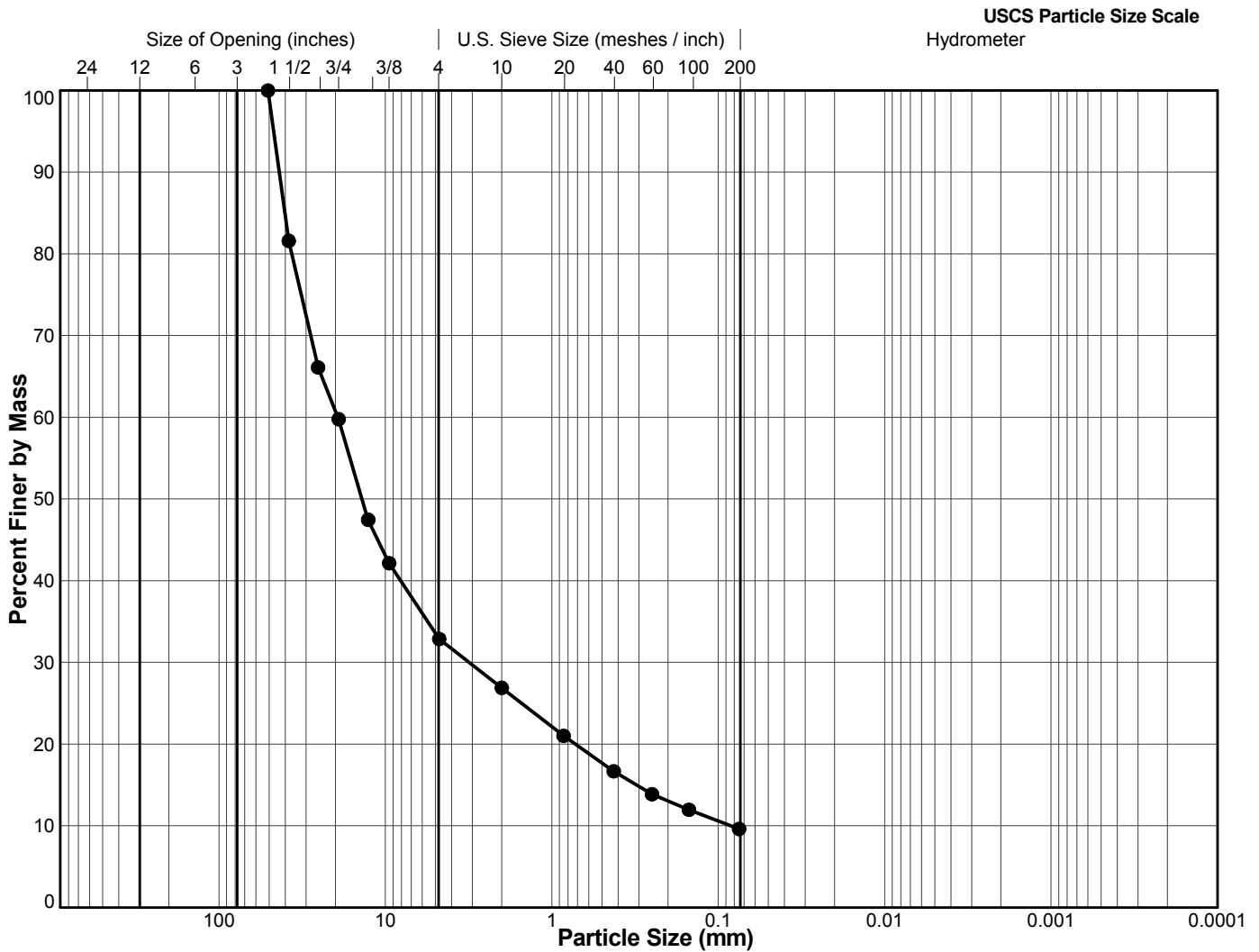
RS	14/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-50
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.30 to 3.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

RS	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

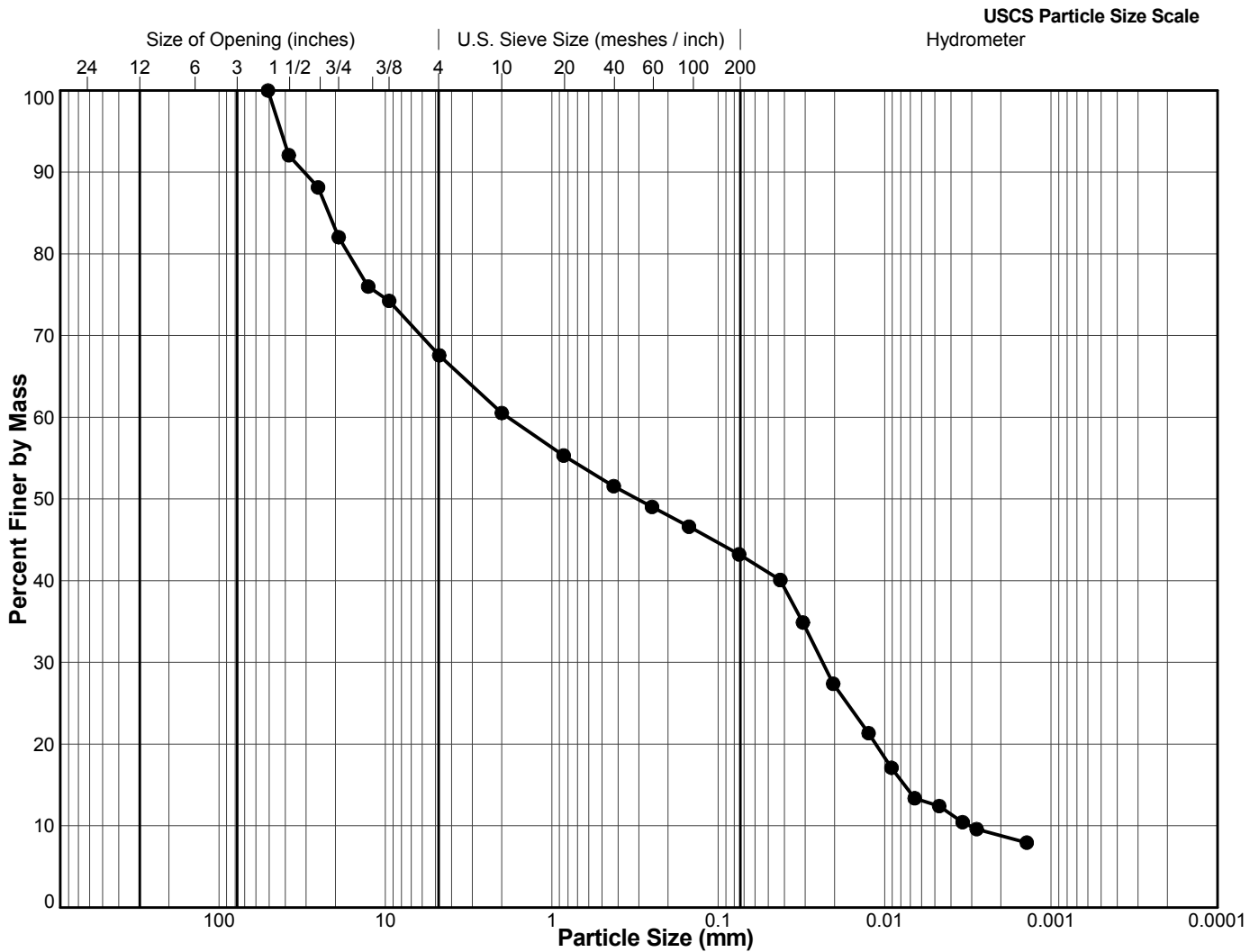
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-51
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.20 to 1.44
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

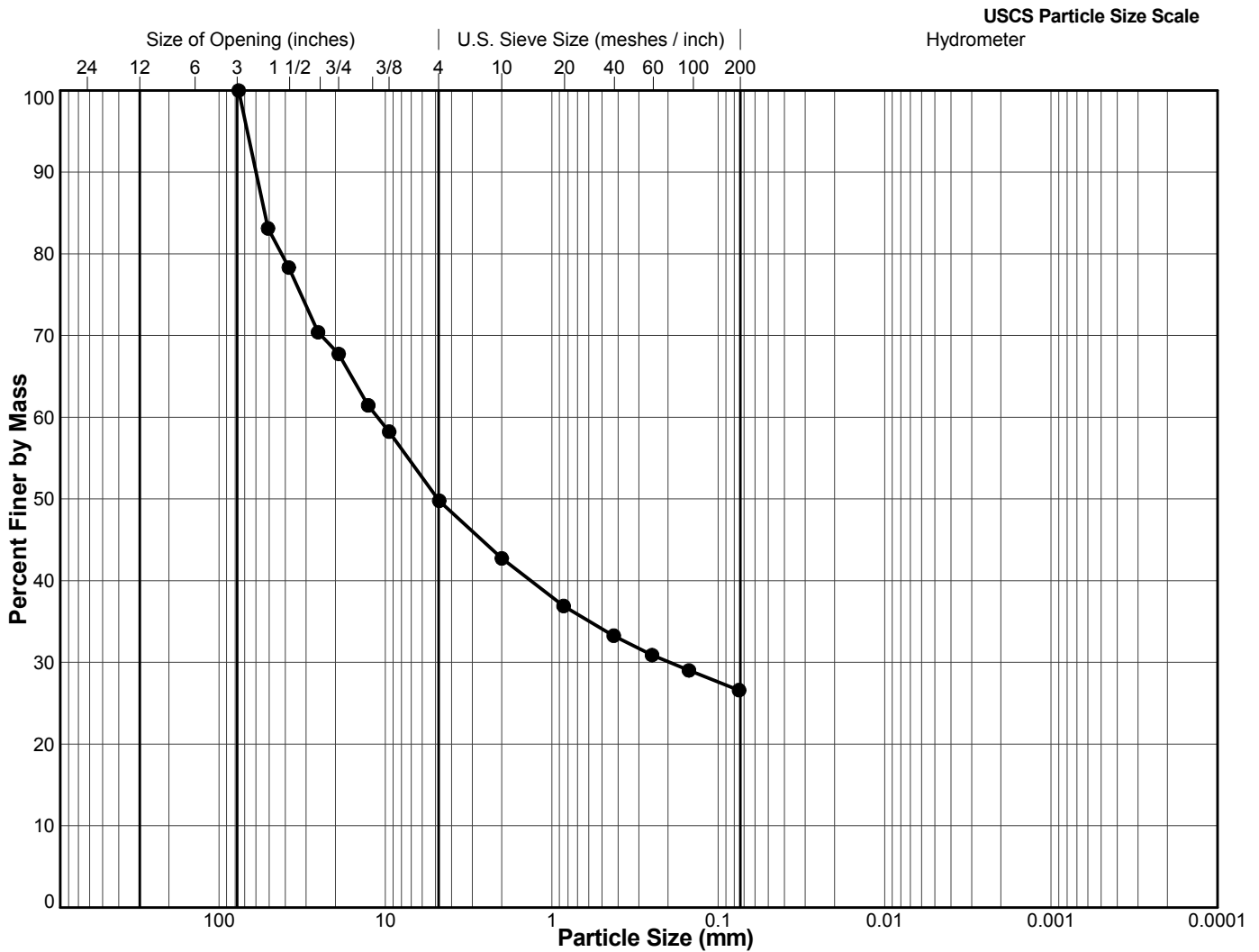
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-51
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.40 to 2.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 76.2	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	27/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

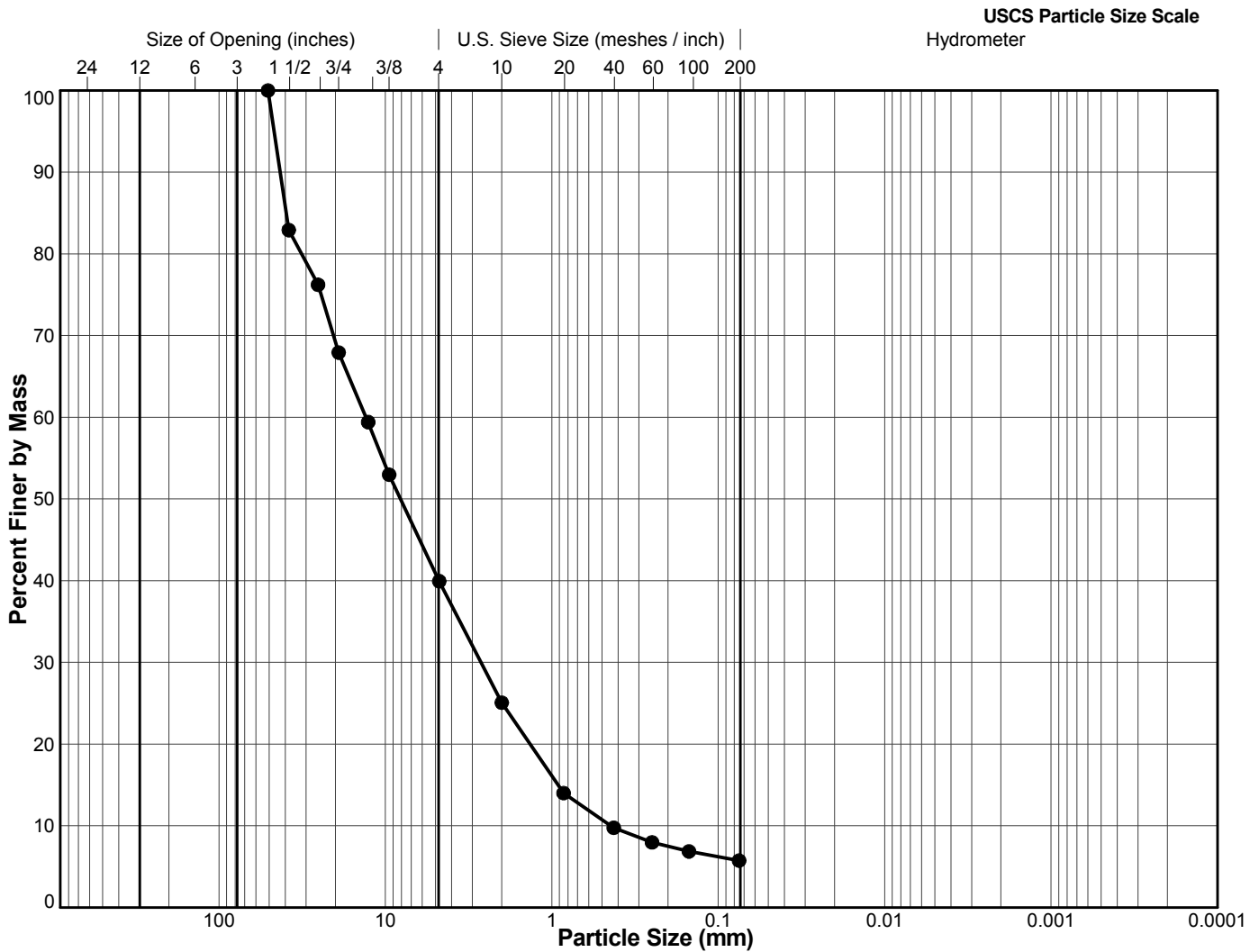
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-51
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 3
Location: Dublin Gulch, Yukon	Depth Interval (m): 4.20 to 4.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

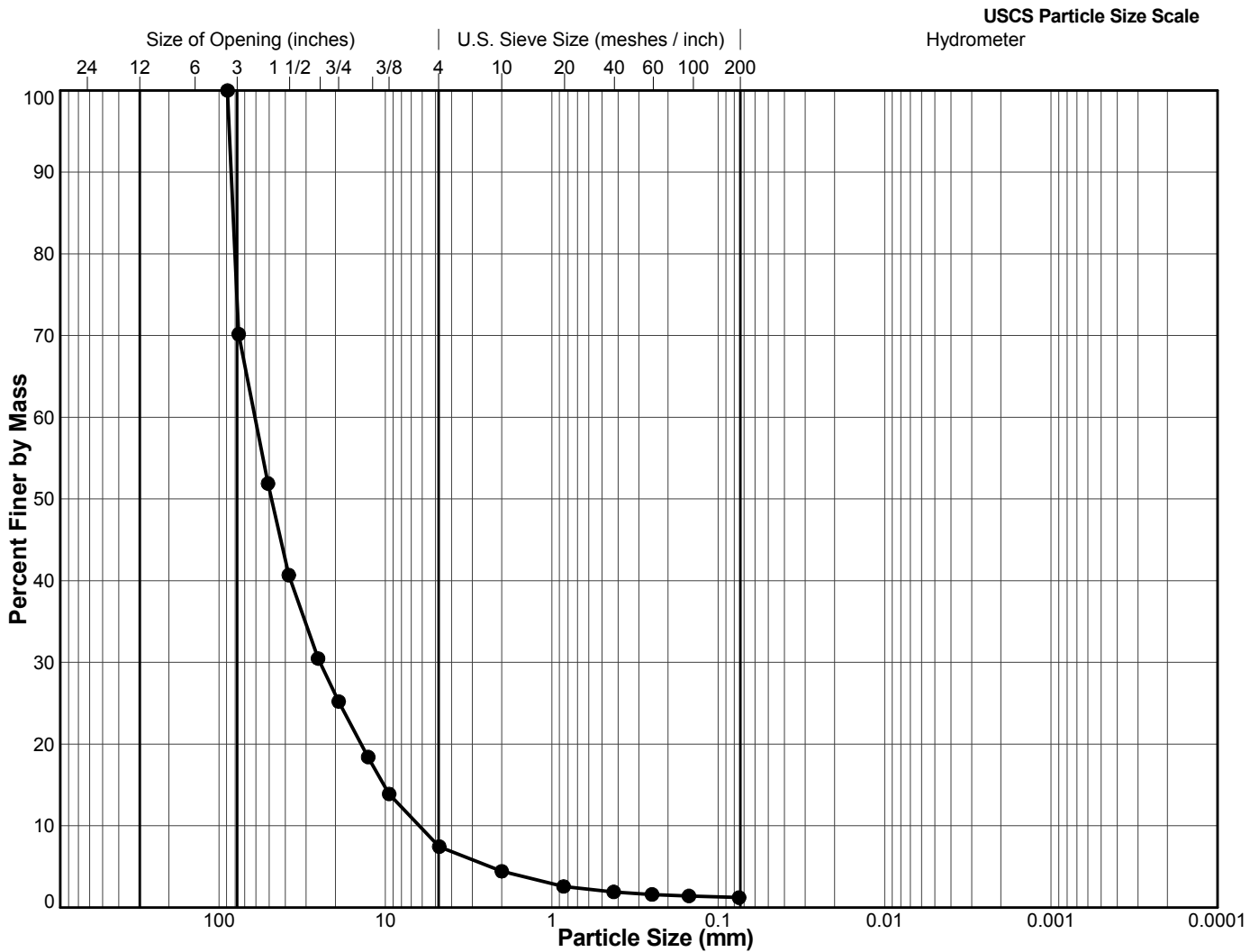
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-52
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.30 to 3.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 88.9	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

SK	06/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

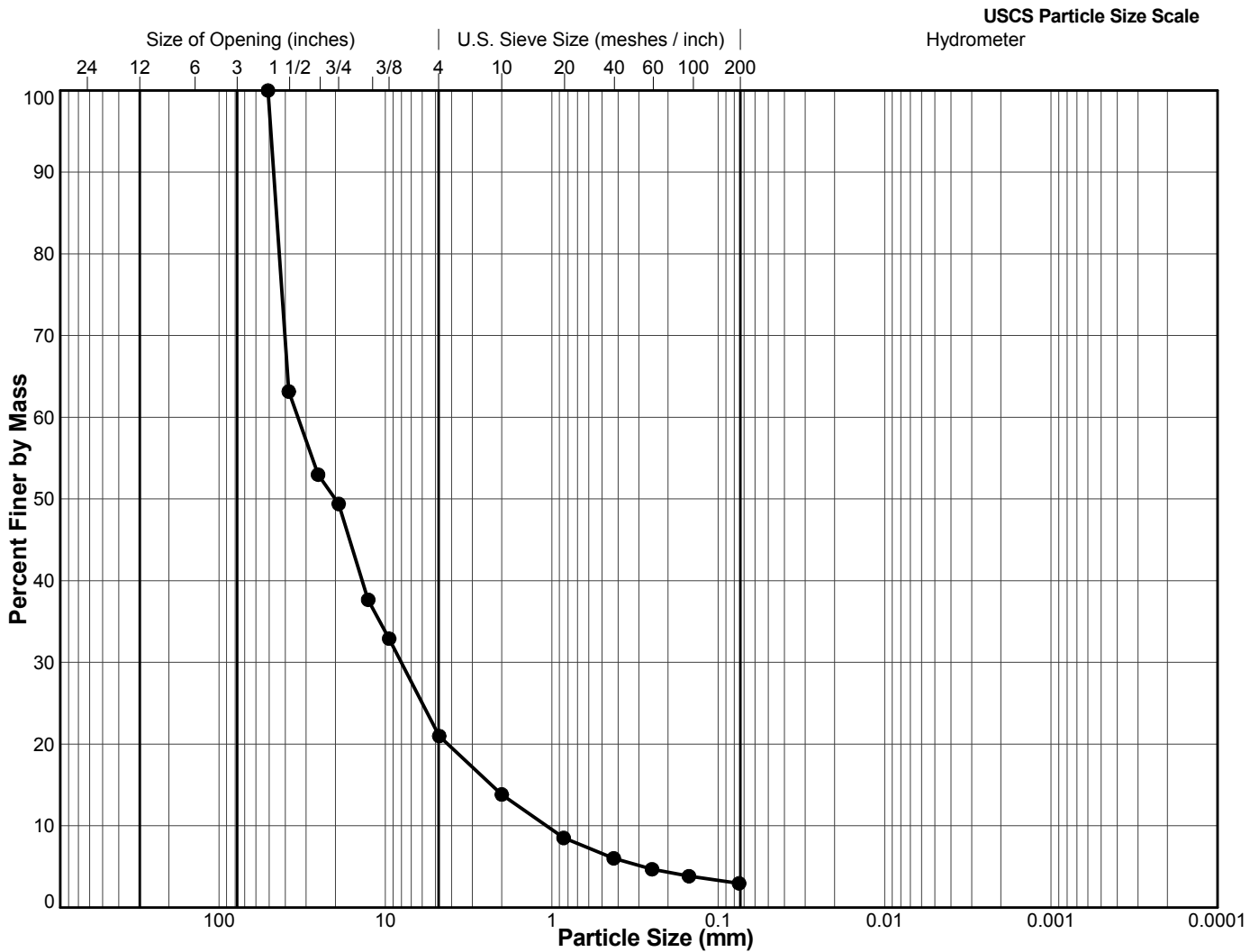
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-53
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.00 to 2.20
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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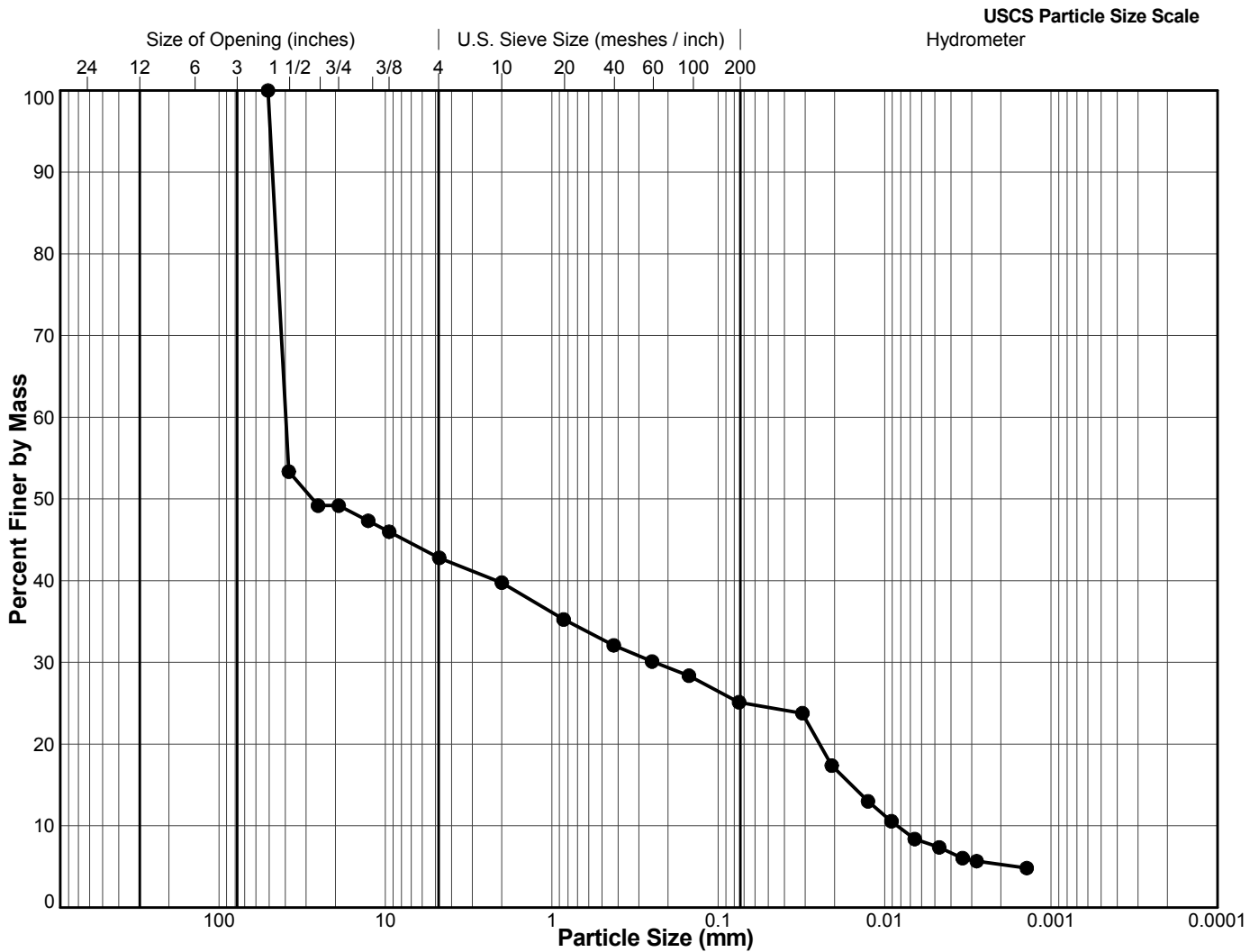
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-54
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

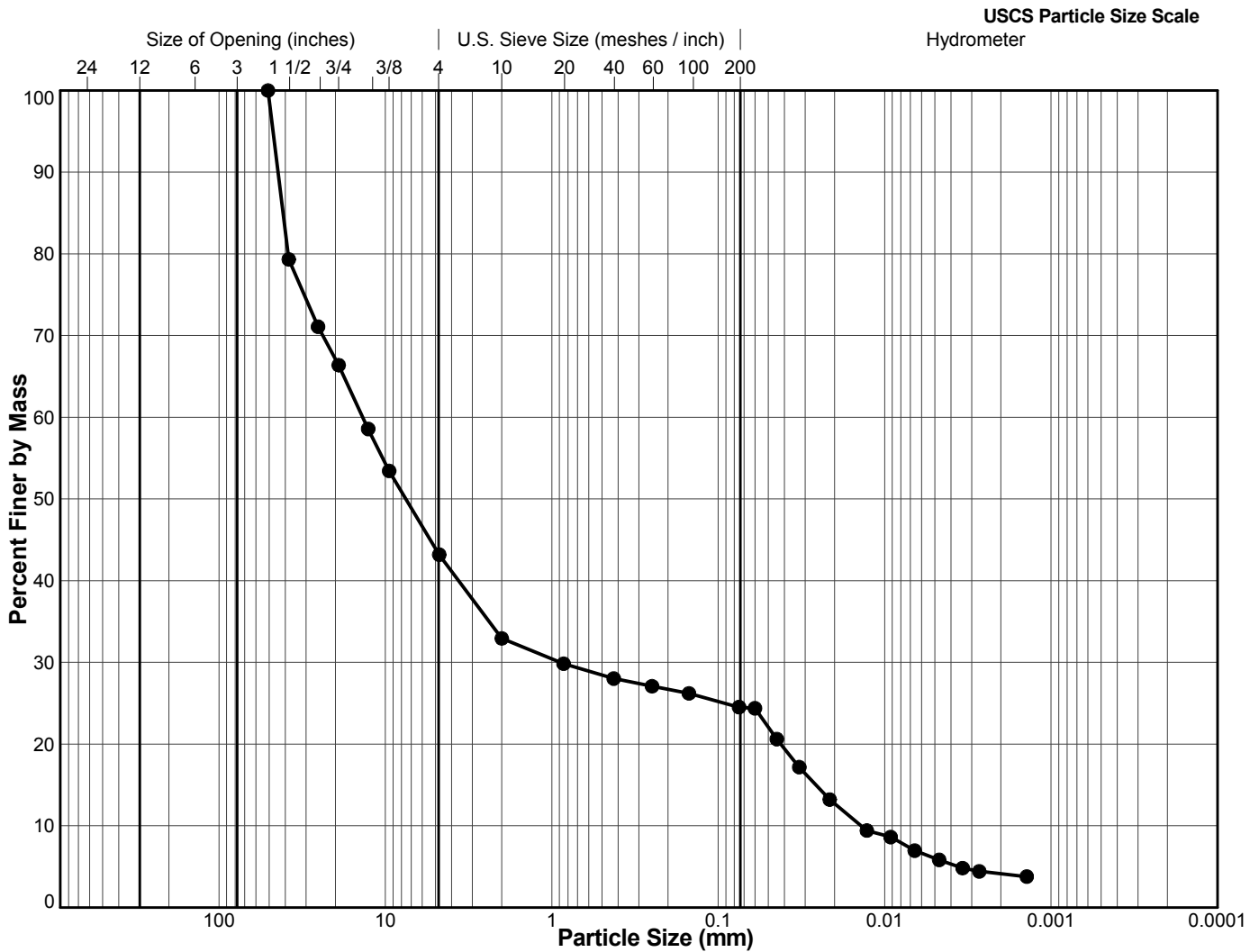
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-55
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

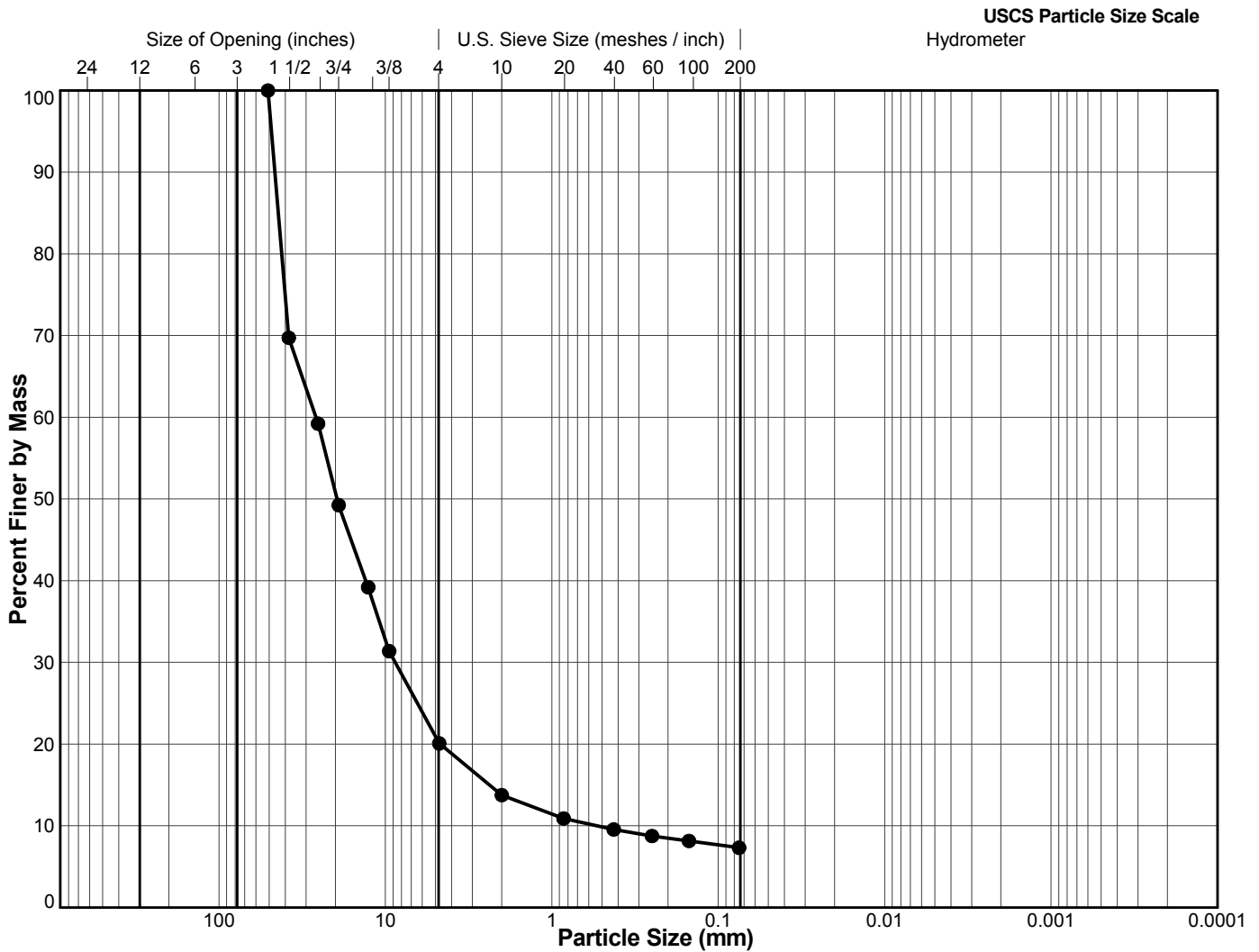
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-55
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.90 to 2.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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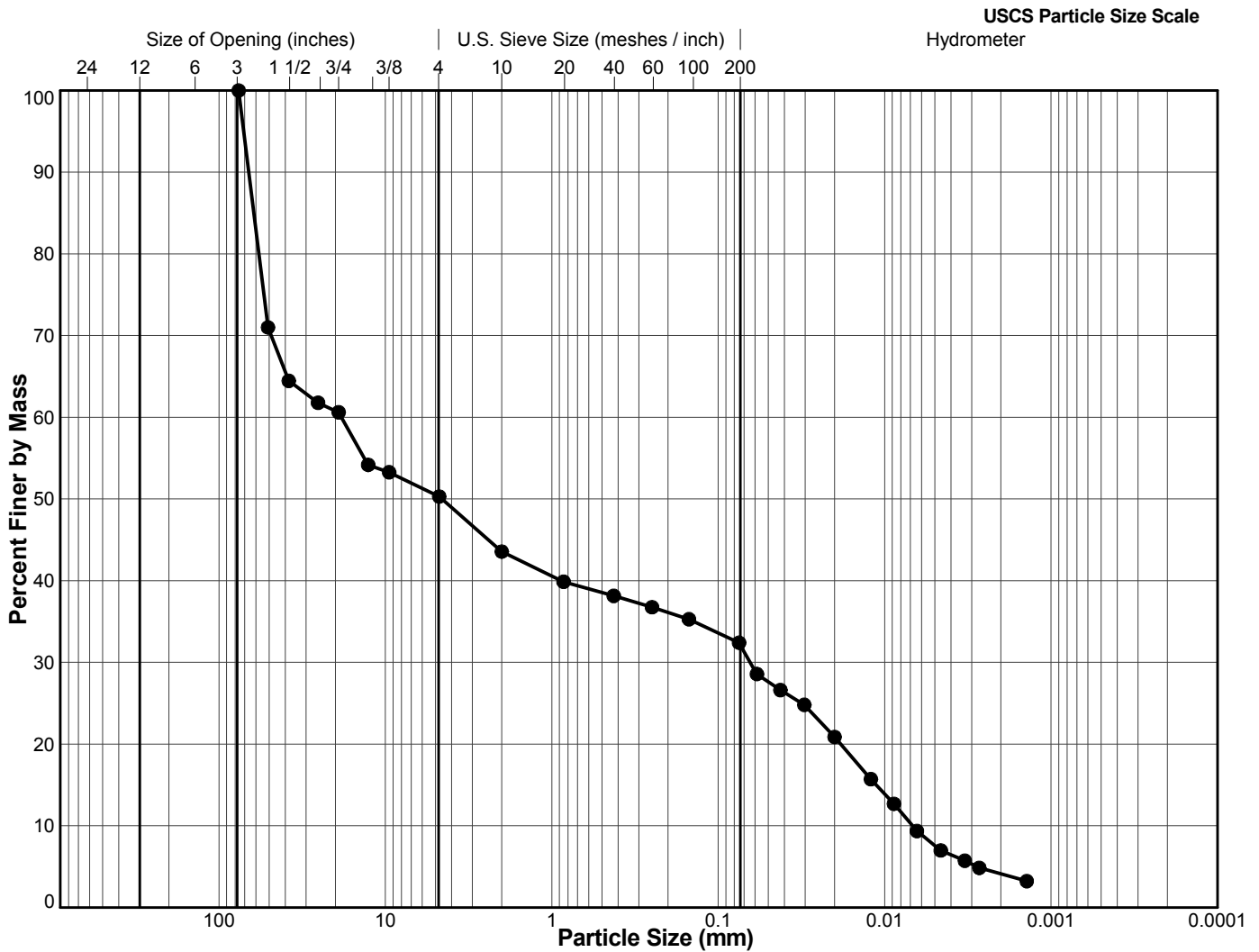
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-57
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.90 to 1.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 76.2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

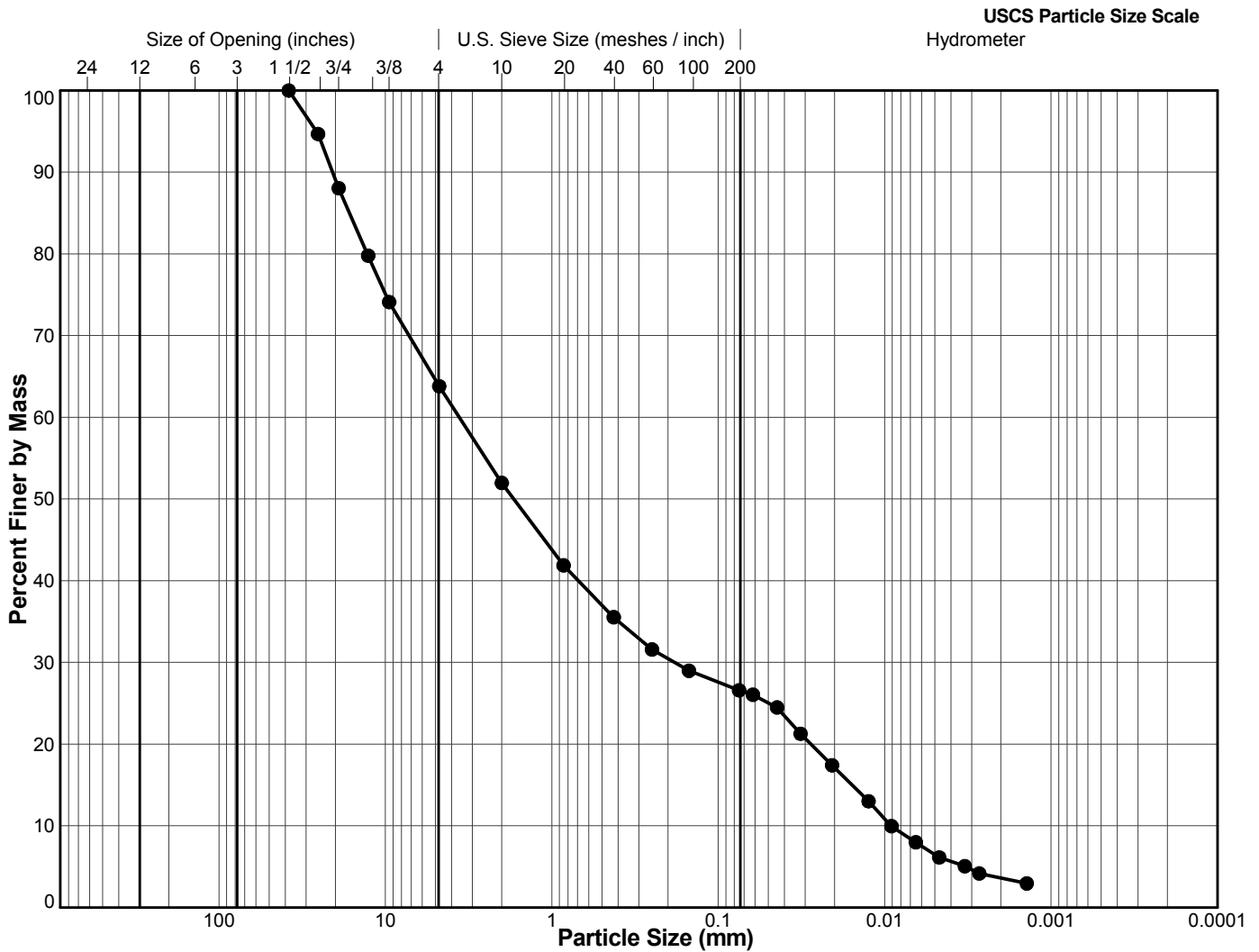
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-58
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

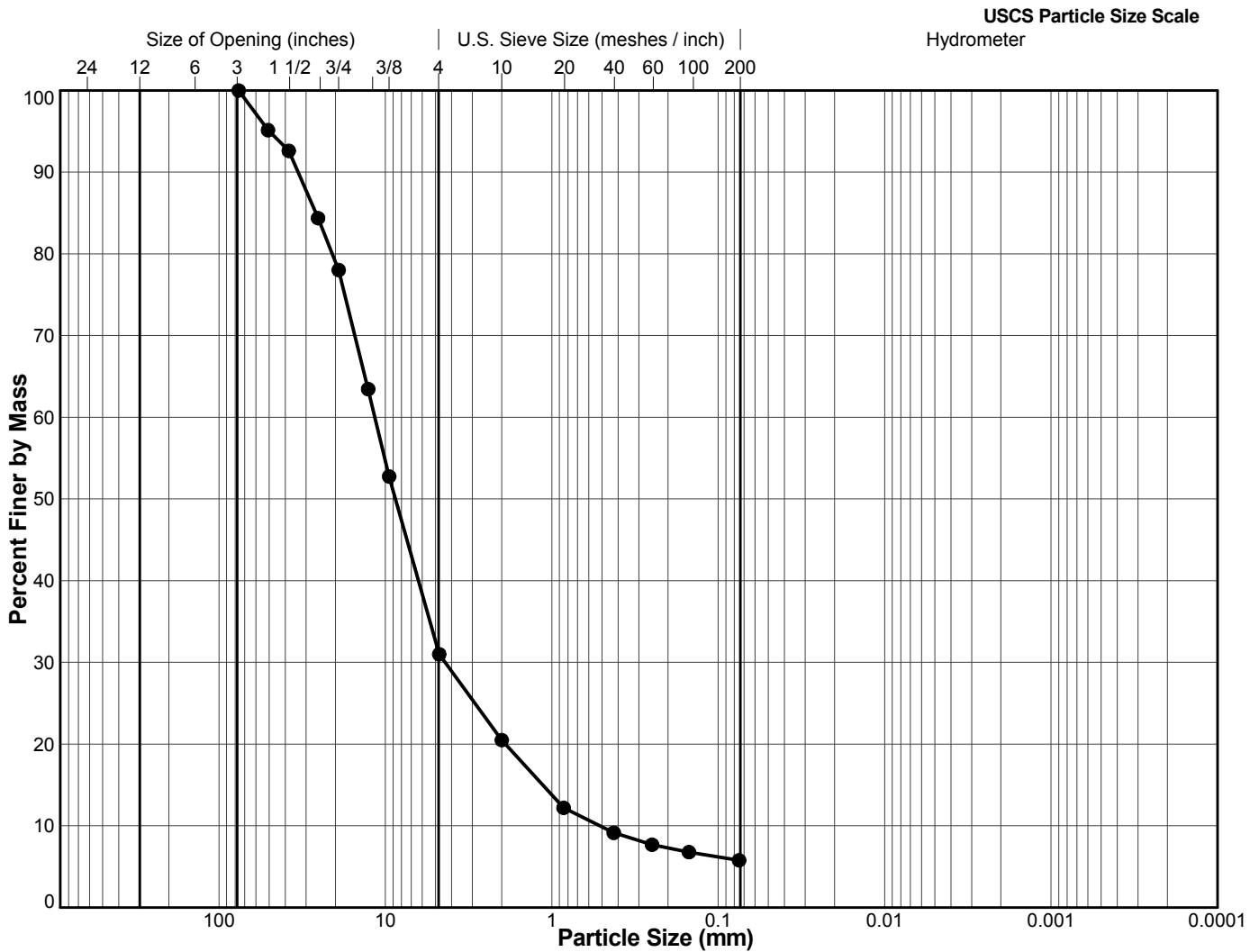
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-58
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.90 to 1.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 76.2	
Method: Split, Washed	



	BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)			
			Coarse	Fine	Coarse	Medium	Fine				

SK	06/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

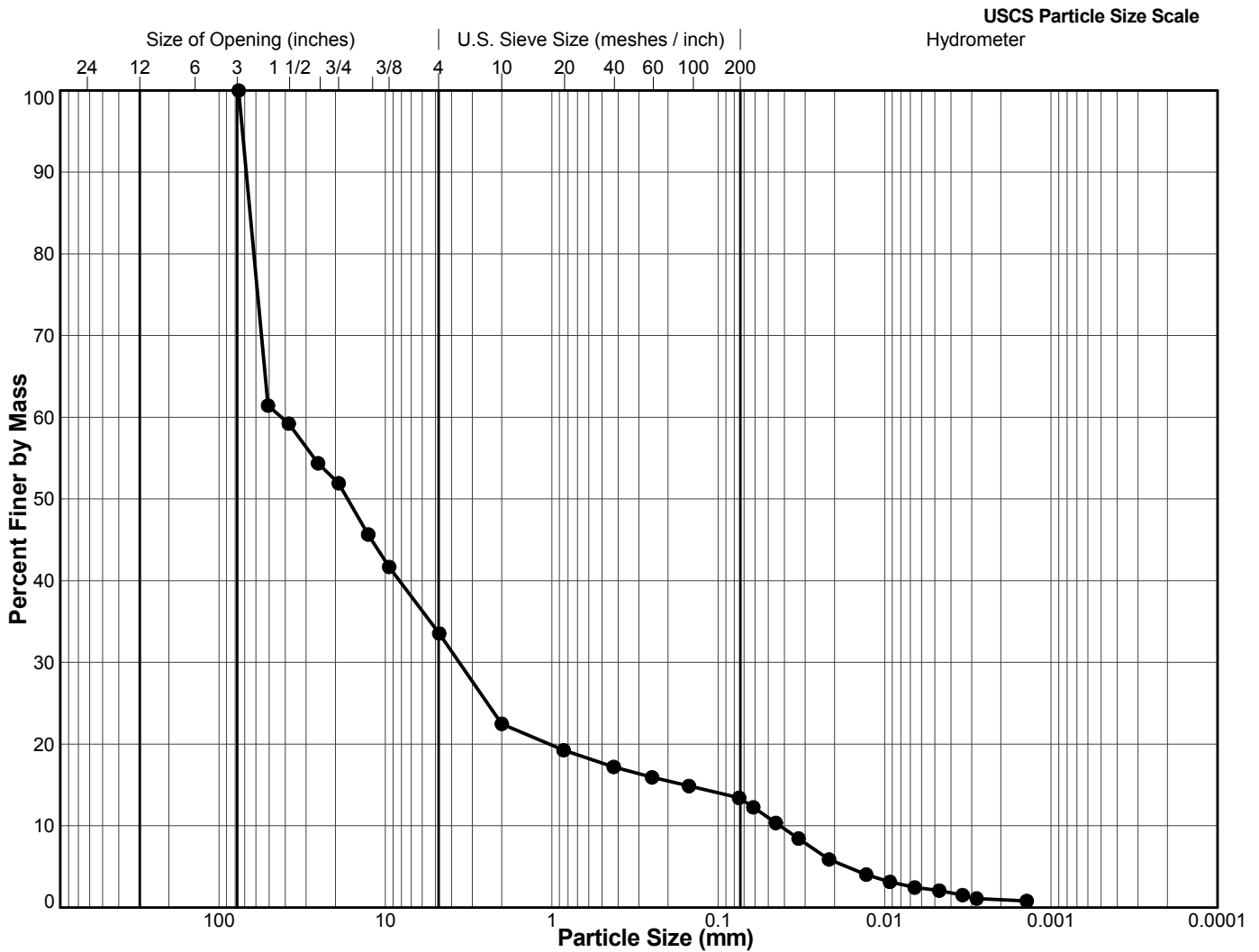
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-59
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 76.2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)		
		Coarse	Fine	Coarse	Medium	Fine			

SK	07/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029\EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

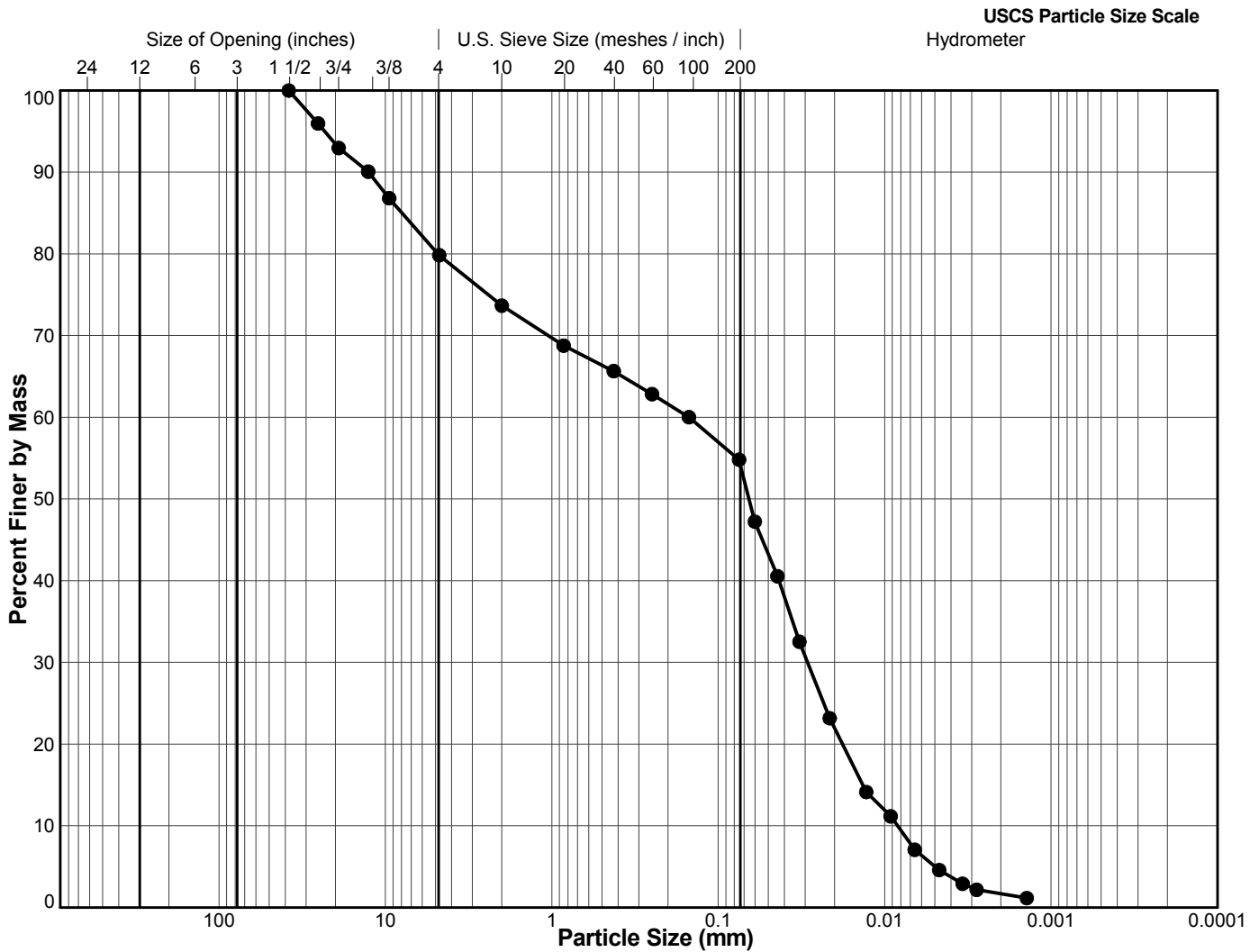
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-60
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	03/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029\EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

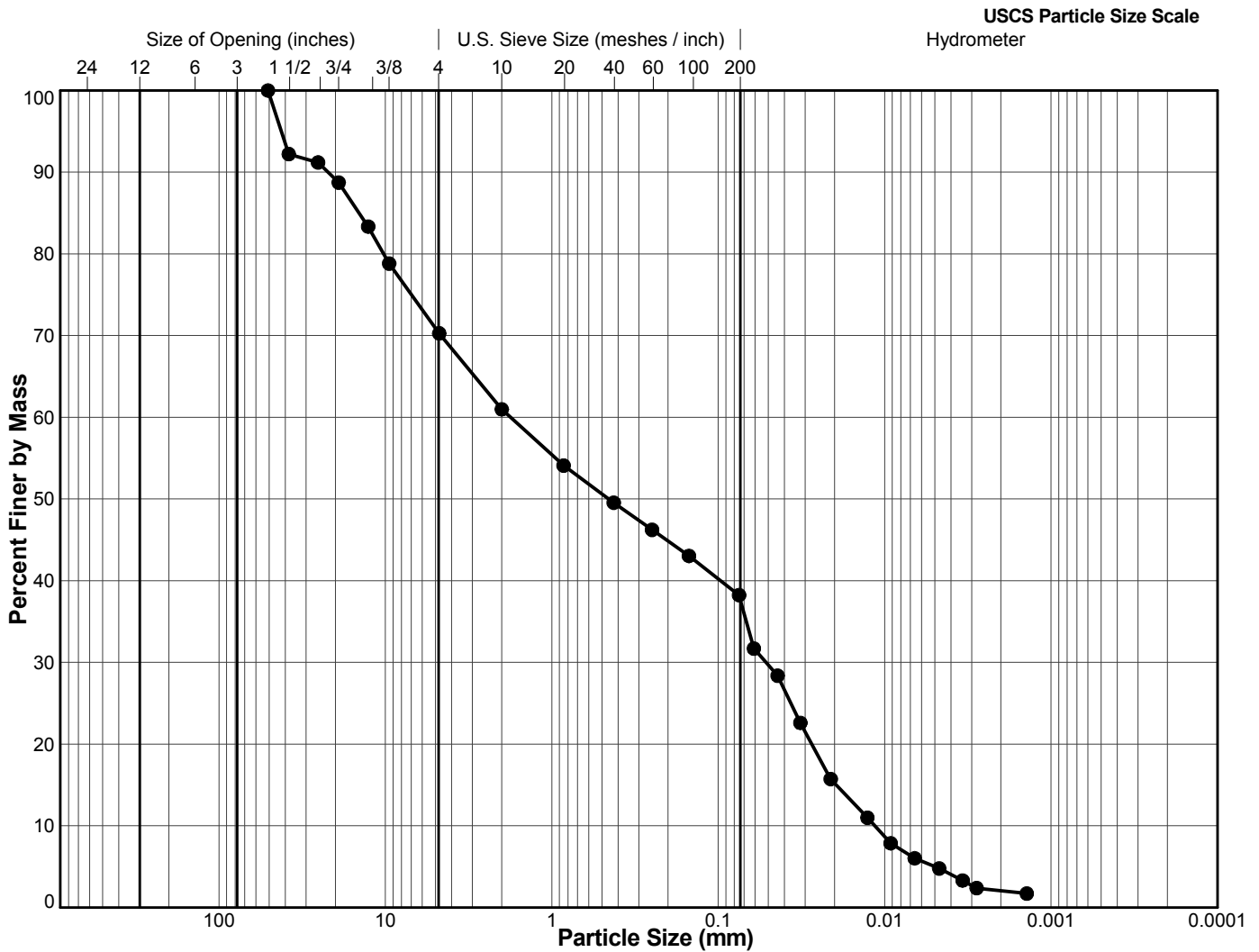
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-61
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

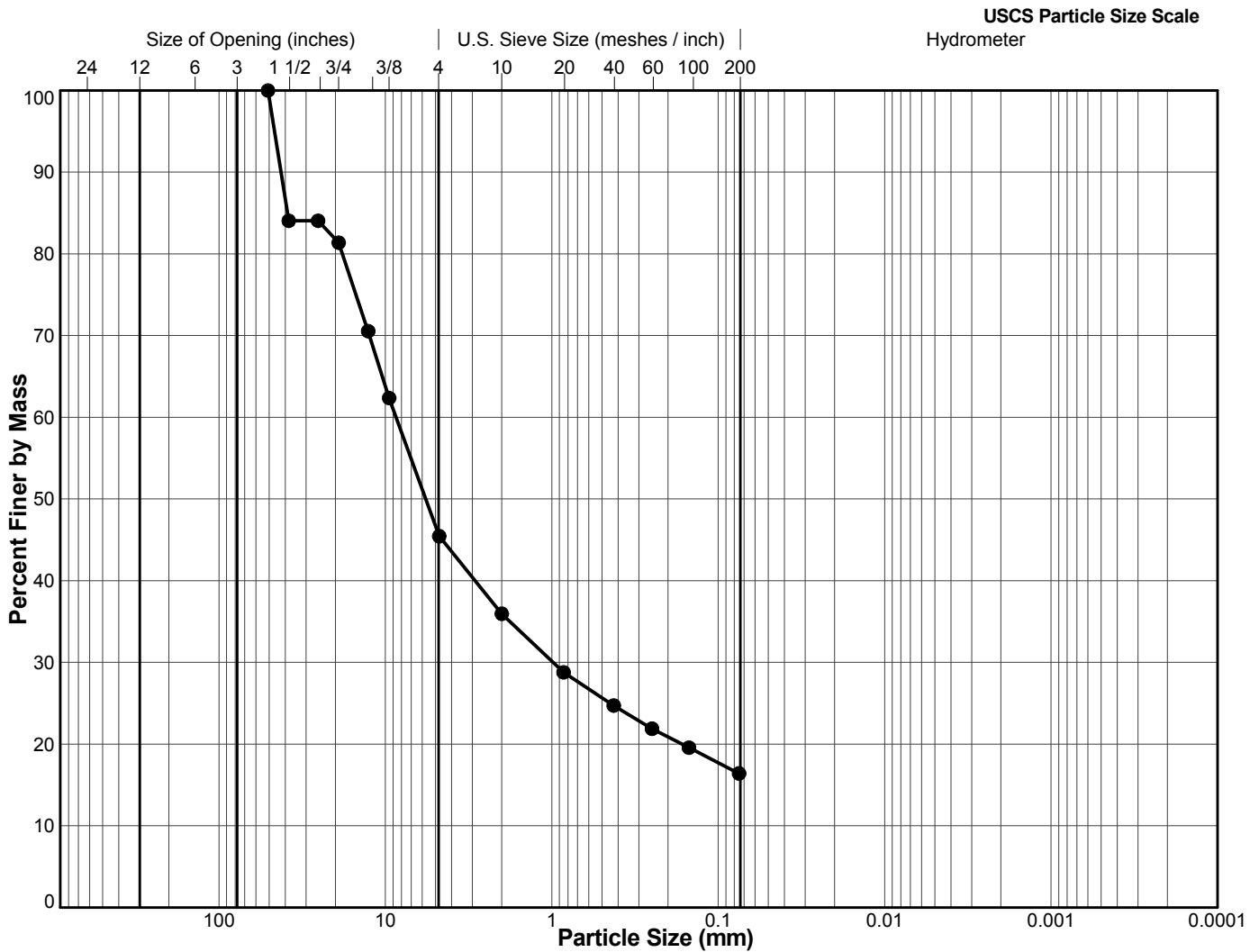
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-60
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.70 to 1.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

SK	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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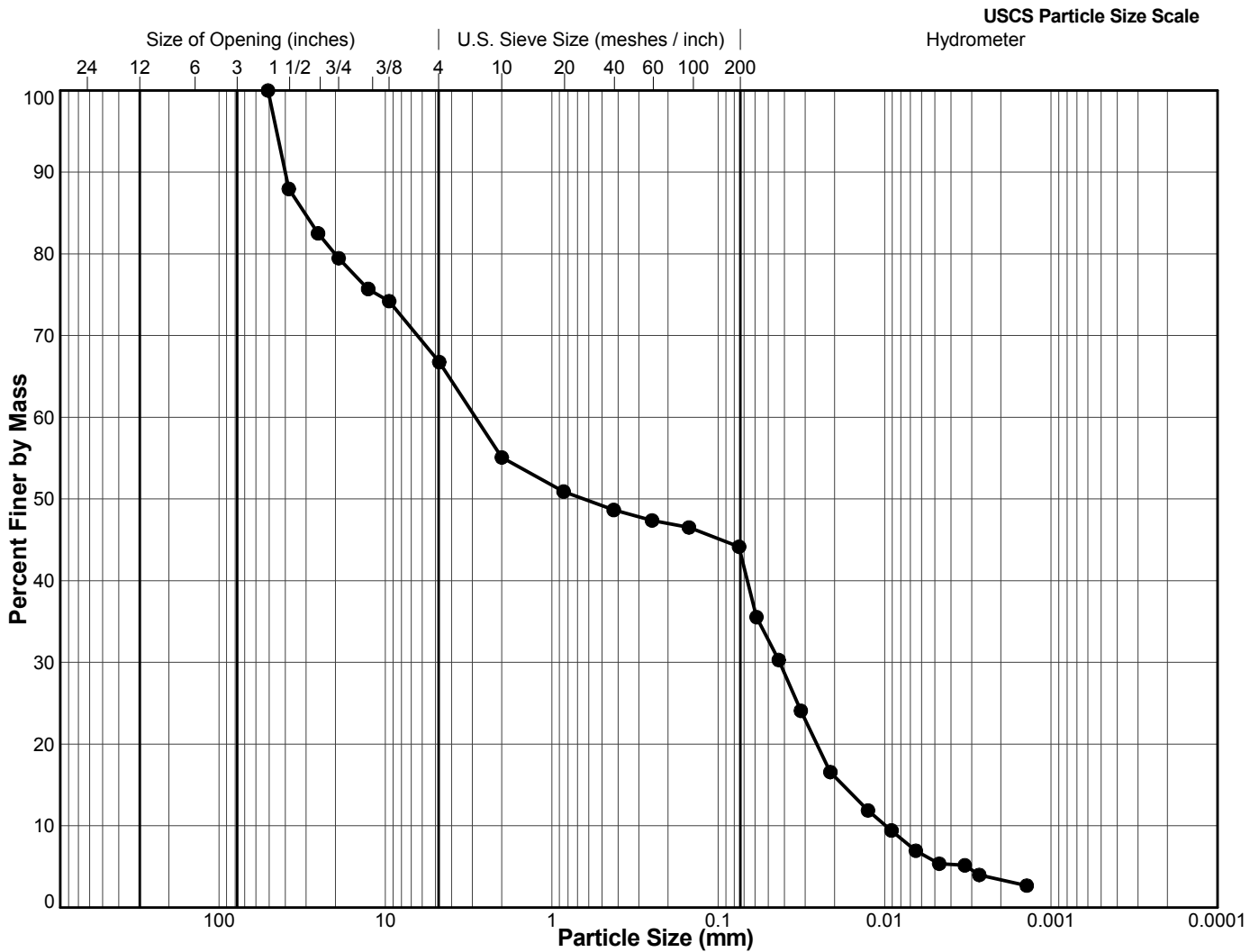
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-62
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.90 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)				
		Coarse	Fine	Coarse	Medium	Fine					

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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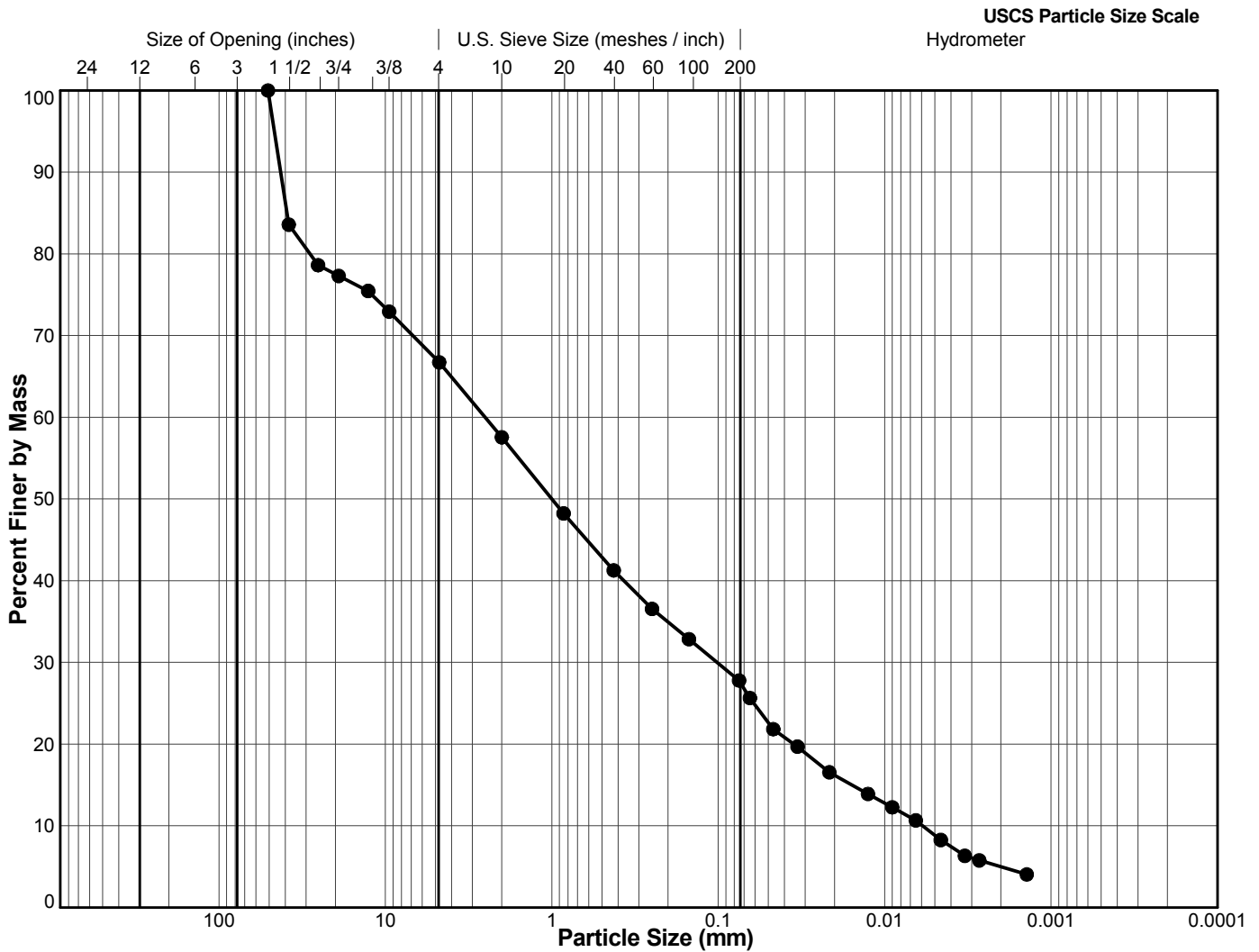
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-62
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.50 to 3.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	03/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

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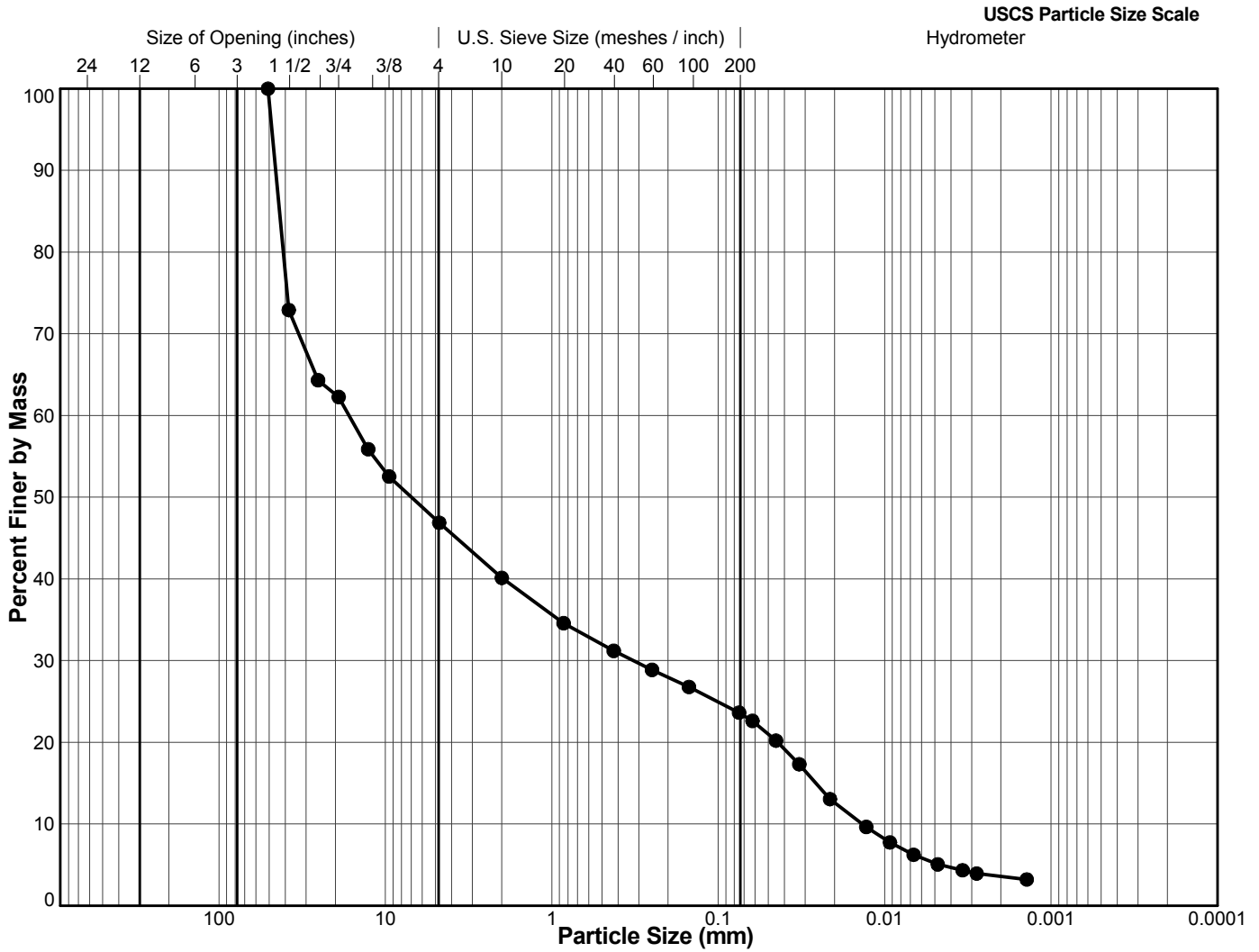
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-63
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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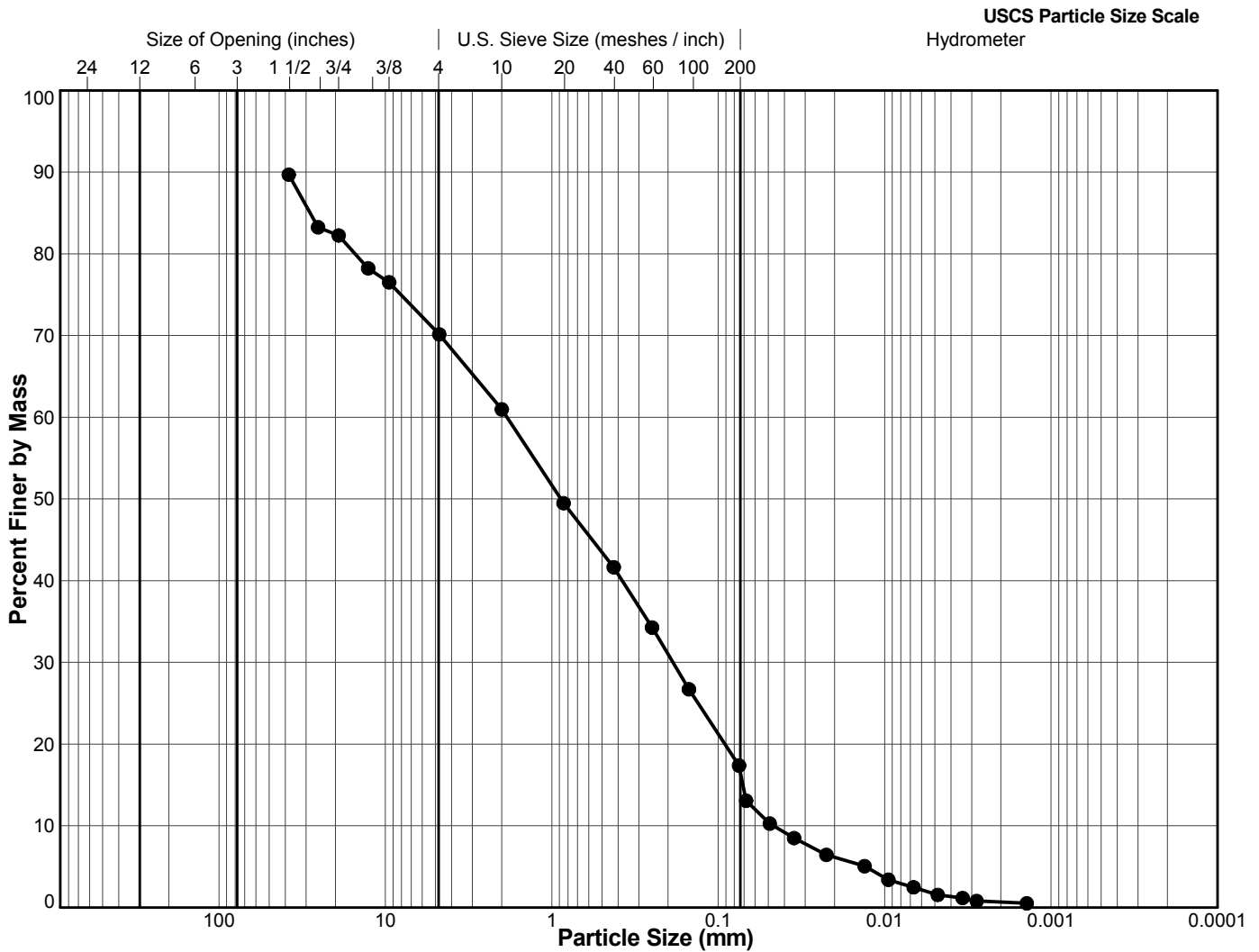
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-64
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm):	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)					
		Coarse	Fine	Coarse	Medium	Fine						

SK	01/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

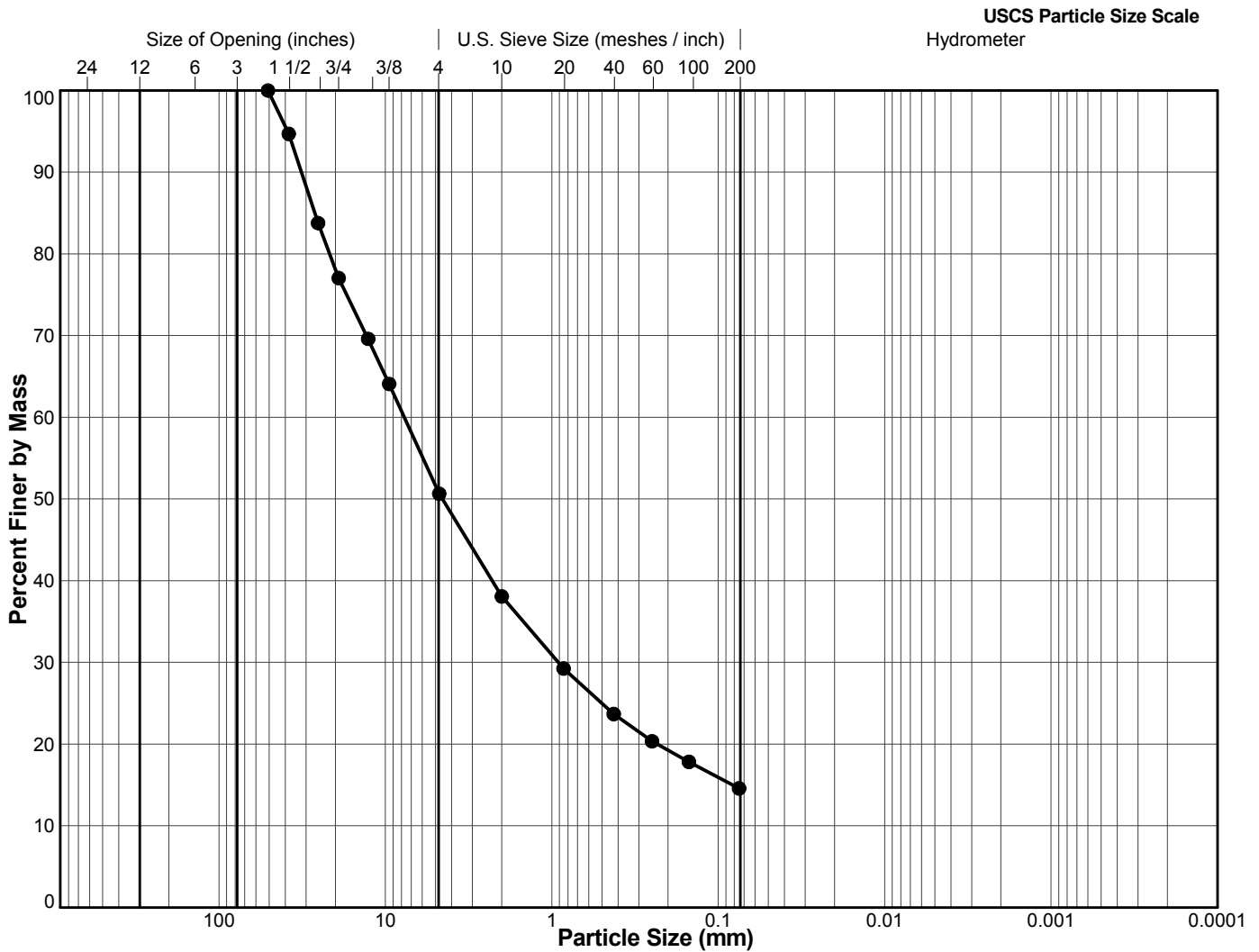
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-65
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	07/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

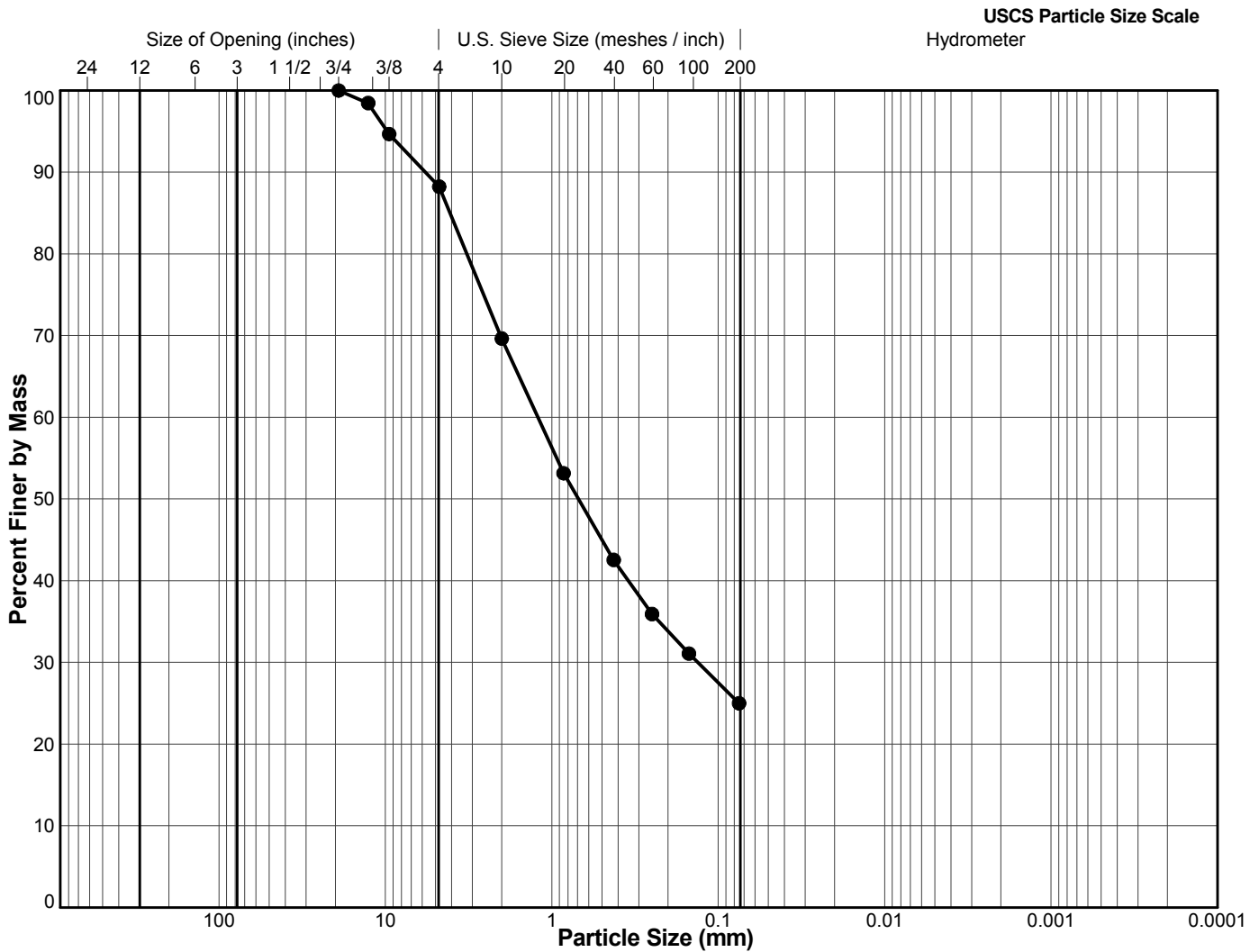
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-66
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	14/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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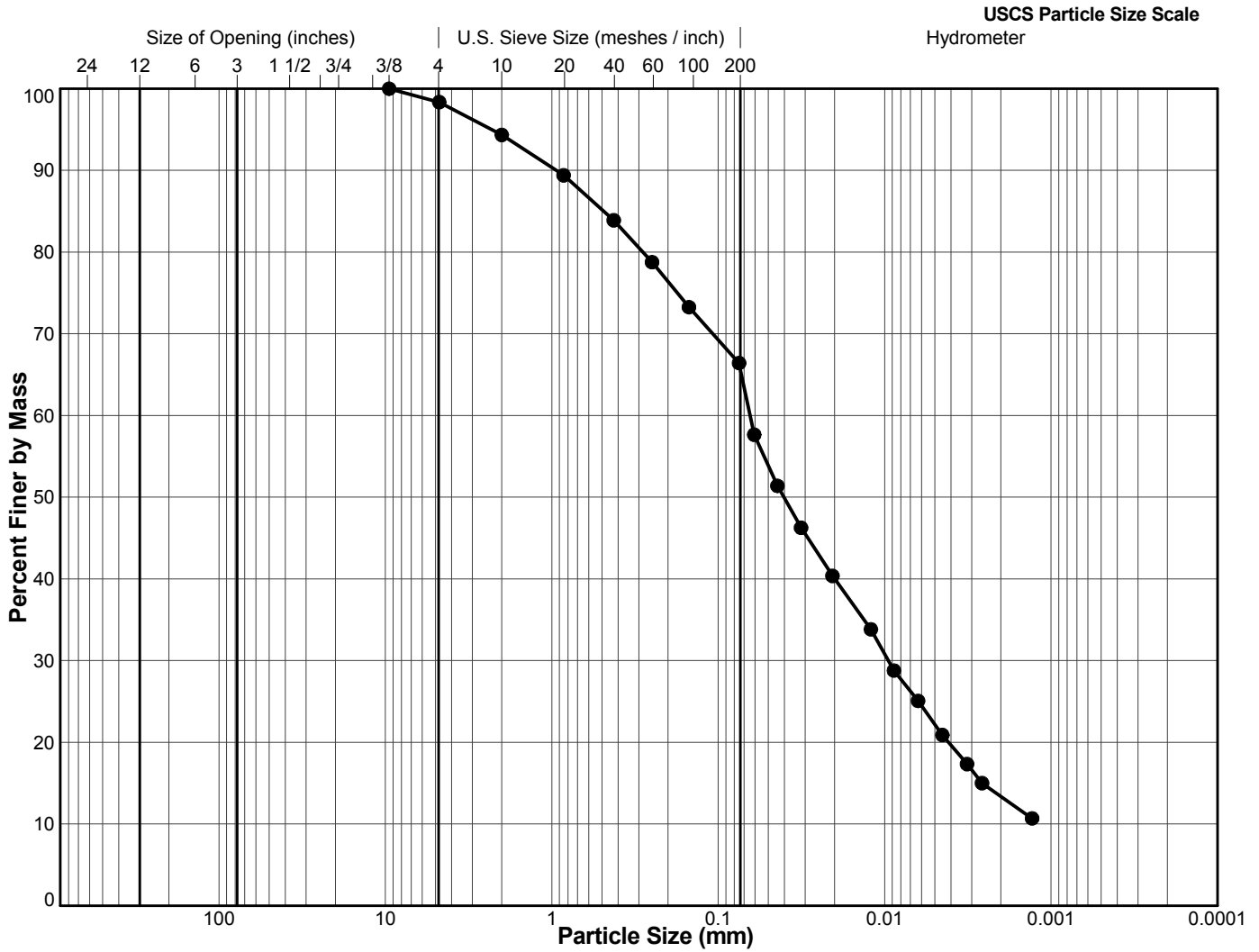
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-66
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.80 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 9.5	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



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RS	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

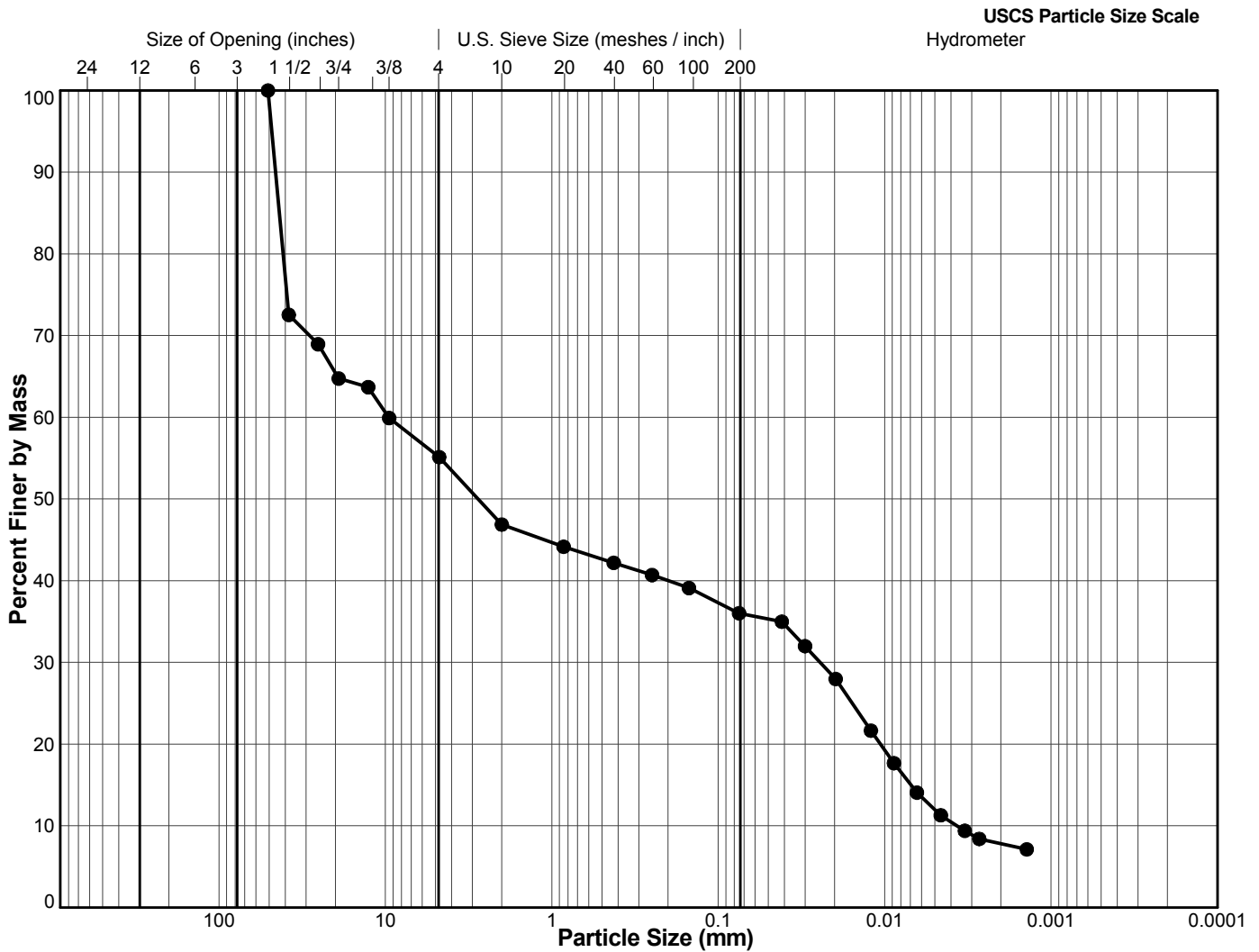
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-67
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

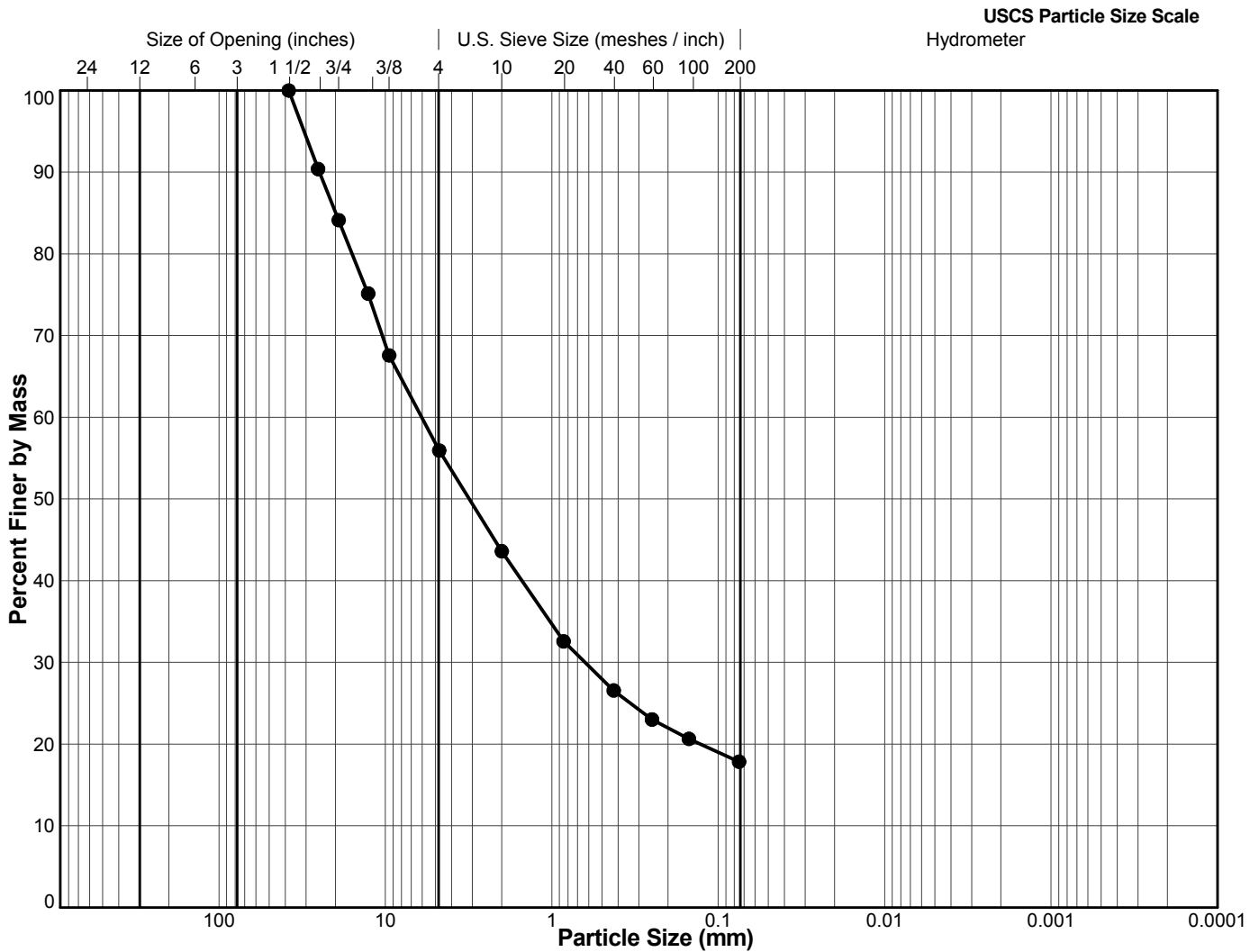
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-68
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.60 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	
Method: Split, Washed	



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SK	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

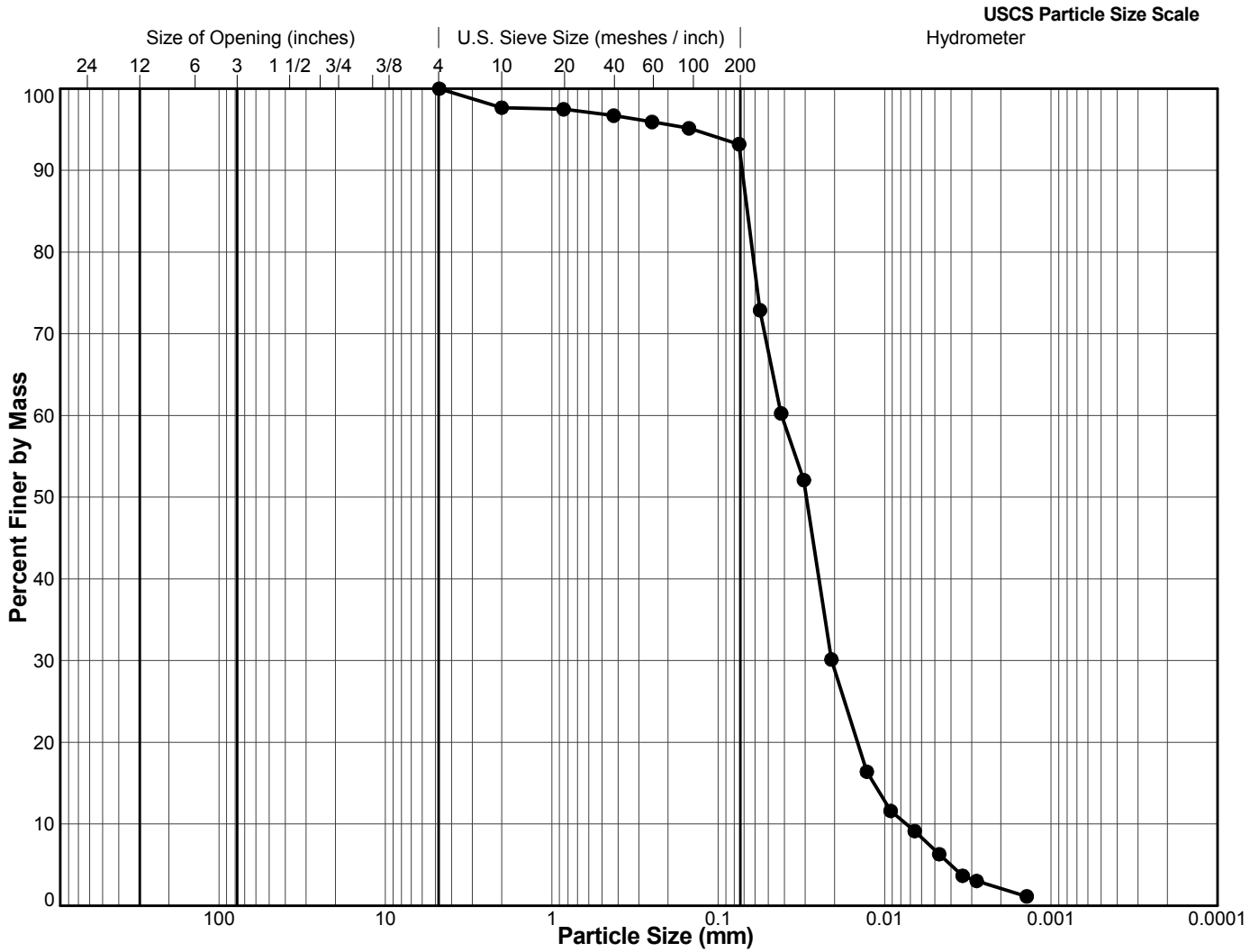
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-69
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 4.75	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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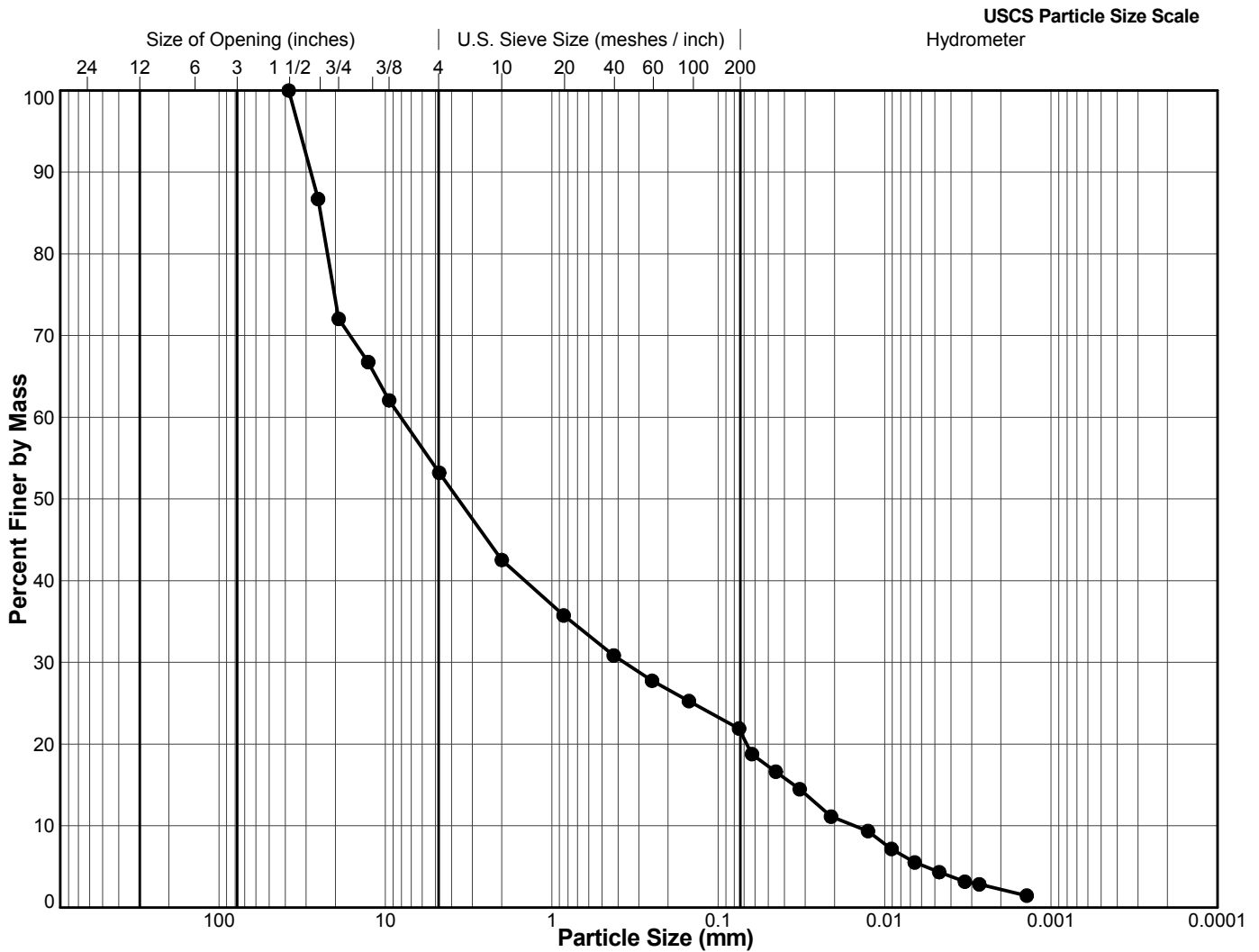
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-69
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.80 to 2.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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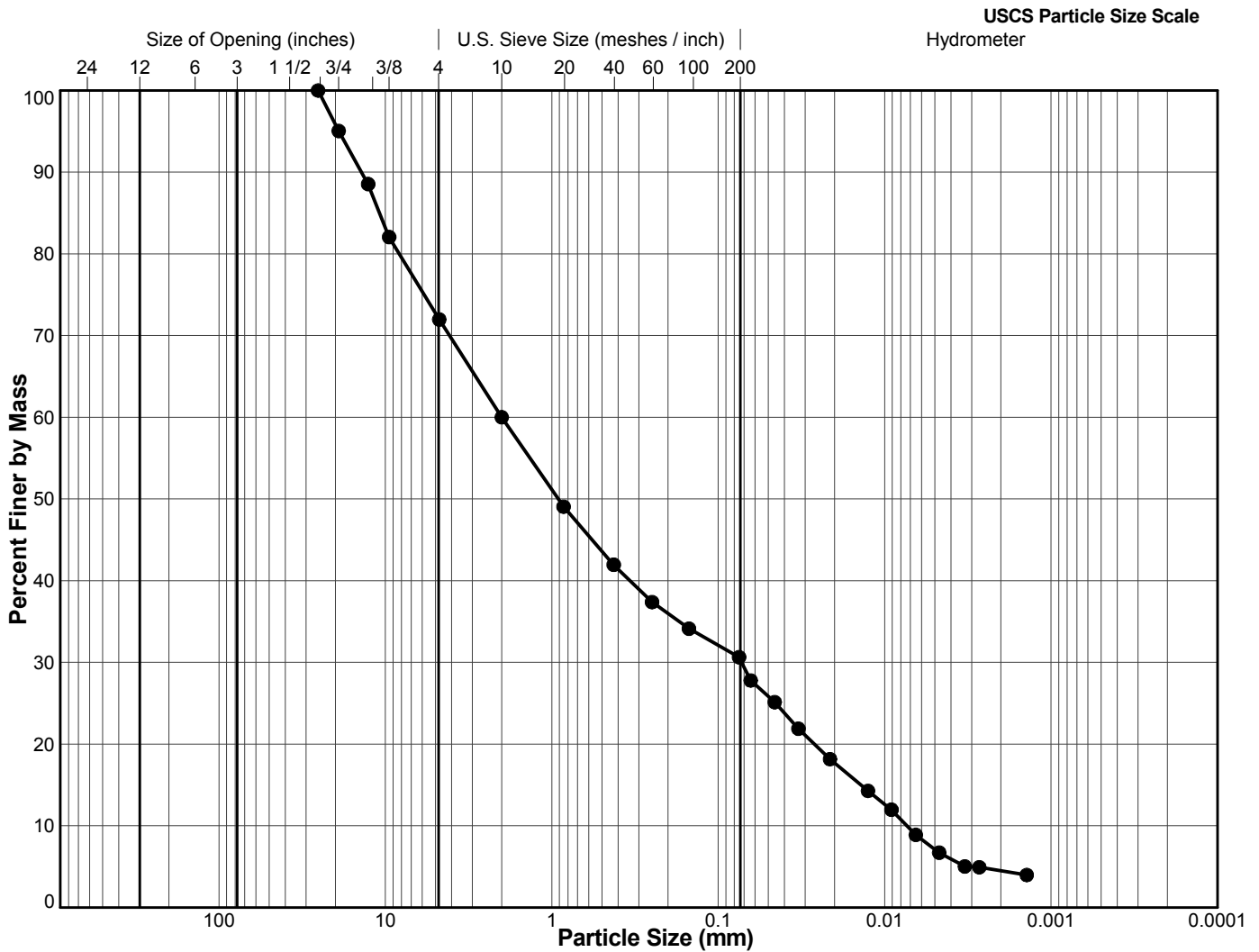
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-71
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 25.4	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

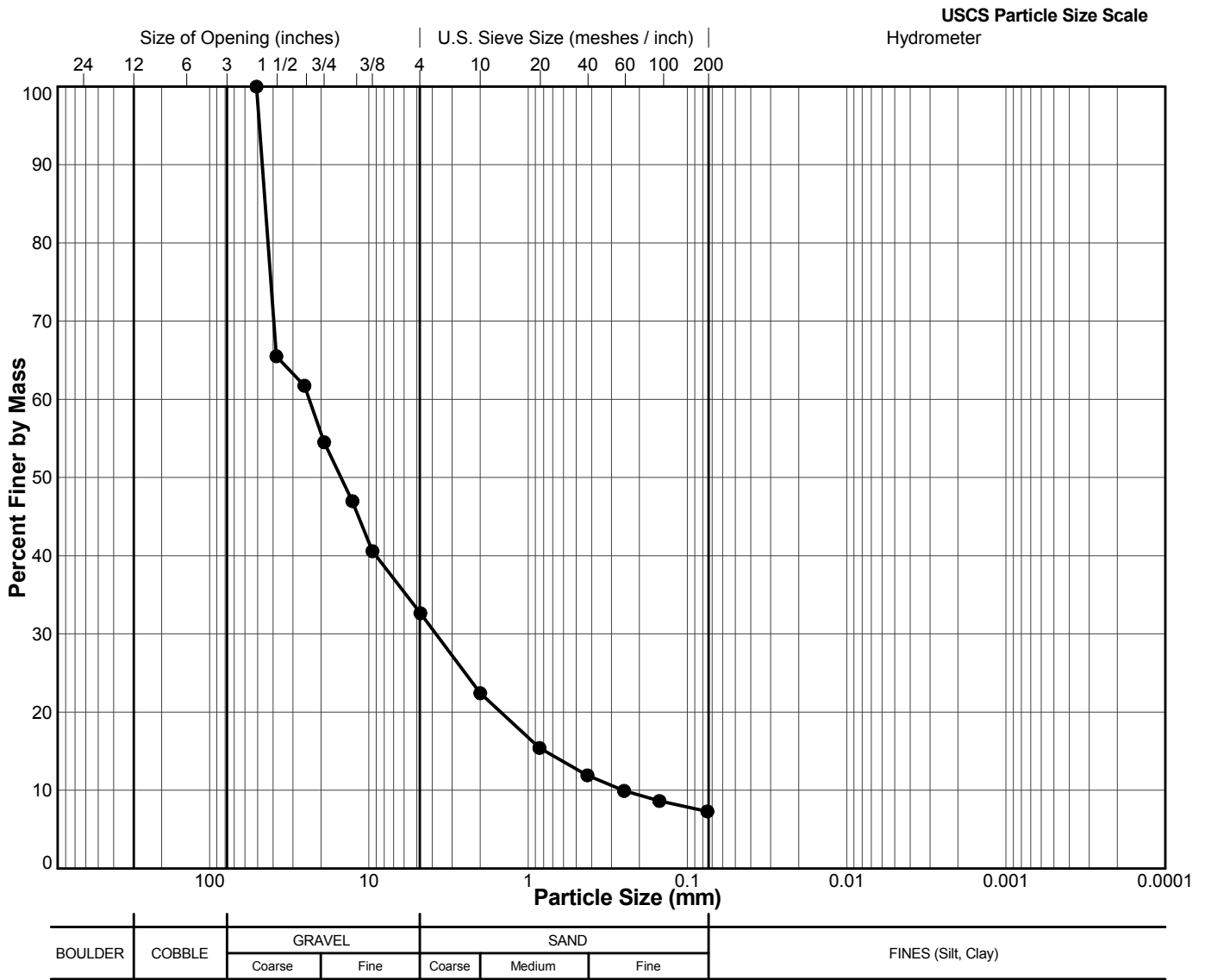
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-72
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.40 to 2.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

RS	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

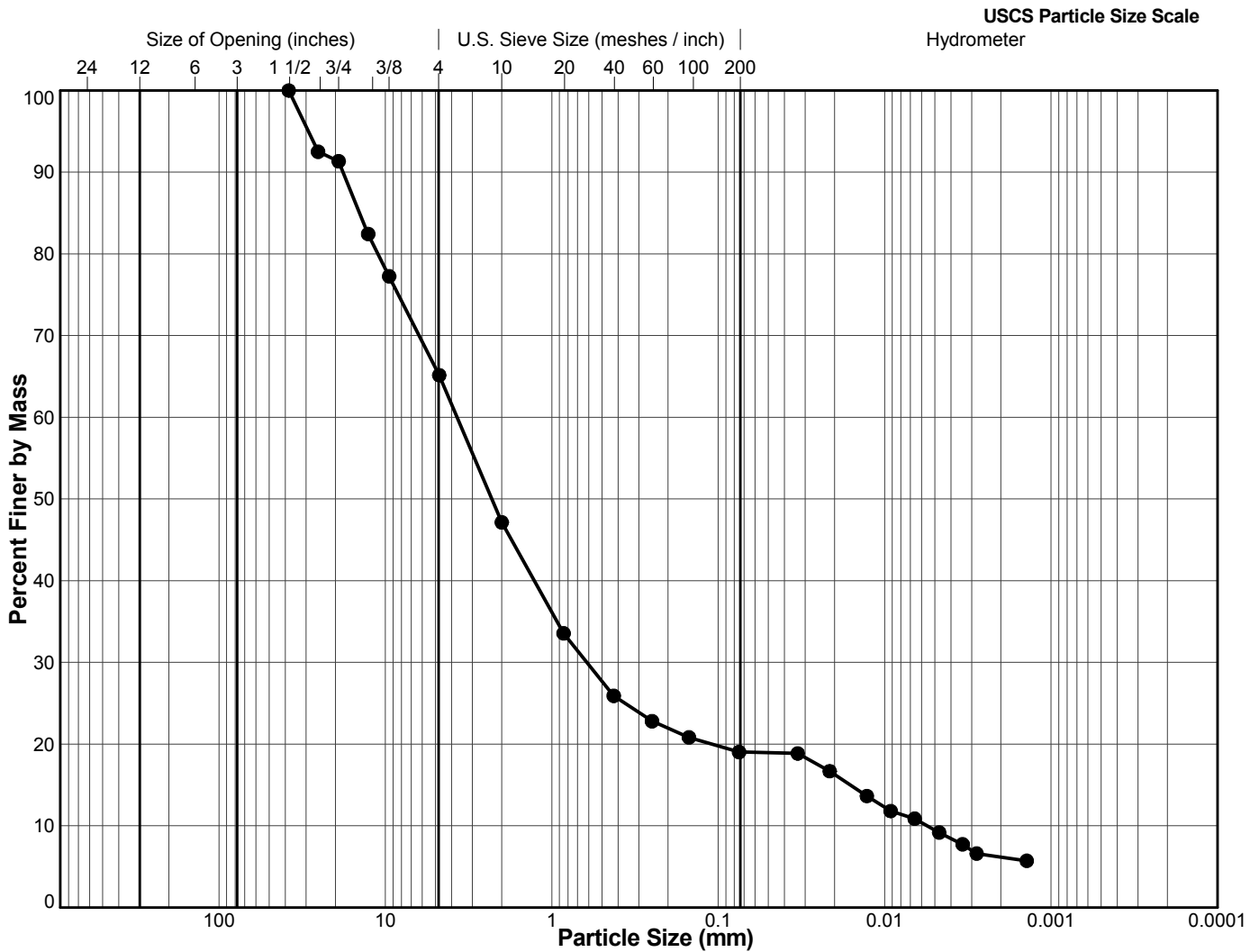
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-72
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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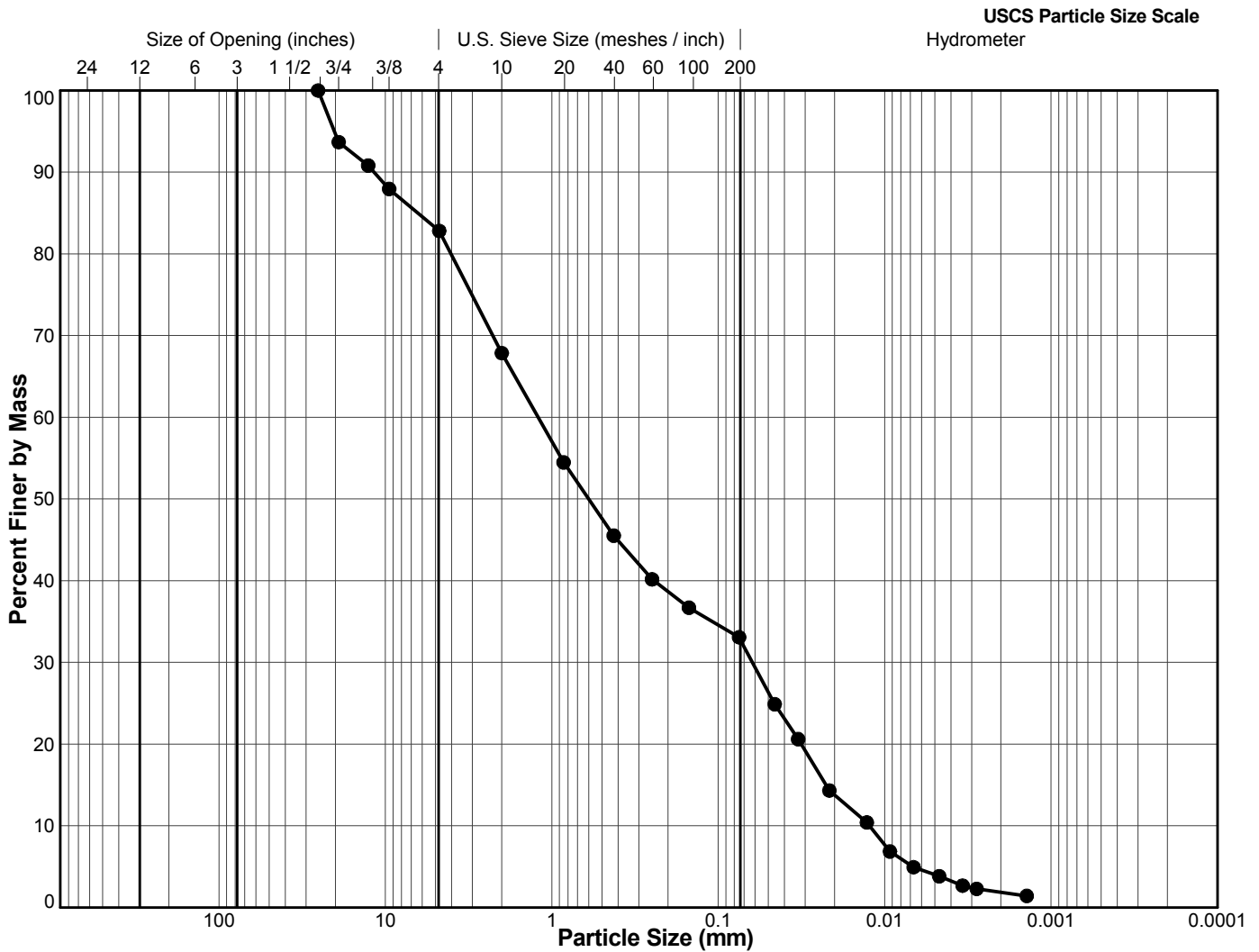
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-73
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 25.4	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029\EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

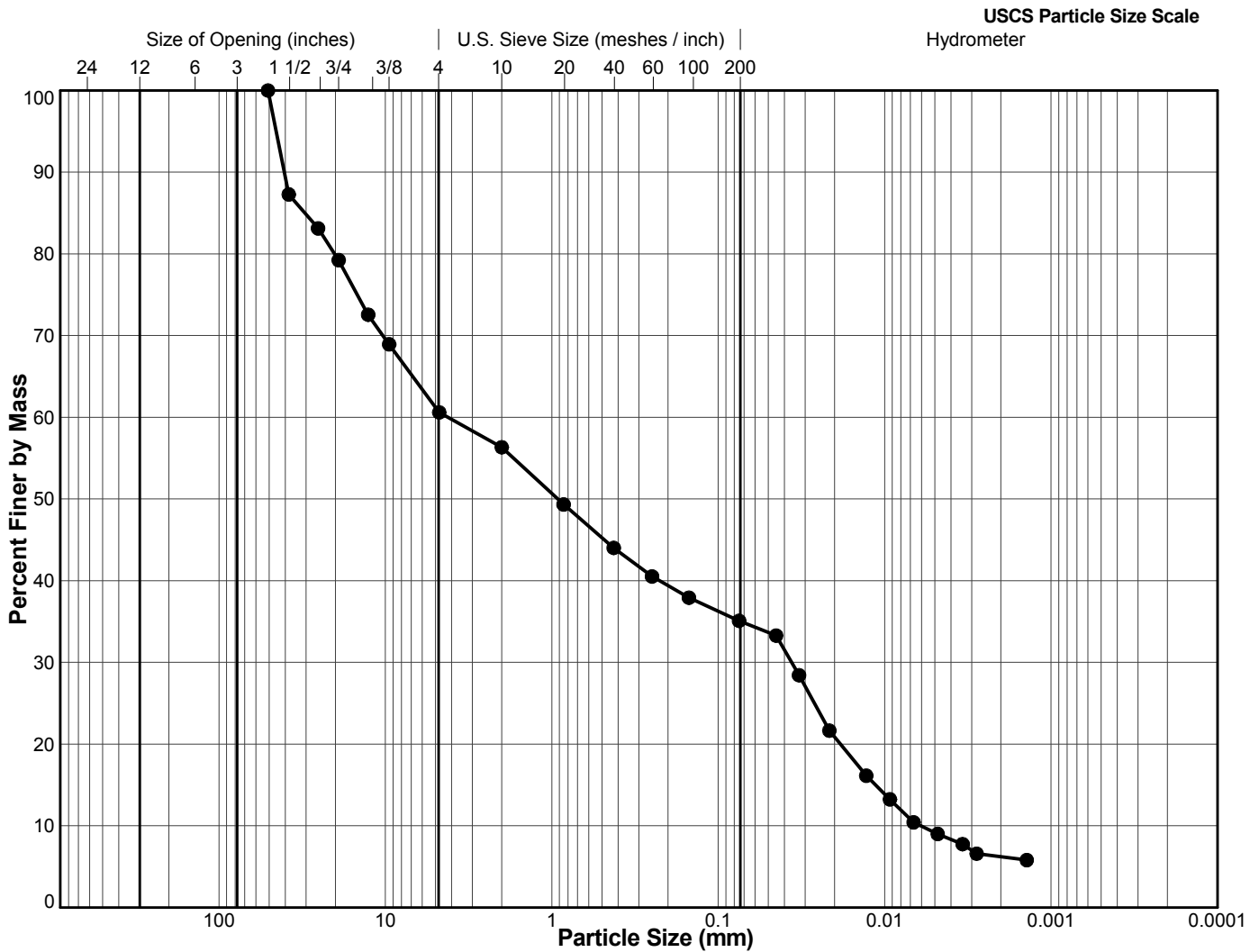
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.20 to 1.40
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)				
		Coarse	Fine	Coarse	Medium	Fine					

SK	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

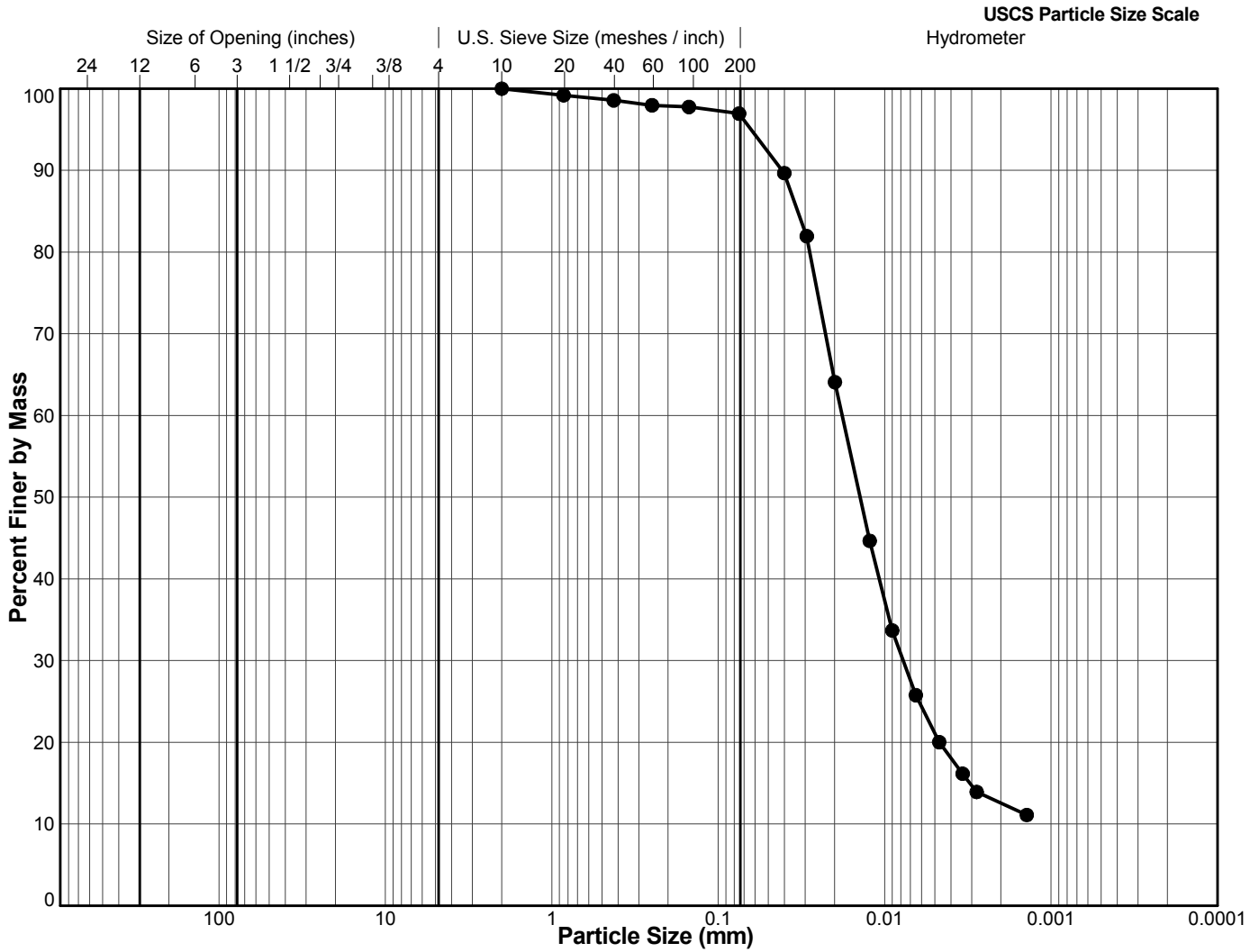
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.00 to 3.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)		
		Coarse	Fine	Coarse	Medium	Fine			

SK	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029\EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

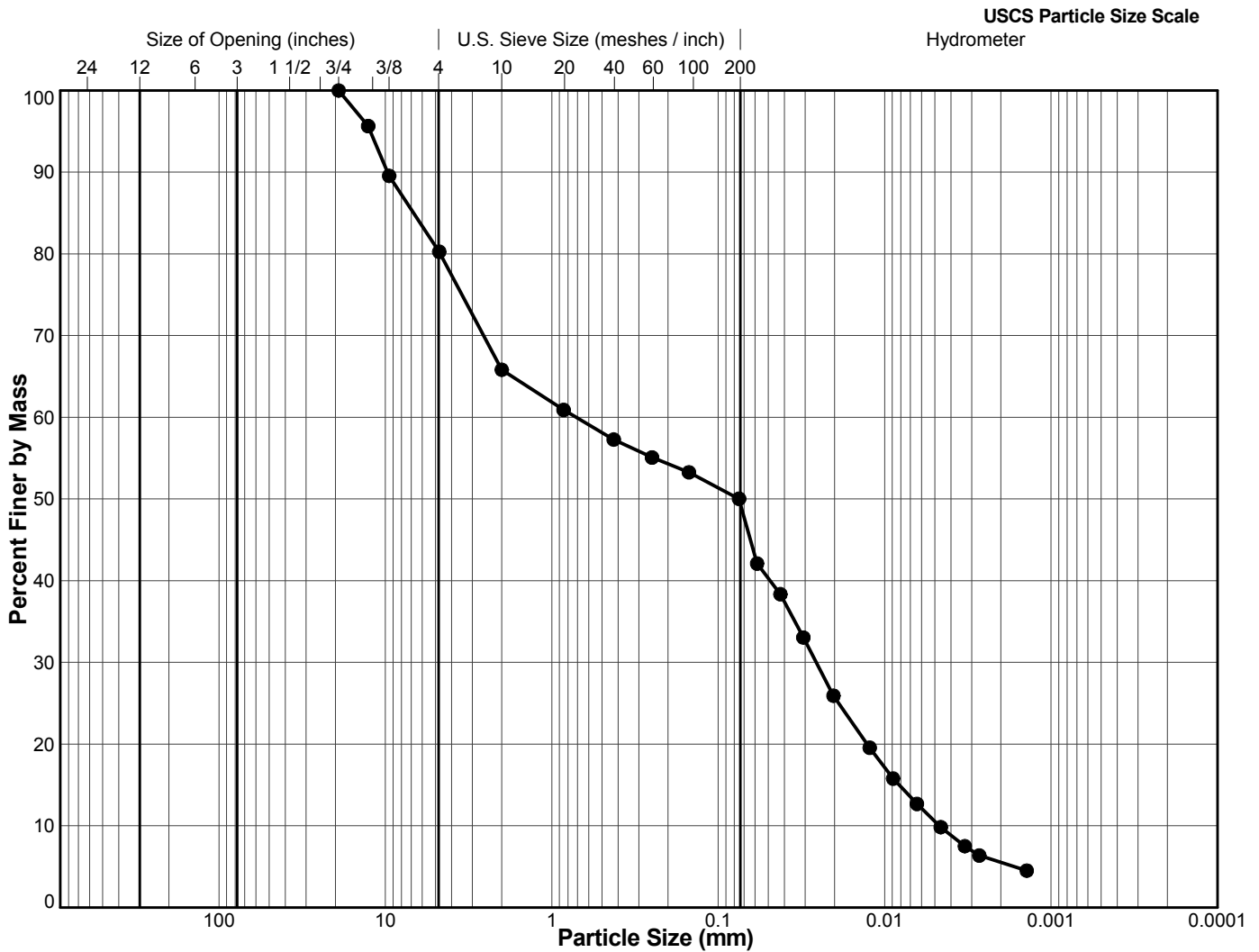
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 3
Location: Dublin Gulch, Yukon	Depth Interval (m): 4.50 to 4.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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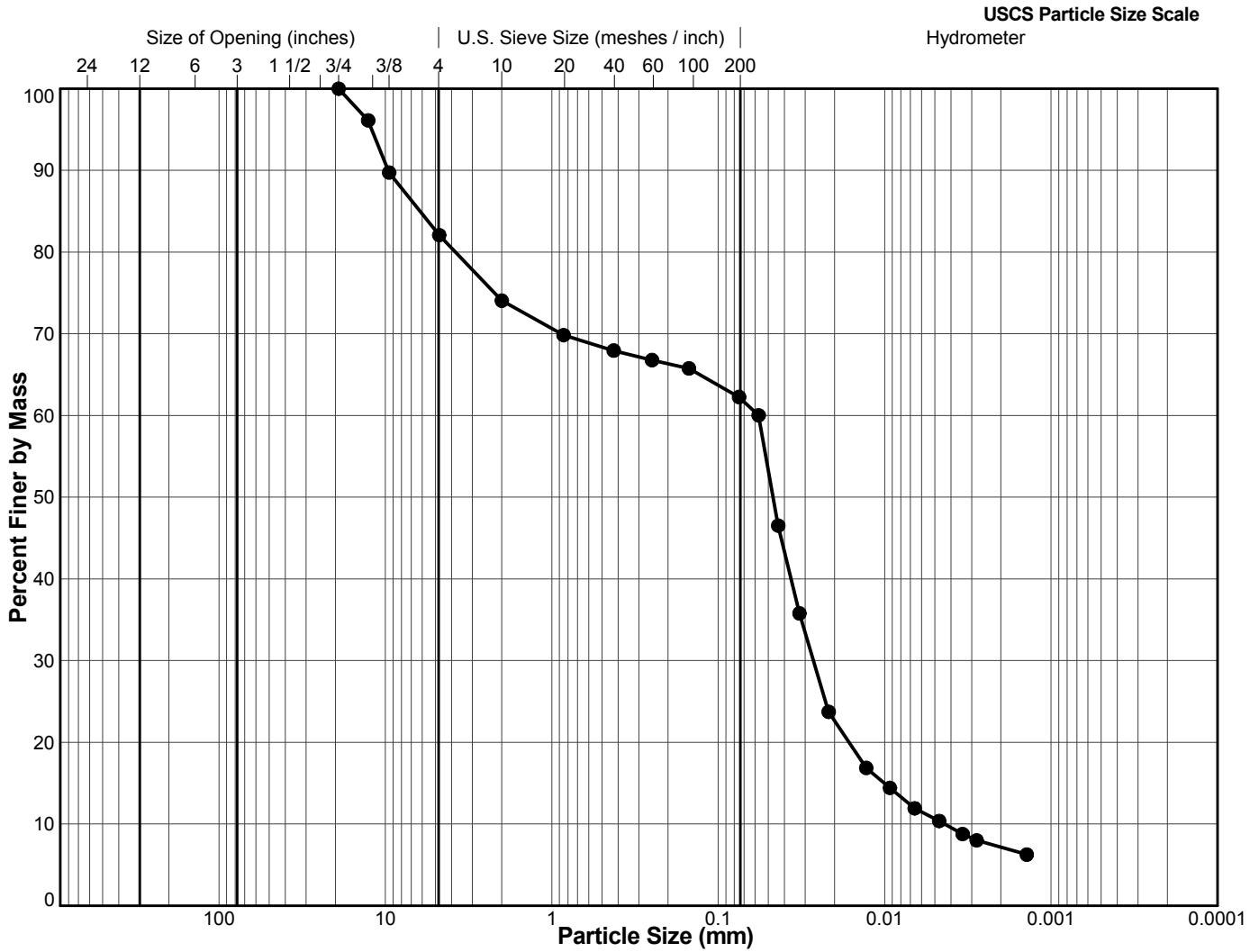
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-76
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	20/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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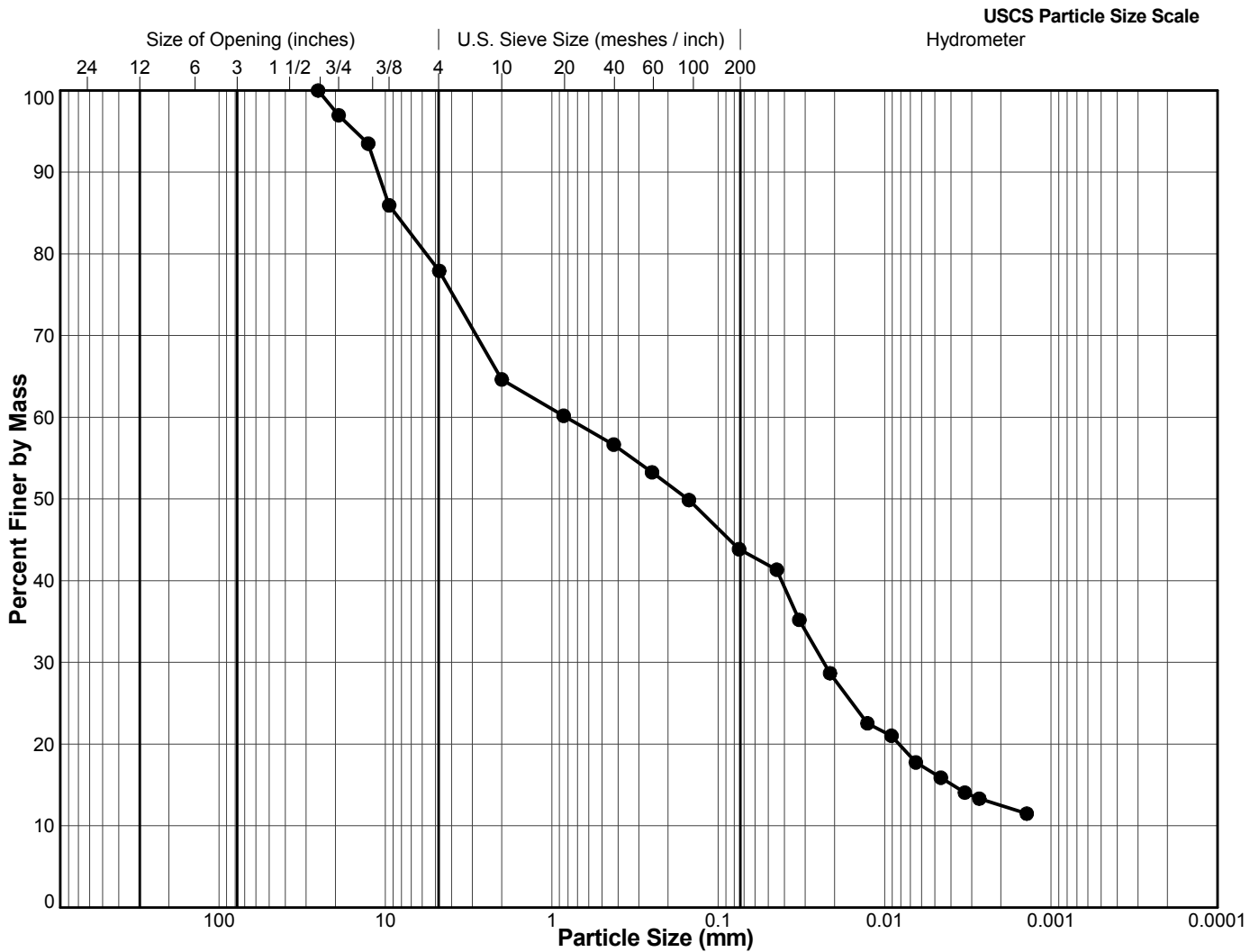
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-77
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 25.4	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)				
		Coarse	Fine	Coarse	Medium	Fine					

RS	03/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

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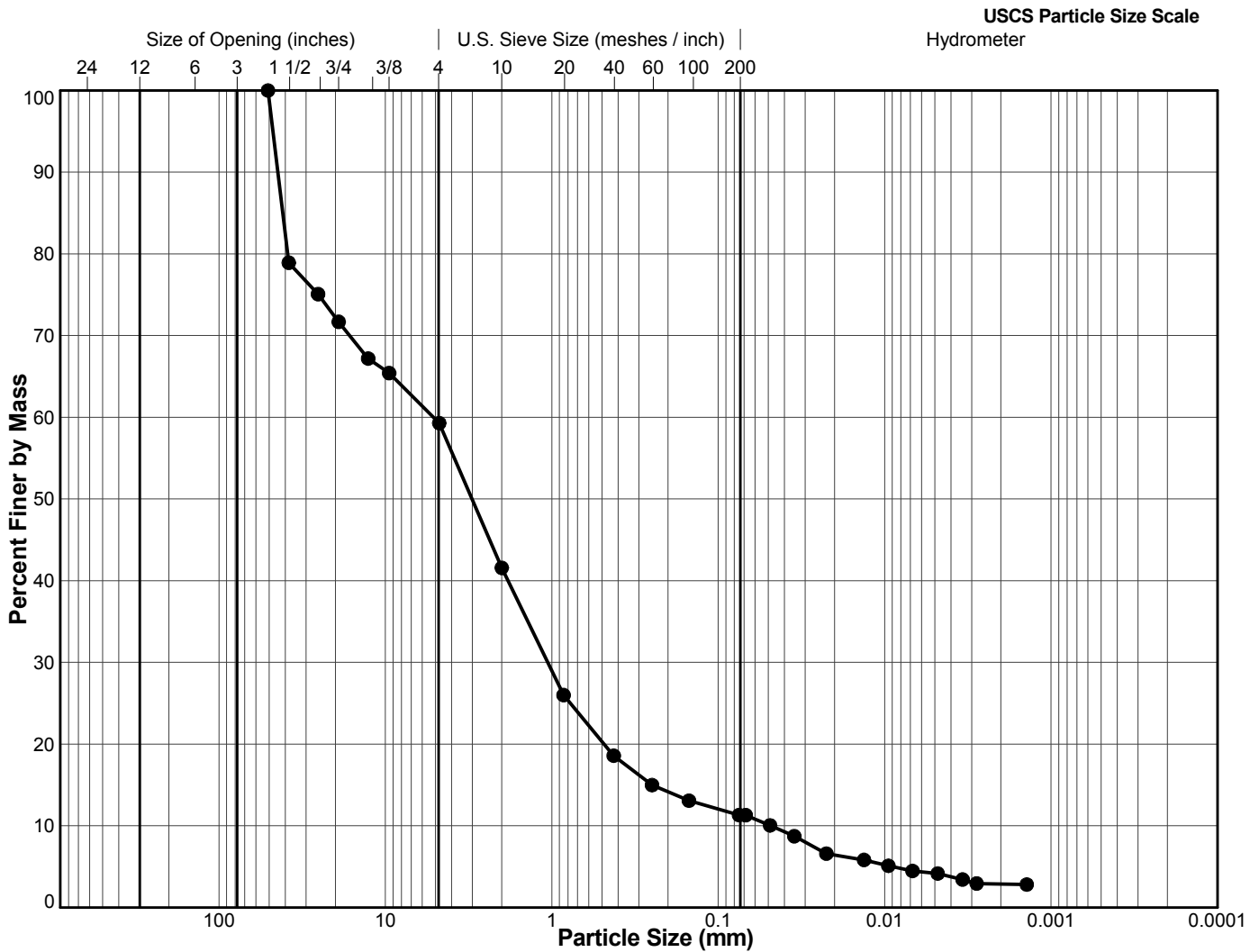
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-79
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

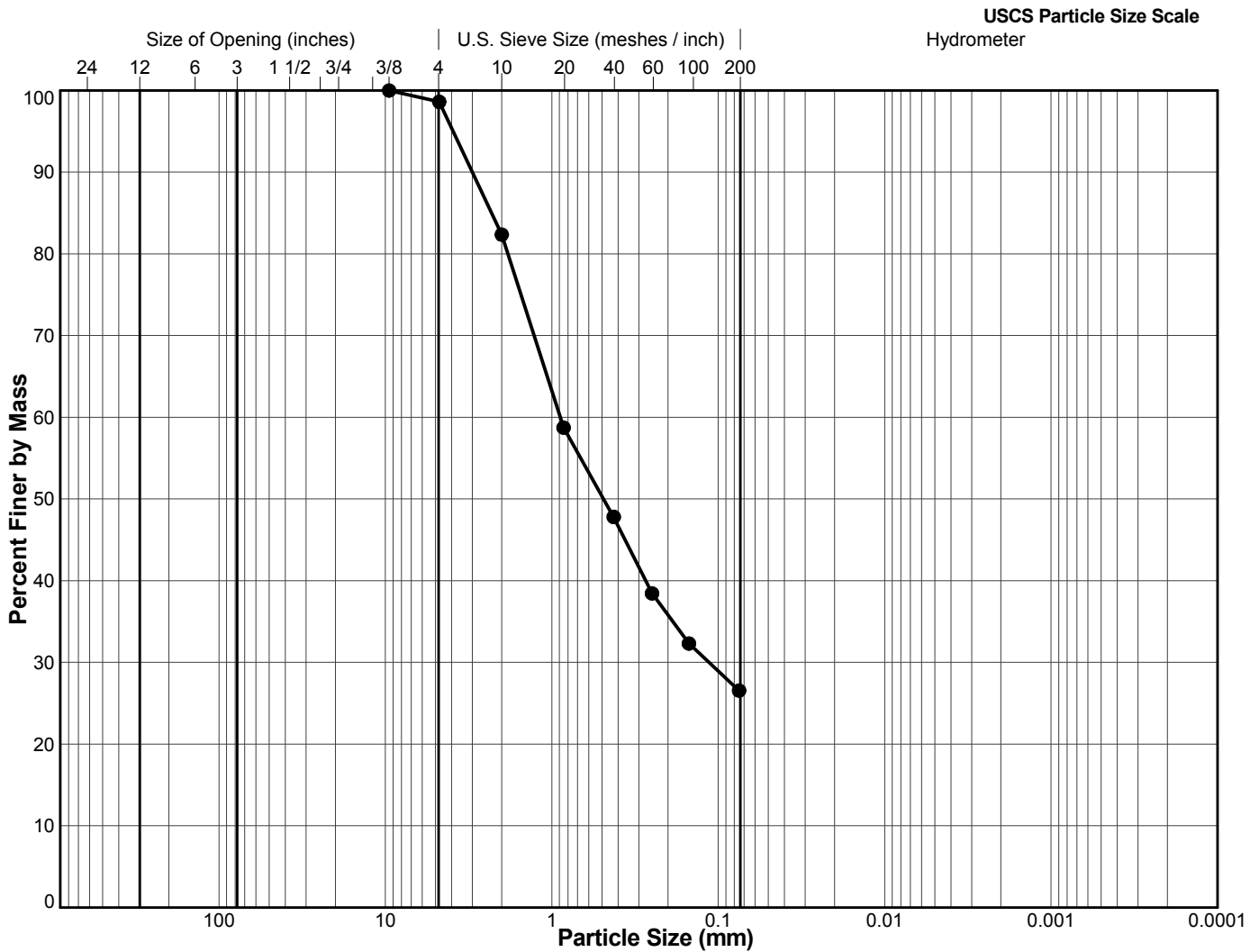
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-79
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.20 to 1.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 9.5	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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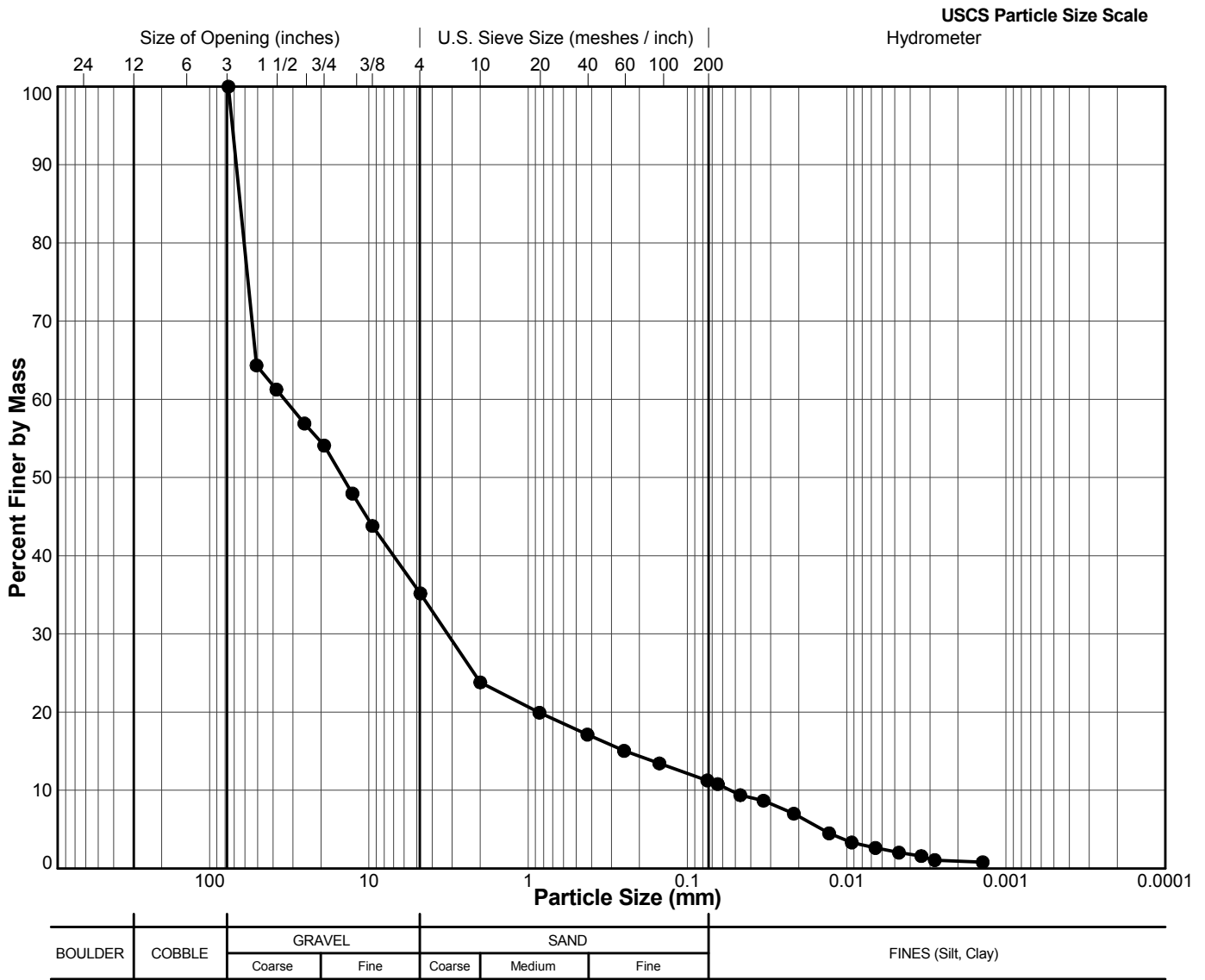
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-81
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 76.2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



File N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

SK	07/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

PARTICLE SIZE ANALYSIS OF SOILS

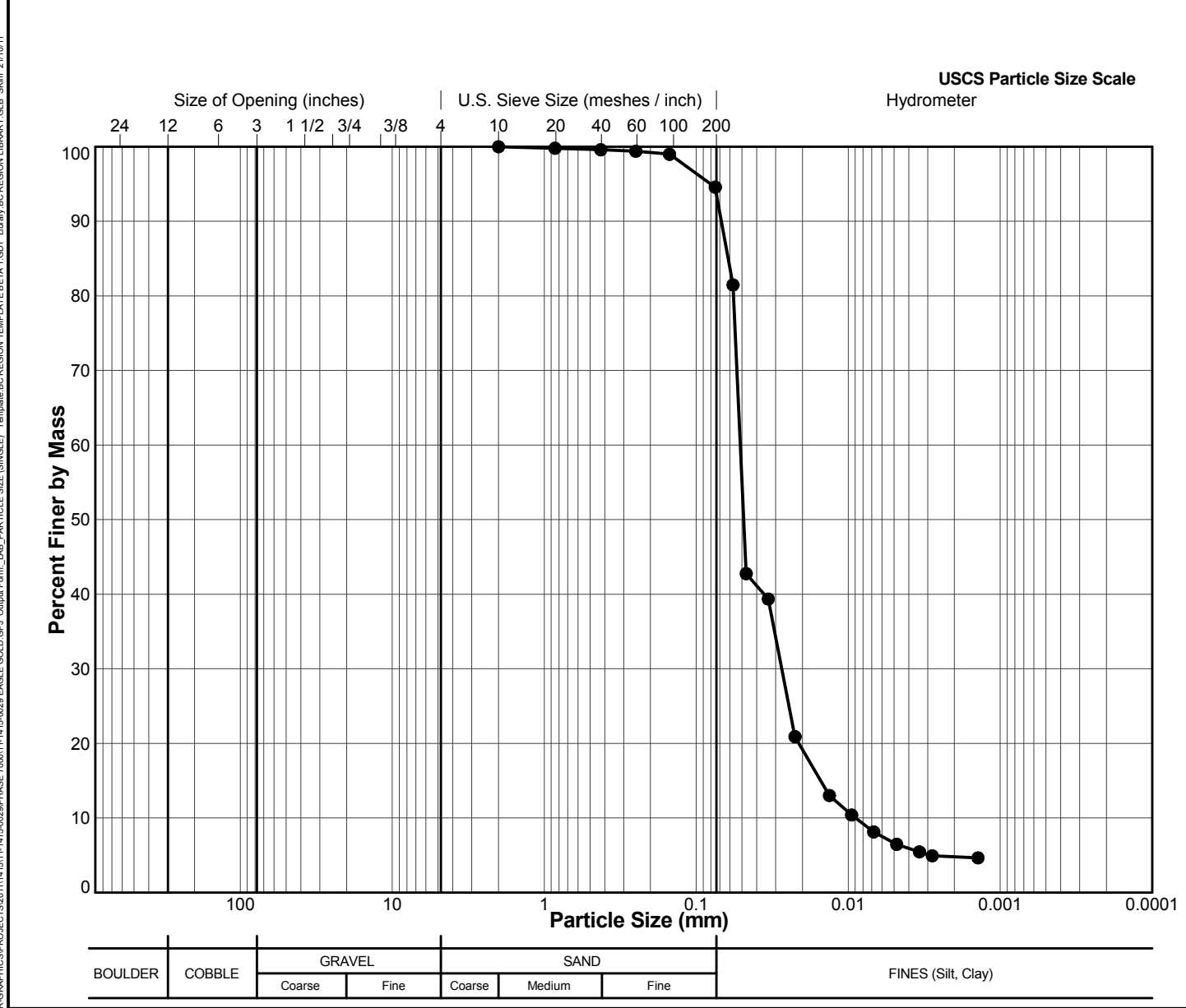
Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M1
Location: Dublin Gulch, Yukon	Depth Interval (m): 4.70 to 4.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1

Hydrometer ID: BURNABY - 87024 (2006)



RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

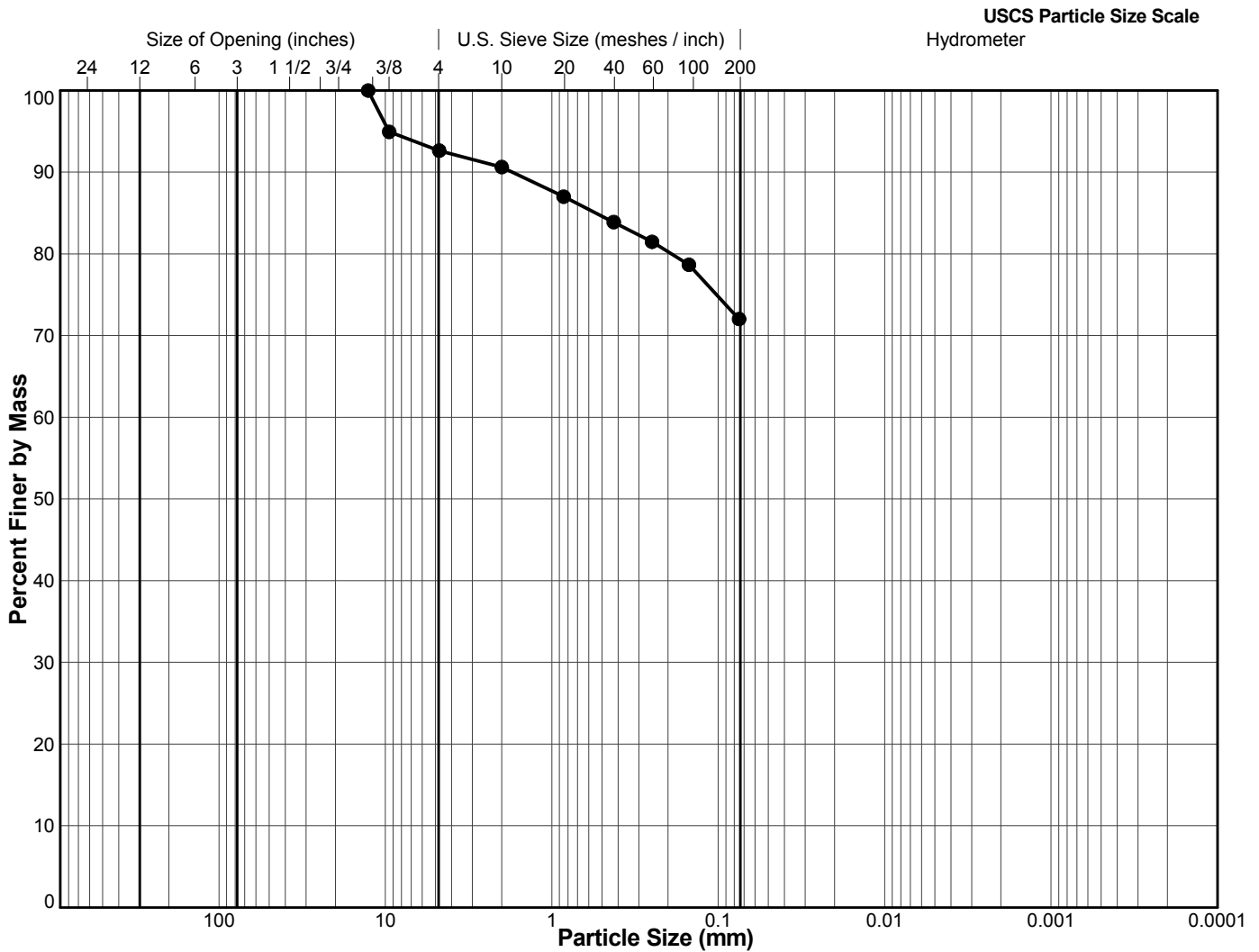
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.00 to 3.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 12.7	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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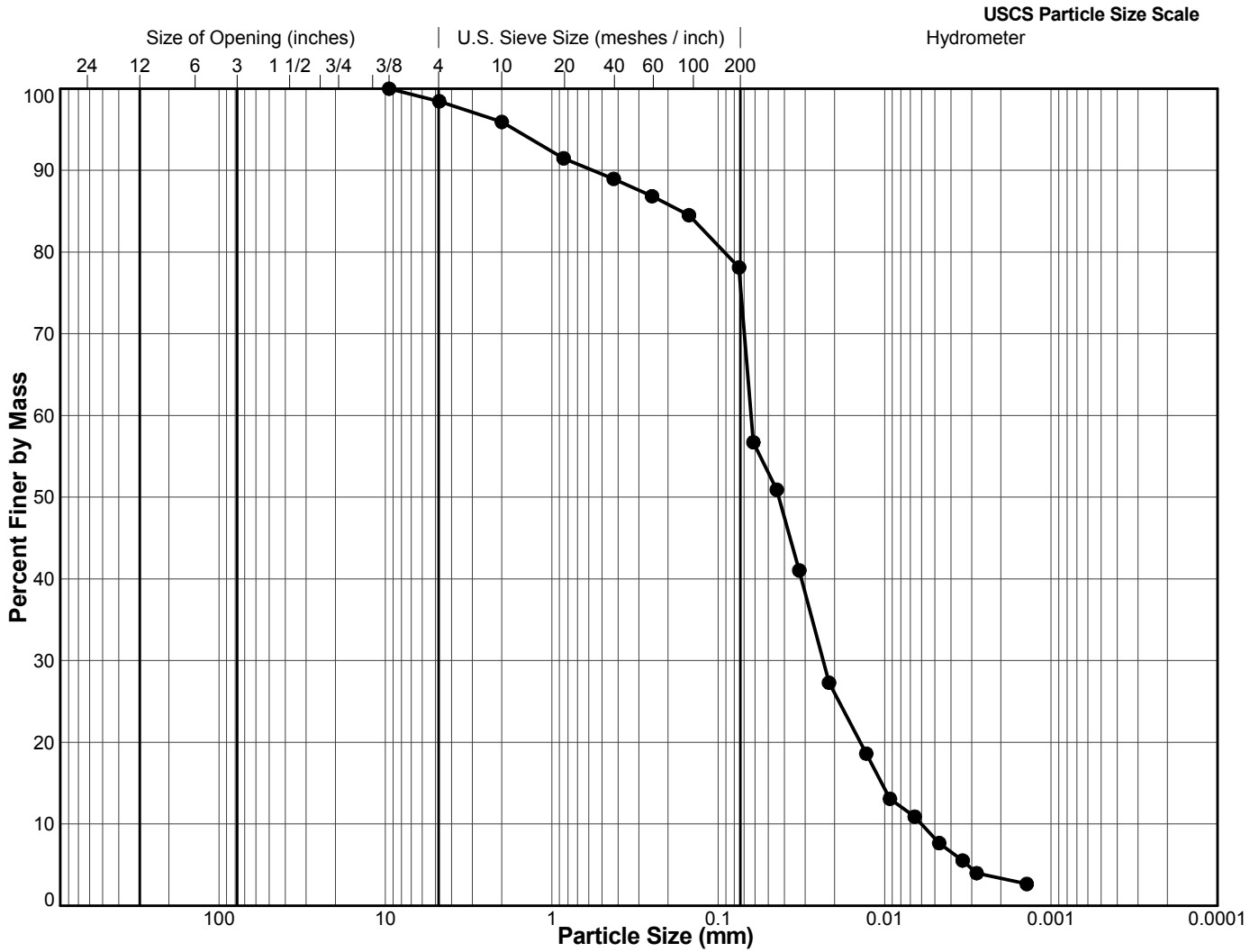
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M3
Location: Dublin Gulch, Yukon	Depth Interval (m): 6.70 to 7.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 9.5	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

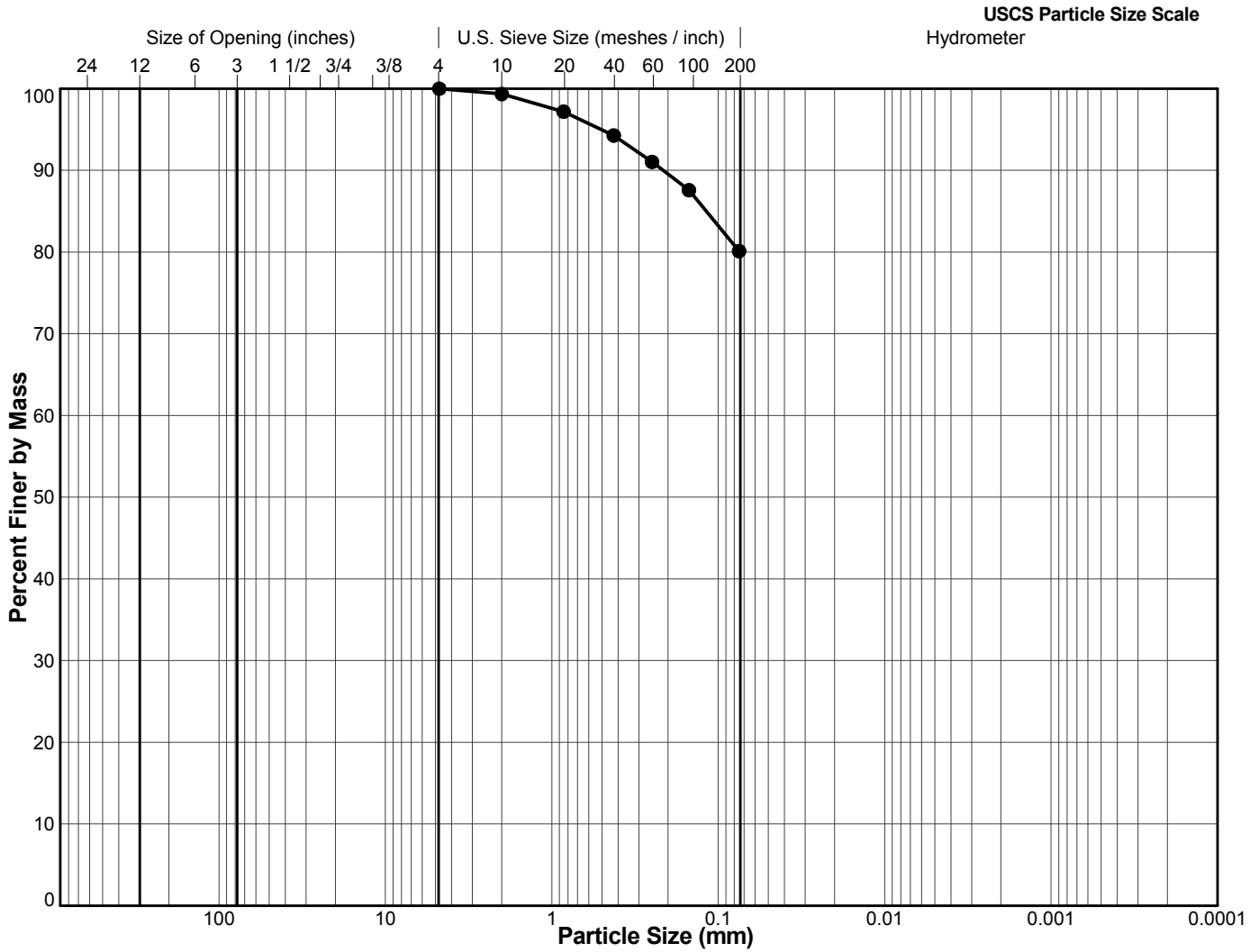
RS	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M4
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 1.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 4.75	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)				
		Coarse	Fine	Coarse	Medium	Fine					

RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

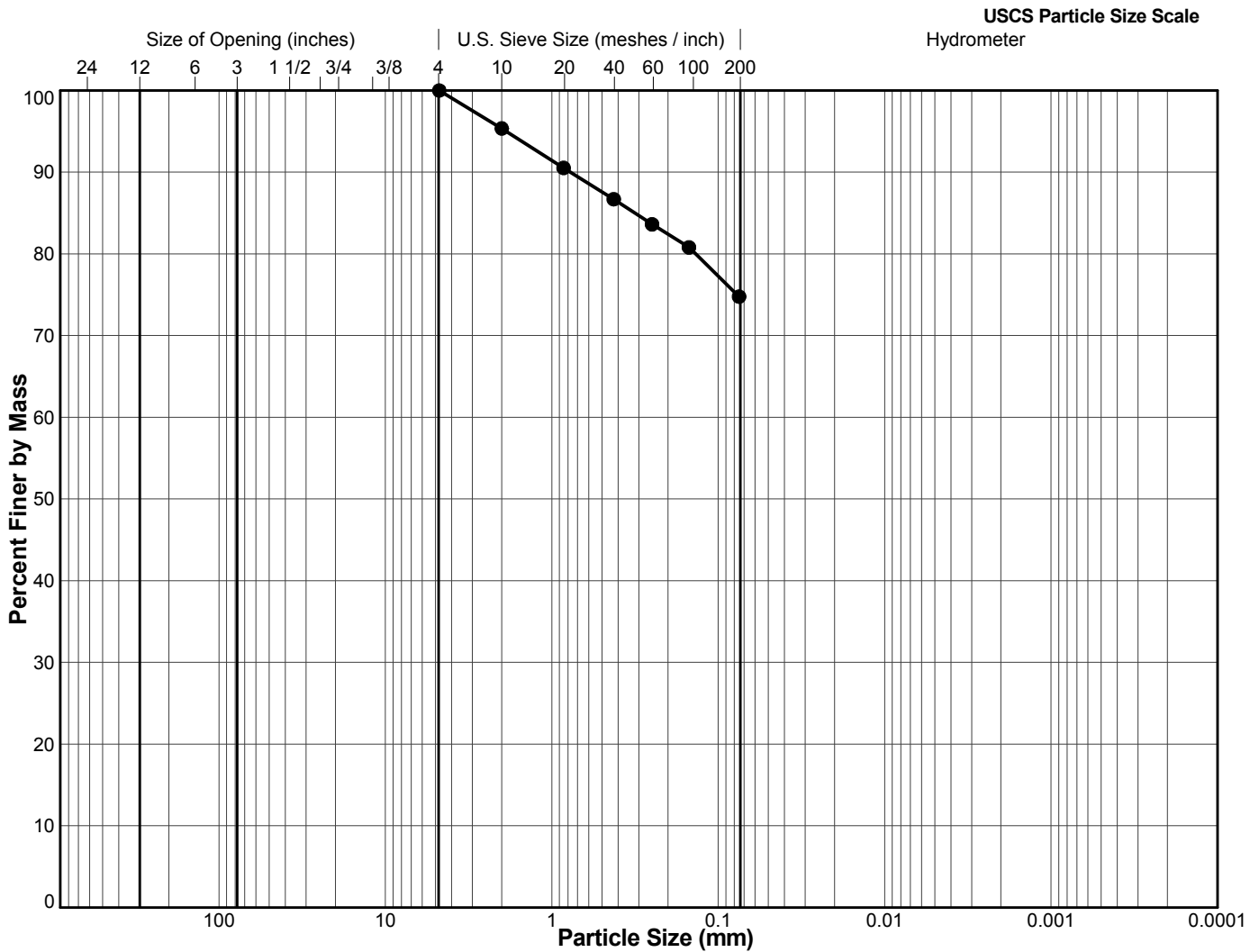
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M5
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.90 to 2.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 4.75	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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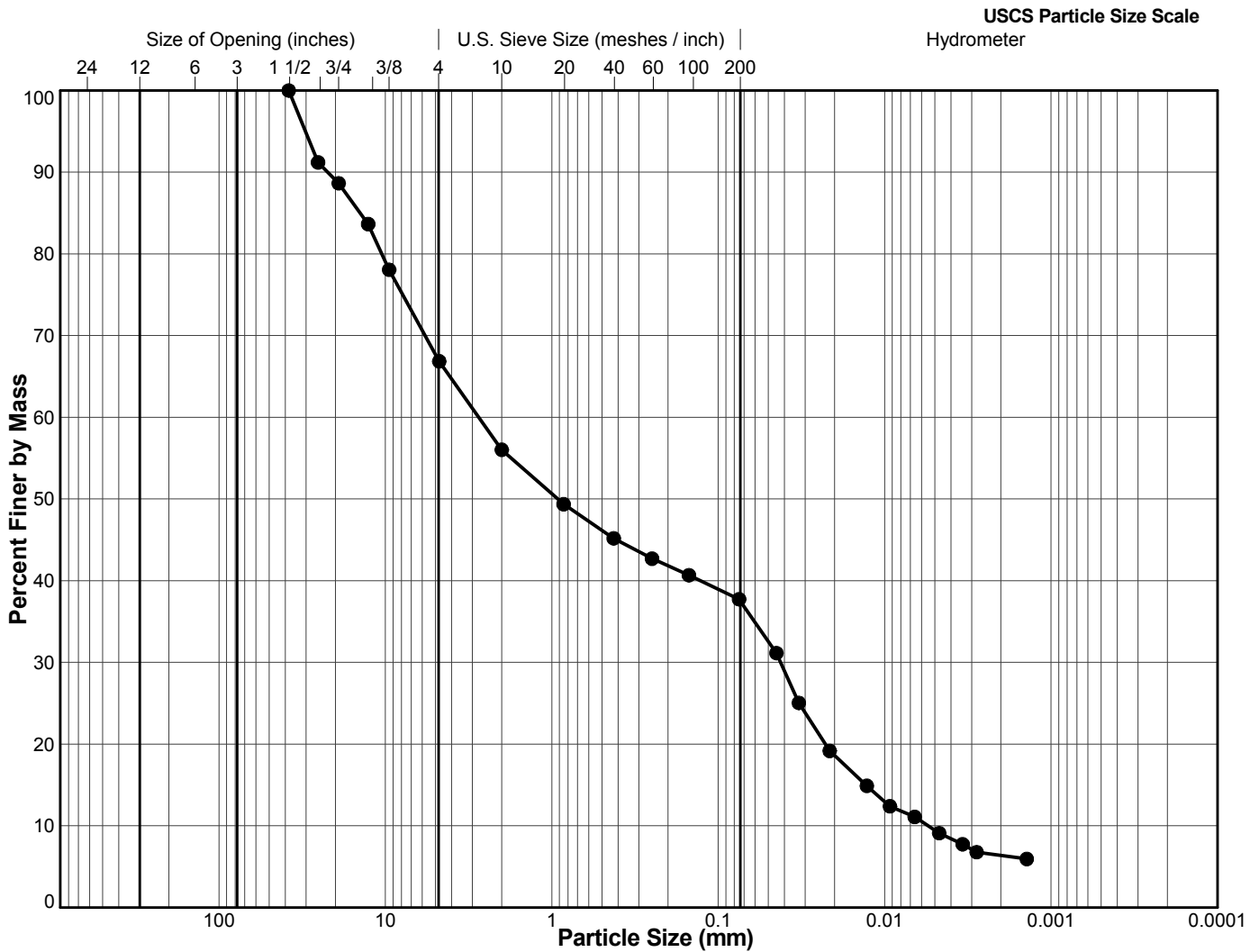
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-83
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



File N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

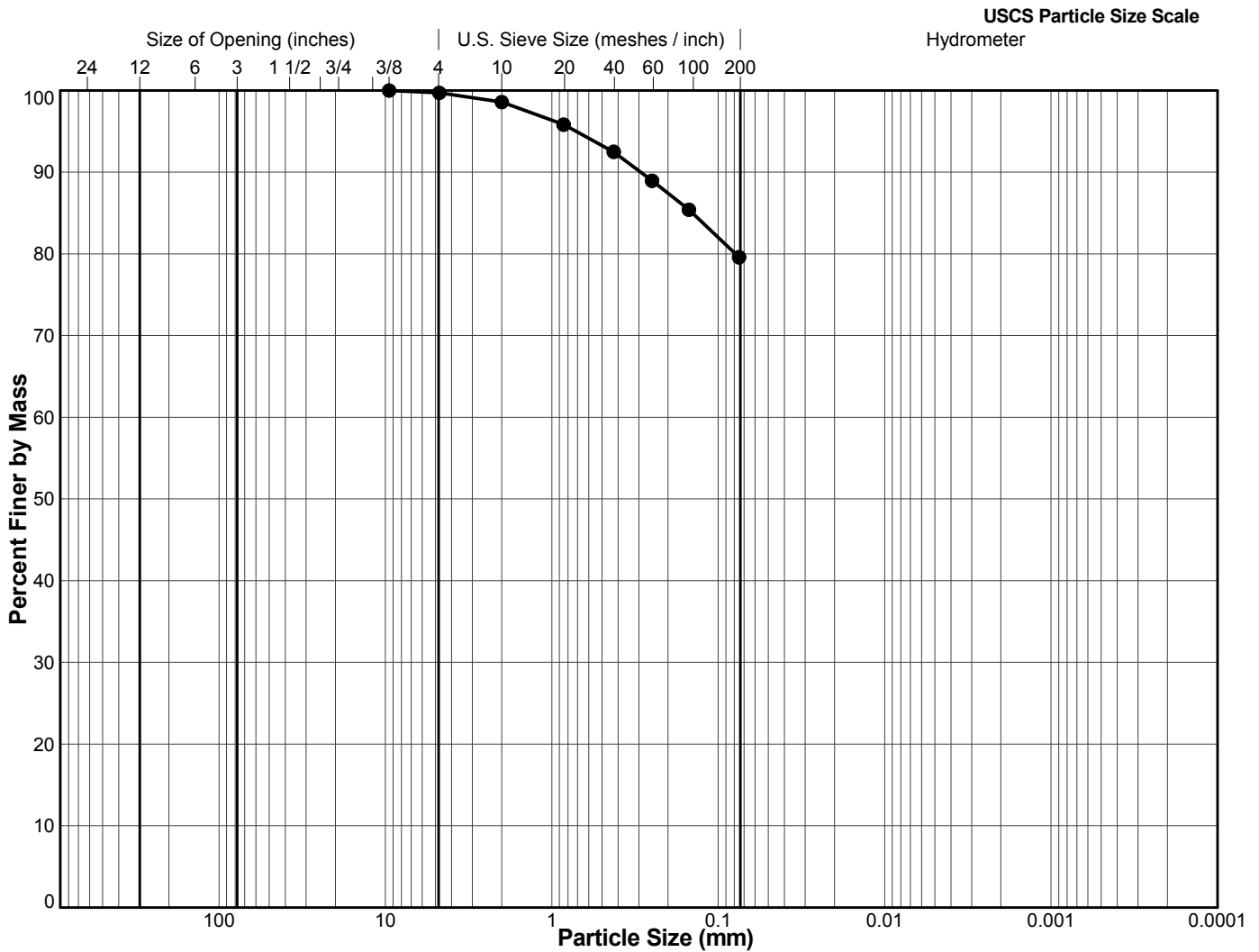
RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-84
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 9.5	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

RS	27/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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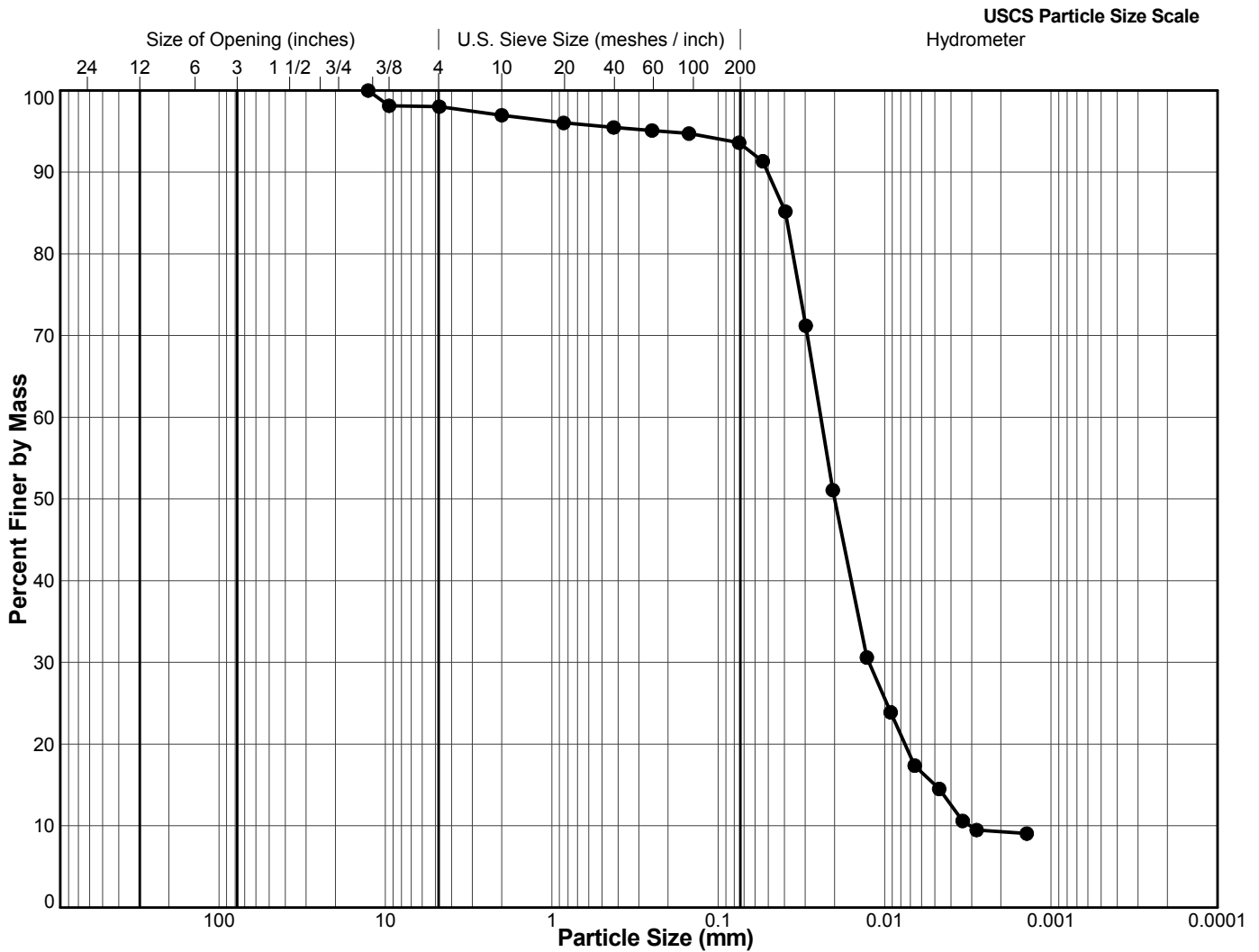
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-84
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.50 to 2.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 12.7	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	03/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

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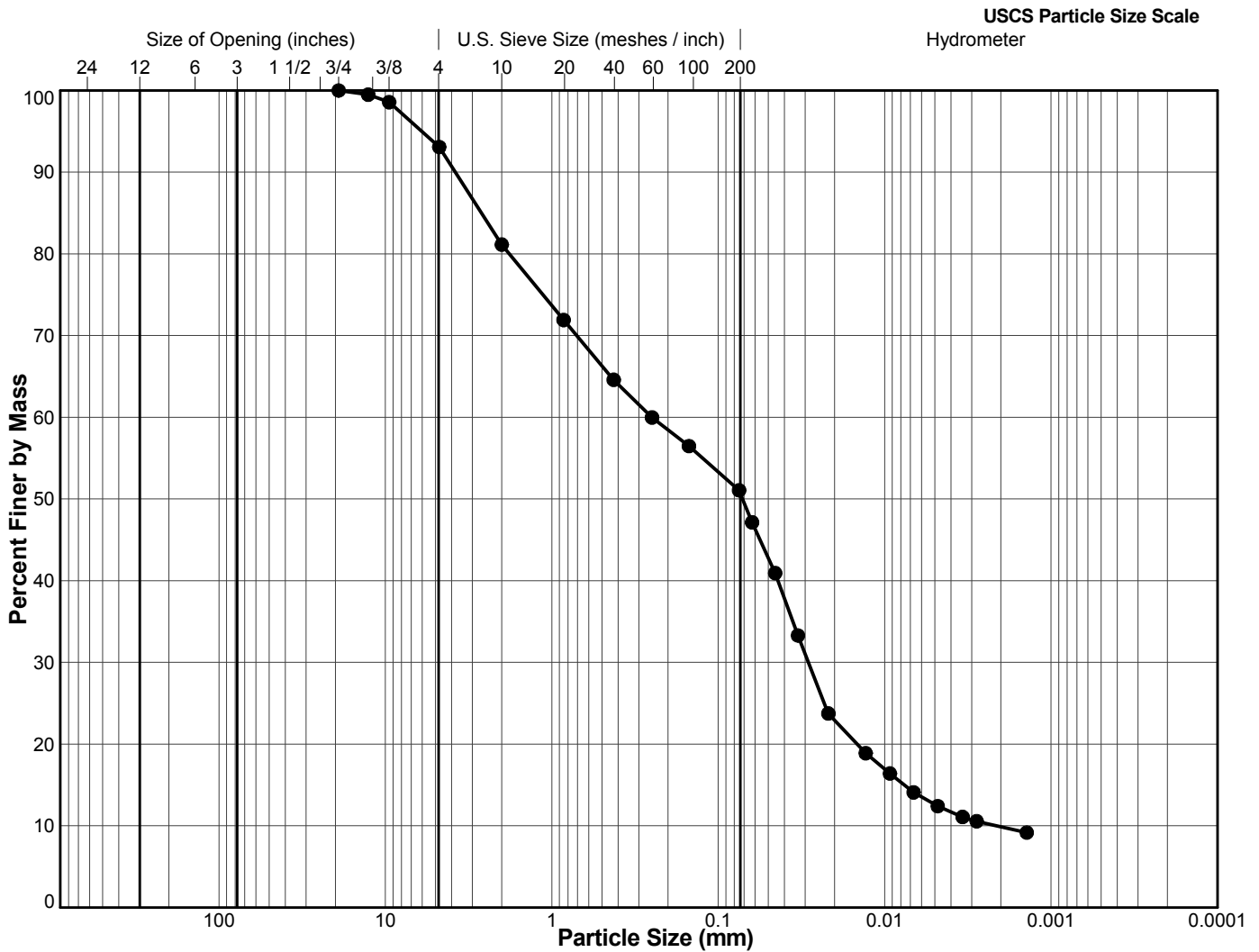
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-85
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

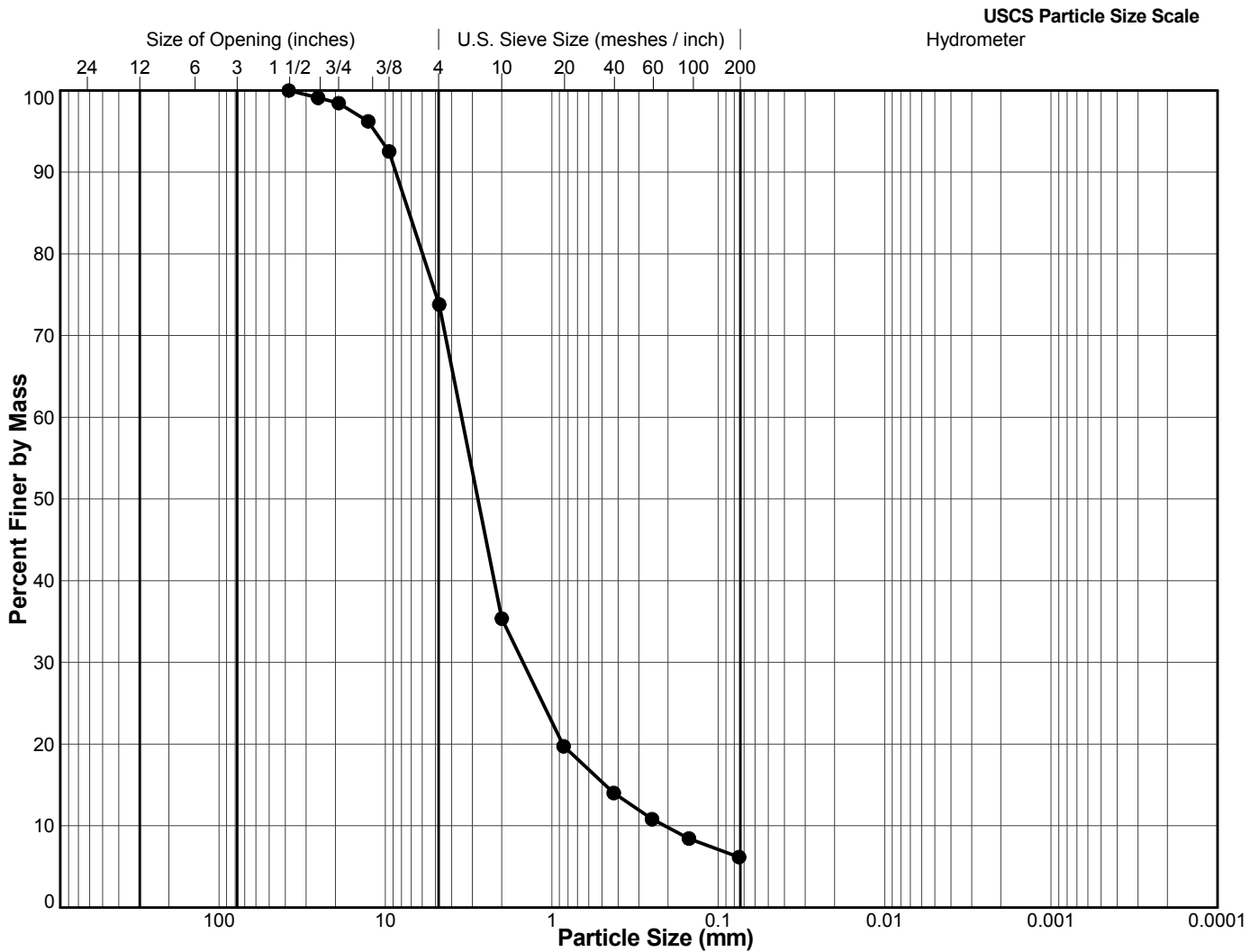
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-87
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

RS	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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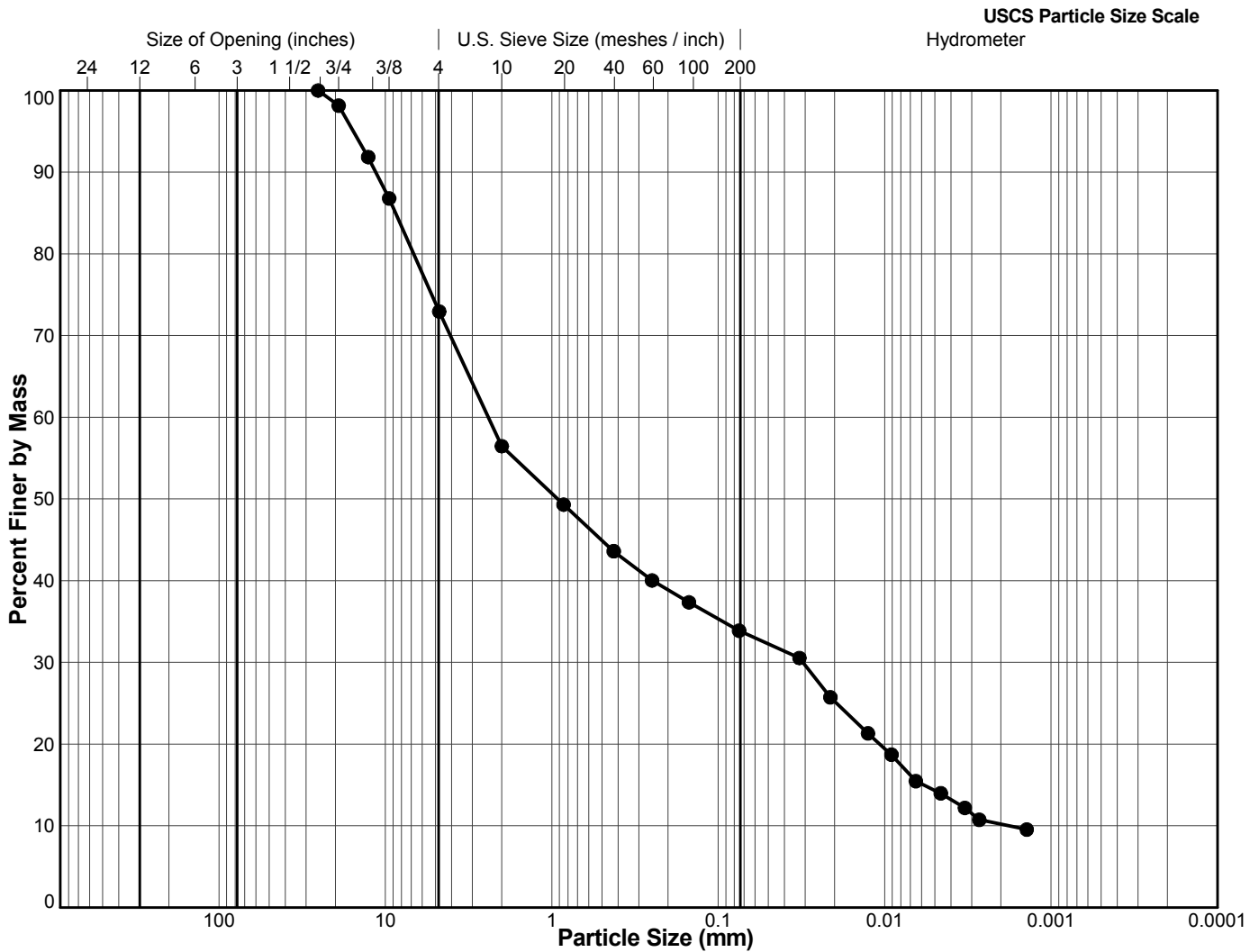
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-88
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 25.4	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

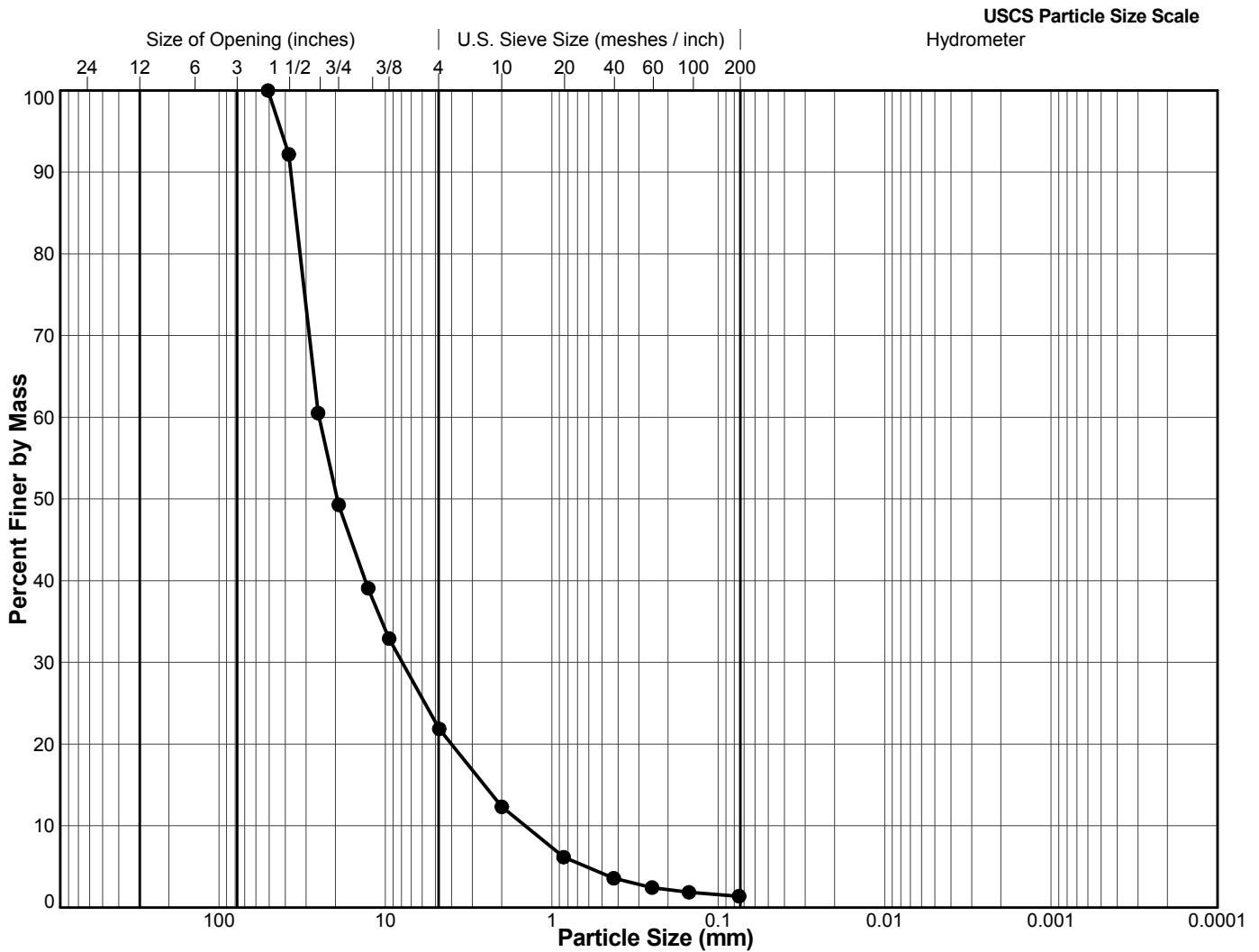
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-89
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

RS	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

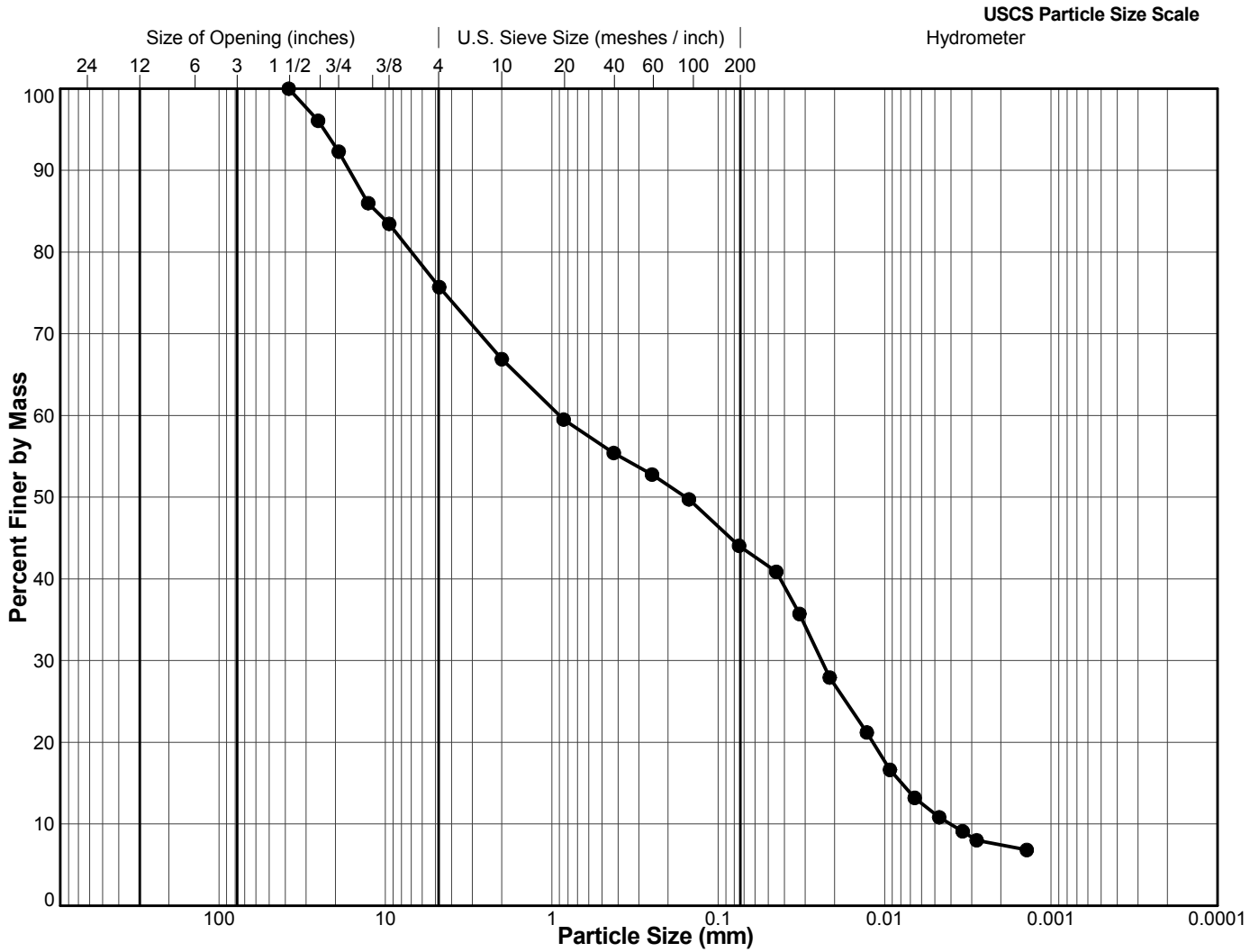
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-89
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 38.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

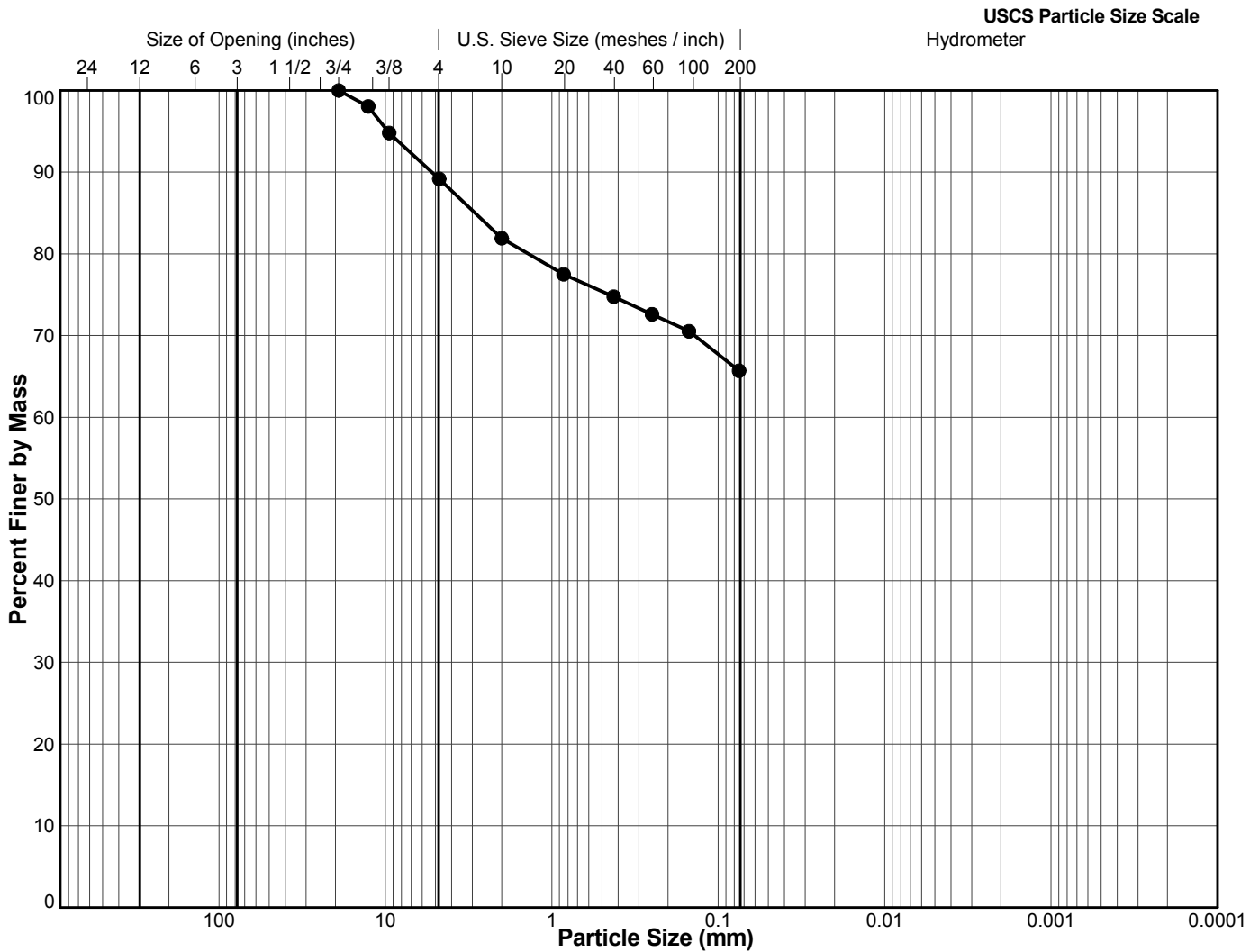
RS	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-90
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

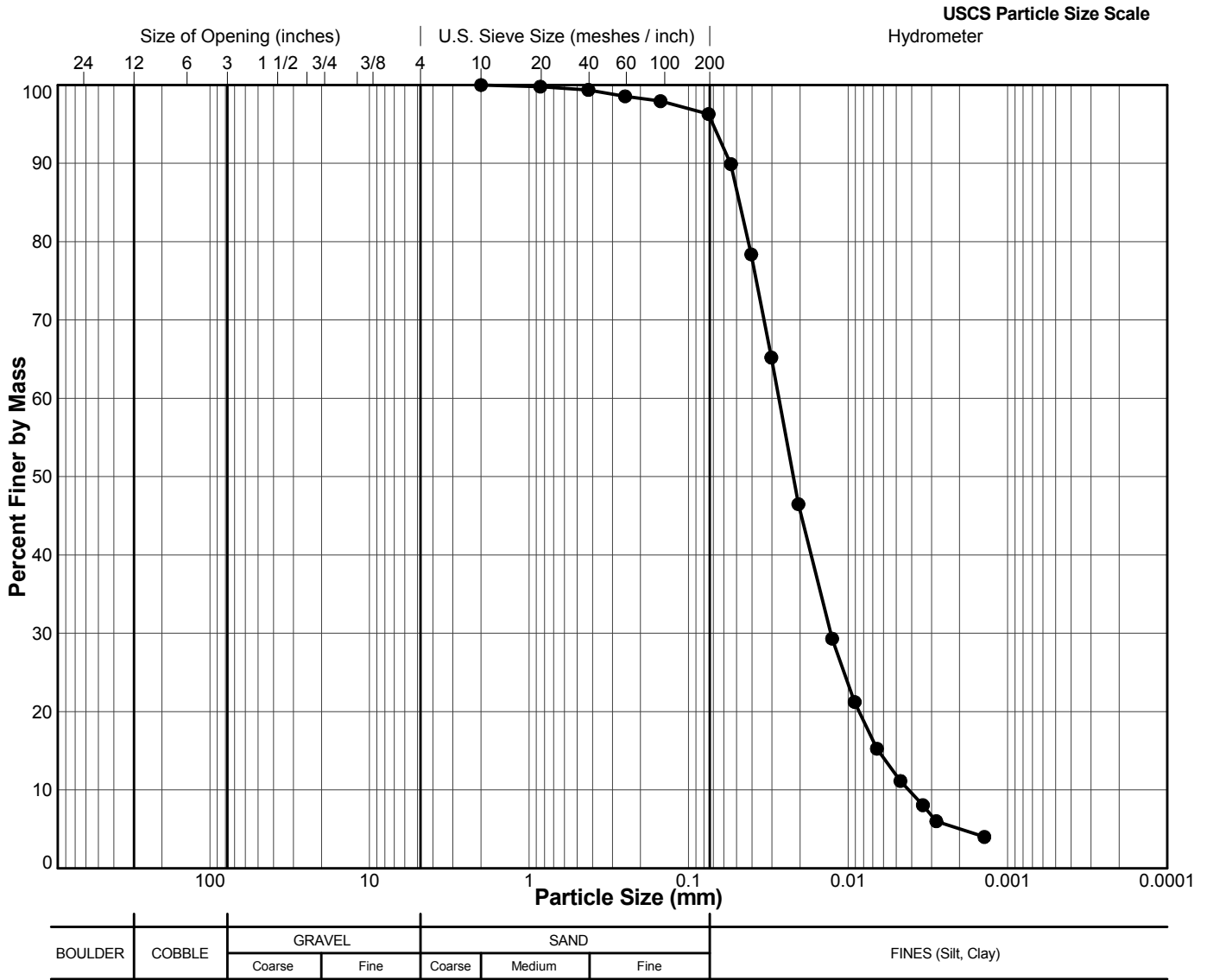
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-90
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.50 to 3.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 541360	



File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

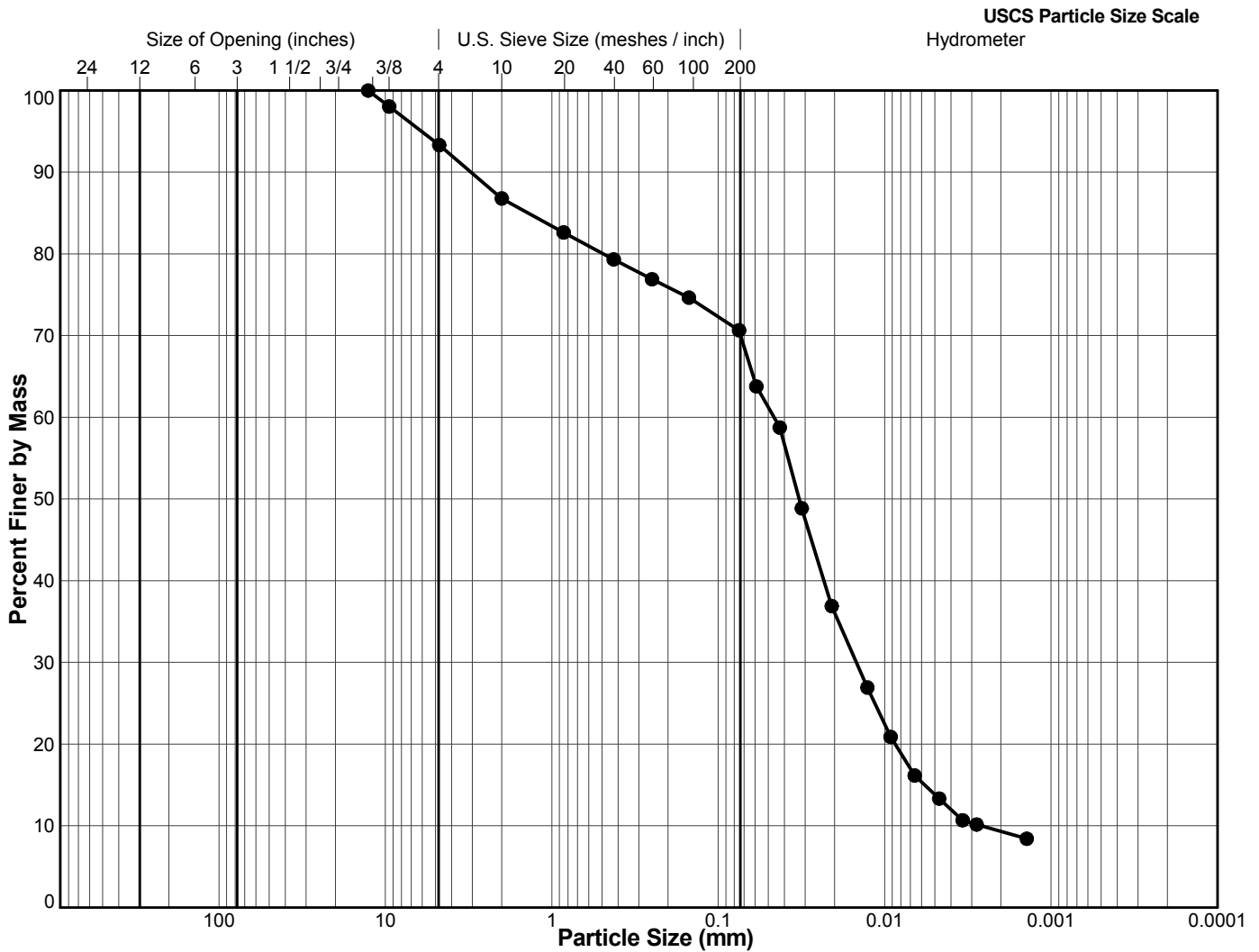
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-91
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 12.7	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)		
		Coarse	Fine	Coarse	Medium	Fine			

RS	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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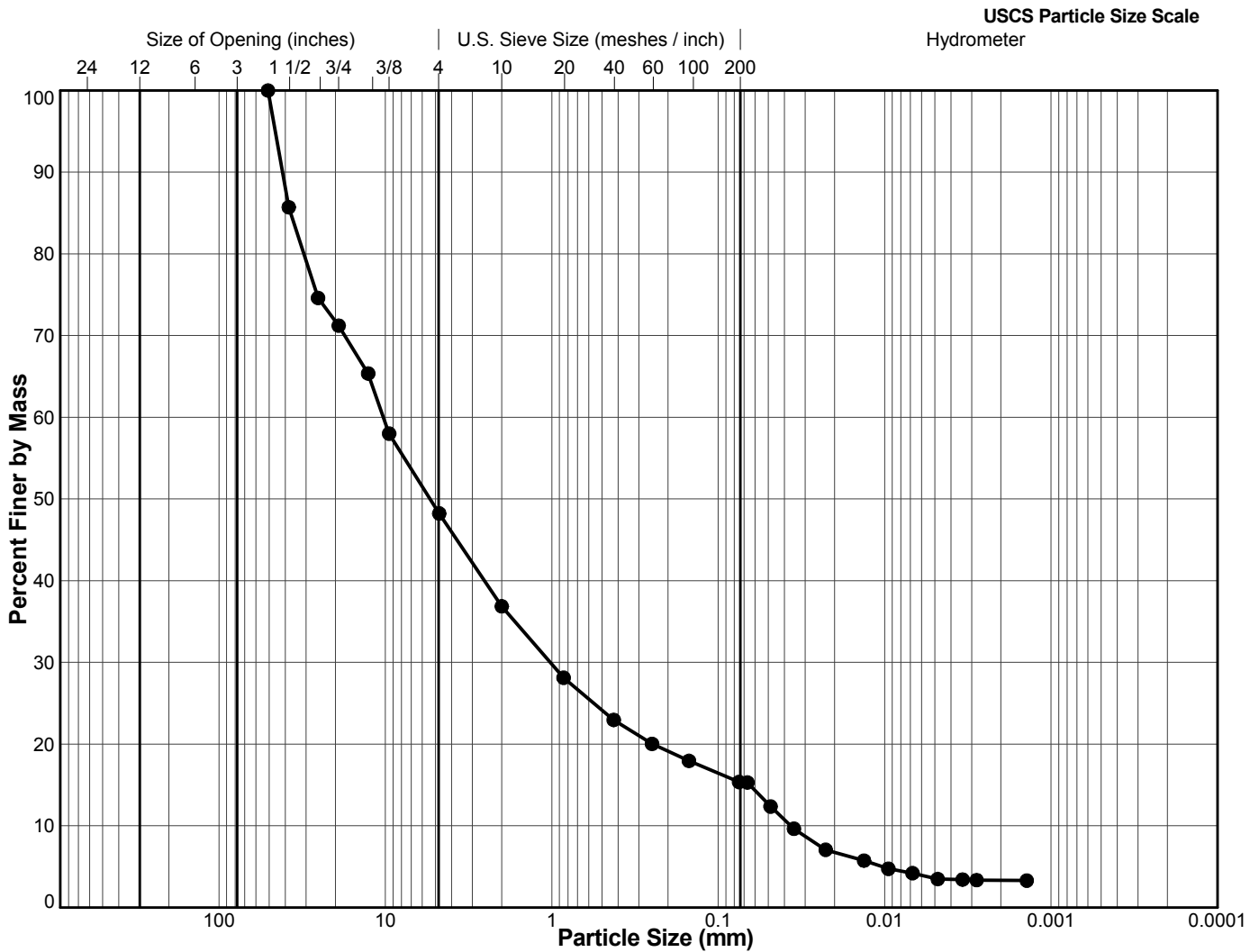
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-91
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.60 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

RS	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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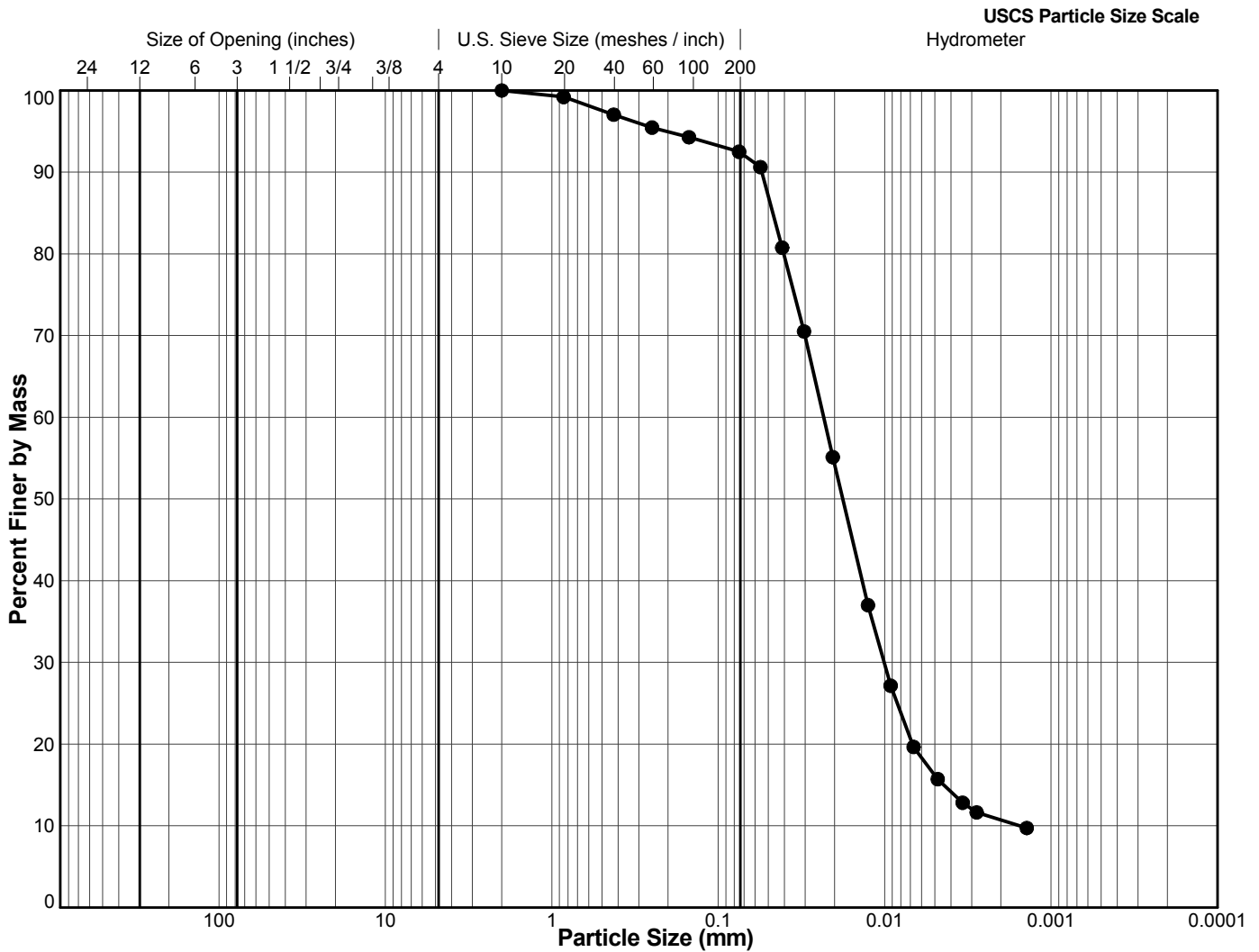
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-92
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)				
		Coarse	Fine	Coarse	Medium	Fine					

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

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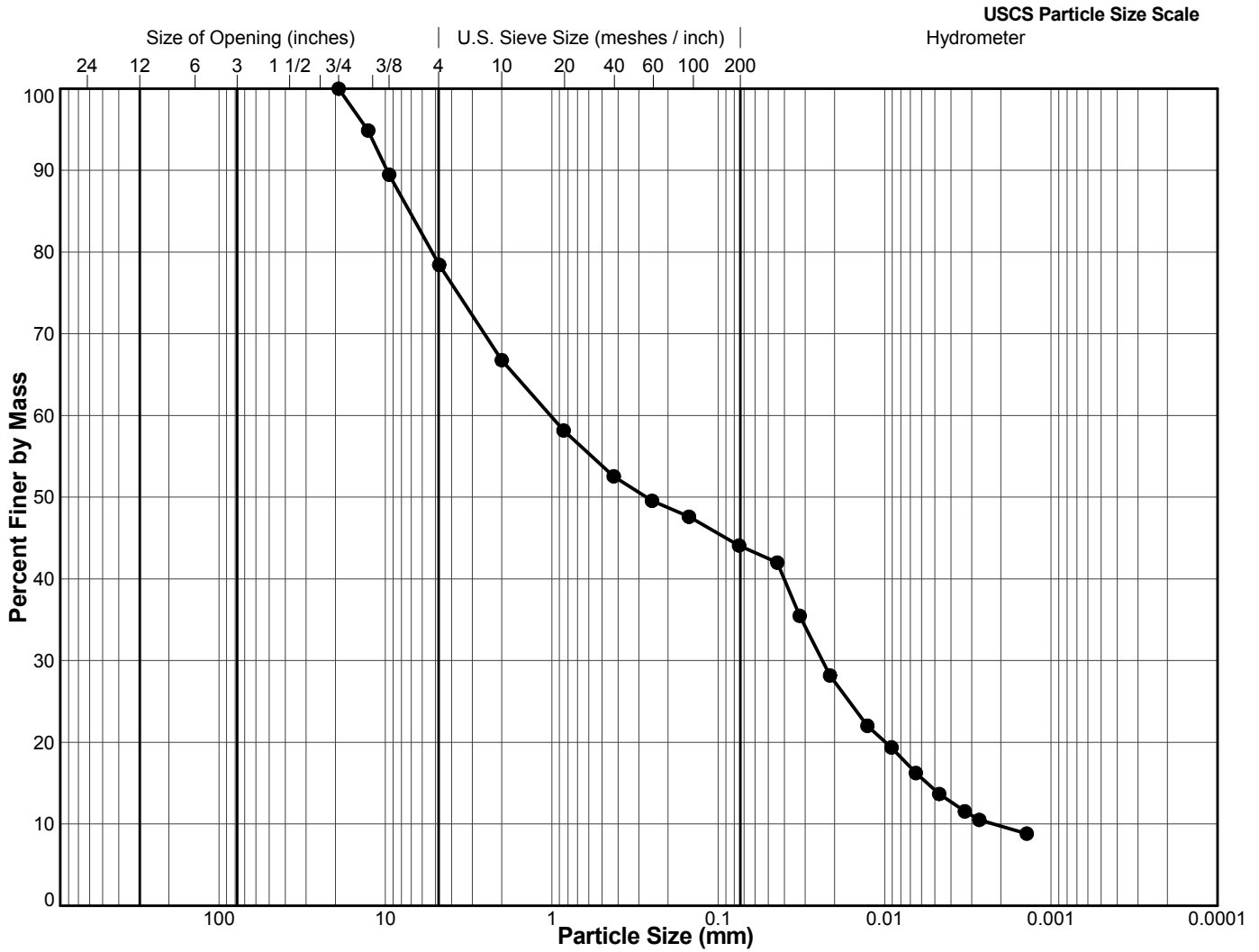
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-94
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.20 to 1.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	27/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

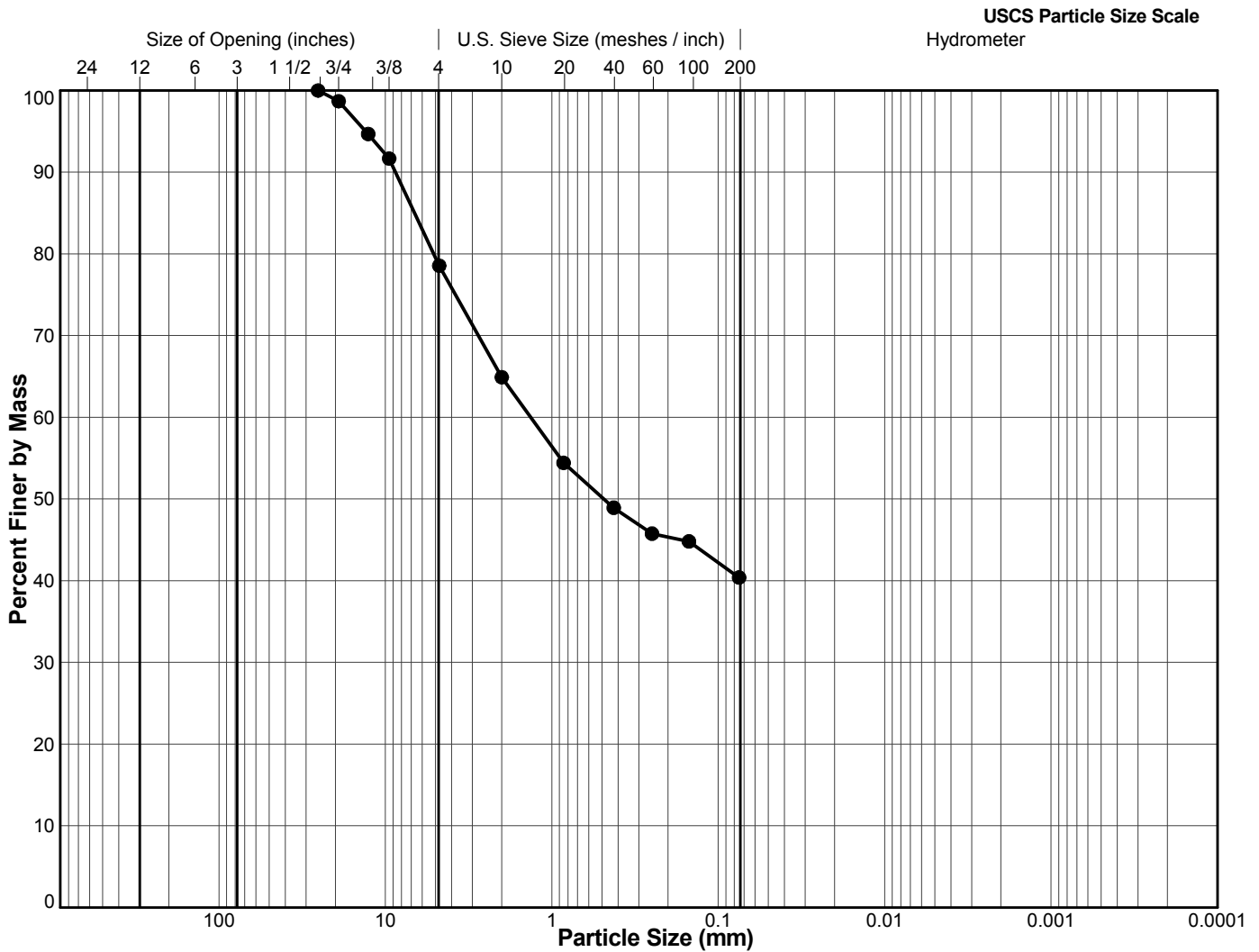
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-94
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.30 to 2.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 25.4	
Method: Split, Washed	



File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

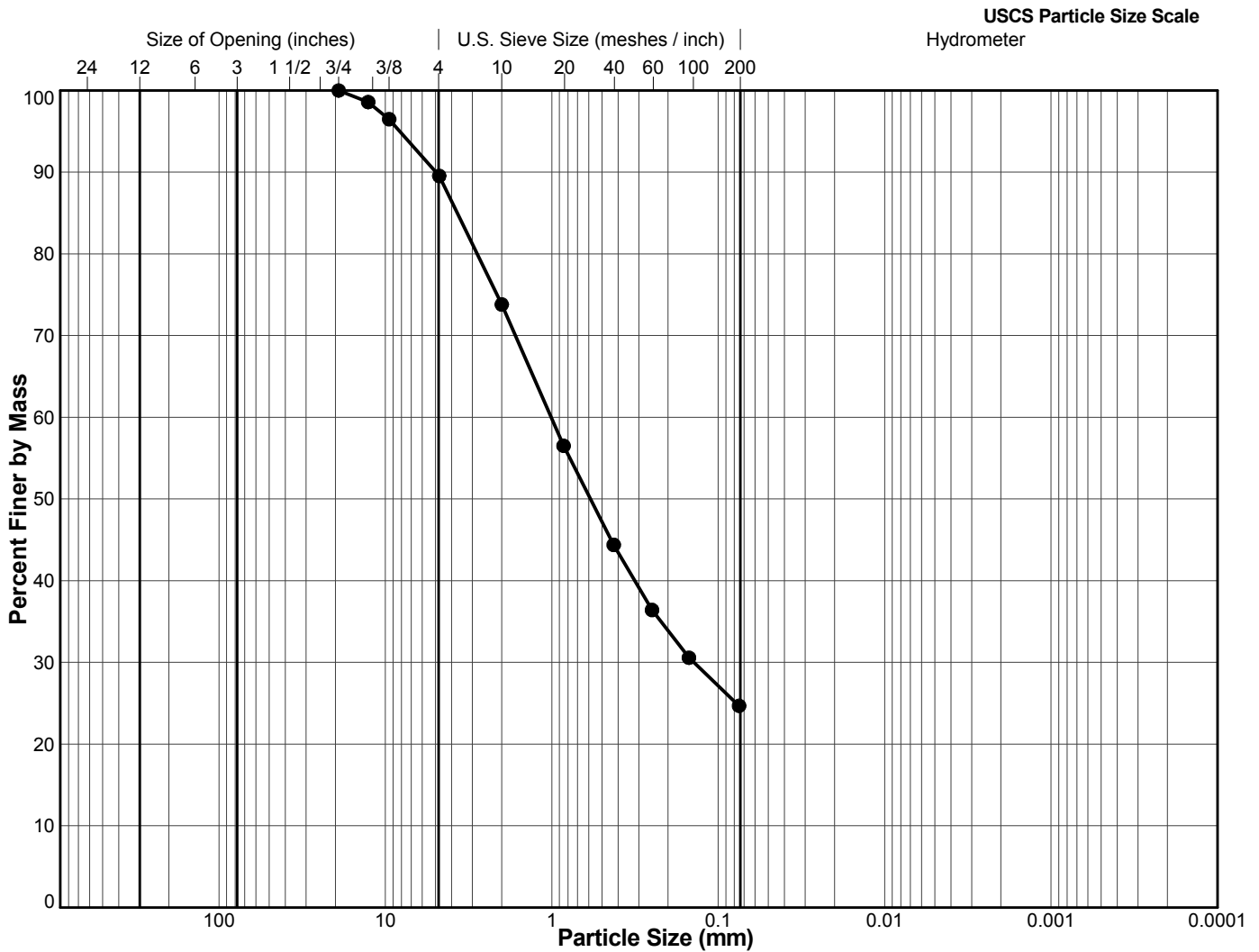
RS	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-95
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.90 to 1.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

SK	01/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

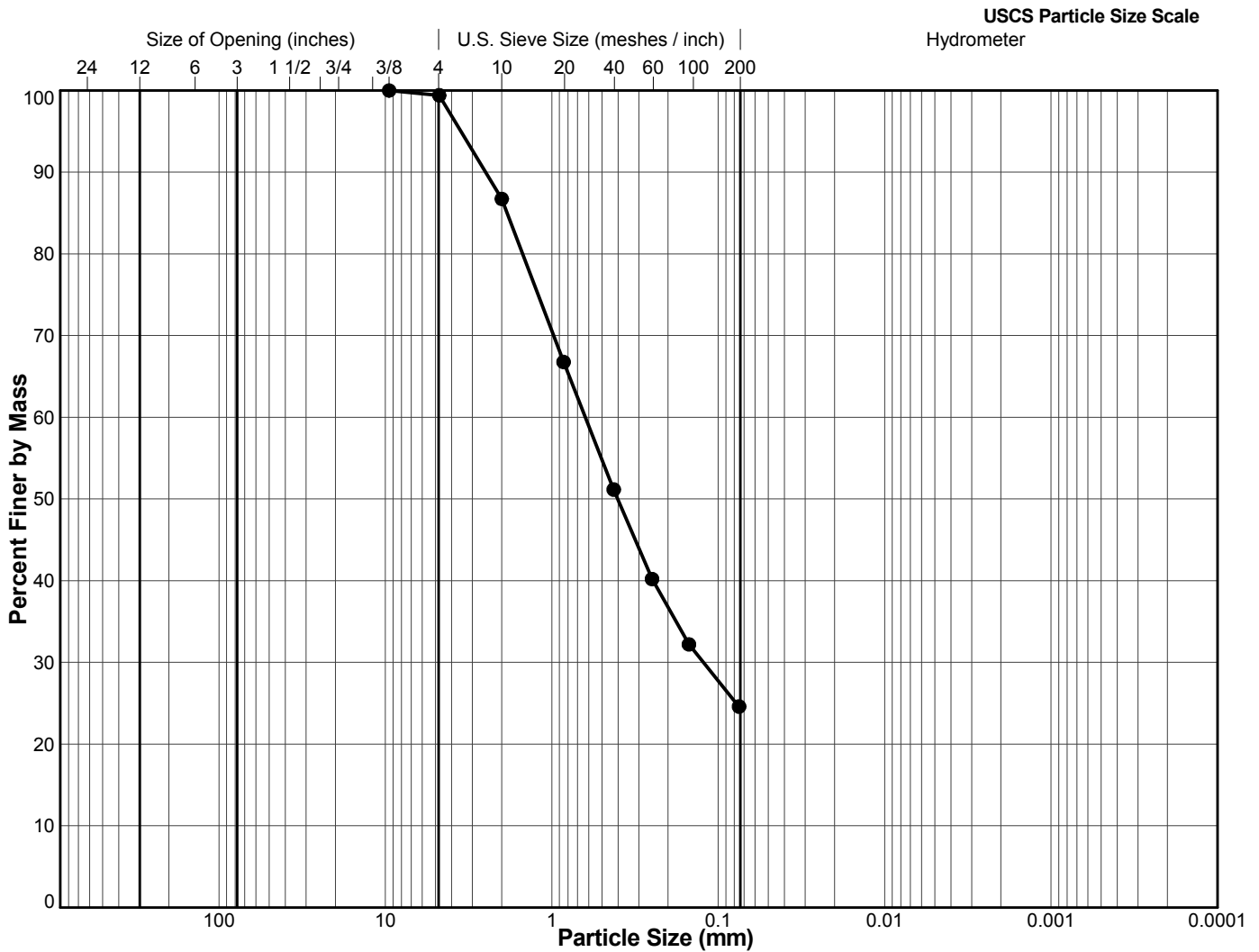
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-95
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.00 to 3.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 9.5	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

SK	01/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

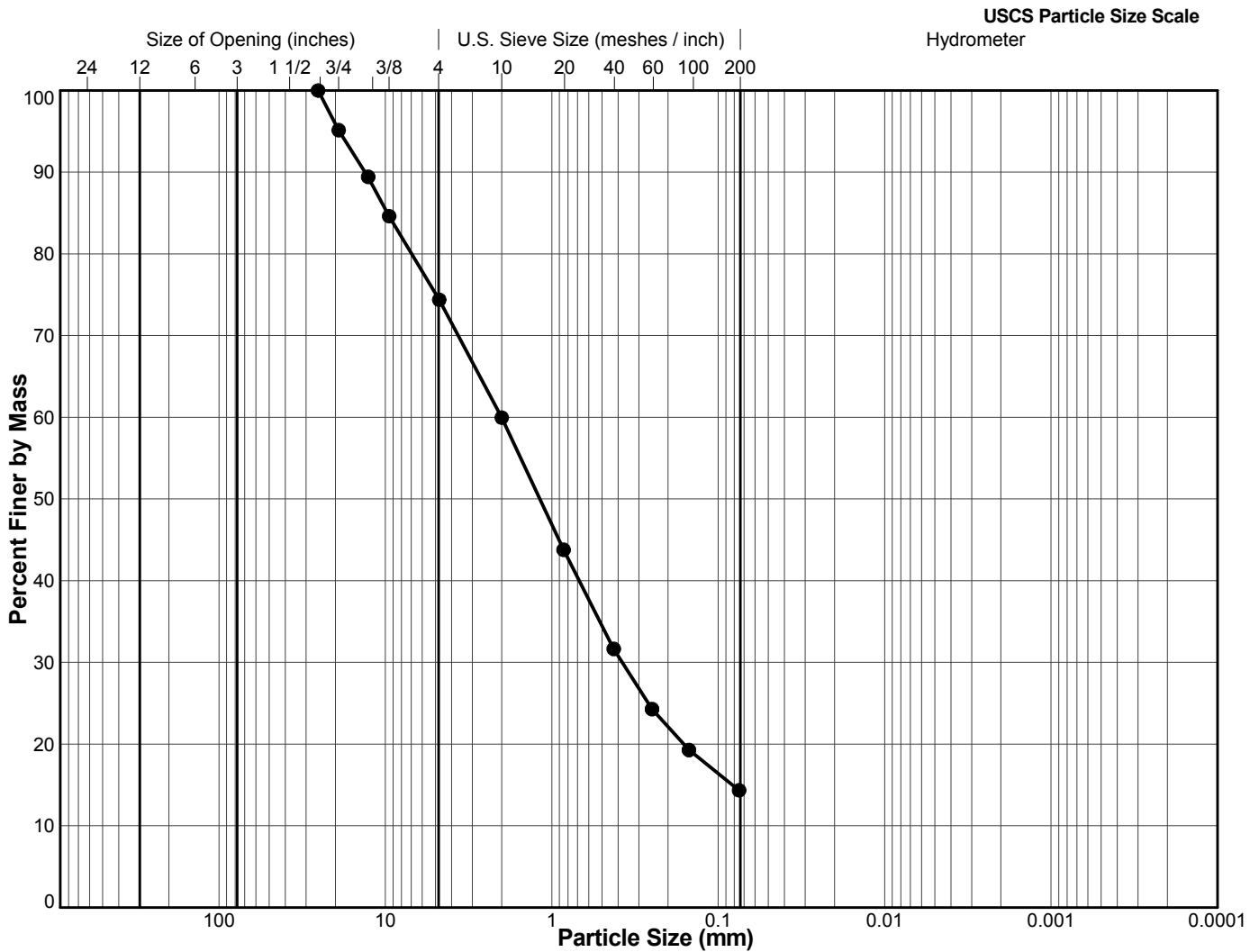
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-96
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 25.4	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	01/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

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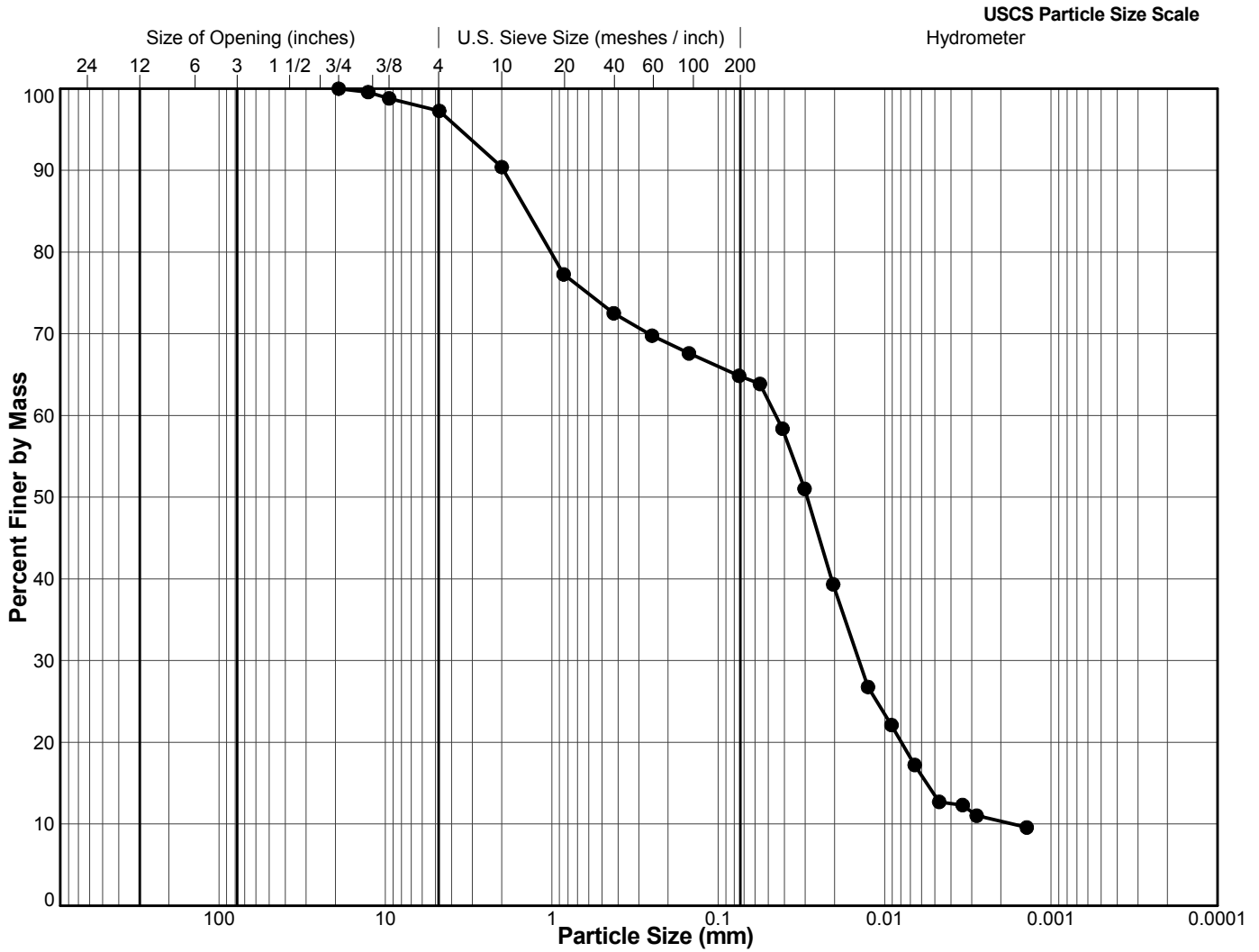
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-96
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 19.1	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

SK	01/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

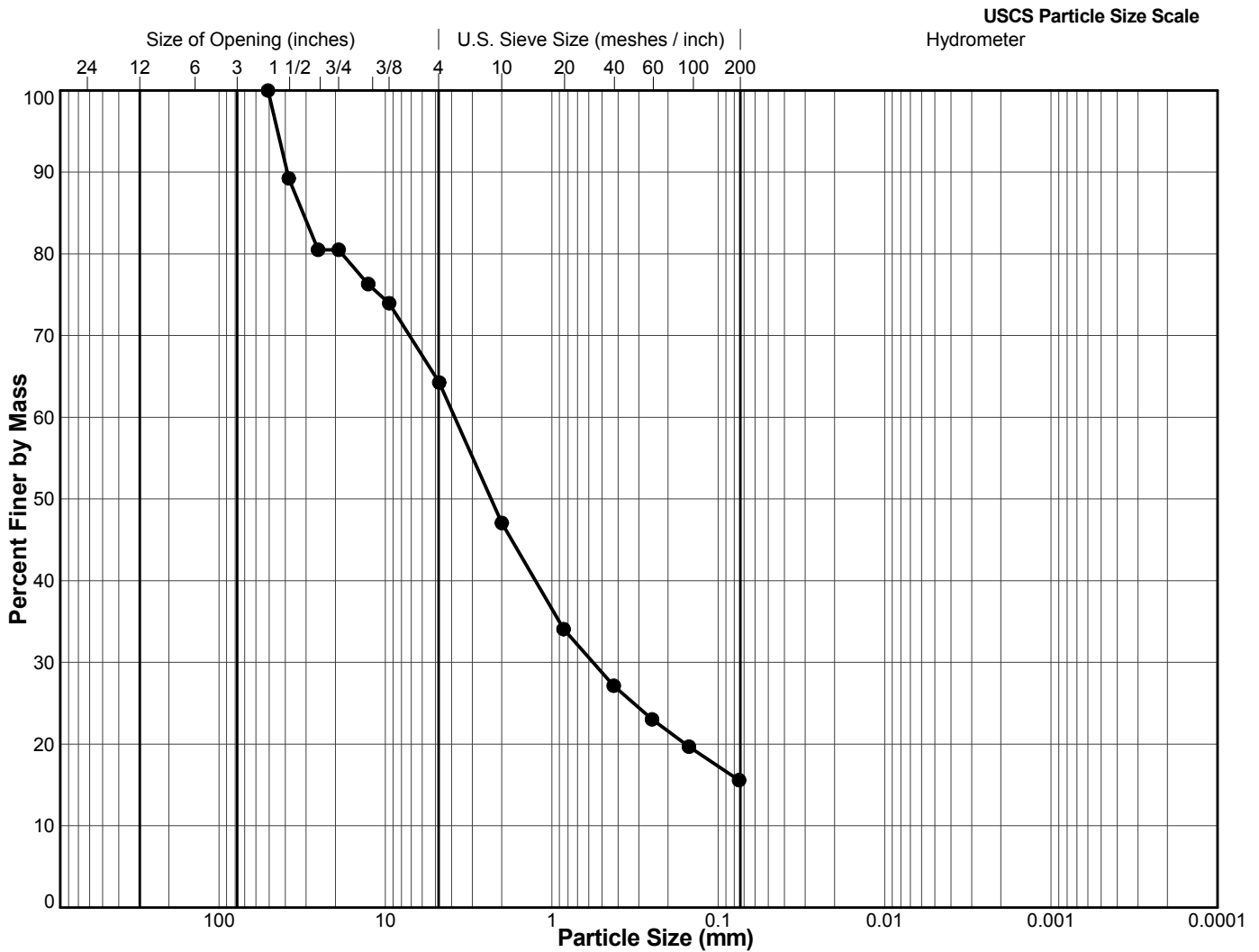
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-96
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 3
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.80 to 2.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Other Remarks: N/A	
Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 50.8	
Method: Split, Washed	



BOULDER	COBBLE	GRAVEL	SAND	FINES (Silt, Clay)
		Coarse Fine	Coarse Medium Fine	

SK	01/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029\EAGLE GOLD.GPJ Output Form: LAB_PARTICLE_SIZE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin_04/11/11



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

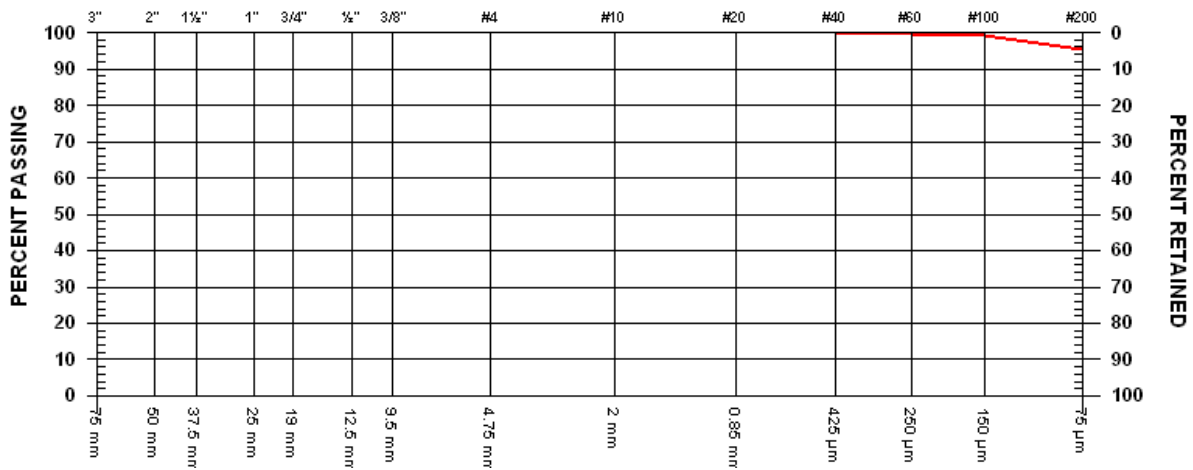
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 26 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39 SAMPLED BY Client
 SOURCE SPT1 @ 1.15 - 1.60m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Sand



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	
1"	25 mm	
3/4"	19 mm	
1/2"	12.5 mm	
3/8"	9.5 mm	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	
No. 10	2.00 mm	100.0
No. 20	850 µm	99.9
No. 40	425 µm	99.9
No. 60	250 µm	99.8
No. 100	150 µm	99.4
No. 200	75 µm	95.4

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

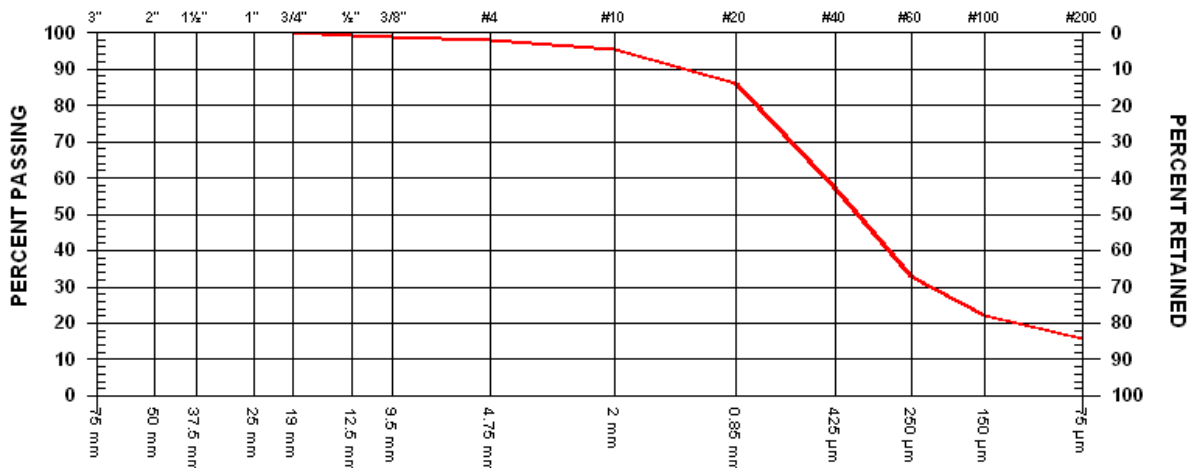
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 35 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39 SAMPLED BY Client
 SOURCE SPT10 @ 8.71 - 9.16m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Sand,gravelly



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	
1"	25 mm	
3/4"	19 mm	100.0
1/2"	12.5 mm	99.1
3/8"	9.5 mm	98.6

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	97.8
No. 10	2.00 mm	95.3
No. 20	850 µm	86.2
No. 40	425 µm	56.9
No. 60	250 µm	32.6
No. 100	150 µm	22.3
No. 200	75 µm	15.6

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

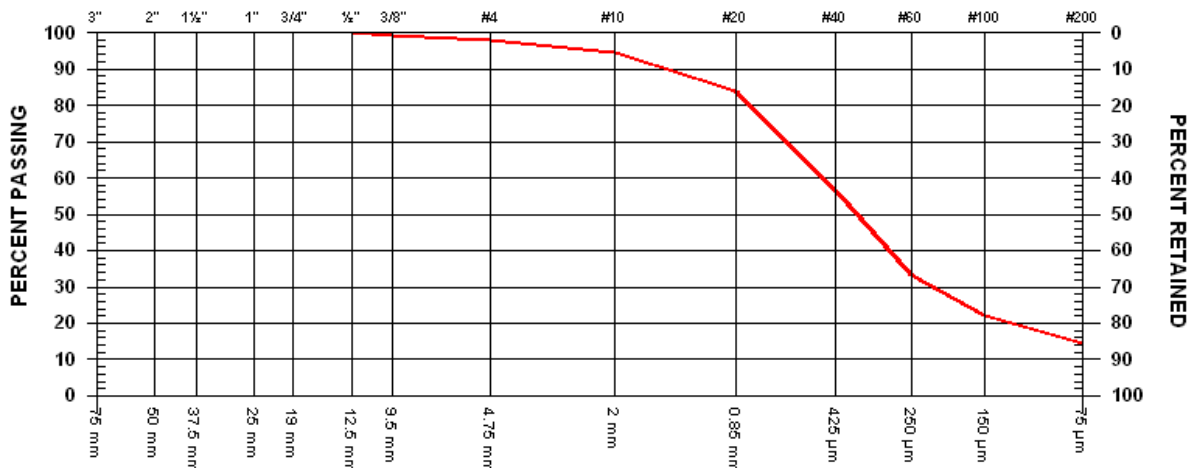
ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing
 Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 36 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39 SAMPLED BY Client
 SOURCE SPT11 @ 9.25 - 9.70m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Sand



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"		
2"		
1 1/2"		
1"		
3/4"		
1/2"	100.0	
3/8"	99.2	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	98.1	
No. 10	94.7	
No. 20	84.0	
No. 40	56.7	
No. 60	33.4	
No. 100	22.0	
No. 200	14.4	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

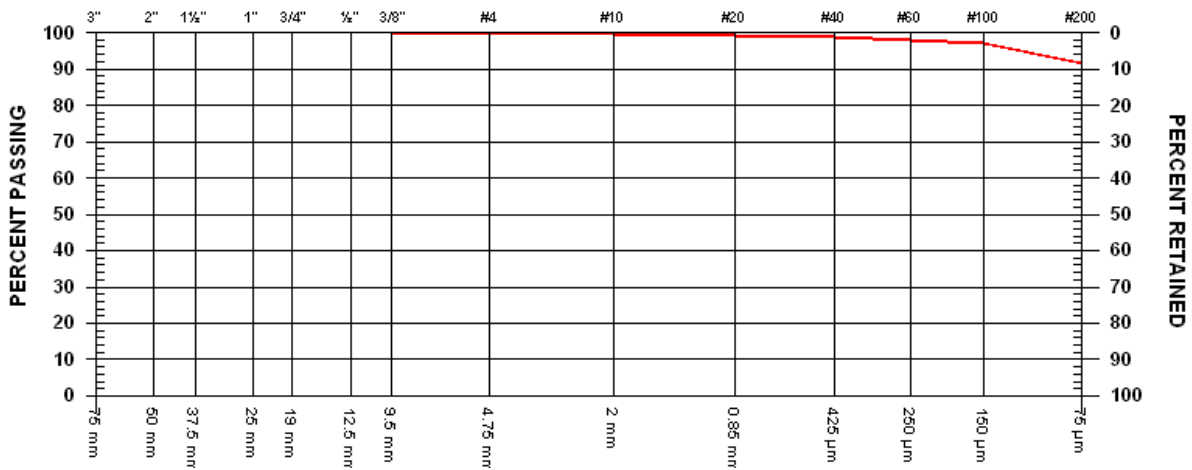
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 27 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39 SAMPLED BY Client
 SOURCE SPT2 @ 1.60 - 2.05m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Sand



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm		
3/4" 19 mm		
1/2" 12.5 mm		
3/8" 9.5 mm	100.0	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	99.8	
No. 10 2.00 mm	99.6	
No. 20 850 µm	99.2	
No. 40 425 µm	98.8	
No. 60 250 µm	98.1	
No. 100 150 µm	97.0	
No. 200 75 µm	91.5	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

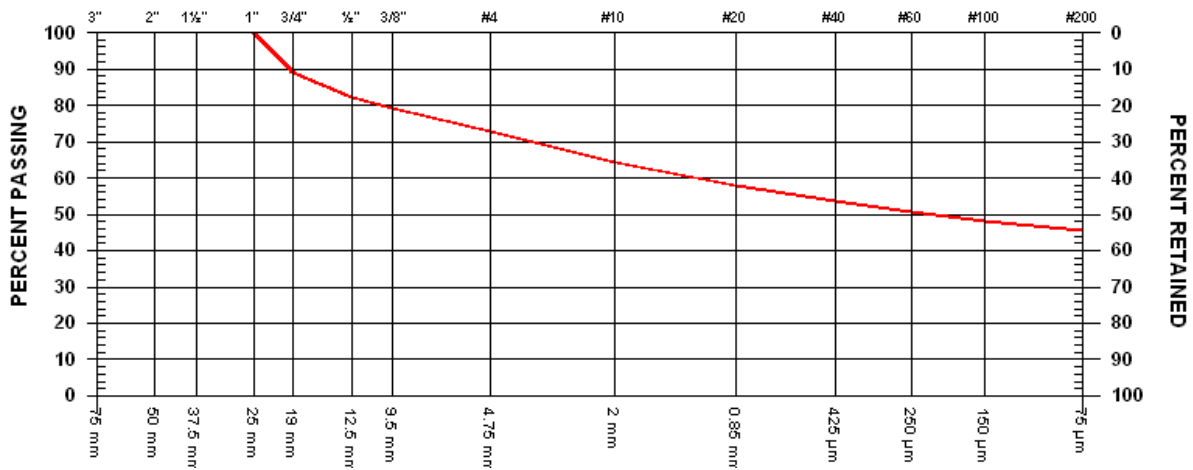
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 28 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39
 SOURCE SPT3 @ 2.28 - 2.73m
 SPECIFICATION
 MATERIAL TYPE Gravel, sandy

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm	100.0	
3/4" 19 mm	88.9	
1/2" 12.5 mm	82.3	
3/8" 9.5 mm	79.2	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	72.7	
No. 10 2.00 mm	64.3	
No. 20 850 µm	57.9	
No. 40 425 µm	53.5	
No. 60 250 µm	50.5	
No. 100 150 µm	48.3	
No. 200 75 µm	45.4	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

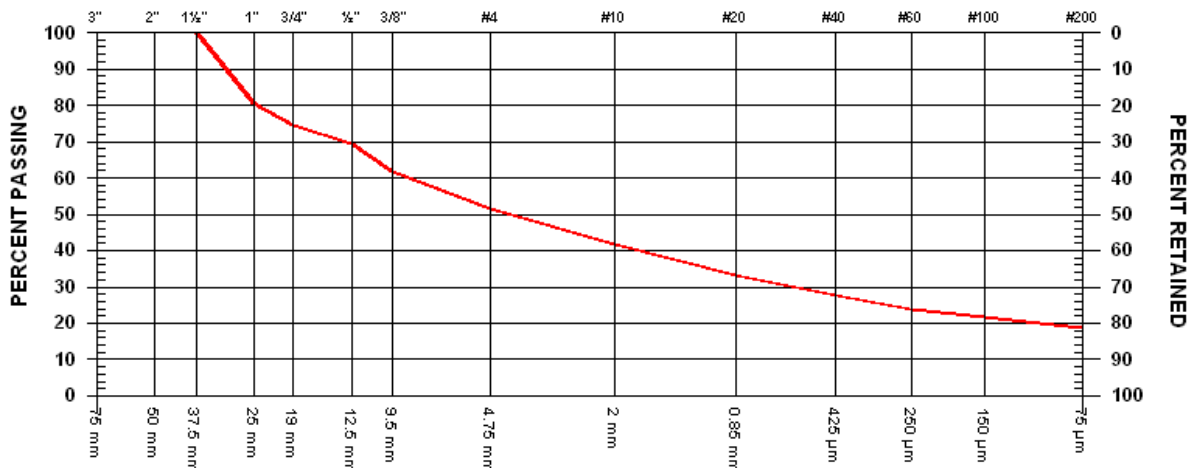
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 29 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39
 SOURCE SPT4 @ 2.75 - 3.20m
 SPECIFICATION
 MATERIAL TYPE Gravel, sandy

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	80.5	
3/4" 19 mm	74.5	
1/2" 12.5 mm	69.4	
3/8" 9.5 mm	61.6	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	51.6	
No. 10 2.00 mm	41.7	
No. 20 850 µm	33.2	
No. 40 425 µm	27.6	
No. 60 250 µm	24.0	
No. 100 150 µm	21.7	
No. 200 75 µm	18.7	

COMMENTS

GeoNorth Engineering Ltd.

3975 18th Avenue Prince George, BC V2N 1B2
 Phone (250)564-4304; Fax (250)564-9323



SIEVE ANALYSIS REPORT 10 20 40 60 SERIES

PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

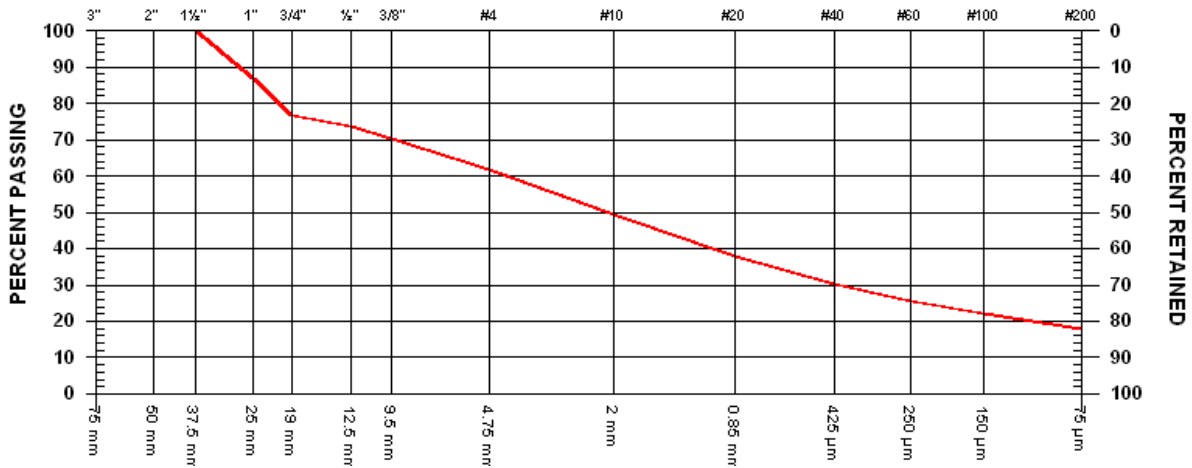
ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing
 Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 30 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER	BH-BGC11-39	SAMPLED BY	Client
SOURCE	SPT5 @ 4.6 - 5.05m	TESTED BY	DJ
SPECIFICATION		TEST METHOD	WASHED
MATERIAL TYPE	Gravel, sandy		



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	86.9	
3/4" 19 mm	76.7	
1/2" 12.5 mm	73.7	
3/8" 9.5 mm	70.2	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	61.6	
No. 10 2.00 mm	49.2	
No. 20 850 µm	37.9	
No. 40 425 µm	30.4	
No. 60 250 µm	25.7	
No. 100 150 µm	22.2	
No. 200 75 µm	17.7	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

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 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

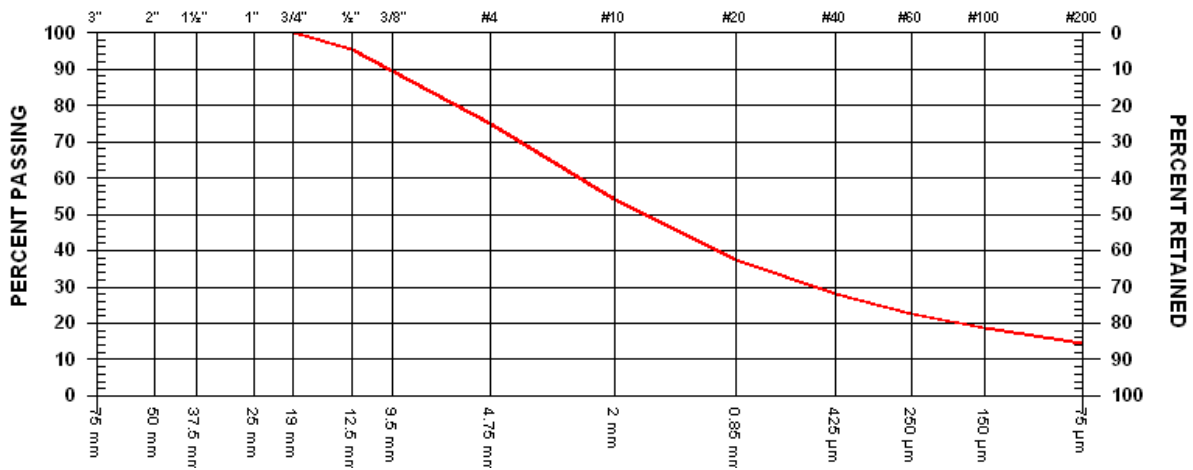
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 31 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER	BH-BGC11-39	SAMPLED BY	Client
SOURCE	SPT6 @ 5.53 - 5.98m	TESTED BY	DJ
SPECIFICATION		TEST METHOD	WASHED
MATERIAL TYPE	Silt		



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	
1"	25 mm	
3/4"	19 mm	100.0
1/2"	12.5 mm	95.5
3/8"	9.5 mm	89.6

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	75.1
No. 10	2.00 mm	54.2
No. 20	850 µm	37.6
No. 40	425 µm	27.9
No. 60	250 µm	22.5
No. 100	150 µm	18.8
No. 200	75 µm	14.5

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

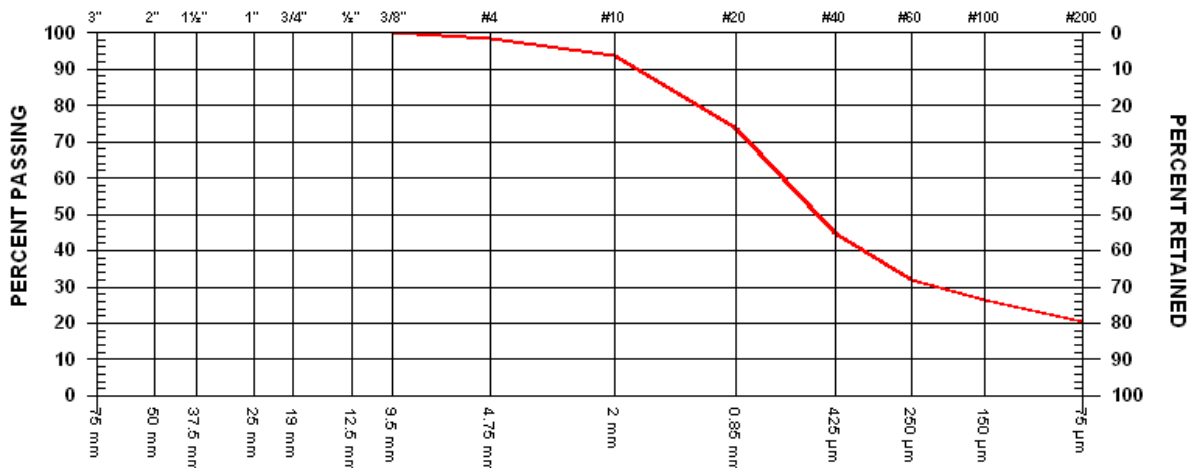
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 32 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER	BH-BGC11-39	SAMPLED BY	Client
SOURCE	SPT7 @ 6.22 - 6.67m	TESTED BY	DJ
SPECIFICATION		TEST METHOD	WASHED
MATERIAL TYPE	Sand		



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	
1"	25 mm	
3/4"	19 mm	
1/2"	12.5 mm	
3/8"	9.5 mm	100.0

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	98.3
No. 10	2.00 mm	93.5
No. 20	850 µm	73.6
No. 40	425 µm	44.5
No. 60	250 µm	31.9
No. 100	150 µm	26.3
No. 200	75 µm	20.5

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
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 BGC Engineering Inc.
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 V6Z 2A9

ATTN: Peter Quinn

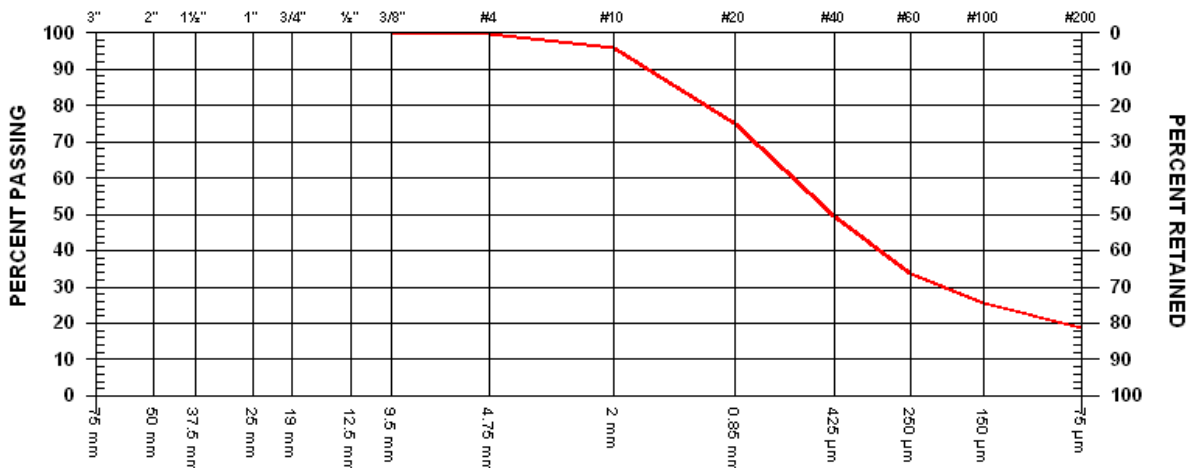
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 33 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39 SAMPLED BY Client
 SOURCE SPT8 @ 7.16 - 7.61m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Sand



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm		
3/4" 19 mm		
1/2" 12.5 mm		
3/8" 9.5 mm	100.0	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	99.7	
No. 10 2.00 mm	96.0	
No. 20 850 µm	74.9	
No. 40 425 µm	49.4	
No. 60 250 µm	33.5	
No. 100 150 µm	25.5	
No. 200 75 µm	18.9	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
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 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

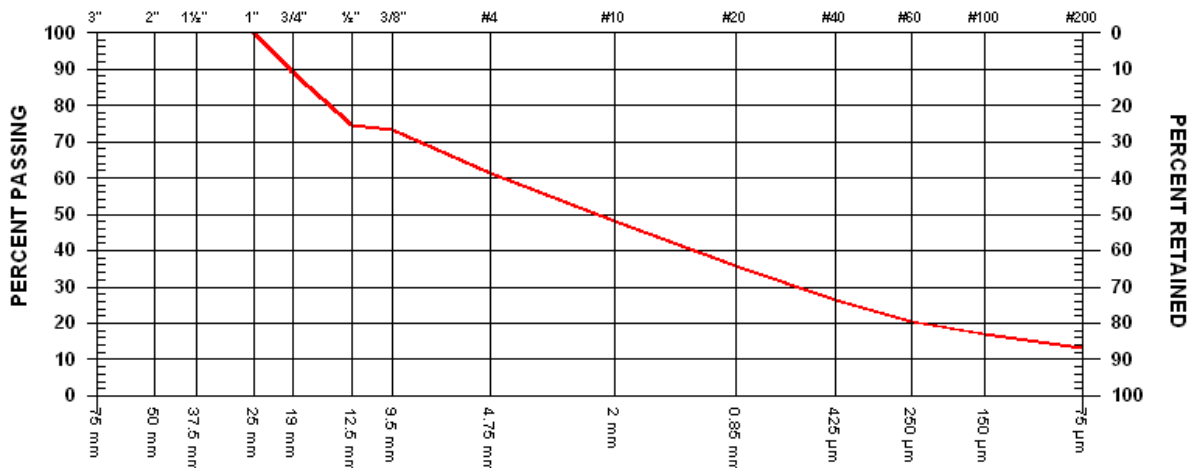
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 34 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Jul.27

SUPPLIER BH-BGC11-39
 SOURCE SPT9 @ 7.75 - 8.20m
 SPECIFICATION
 MATERIAL TYPE Sand, gravelly

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED

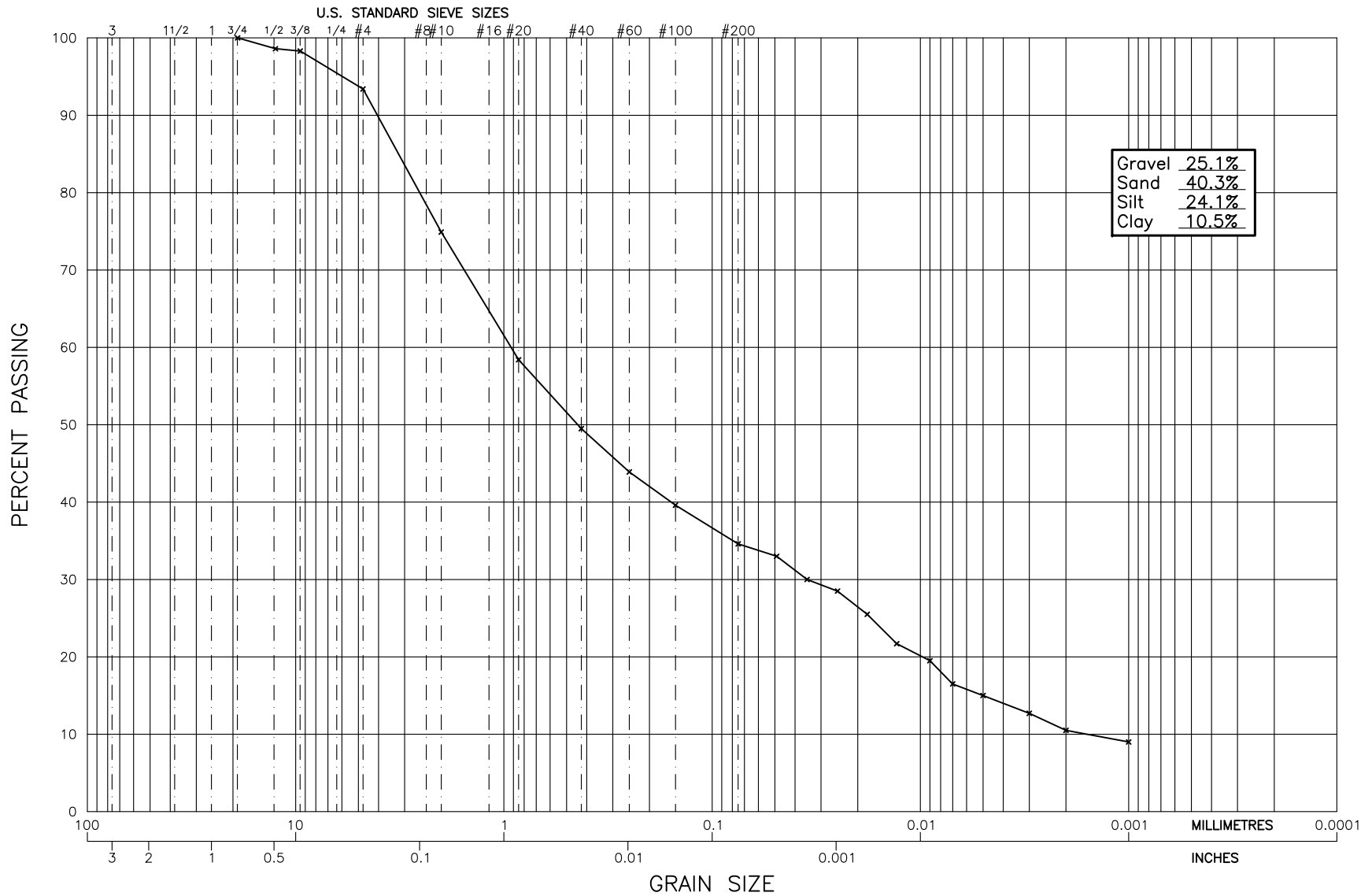


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm	100.0	
3/4" 19 mm	88.9	
1/2" 12.5 mm	74.7	
3/8" 9.5 mm	73.4	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	61.4	
No. 10 2.00 mm	48.3	
No. 20 850 µm	35.6	
No. 40 425 µm	26.2	
No. 60 250 µm	20.5	
No. 100 150 µm	16.9	
No. 200 75 µm	13.0	

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-42-S2

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH42-S2

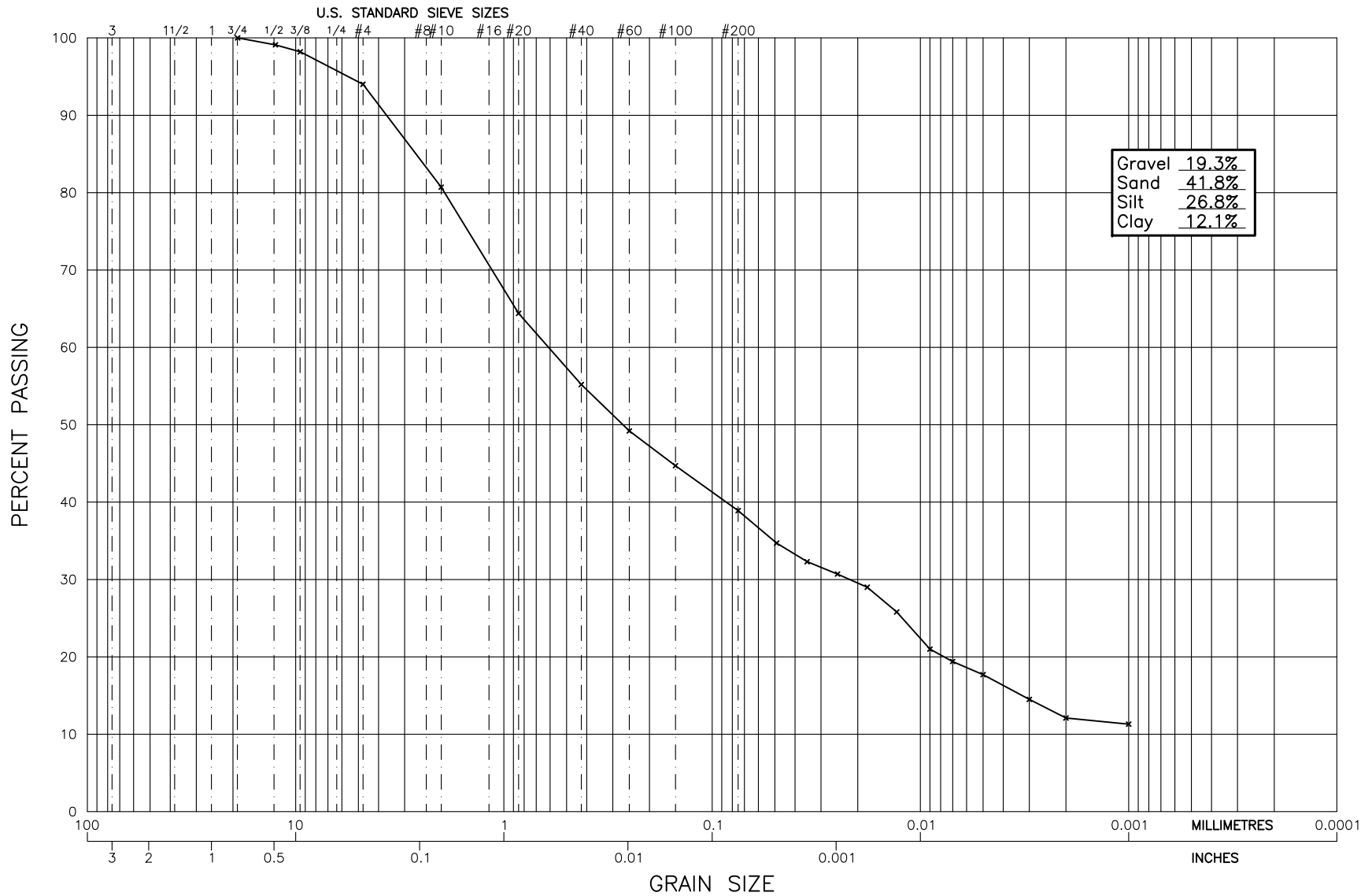
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 8, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand, gravelly, silty, trace clay				
Sample #: 2		Test #:		Hole #: BH-BGC11-42		Depth: 1.8 - 2.27m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: July 29, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.7, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	74.9	
Wet Wt. & Tare	1350.5	25.4				20	11.0	39.0	78.0	58.4	
Dry Wt. & Tare	1180.8	19.0		984.0	100.0	40	6.0	33.0	66.0	49.5	
Water Wt.	169.7	12.5	14.2	969.8	98.6	60	3.7	29.3	58.6	43.9	
Tare Wt.	196.8	9.5	2.1	967.7	98.3	100	2.9	26.4	52.8	39.6	
Wt. Of Dry Soil	984.0	4.75	48.3	919.4	93.4	200	3.3	23.1	46.2	34.6	
Moisture Content %	17.2	10	182.1	737.3	74.9	Pan	23.1				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	984.0			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.749	0.5	30.0	20.0	0.01365	23.0	12.5	5.000	0.068	46.0	34.5
50.0	0.749	1	29.0	20.0	0.01365	22.0	12.7	3.559	0.049	44.0	33.0
50.0	0.749	2	27.0	20.0	0.01365	20.0	13.0	2.549	0.035	40.0	30.0
50.0	0.749	4	26.0	20.0	0.01365	19.0	13.2	1.814	0.025	38.0	28.5
50.0	0.749	8	24.0	20.0	0.01365	17.0	13.5	1.299	0.018	34.0	25.5
50.0	0.749	15	21.5	20.0	0.01365	14.5	13.9	0.963	0.013	29.0	21.7
50.0	0.749	30	20.0	20.0	0.01365	13.0	14.2	0.687	0.009	26.0	19.5
50.0	0.749	60	18.0	20.0	0.01365	11.0	14.5	0.491	0.007	22.0	16.5
50.0	0.749	120	17.0	21.0	0.01348	10.0	14.6	0.349	0.005	20.0	15.0
50.0	0.749	240	15.5	21.0	0.01348	8.5	14.9	0.249	0.003	17.0	12.7
50.0	0.749	480	14.0	21.0	0.01348	7.0	15.1	0.178	0.002	14.0	10.5
50.0	0.749	1440	13.0	21.0	0.01348	6.0	15.3	0.103	0.001	12.0	9.0
Hydrometer #: 932452			Graduate #: 5			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	19.3%
Sand	41.8%
Silt	26.8%
Clay	12.1%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-42-S5

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH42-S5

GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 8, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand,silty,some gravel and clay				
Sample #: 5		Test #:		Hole #: BH-BGC11-42		Depth: 6.4 - 6.84m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: July 29, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.7, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	80.7	
Wet Wt. & Tare	1265.1	25.4				20	10.1	39.9	79.8	64.4	
Dry Wt. & Tare	1136.5	19.0		945.4	100.0	40	5.7	34.2	68.4	55.2	
Water Wt.	128.6	12.5	8.4	937.0	99.1	60	3.7	30.5	61.0	49.2	
Tare Wt.	191.1	9.5	8.7	928.3	98.2	100	2.8	27.7	55.4	44.7	
Wt. Of Dry Soil	945.4	4.75	39.3	889.0	94.0	200	3.6	24.1	48.2	38.9	
Moisture Content %	13.6	10	126.3	762.7	80.7	Pan	24.1				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	945.4			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.807	0.5	31.0	20.0	0.01365	24.0	12.3	4.967	0.068	48.0	38.7
50.0	0.807	1	28.5	20.0	0.01365	21.5	12.7	3.570	0.049	43.0	34.7
50.0	0.807	2	27.0	20.0	0.01365	20.0	13.0	2.549	0.035	40.0	32.3
50.0	0.807	4	26.0	20.0	0.01365	19.0	13.2	1.814	0.025	38.0	30.7
50.0	0.807	8	25.0	20.0	0.01365	18.0	13.3	1.291	0.018	36.0	29.0
50.0	0.807	15	23.0	20.0	0.01365	16.0	13.7	0.954	0.013	32.0	25.8
50.0	0.807	30	20.0	20.0	0.01365	13.0	14.2	0.687	0.009	26.0	21.0
50.0	0.807	60	19.0	20.0	0.01365	12.0	14.3	0.488	0.007	24.0	19.4
50.0	0.807	120	18.0	21.0	0.01348	11.0	14.5	0.347	0.005	22.0	17.7
50.0	0.807	240	16.0	21.0	0.01348	9.0	14.8	0.248	0.003	18.0	14.5
50.0	0.807	480	14.5	21.0	0.01348	7.5	15.1	0.177	0.002	15.0	12.1
50.0	0.807	1440	14.0	21.0	0.01348	7.0	15.1	0.103	0.001	14.0	11.3
Hydrometer #: 932452			Graduate #: 4			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

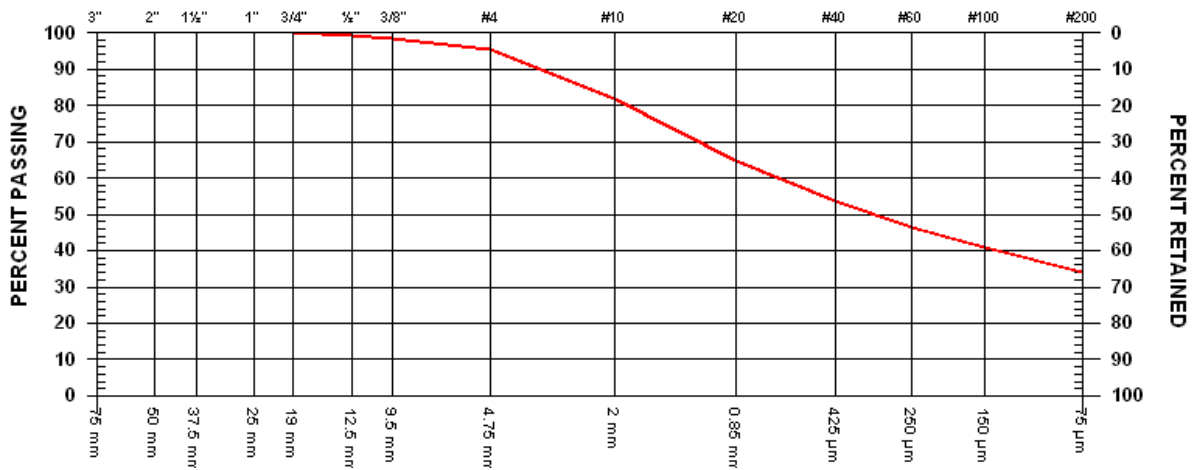
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 8 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.02 DATE SAMPLED 2011.Jul.07

SUPPLIER BH-BGC11-42 SAMPLED BY Client
 SOURCE SA#7 @ 9.4 - 9.88m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	
1"	25 mm	
3/4"	19 mm	100.0
1/2"	12.5 mm	99.3
3/8"	9.5 mm	98.5

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	95.2
No. 10	2.00 mm	81.7
No. 20	850 µm	64.9
No. 40	425 µm	53.7
No. 60	250 µm	46.3
No. 100	150 µm	40.8
No. 200	75 µm	33.9

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

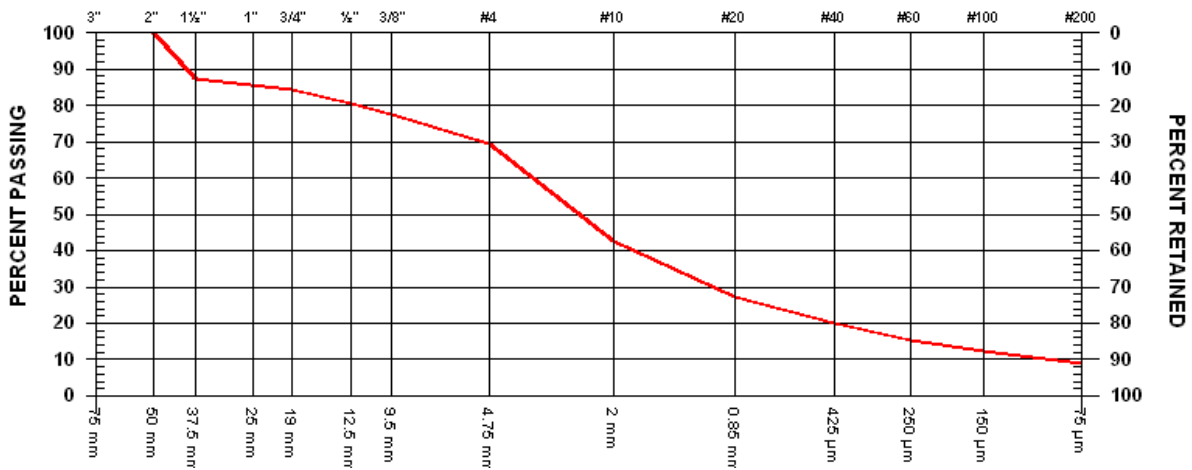
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 9 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.02 DATE SAMPLED 2011.Jul.30

SUPPLIER BH-BGC11-42
 SOURCE SA#12 @ 13.3 - 13.65m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED

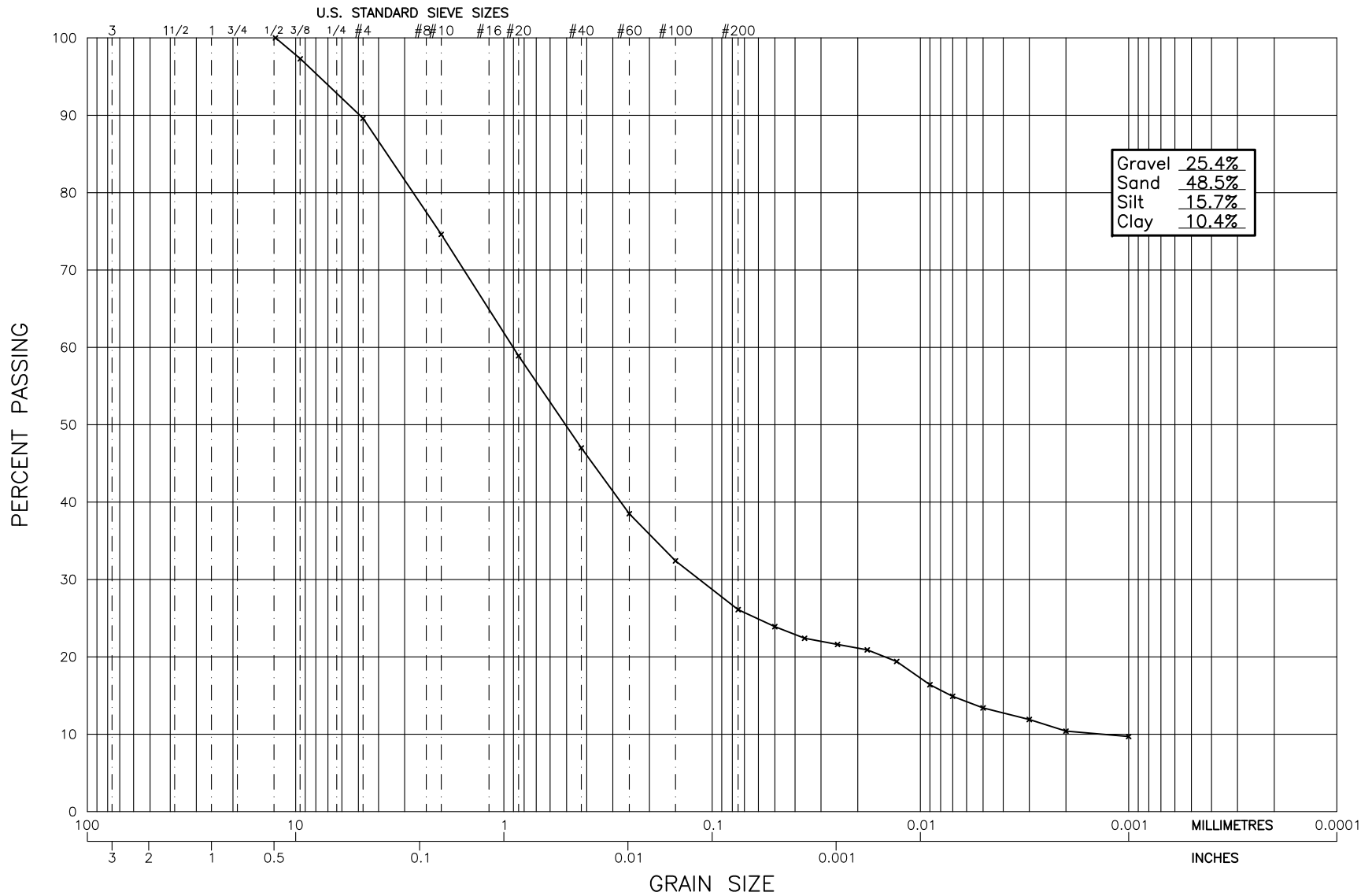


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm	100.0	
1 1/2" 37.5 mm	87.1	
1" 25 mm	84.1	
3/4" 19 mm	80.5	
1/2" 12.5 mm	77.6	
3/8" 9.5 mm		

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	69.2	
No. 10 2.00 mm	42.5	
No. 20 850 µm	27.1	
No. 40 425 µm	19.8	
No. 60 250 µm	15.4	
No. 100 150 µm	12.3	
No. 200 75 µm	8.8	

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	25.4%
Sand	48.5%
Silt	15.7%
Clay	10.4%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-42-S20

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH42-S20

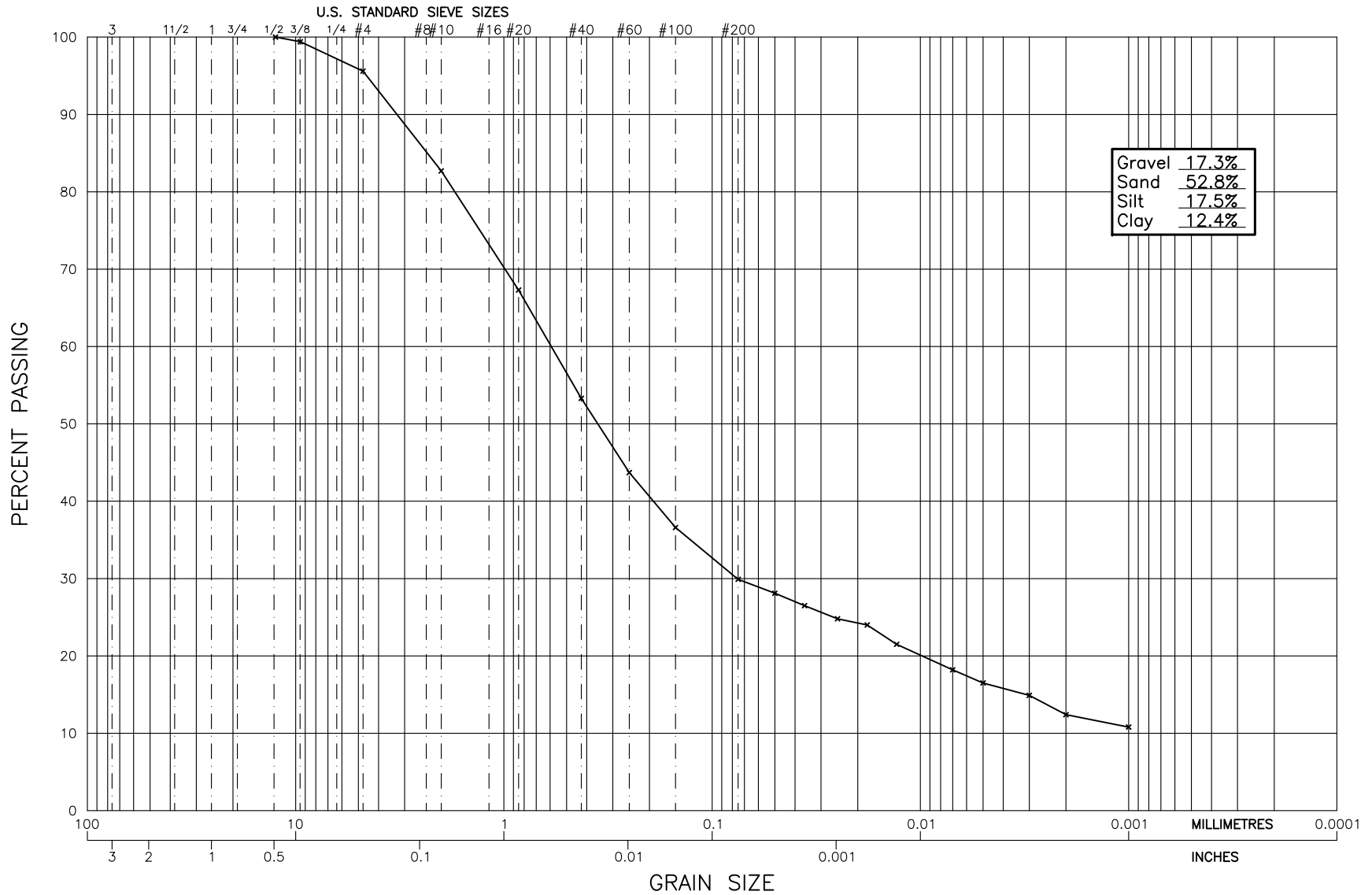
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 8, 2011					
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300					
Source/Location: Dublin Gulch, Yukon							Type: Sand,gravelly,some silt and clay					
Sample #: 20		Test #:		Hole #: BH-BGC11-42		Depth: 19.7 - 20.0m		Time:				
Sampled By: Client				Tested By: DJ				Checked By: DJ				
Date Sampled: July 30, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.8, 2011				
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis					
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.			38.1				10		50.0	100.0	74.6	
Wet Wt. & Tare	597.3		25.4				20	10.5	39.5	79.0	58.9	
Dry Wt. & Tare	532.7		19.0				40	8.0	31.5	63.0	47.0	
Water Wt.	64.6		12.5		353.5	100.0	60	5.7	25.8	51.6	38.5	
Tare Wt.	179.2		9.5	9.6	343.9	97.3	100	4.1	21.7	43.4	32.4	
Wt. Of Dry Soil	353.5		4.75	27.1	316.8	89.6	200	4.2	17.5	35.0	26.1	
Moisture Content %	18.3		10	53.2	263.6	74.6	Pan	17.5				
Dry Wt. Of Sample from Initial Moisture							Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=					
			Total	353.5			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)	
50.0	0.746	0.5	24.5	20.0	0.01365	17.5	13.4	5.178	0.071	35.0	26.1	
50.0	0.746	1	23.0	20.0	0.01365	16.0	13.7	3.695	0.050	32.0	23.9	
50.0	0.746	2	22.0	20.0	0.01365	15.0	13.8	2.629	0.036	30.0	22.4	
50.0	0.746	4	21.5	20.0	0.01365	14.5	13.9	1.864	0.025	29.0	21.6	
50.0	0.746	8	21.0	20.0	0.01365	14.0	14.0	1.322	0.018	28.0	20.9	
50.0	0.746	15	20.0	20.0	0.01365	13.0	14.2	0.971	0.013	26.0	19.4	
50.0	0.746	30	18.0	20.0	0.01365	11.0	14.5	0.695	0.009	22.0	16.4	
50.0	0.746	60	17.0	20.0	0.01365	10.0	14.6	0.494	0.007	20.0	14.9	
50.0	0.746	120	16.0	21.0	0.01348	9.0	14.8	0.351	0.005	18.0	13.4	
50.0	0.746	240	15.0	21.0	0.01348	8.0	15.0	0.250	0.003	16.0	11.9	
50.0	0.746	480	14.0	21.0	0.01348	7.0	15.1	0.178	0.002	14.0	10.4	
50.0	0.746	1440	13.5	21.0	0.01348	6.5	15.2	0.103	0.001	13.0	9.7	
Hydrometer #: 932452			Graduate #: 3			Dispersing Agent: Sodium Hex			Amount: 125ml			
Density of Solids:												
Description of Sample:												

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-42-S22

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH42-S22

GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 8, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand, some gravel,silt,clay				
Sample #: 22		Test #:		Hole #: BH-BGC11-42		Depth: 21.55 - 21.90m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: July 30, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.7, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	82.7	
Wet Wt. & Tare	653.2	25.4				20	9.3	40.7	81.4	67.3	
Dry Wt. & Tare	576.8	19.0				40	8.5	32.2	64.4	53.3	
Water Wt.	76.4	12.5		395.6	100.0	60	5.8	26.4	52.8	43.7	
Tare Wt.	181.2	9.5	2.5	393.1	99.4	100	4.3	22.1	44.2	36.6	
Wt. Of Dry Soil	395.6	4.75	14.8	378.3	95.6	200	4.0	18.1	36.2	29.9	
Moisture Content %	19.3	10	51.1	327.2	82.7	Pan	18.1				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	395.6			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.827	0.5	25.0	20.0	0.01365	18.0	13.3	5.162	0.070	36.0	29.8
50.0	0.827	1	24.0	20.0	0.01365	17.0	13.5	3.673	0.050	34.0	28.1
50.0	0.827	2	23.0	20.0	0.01365	16.0	13.7	2.613	0.036	32.0	26.5
50.0	0.827	4	22.0	20.0	0.01365	15.0	13.8	1.859	0.025	30.0	24.8
50.0	0.827	8	21.5	20.0	0.01365	14.5	13.9	1.318	0.018	29.0	24.0
50.0	0.827	15	20.0	20.0	0.01365	13.0	14.2	0.971	0.013	26.0	21.5
50.0	0.827	30	20.0	20.0	0.01365	13.0	14.2	0.687	0.009	26.0	21.5
50.0	0.827	60	18.0	20.0	0.01365	11.0	14.5	0.491	0.007	22.0	18.2
50.0	0.827	120	17.0	21.0	0.01348	10.0	14.6	0.349	0.005	20.0	16.5
50.0	0.827	240	16.0	21.0	0.01348	9.0	14.8	0.248	0.003	18.0	14.9
50.0	0.827	480	14.5	21.0	0.01348	7.5	15.1	0.177	0.002	15.0	12.4
50.0	0.827	1440	13.5	21.0	0.01348	6.5	15.2	0.103	0.001	13.0	10.8
Hydrometer #: 932452			Graduate #: 2			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

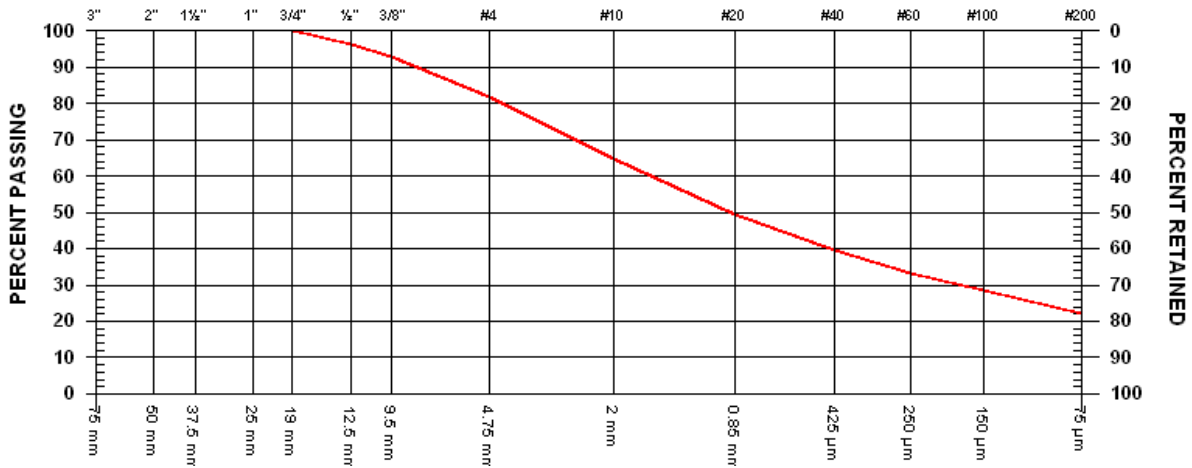
ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing
 Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 10 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.02 DATE SAMPLED 2011.Jul.30

SUPPLIER BH-BGC11-42 SAMPLED BY Client
 SOURCE SA#26 @ 27.60 - 28.0m TESTED BY DJ
 SPECIFICATION MATERIAL TYPE Gravel TEST METHOD WASHED

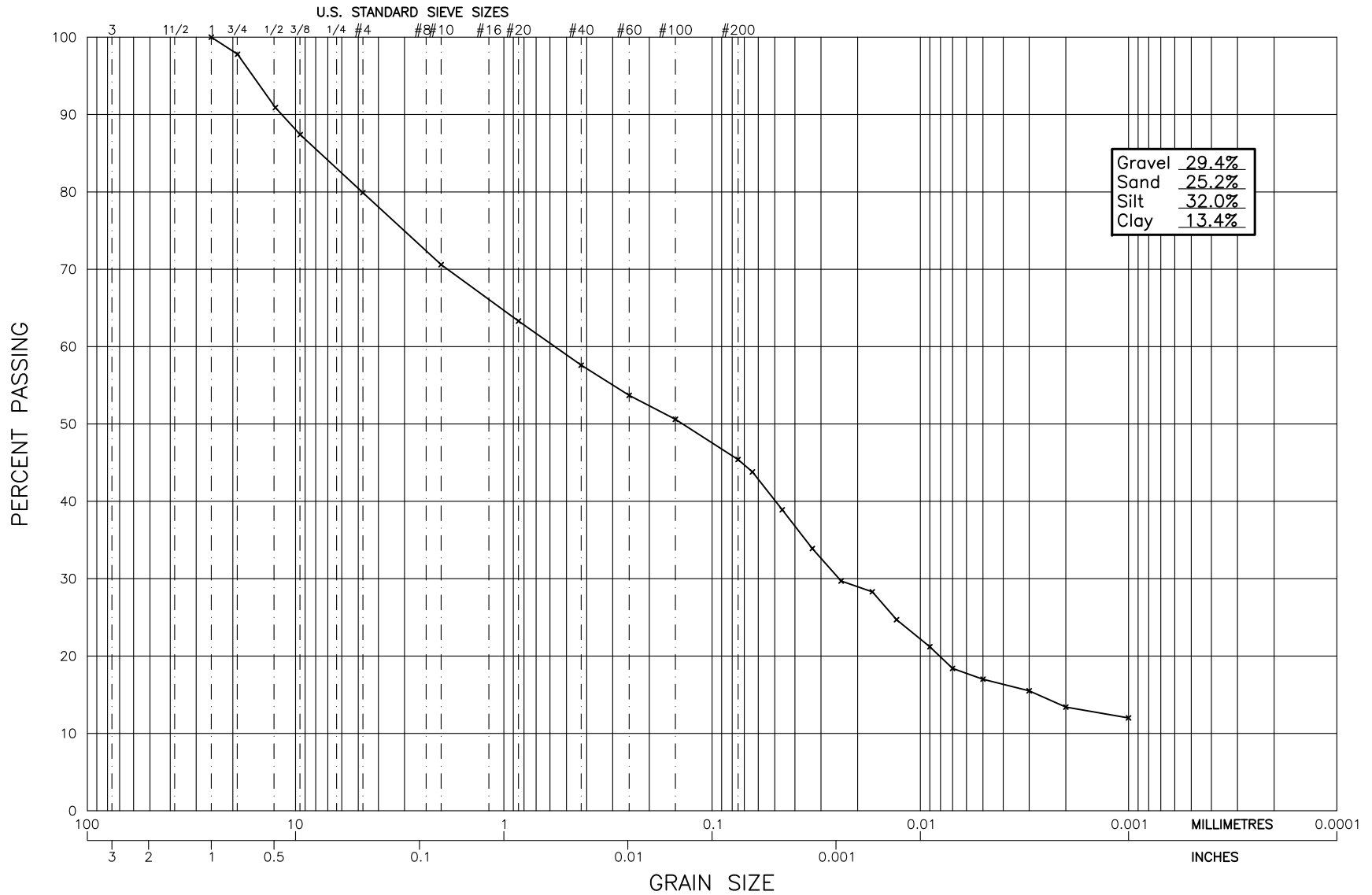


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm		
3/4" 19 mm	100.0	
1/2" 12.5 mm	96.4	
3/8" 9.5 mm	92.7	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	81.6	
No. 10 2.00 mm	64.9	
No. 20 850 µm	49.4	
No. 40 425 µm	39.7	
No. 60 250 µm	33.4	
No. 100 150 µm	28.5	
No. 200 75 µm	22.3	

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-44-S1

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH44-S1

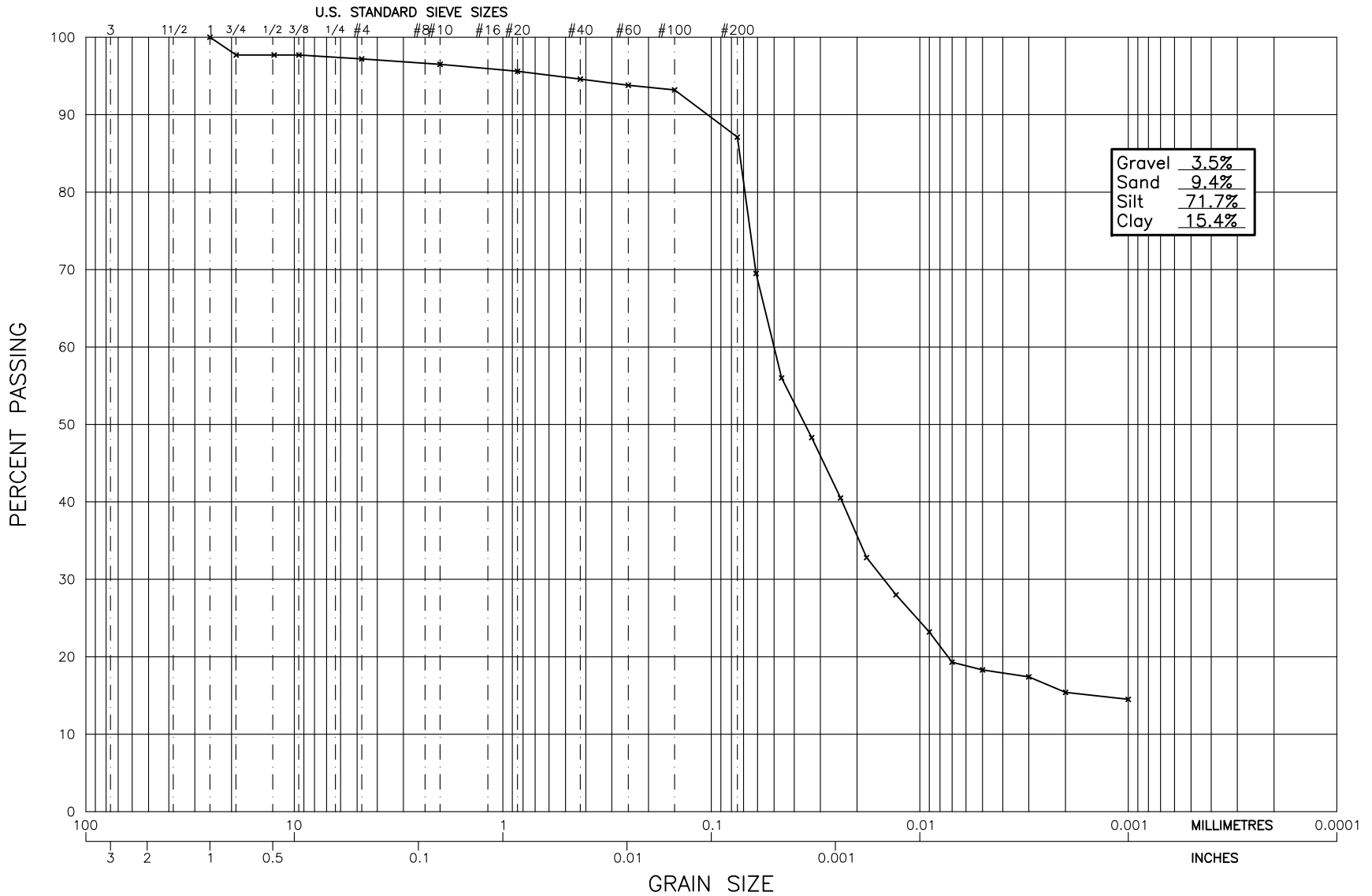
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Sept.28, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt/Gravel/Sand,some clay				
Sample #: S1		Test #:		Hole #: BH-BGC11-44		Depth: 0.2 - 0.4m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 3, 2011				Date Received: September 13, 2011				Date Tested: Sept.27, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	70.6	
Wet Wt. & Tare	976.4	25.4		696.3	100.0	20	5.2	44.8	89.6	63.3	
Dry Wt. & Tare	876.8	19.0	15.1	681.2	97.8	40	4.0	40.8	81.6	57.6	
Water Wt.	99.6	12.5	48.2	633.0	90.9	60	2.8	38.0	76.0	53.7	
Tare Wt.	180.5	9.5	24.5	608.5	87.4	100	2.2	35.8	71.6	50.6	
Wt. Of Dry Soil	696.3	4.75	51.9	556.6	79.9	200	3.7	32.1	64.2	45.4	
Moisture Content %	14.3	10	64.7	491.9	70.6	Pan	32.1				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	696.3			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.706	0.5	38.0	21.0	0.01348	31.0	11.2	4.729	0.064	62.0	43.8
50.0	0.706	1	34.5	21.0	0.01348	27.5	11.8	3.429	0.046	55.0	38.9
50.0	0.706	2	31.0	21.0	0.01348	24.0	12.3	2.483	0.033	48.0	33.9
50.0	0.706	4	28.0	21.0	0.01348	21.0	12.8	1.791	0.024	42.0	29.7
50.0	0.706	8	27.0	21.0	0.01348	20.0	13.0	1.275	0.017	40.0	28.3
50.0	0.706	15	24.5	21.0	0.01348	17.5	13.4	0.945	0.013	35.0	24.7
50.0	0.706	30	22.0	21.0	0.01348	15.0	13.8	0.679	0.009	30.0	21.2
50.0	0.706	60	20.0	21.0	0.01348	13.0	14.2	0.486	0.007	26.0	18.4
50.0	0.706	120	19.0	21.0	0.01348	12.0	14.3	0.345	0.005	24.0	17.0
50.0	0.706	240	18.0	21.0	0.01348	11.0	14.5	0.246	0.003	22.0	15.5
50.0	0.706	480	16.0	22.0	0.01332	9.5	14.7	0.175	0.002	19.0	13.4
50.0	0.706	1440	15.0	22.0	0.01332	8.5	14.9	0.102	0.001	17.0	12.0
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-44-S3

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH44-S3

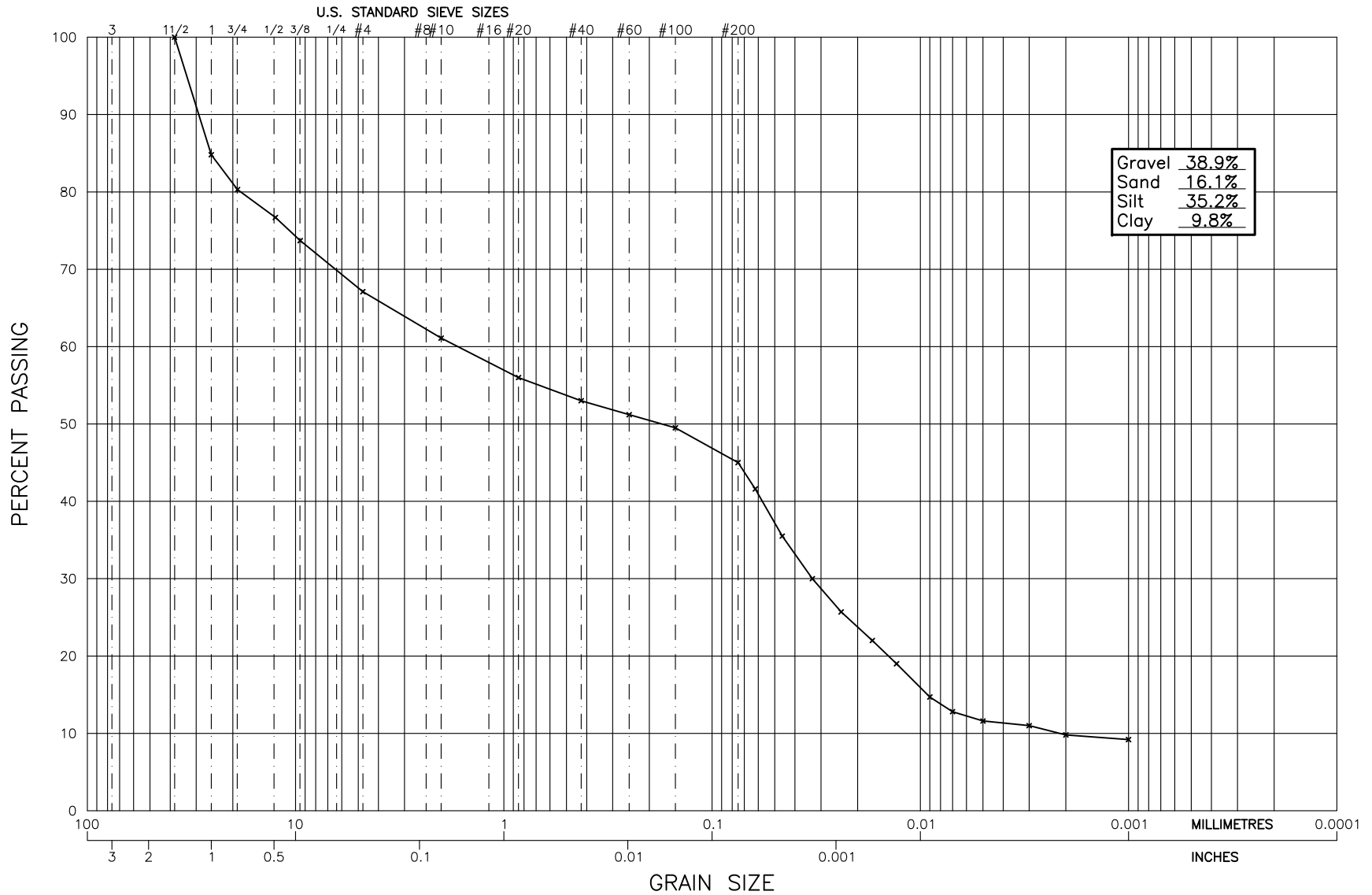
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Sept.28, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt,clayey				
Sample #: S3		Test #:		Hole #: BH-BGC11-44		Depth: 1.6 - 2.0m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 3, 2011				Date Received: September 13, 2011				Date Tested: Sept.27, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	96.5	
Wet Wt. & Tare	1044.3	25.4		680.4	100.0	20	0.5	49.5	99.0	95.6	
Dry Wt. & Tare	885.5	19.0	15.6	664.8	97.7	40	0.5	49.0	98.0	94.6	
Water Wt.	158.8	12.5	0.0	664.8	97.7	60	0.4	48.6	97.2	93.8	
Tare Wt.	205.1	9.5	0.0	664.8	97.7	100	0.3	48.3	96.6	93.2	
Wt. Of Dry Soil	680.4	4.75	3.7	661.1	97.2	200	3.2	45.1	90.2	87.1	
Moisture Content %	23.3	10	4.3	656.8	96.5	Pan	45.1				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	680.4			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.965	0.5	43.0	21.0	0.01348	36.0	10.4	4.551	0.061	72.0	69.5
50.0	0.965	1	36.0	21.0	0.01348	29.0	11.5	3.393	0.046	58.0	56.0
50.0	0.965	2	32.0	21.0	0.01348	25.0	12.2	2.467	0.033	50.0	48.3
50.0	0.965	4	28.0	21.0	0.01348	21.0	12.8	1.791	0.024	42.0	40.5
50.0	0.965	8	24.0	21.0	0.01348	17.0	13.5	1.299	0.018	34.0	32.8
50.0	0.965	15	21.5	21.0	0.01348	14.5	13.9	0.963	0.013	29.0	28.0
50.0	0.965	30	19.0	21.0	0.01348	12.0	14.3	0.691	0.009	24.0	23.2
50.0	0.965	60	17.0	21.0	0.01348	10.0	14.6	0.494	0.007	20.0	19.3
50.0	0.965	120	16.5	21.0	0.01348	9.5	14.7	0.350	0.005	19.0	18.3
50.0	0.965	240	16.0	21.0	0.01348	9.0	14.8	0.248	0.003	18.0	17.4
50.0	0.965	480	14.5	22.0	0.01332	8.0	15.0	0.177	0.002	16.0	15.4
50.0	0.965	1440	14.0	22.0	0.01332	7.5	15.1	0.102	0.001	15.0	14.5
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-44-S5

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH44-S5

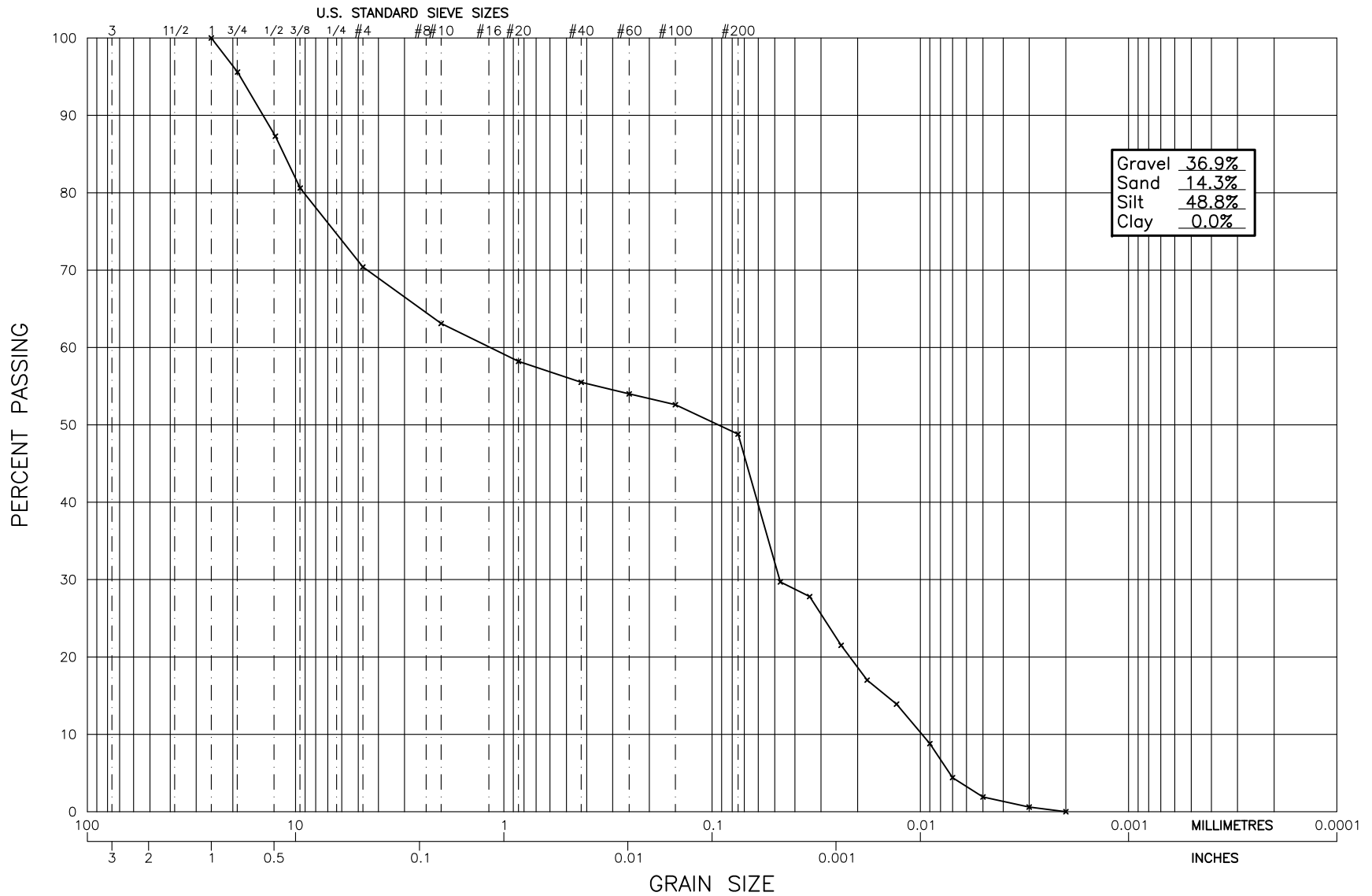
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Sept.28, 2011					
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300					
Source/Location: Dublin Gulch, Yukon							Type: Gravel,silty					
Sample #: S5		Test #:		Hole #: BH-BGC11-44		Depth: 4.8 - 5.1m		Time:				
Sampled By: Client				Tested By: DJ				Checked By:				
Date Sampled: August 3, 2011				Date Received: September 13, 2011				Date Tested: Sept.27, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis						
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.		
Tare No.		38.1		1268.4	100.0	10		50.0	100.0	61.1		
Wet Wt. & Tare	1634.4	25.4	193.1	1075.3	84.8	20	4.2	45.8	91.6	56.0		
Dry Wt. & Tare	1461.3	19.0	57.0	1018.3	80.3	40	2.5	43.3	86.6	53.0		
Water Wt.	173.1	12.5	45.6	972.7	76.7	60	1.4	41.9	83.8	51.2		
Tare Wt.	192.9	9.5	37.9	934.8	73.7	100	1.4	40.5	81.0	49.5		
Wt. Of Dry Soil	1268.4	4.75	83.5	851.3	67.1	200	3.7	36.8	73.6	45.0		
Moisture Content %	13.6	10	75.7	775.6	61.1	Pan	36.8					
Dry Wt. Of Sample from Initial Moisture						Total	50.0					
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=						
		Total	1268.4			Tare		Wt. Passing #200 =				
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)	
50.0	0.611	0.5	41.0	21.0	0.01348	34.0	10.7	4.623	0.062	68.0	41.6	
50.0	0.611	1	36.0	21.0	0.01348	29.0	11.5	3.393	0.046	58.0	35.5	
50.0	0.611	2	31.5	21.0	0.01348	24.5	12.3	2.475	0.033	49.0	30.0	
50.0	0.611	4	28.0	21.0	0.01348	21.0	12.8	1.791	0.024	42.0	25.7	
50.0	0.611	8	25.0	21.0	0.01348	18.0	13.3	1.291	0.017	36.0	22.0	
50.0	0.611	15	22.5	21.0	0.01348	15.5	13.7	0.957	0.013	31.0	19.0	
50.0	0.611	30	19.0	21.0	0.01348	12.0	14.3	0.691	0.009	24.0	14.7	
50.0	0.611	60	17.5	21.0	0.01348	10.5	14.6	0.493	0.007	21.0	12.8	
50.0	0.611	120	16.5	21.0	0.01348	9.5	14.7	0.350	0.005	19.0	11.6	
50.0	0.611	240	16.0	21.0	0.01348	9.0	14.8	0.248	0.003	18.0	11.0	
50.0	0.611	480	14.5	22.0	0.01332	8.0	15.0	0.177	0.002	16.0	9.8	
50.0	0.611	1440	14.0	22.0	0.01332	7.5	15.1	0.102	0.001	15.0	9.2	
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml			
Density of Solids:												
Description of Sample:												

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	36.9%
Sand	14.3%
Silt	48.8%
Clay	0.0%

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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-44-S7

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH44-S7

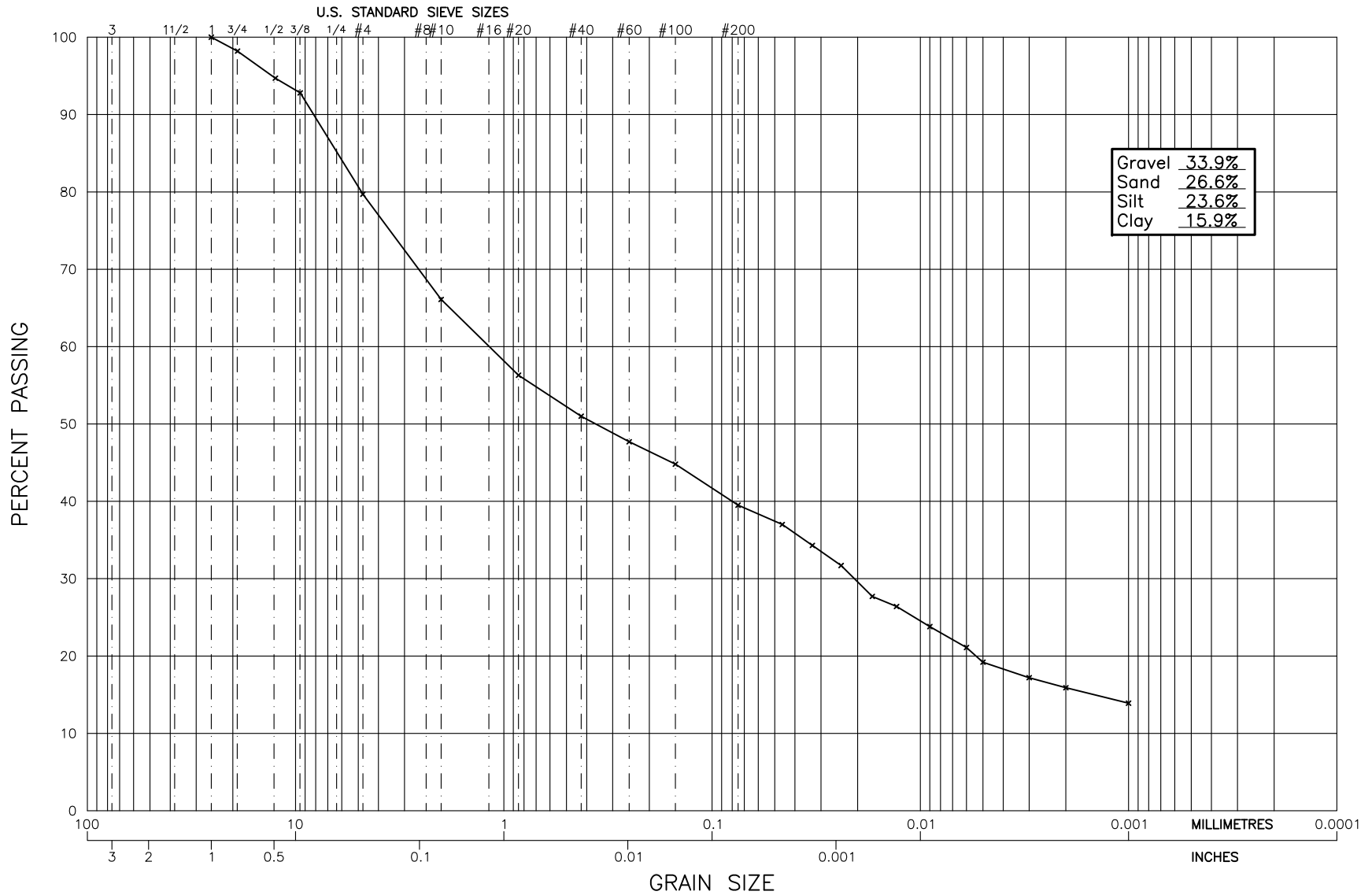
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Sept.29, 2011					
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300					
Source/Location: Dublin Gulch, Yukon							Type: Silt/Gravel,some sand					
Sample #: S7		Test #:		Hole #: BH-BGC11-44		Depth: 6.3 - 6.6m		Time:				
Sampled By: Client				Tested By: DJ				Checked By:				
Date Sampled: August 3, 2011				Date Received: September 13, 2011				Date Tested: Sept.28, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis						
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.		
Tare No.		38.1				10		50.0	100.0	63.1		
Wet Wt. & Tare	1543.7	25.4		1089.2	100.0	20	3.9	46.1	92.2	58.2		
Dry Wt. & Tare	1280.7	19.0	48.1	1041.1	95.6	40	2.1	44.0	88.0	55.5		
Water Wt.	263.0	12.5	90.3	950.8	87.3	60	1.2	42.8	85.6	54.0		
Tare Wt.	191.5	9.5	72.9	877.9	80.6	100	1.1	41.7	83.4	52.6		
Wt. Of Dry Soil	1089.2	4.75	110.6	767.3	70.4	200	3.0	38.7	77.4	48.8		
Moisture Content %	24.1	10	80.1	687.2	63.1	Pan	38.7					
Dry Wt. Of Sample from Initial Moisture						Total	50.0					
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=						
		Total	1089.2			Tare		Wt. Passing #200 =				
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)	
50.0	0.631	0.5	35.0	22.0	0.01332	28.5	11.6	4.815	0.064	57.0	36.0	
50.0	0.631	1	30.0	22.0	0.01332	23.5	12.4	3.524	0.047	47.0	29.7	
50.0	0.631	2	28.5	22.0	0.01332	22.0	12.7	2.516	0.034	44.0	27.8	
50.0	0.631	4	23.5	22.0	0.01332	17.0	13.5	1.836	0.024	34.0	21.5	
50.0	0.631	8	20.0	22.0	0.01332	13.5	14.1	1.326	0.018	27.0	17.0	
50.0	0.631	15	17.5	22.0	0.01332	11.0	14.5	0.983	0.013	22.0	13.9	
50.0	0.631	30	13.5	22.0	0.01332	7.0	15.1	0.710	0.009	14.0	8.8	
50.0	0.631	60	10.0	22.0	0.01332	3.5	15.7	0.512	0.007	7.0	4.4	
50.0	0.631	120	8.0	22.0	0.01332	1.5	16.0	0.366	0.005	3.0	1.9	
50.0	0.631	240	6.5	24.0	0.01301	0.5	16.2	0.260	0.003	1.0	0.6	
50.0	0.631	480	6.0	24.0	0.01301	0.0	16.3	0.184	0.002	0.0	0.0	
50.0	0.631	1440	6.0	24.0	0.01301	0.0	16.3	0.106	0.001	0.0	0.0	
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml			
Density of Solids:												
Description of Sample:												

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-44-S11

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH44-S11

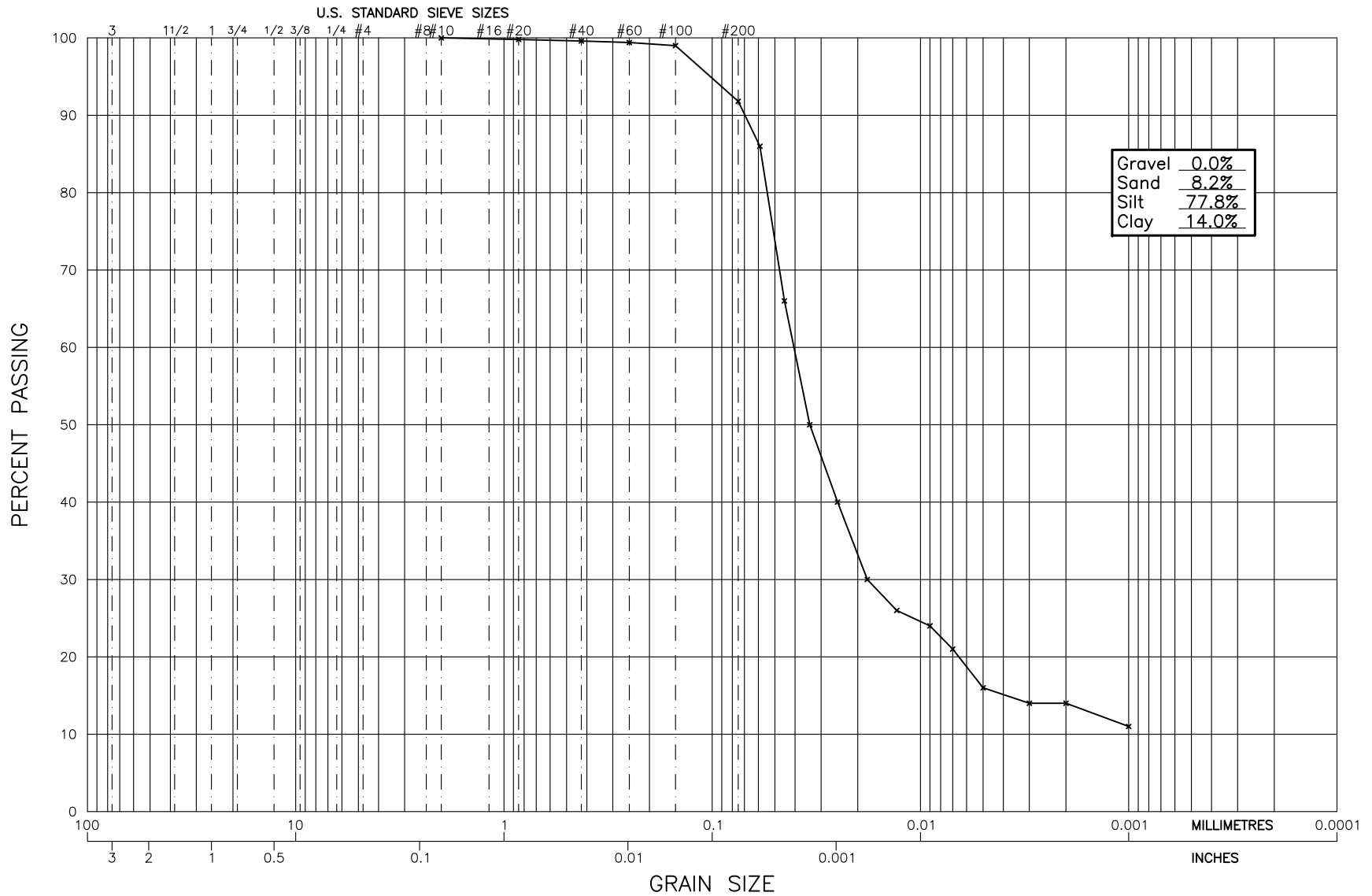
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Sept.28, 2011					
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300					
Source/Location: Dublin Gulch, Yukon							Type: Gravel, sandy, silty, some clay					
Sample #: S11		Test #:		Hole #: Bh-BGC11-44		Depth: 9.6 - 9.9m		Time:				
Sampled By: Client				Tested By: DJ				Checked By: DJ				
Date Sampled: August 3, 2011				Date Received: September 13, 2011				Date Tested: Sept.27, 2011				
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis					
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.			38.1				10		50.0	100.0	66.1	
Wet Wt. & Tare	1079.5		25.4		771.2	100.0	20	7.4	42.6	85.2	56.3	
Dry Wt. & Tare	950.9		19.0	13.7	757.5	98.2	40	4.0	38.6	77.2	51.0	
Water Wt.	128.6		12.5	27.1	730.4	94.7	60	2.5	36.1	72.2	47.7	
Tare Wt.	179.7		9.5	14.6	715.8	92.8	100	2.2	33.9	67.8	44.8	
Wt. Of Dry Soil	771.2		4.75	101.2	614.6	79.7	200	4.0	29.9	59.8	39.5	
Moisture Content %	16.7		10	105.2	509.4	66.1	Pan	29.9				
Dry Wt. Of Sample from Initial Moisture							Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=					
			Total	771.2			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)	
50.0	0.661	0.5	36.5	21.0	0.01348	29.5	11.4	4.781	0.064	59.0	39.0	
50.0	0.661	1	35.0	21.0	0.01348	28.0	11.7	3.417	0.046	56.0	37.0	
50.0	0.661	2	33.0	21.0	0.01348	26.0	12.0	2.450	0.033	52.0	34.3	
50.0	0.661	4	31.0	21.0	0.01348	24.0	12.3	1.756	0.024	48.0	31.7	
50.0	0.661	8	28.0	21.0	0.01348	21.0	12.8	1.266	0.017	42.0	27.7	
50.0	0.661	15	27.0	21.0	0.01348	20.0	13.0	0.931	0.013	40.0	26.4	
50.0	0.661	30	25.0	21.0	0.01348	18.0	13.3	0.666	0.009	36.0	23.8	
50.0	0.661	60	23.0	21.0	0.01348	16.0	13.7	0.477	0.006	32.0	21.1	
50.0	0.661	120	21.5	21.0	0.01348	14.5	13.9	0.340	0.005	29.0	19.2	
50.0	0.661	240	20.0	21.0	0.01348	13.0	14.2	0.243	0.003	26.0	17.2	
50.0	0.661	480	18.5	22.0	0.01332	12.0	14.3	0.173	0.002	24.0	15.9	
50.0	0.661	1440	17.0	22.0	0.01332	10.5	14.6	0.101	0.001	21.0	13.9	
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml			
Density of Solids:												
Description of Sample:												

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	0.0%
Sand	8.2%
Silt	77.8%
Clay	14.0%

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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-47-G4

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH47-G3

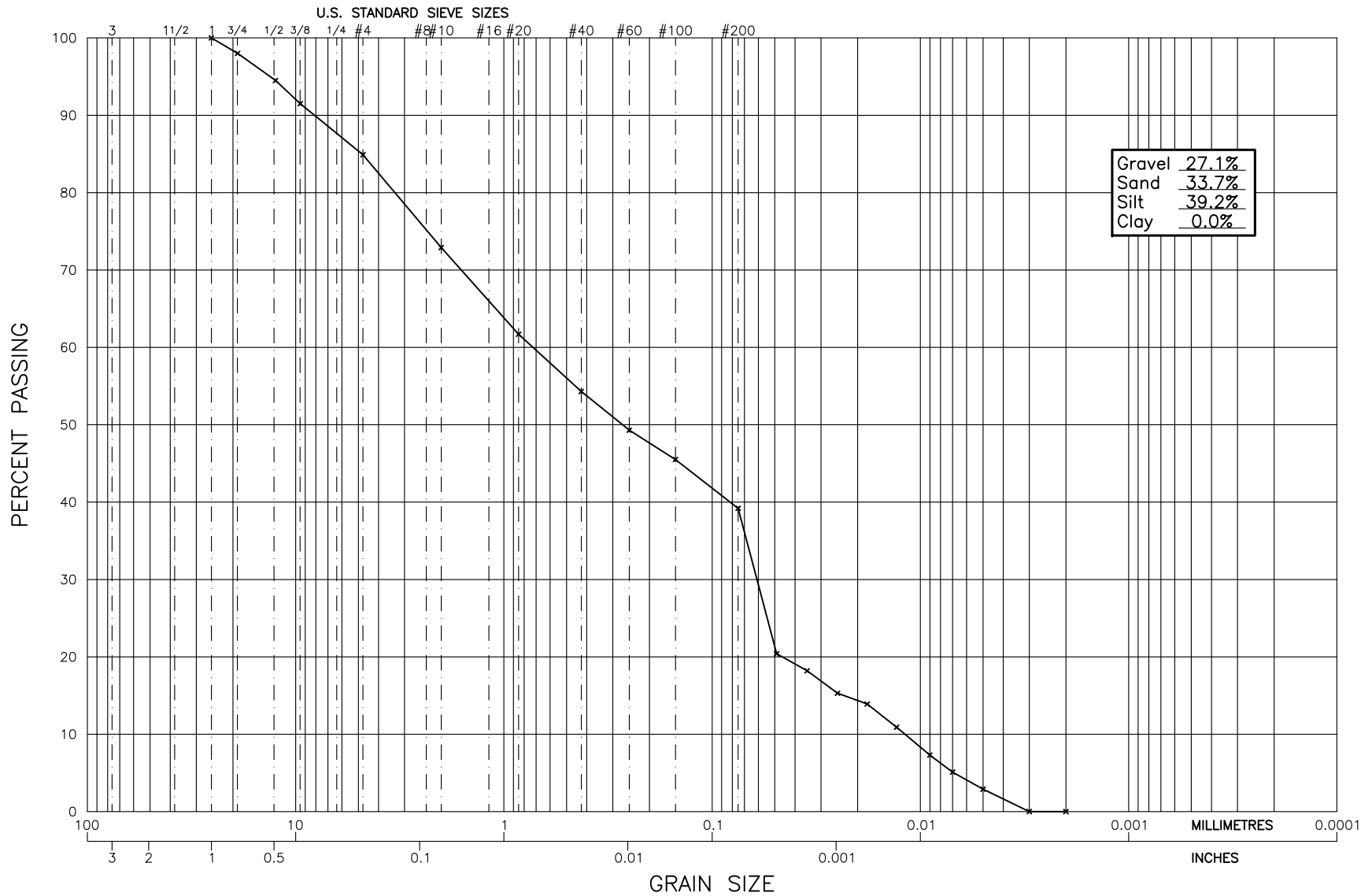
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 27, 2011				
Project Name: Eagle Gold Soil and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt, some clay				
Sample #: G3		Test #:		Hole #: BH-BGC11-47		Depth: 2.0 - 2.27m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 4, 2011				Date Received: September 13, 2011				Date Tested: Sept.26, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	100.0
Wet Wt. & Tare	924.7		25.4				20	0.1	49.9	99.8	99.8
Dry Wt. & Tare	764.9		19.0				40	0.1	49.8	99.6	99.6
Water Wt.	159.8		12.5				60	0.1	49.7	99.4	99.4
Tare Wt.	181.1		9.5				100	0.2	49.5	99.0	99.0
Wt. Of Dry Soil	583.8		4.75				200	3.6	45.9	91.8	91.8
Moisture Content %	27.4		10		583.8	100.0	Pan	45.9			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =			Total	583.8			Unwashed Wt.=				
							Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	1.000	0.5	50.0	20.0	0.01365	43.0	9.2	4.290	0.059	86.0	86.0
50.0	1.000	1	40.0	20.0	0.01365	33.0	10.9	3.294	0.045	66.0	66.0
50.0	1.000	2	32.0	20.0	0.01365	25.0	12.2	2.467	0.034	50.0	50.0
50.0	1.000	4	27.0	20.0	0.01365	20.0	13.0	1.802	0.025	40.0	40.0
50.0	1.000	8	22.0	20.0	0.01365	15.0	13.8	1.314	0.018	30.0	30.0
50.0	1.000	15	20.0	20.0	0.01365	13.0	14.2	0.971	0.013	26.0	26.0
50.0	1.000	30	19.0	20.0	0.01365	12.0	14.3	0.691	0.009	24.0	24.0
50.0	1.000	60	17.5	20.0	0.01365	10.5	14.6	0.493	0.007	21.0	21.0
50.0	1.000	120	15.0	20.0	0.01365	8.0	15.0	0.353	0.005	16.0	16.0
50.0	1.000	240	14.0	20.0	0.01365	7.0	15.1	0.251	0.003	14.0	14.0
50.0	1.000	480	13.0	23.0	0.01317	7.0	15.1	0.178	0.002	14.0	14.0
50.0	1.000	1440	12.5	21.0	0.01348	5.5	15.4	0.103	0.001	11.0	11.0
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-47-G10

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH47-G10

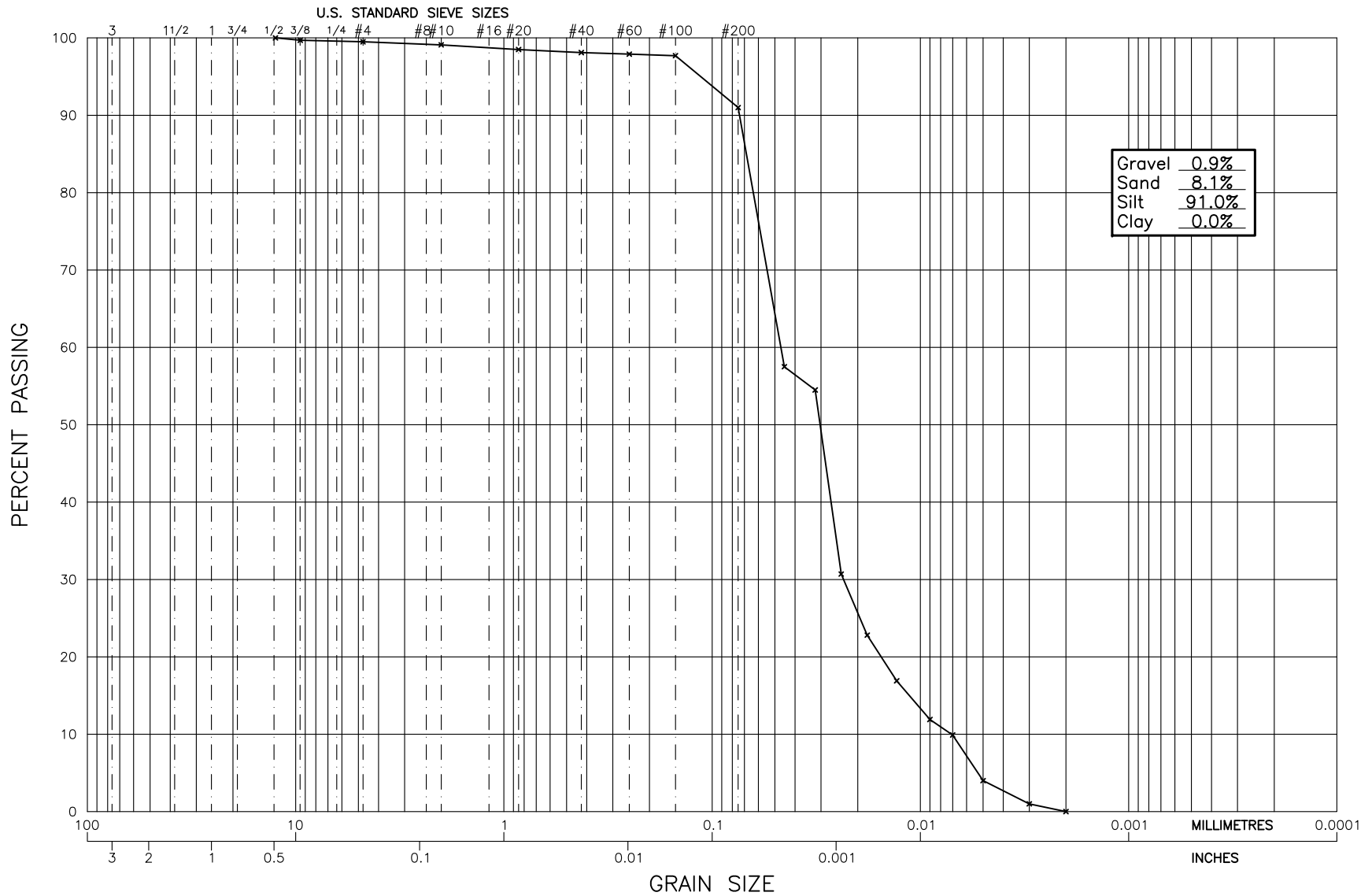
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt/Sand/Gravel				
Sample #: G10		Test #:		Hole #: BH-BGC11-47		Depth: 11.8 - 12.1m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 4, 2011				Date Received: September 13, 2011				Date Tested: Sept. 23, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	72.9
Wet Wt. & Tare	1089.1		25.4		814.4	100.0	20	7.7	42.3	84.6	61.7
Dry Wt. & Tare	995.9		19.0	15.9	798.5	98.0	40	5.1	37.2	74.4	54.3
Water Wt.	93.2		12.5	29.1	769.4	94.5	60	3.4	33.8	67.6	49.3
Tare Wt.	181.5		9.5	23.9	745.5	91.5	100	2.6	31.2	62.4	45.5
Wt. Of Dry Soil	814.4		4.75	54.0	691.5	84.9	200	4.3	26.9	53.8	39.2
Moisture Content %	11.4		10	97.6	593.9	72.9	Pan	26.9			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =			Total	814.4			Unwashed Wt.=				
							Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.729	0.5	25.0	23.0	0.01317	19.0	13.2	5.130	0.068	38.0	27.7
50.0	0.729	1	20.0	23.0	0.01317	14.0	14.0	3.740	0.049	28.0	20.4
50.0	0.729	2	18.5	23.0	0.01317	12.5	14.2	2.668	0.035	25.0	18.2
50.0	0.729	4	16.5	23.0	0.01317	10.5	14.6	1.908	0.025	21.0	15.3
50.0	0.729	8	15.5	23.0	0.01317	9.5	14.7	1.357	0.018	19.0	13.9
50.0	0.729	15	13.5	23.0	0.01317	7.5	15.1	1.002	0.013	15.0	10.9
50.0	0.729	30	11.0	23.0	0.01317	5.0	15.5	0.718	0.009	10.0	7.3
50.0	0.729	60	9.5	23.0	0.01317	3.5	15.7	0.512	0.007	7.0	5.1
50.0	0.729	120	8.0	23.0	0.01317	2.0	16.0	0.365	0.005	4.0	2.9
50.0	0.729	240	6.0	23.0	0.01317	0.0	16.3	0.261	0.003	0.0	0.0
50.0	0.729	480	6.0	23.0	0.01317	0.0	16.3	0.184	0.002	0.0	0.0
50.0	0.729	1469	6.0	23.0	0.01317	0.0	16.3	0.105	0.001	0.0	0.0
Hydrometer #: 932452			23		Dispersing Agent: Sodium Hex				Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-47-SPT1

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH47-SPT1

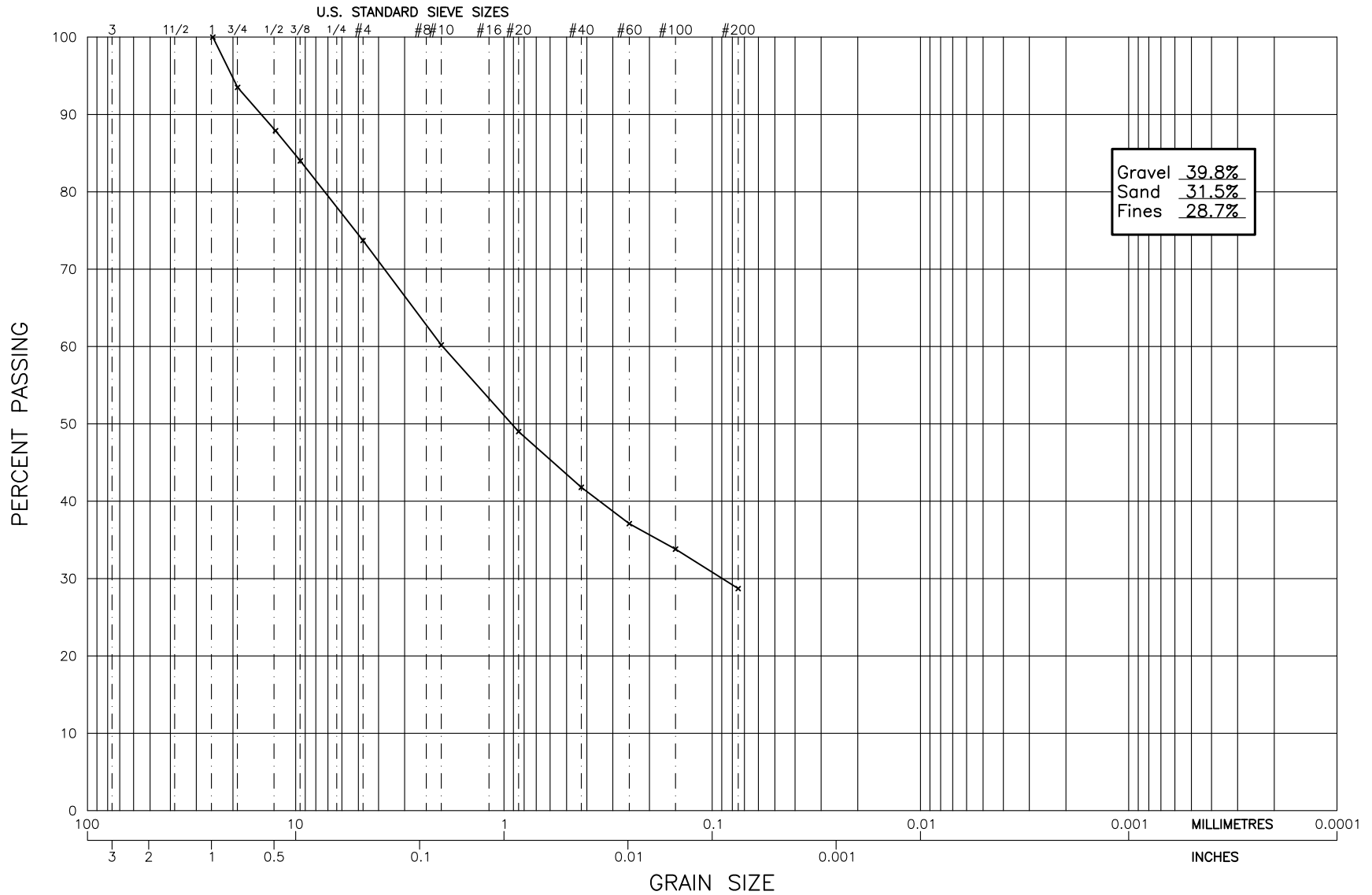
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold Soil and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt				
Sample #: SPT1		Test #:		Hole #: BH-BGC11-47		Depth: 0.75 - 1.2m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 4, 2011				Date Received: September 13, 2011				Date Tested: Sept.23, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	99.1
Wet Wt. & Tare	894.6		25.4				20	0.3	49.7	99.4	98.5
Dry Wt. & Tare	747.3		19.0				40	0.2	49.5	99.0	98.1
Water Wt.	147.3		12.5		566.9	100.0	60	0.1	49.4	98.8	97.9
Tare Wt.	180.4		9.5	1.7	565.2	99.7	100	0.1	49.3	98.6	97.7
Wt. Of Dry Soil	566.9		4.75	1.4	563.8	99.5	200	3.4	45.9	91.8	91.0
Moisture Content %	26.0		10	1.8	562.0	99.1	Pan	45.9			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =			Total	566.9			Unwashed Wt.=				
							Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.991	0.5	42.0	23.0	0.01317	36.0	10.4	4.551	0.060	72.0	71.4
50.0	0.991	1	35.0	23.0	0.01317	29.0	11.5	3.393	0.045	58.0	57.5
50.0	0.991	2	33.5	23.0	0.01317	27.5	11.8	2.425	0.032	55.0	54.5
50.0	0.991	4	21.5	23.0	0.01317	15.5	13.7	1.853	0.024	31.0	30.7
50.0	0.991	8	17.5	23.0	0.01317	11.5	14.4	1.342	0.018	23.0	22.8
50.0	0.991	15	14.5	23.0	0.01317	8.5	14.9	0.996	0.013	17.0	16.9
50.0	0.991	30	12.0	23.0	0.01317	6.0	15.3	0.714	0.009	12.0	11.9
50.0	0.991	60	11.0	23.0	0.01317	5.0	15.5	0.508	0.007	10.0	9.9
50.0	0.991	120	8.0	23.0	0.01317	2.0	16.0	0.365	0.005	4.0	4.0
50.0	0.991	240	6.5	23.0	0.01317	0.5	16.2	0.260	0.003	1.0	1.0
50.0	0.991	480	6.0	23.0	0.01317	0.0	16.3	0.184	0.002	0.0	0.0
50.0	0.991	1464	6.0	23.0	0.01317	0.0	16.3	0.106	0.001	0.0	0.0
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-47-SPT4

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH47-SPT4



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 630 - 1718 Argyle Street
 Halifax, Nova Scotia
 B3J 3N6

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

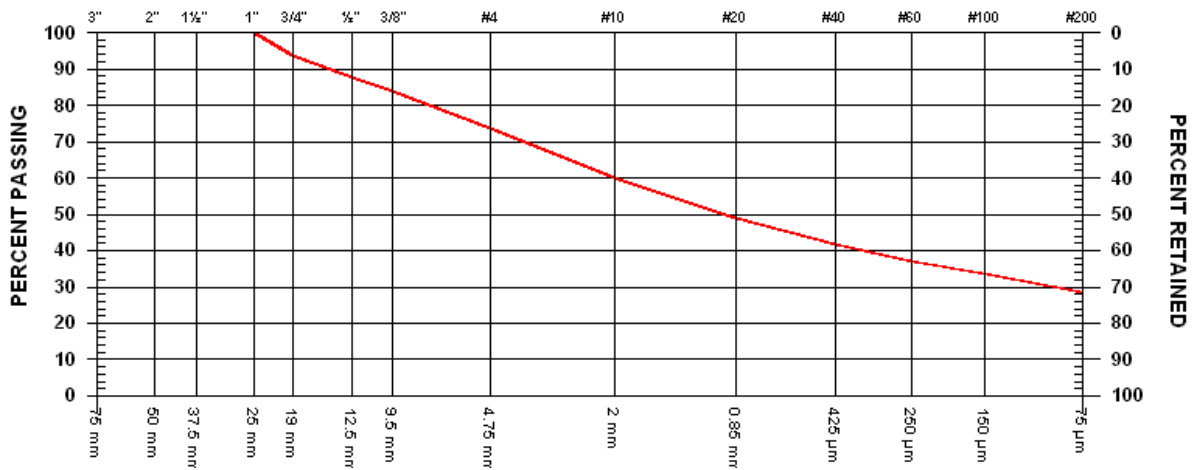
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 3 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Sep.21 DATE SAMPLED 2011.Aug.04

SUPPLIER BH-BGC11-47-SPT4
 SOURCE 5.33 - 5.88m
 SPECIFICATION
 MATERIAL TYPE Sand/Silt

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED

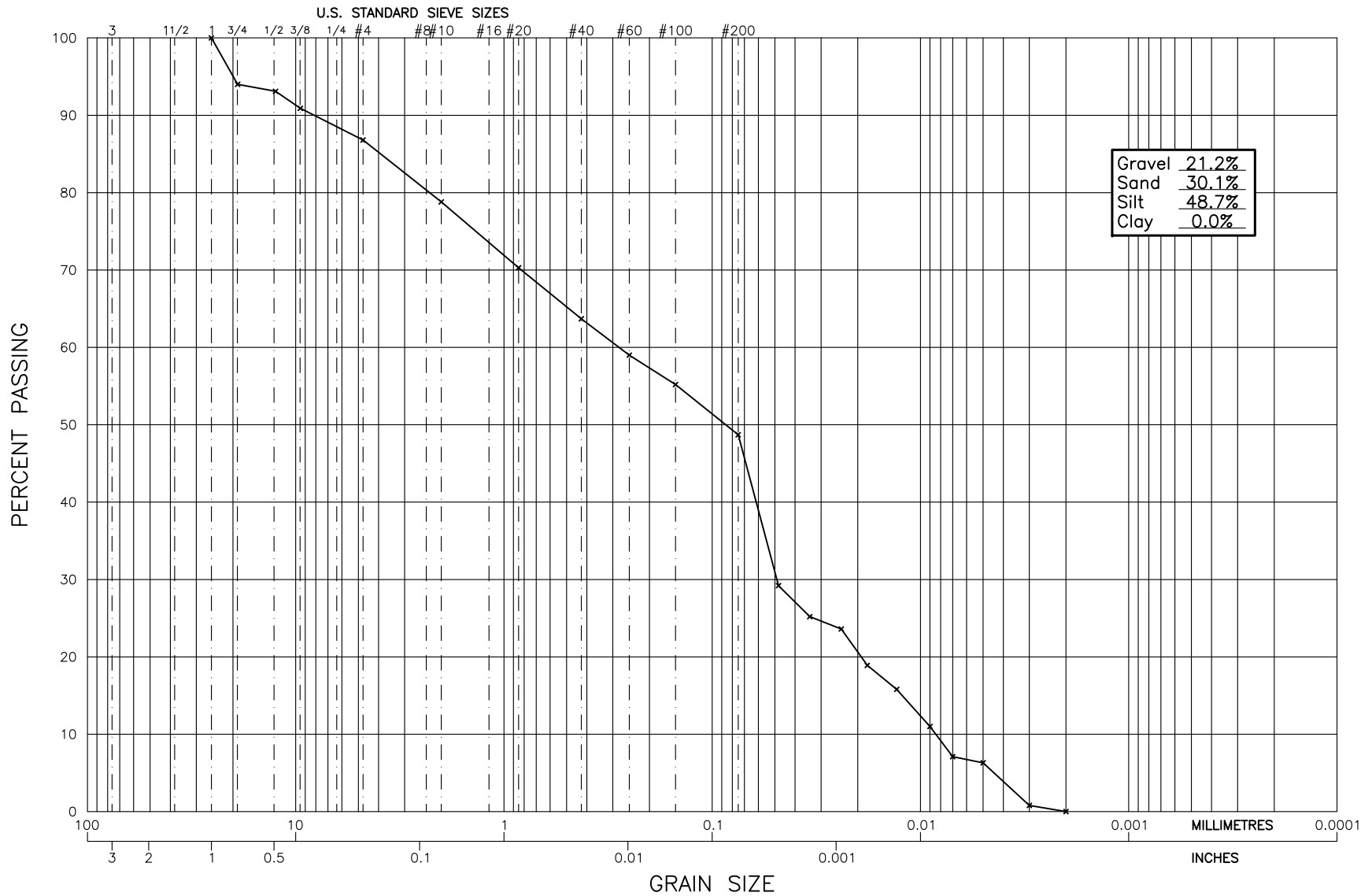


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm	100.0	
3/4" 19 mm	93.5	
1/2" 12.5 mm	87.9	
3/8" 9.5 mm	84.0	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	73.7	
No. 10 2.00 mm	60.2	
No. 20 850 µm	49.0	
No. 40 425 µm	41.8	
No. 60 250 µm	37.1	
No. 100 150 µm	33.8	
No. 200 75 µm	28.7	

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	21.2%
Sand	30.1%
Silt	48.7%
Clay	0.0%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-47-SPT6

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH47-SPT6

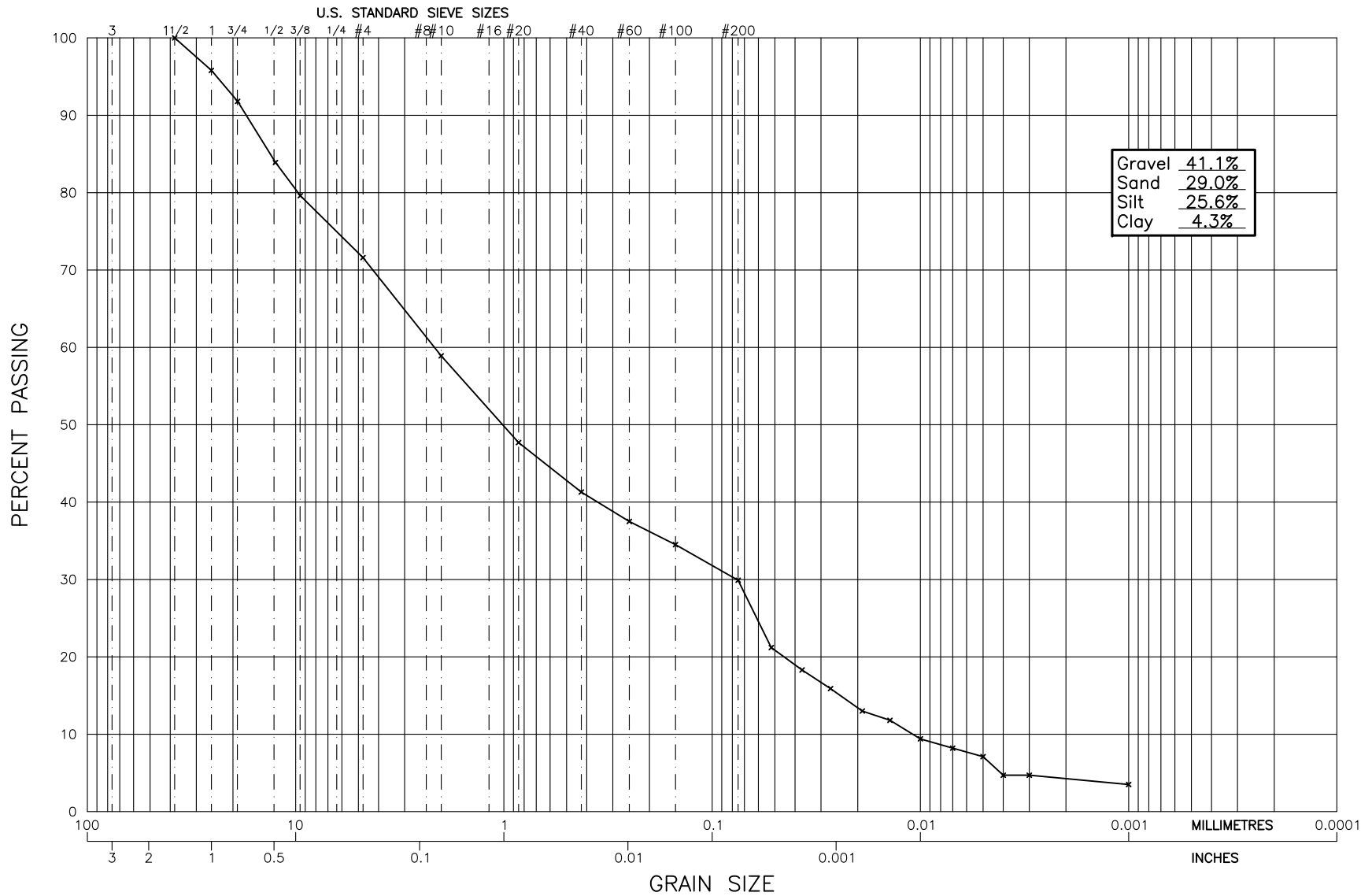
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept.26, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt and Sand, gravelly				
Sample #: SPT6		Test #:		Hole #: BH-BGC11-47		Depth: 8.38 - 8.58m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 4, 2011				Date Received: September 13, 2011				Date Tested: Sept.21, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	78.8
Wet Wt. & Tare	583.9		25.4		349.8	100.0	20	5.4	44.6	89.2	70.3
Dry Wt. & Tare	542.7		19.0	21.0	328.8	94.0	40	4.2	40.4	80.8	63.7
Water Wt.	41.2		12.5	3.1	325.7	93.1	60	3.0	37.4	74.8	59.0
Tare Wt.	192.9		9.5	7.7	318.0	90.9	100	2.4	35.0	70.0	55.2
Wt. Of Dry Soil	349.8		4.75	14.5	303.5	86.8	200	4.1	30.9	61.8	48.7
Moisture Content %	11.8		10	27.8	275.7	78.8	Pan	30.9			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =			Total	349.8			Unwashed Wt.=				
							Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.788	0.5	28.0	23.0	0.01317	22.0	12.7	5.033	0.066	44.0	34.7
50.0	0.788	1	24.5	23.0	0.01317	18.5	13.2	3.639	0.048	37.0	29.2
50.0	0.788	2	22.0	23.0	0.01317	16.0	13.7	2.613	0.034	32.0	25.2
50.0	0.788	4	21.0	23.0	0.01317	15.0	13.8	1.859	0.024	30.0	23.6
50.0	0.788	8	18.0	23.0	0.01317	12.0	14.3	1.338	0.018	24.0	18.9
50.0	0.788	15	16.0	23.0	0.01317	10.0	14.6	0.988	0.013	20.0	15.8
50.0	0.788	30	13.0	23.0	0.01317	7.0	15.1	0.710	0.009	14.0	11.0
50.0	0.788	60	10.5	23.0	0.01317	4.5	15.6	0.509	0.007	9.0	7.1
50.0	0.788	120	10.5	22.0	0.01332	4.0	15.6	0.361	0.005	8.0	6.3
50.0	0.788	240	7.0	22.0	0.01332	0.5	16.2	0.260	0.003	1.0	0.8
50.0	0.788	480	6.5	22.0	0.01332	0.0	16.3	0.184	0.002	0.0	0.0
50.0	0.788	1440	6.5	22.0	0.01332	0.0	16.3	0.106	0.001	0.0	0.0
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-49-G4

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH49-G4

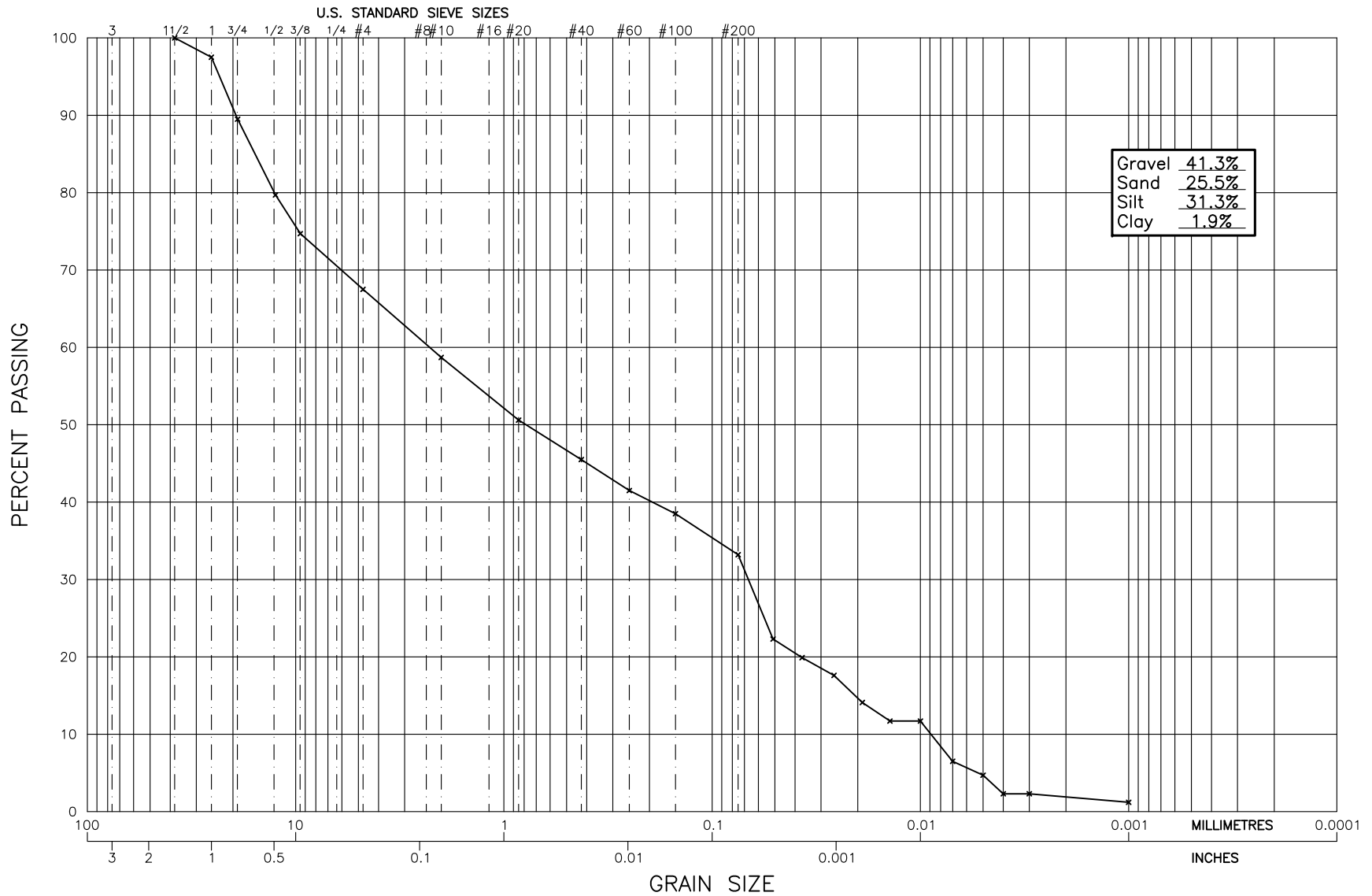
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: November 15, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel, sandy and silty				
Sample #: G4		Test #:		Hole #: BH-BGC11-49		Depth: 3.4 - 3.6m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: Aug.5, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.14, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		1449.0	100.0	10		50.0	100.0	58.9	
Wet Wt. & Tare	1798.8	25.4	60.3	1388.7	95.8	20	9.5	40.5	81.0	47.7	
Dry Wt. & Tare	1630.5	19.0	57.8	1330.9	91.8	40	5.4	35.1	70.2	41.3	
Water Wt.	168.3	12.5	115.2	1215.7	83.9	60	3.3	31.8	63.6	37.5	
Tare Wt.	181.5	9.5	62.0	1153.7	79.6	100	2.5	29.3	58.6	34.5	
Wt. Of Dry Soil	1449.0	4.75	116.7	1037.0	71.6	200	3.9	25.4	50.8	29.9	
Moisture Content %	11.6	10	183.6	853.4	58.9	Pan	25.4				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	1449.0			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.589	0.5	32.0	17.0	0.01417	23.0	12.5	5.000	0.071	46.0	27.1
50.0	0.589	1	27.0	17.0	0.01417	18.0	13.3	3.650	0.052	36.0	21.2
50.0	0.589	2	24.5	17.0	0.01417	15.5	13.7	2.621	0.037	31.0	18.3
50.0	0.589	4	22.5	17.0	0.01417	13.5	14.1	1.875	0.027	27.0	15.9
50.0	0.589	8	20.0	17.0	0.01417	11.0	14.5	1.345	0.019	22.0	13.0
50.0	0.589	15	19.0	17.0	0.01417	10.0	14.6	0.988	0.014	20.0	11.8
50.0	0.589	30	17.0	17.0	0.01417	8.0	15.0	0.707	0.010	16.0	9.4
50.0	0.589	60	16.0	17.0	0.01417	7.0	15.1	0.502	0.007	14.0	8.2
50.0	0.589	120	15.0	17.0	0.01417	6.0	15.3	0.357	0.005	12.0	7.1
50.0	0.589	240	13.0	17.0	0.01417	4.0	15.6	0.255	0.004	8.0	4.7
50.0	0.589	480	12.0	18.0	0.01399	4.0	15.6	0.180	0.003	8.0	4.7
50.0	0.589	1440	11.0	18.0	0.01399	3.0	15.8	0.105	0.001	6.0	3.5
Hydrometer #: 932452			Graduate #: 2			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	41.3%
Sand	25.5%
Silt	31.3%
Clay	1.9%

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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-49-G6

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH49-G6

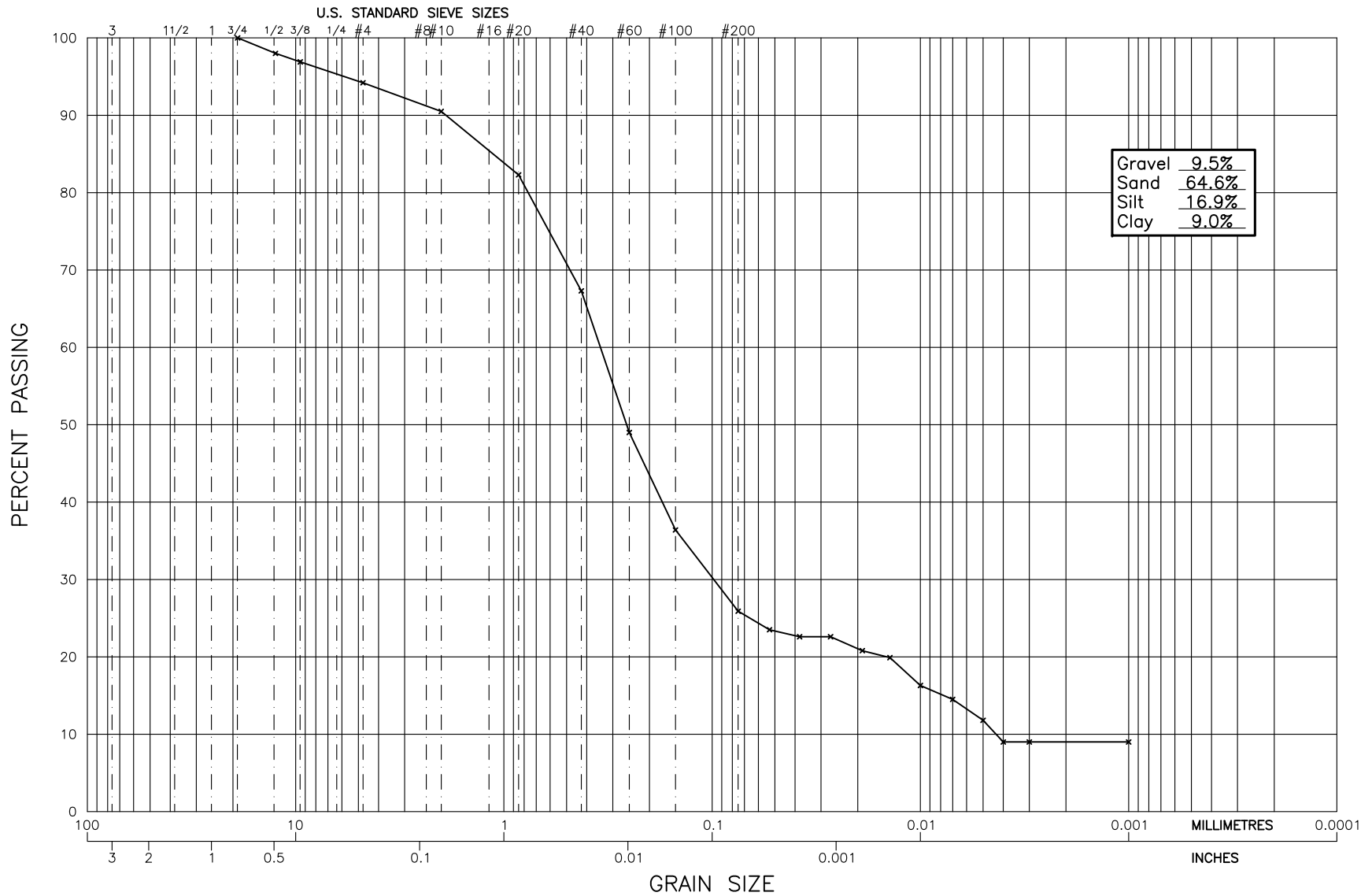
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: November 15, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel and Silt, sandy				
Sample #: G6		Test #:		Hole #: BH-BGC11-49		Depth: 6.4 - 6.6m		Time:			
Sampled By: Client			Tested By: DJ				Checked By: DJ				
Date Sampled: Aug.5, 2011			Date Received: Sept.13, 2011				Date Tested: Nov.14, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		938.3	100.0	10		50.0	100.0	58.7	
Wet Wt. & Tare	1241.4	25.4	23.9	914.4	97.5	20	6.9	43.1	86.2	50.6	
Dry Wt. & Tare	1118.7	19.0	74.9	839.5	89.5	40	4.3	38.8	77.6	45.5	
Water Wt.	122.7	12.5	92.1	747.4	79.7	60	3.4	35.4	70.8	41.5	
Tare Wt.	180.4	9.5	46.5	700.9	74.7	100	2.6	32.8	65.6	38.5	
Wt. Of Dry Soil	938.3	4.75	67.4	633.5	67.5	200	4.5	28.3	56.6	33.2	
Moisture Content %	13.1	10	83.0	550.5	58.7	Pan	28.3				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	938.3			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.587	0.5	33.0	17.0	0.01417	24.0	12.3	4.967	0.070	48.0	28.2
50.0	0.587	1	28.0	17.0	0.01417	19.0	13.2	3.628	0.051	38.0	22.3
50.0	0.587	2	26.0	17.0	0.01417	17.0	13.5	2.597	0.037	34.0	19.9
50.0	0.587	4	24.0	17.0	0.01417	15.0	13.8	1.859	0.026	30.0	17.6
50.0	0.587	8	21.0	17.0	0.01417	12.0	14.3	1.338	0.019	24.0	14.1
50.0	0.587	15	19.0	17.0	0.01417	10.0	14.6	0.988	0.014	20.0	11.7
50.0	0.587	30	19.0	17.0	0.01417	10.0	14.6	0.699	0.010	20.0	11.7
50.0	0.587	60	14.5	17.0	0.01417	5.5	15.4	0.506	0.007	11.0	6.5
50.0	0.587	120	13.0	17.0	0.01417	4.0	15.6	0.361	0.005	8.0	4.7
50.0	0.587	240	11.0	17.0	0.01417	2.0	16.0	0.258	0.004	4.0	2.3
50.0	0.587	480	10.0	18.0	0.01399	2.0	16.0	0.182	0.003	4.0	2.3
50.0	0.587	1440	9.0	18.0	0.01399	1.0	16.1	0.106	0.001	2.0	1.2
Hydrometer #: 932452			Graduate #: 3			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	9.5%
Sand	64.6%
Silt	16.9%
Clay	9.0%

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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-49-G8

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH49-G8

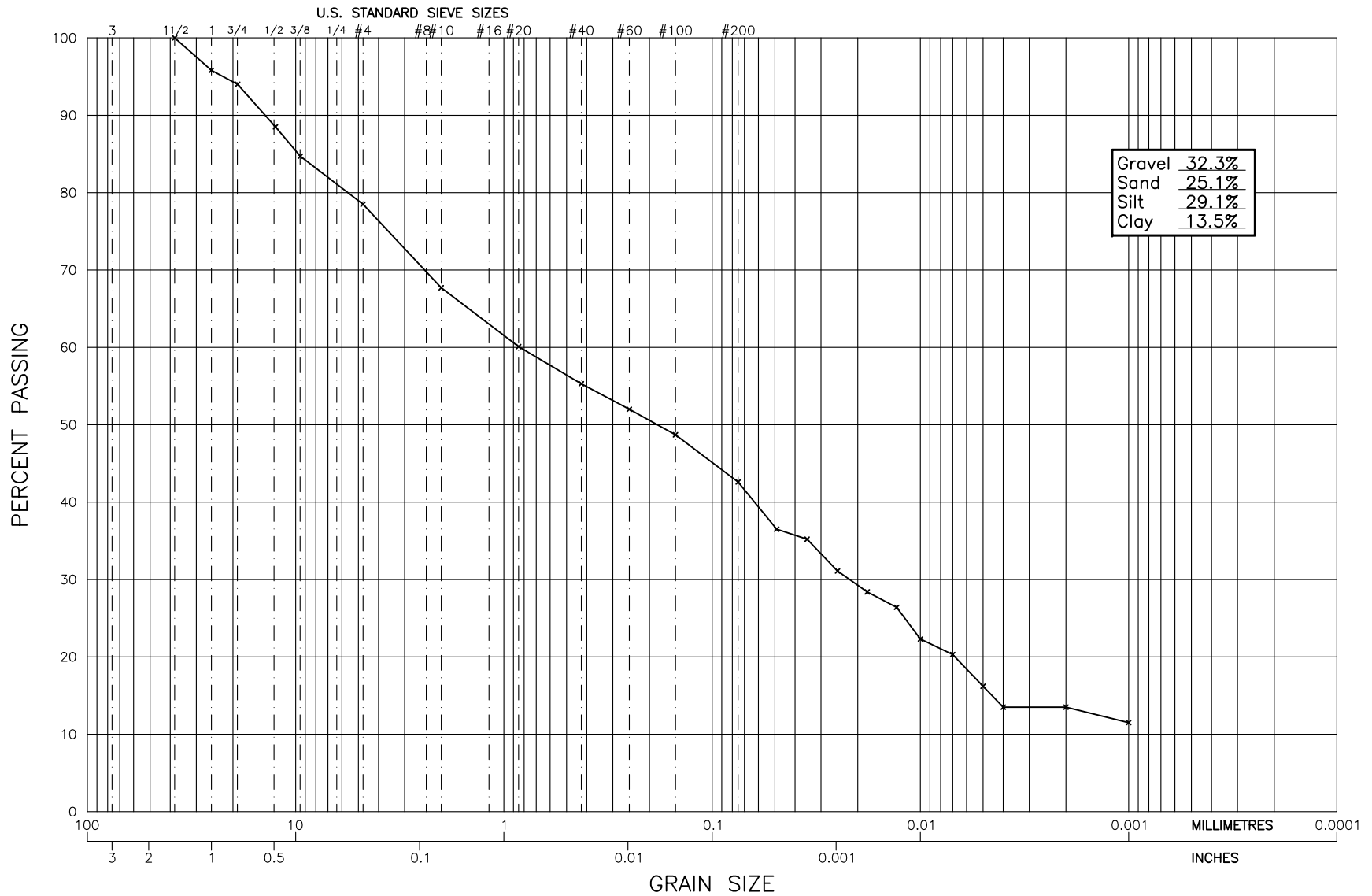
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: November 15, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand, silty, trace gravel and clay				
Sample #: G8		Test #:		Hole #: BH-BGC11-49		Depth: 9.4 - 9.6m		Time:			
Sampled By: Client			Tested By: DJ				Checked By: DJ				
Date Sampled: Aug.5, 2011			Date Received: Sept.13, 2011				Date Tested: Nov.14, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	90.5	
Wet Wt. & Tare	1302.8	25.4				20	4.5	45.5	91.0	82.3	
Dry Wt. & Tare	1072.7	19.0		891.9	100.0	40	8.3	37.2	74.4	67.3	
Water Wt.	230.1	12.5	17.5	874.4	98.0	60	10.1	27.1	54.2	49.0	
Tare Wt.	180.8	9.5	10.4	864.0	96.9	100	7.0	20.1	40.2	36.4	
Wt. Of Dry Soil	891.9	4.75	23.9	840.1	94.2	200	5.8	14.3	28.6	25.9	
Moisture Content %	25.8	10	33.3	806.8	90.5	Pan	14.3				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	891.9			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.905	0.5	23.0	17.0	0.01417	14.0	14.0	5.289	0.075	28.0	25.3
50.0	0.905	1	22.0	17.0	0.01417	13.0	14.2	3.762	0.053	26.0	23.5
50.0	0.905	2	21.5	17.0	0.01417	12.5	14.2	2.668	0.038	25.0	22.6
50.0	0.905	4	21.5	17.0	0.01417	12.5	14.2	1.886	0.027	25.0	22.6
50.0	0.905	8	20.5	17.0	0.01417	11.5	14.4	1.342	0.019	23.0	20.8
50.0	0.905	15	20.0	17.0	0.01417	11.0	14.5	0.983	0.014	22.0	19.9
50.0	0.905	30	18.0	17.0	0.01417	9.0	14.8	0.703	0.010	18.0	16.3
50.0	0.905	60	17.0	17.0	0.01417	8.0	15.0	0.500	0.007	16.0	14.5
50.0	0.905	120	15.5	17.0	0.01417	6.5	15.2	0.356	0.005	13.0	11.8
50.0	0.905	240	14.0	17.0	0.01417	5.0	15.5	0.254	0.004	10.0	9.0
50.0	0.905	480	13.0	18.0	0.01399	5.0	15.5	0.180	0.003	10.0	9.0
50.0	0.905	1440	13.0	18.0	0.01399	5.0	15.5	0.104	0.001	10.0	9.0
Hydrometer #: 932452			Graduate #: 4			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 GRAIN SIZE ANALYSIS OF BH-BGC11-49-G10

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH49-G10

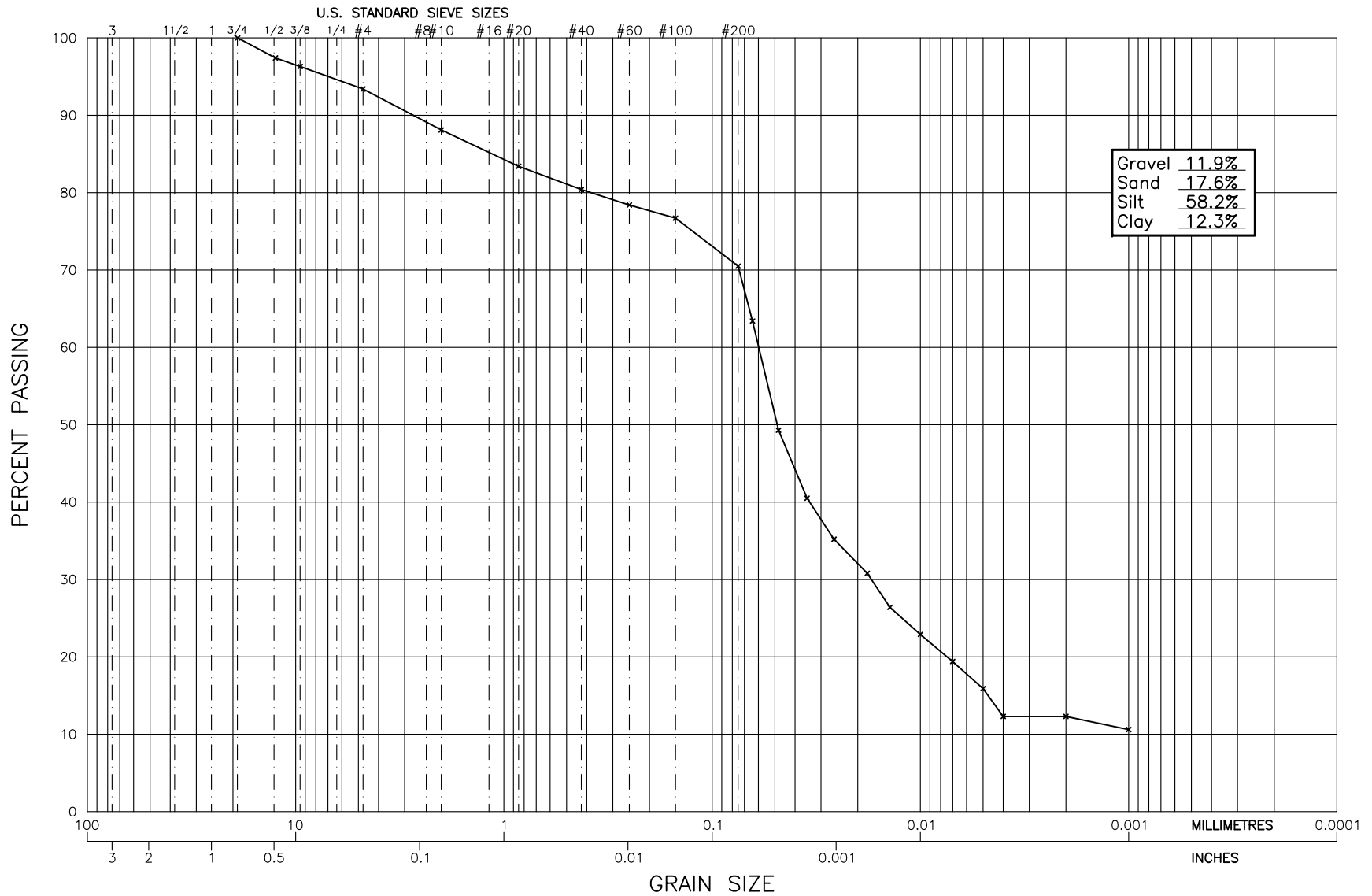
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: November 15, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel, silty and sandy, some clay				
Sample #: G10		Test #:		Hole #: BH-BGC11-49		Depth: 12.4 - 12.6m		Time:			
Sampled By: Client			Tested By: DJ				Checked By: DJ				
Date Sampled: Aug.5, 2011			Date Received: Sept.13, 2011				Date Tested: Nov.14, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		928.1	100.0	10		50.0	100.0	67.7	
Wet Wt. & Tare	1240.8	25.4	38.8	889.3	95.8	20	5.6	44.4	88.8	60.1	
Dry Wt. & Tare	1133.3	19.0	17.1	872.2	94.0	40	3.5	40.9	81.8	55.3	
Water Wt.	107.5	12.5	51.0	821.2	88.5	60	2.5	38.4	76.8	52.0	
Tare Wt.	205.2	9.5	35.3	785.9	84.7	100	2.4	36.0	72.0	48.7	
Wt. Of Dry Soil	928.1	4.75	57.0	728.9	78.5	200	4.5	31.5	63.0	42.6	
Moisture Content %	11.6	10	101.0	627.9	67.7	Pan	31.5				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	928.1			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.677	0.5	40.0	17.0	0.01417	31.0	11.2	4.729	0.067	62.0	41.9
50.0	0.677	1	36.0	17.0	0.01417	27.0	11.8	3.441	0.049	54.0	36.5
50.0	0.677	2	35.0	17.0	0.01417	26.0	12.0	2.450	0.035	52.0	35.2
50.0	0.677	4	32.0	17.0	0.01417	23.0	12.5	1.768	0.025	46.0	31.1
50.0	0.677	8	30.0	17.0	0.01417	21.0	12.8	1.266	0.018	42.0	28.4
50.0	0.677	15	28.5	17.0	0.01417	19.5	13.1	0.934	0.013	39.0	26.4
50.0	0.677	30	25.5	17.0	0.01417	16.5	13.6	0.673	0.010	33.0	22.3
50.0	0.677	60	24.0	17.0	0.01417	15.0	13.8	0.480	0.007	30.0	20.3
50.0	0.677	120	21.0	17.0	0.01417	12.0	14.3	0.345	0.005	24.0	16.2
50.0	0.677	240	19.0	17.0	0.01417	10.0	14.6	0.247	0.004	20.0	13.5
50.0	0.677	480	18.0	18.0	0.01399	10.0	14.6	0.175	0.002	20.0	13.5
50.0	0.677	1440	16.5	18.0	0.01399	8.5	14.9	0.102	0.001	17.0	11.5
Hydrometer #: 932452			Graduate #: 5			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-49-SPT2

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH49-SPT2

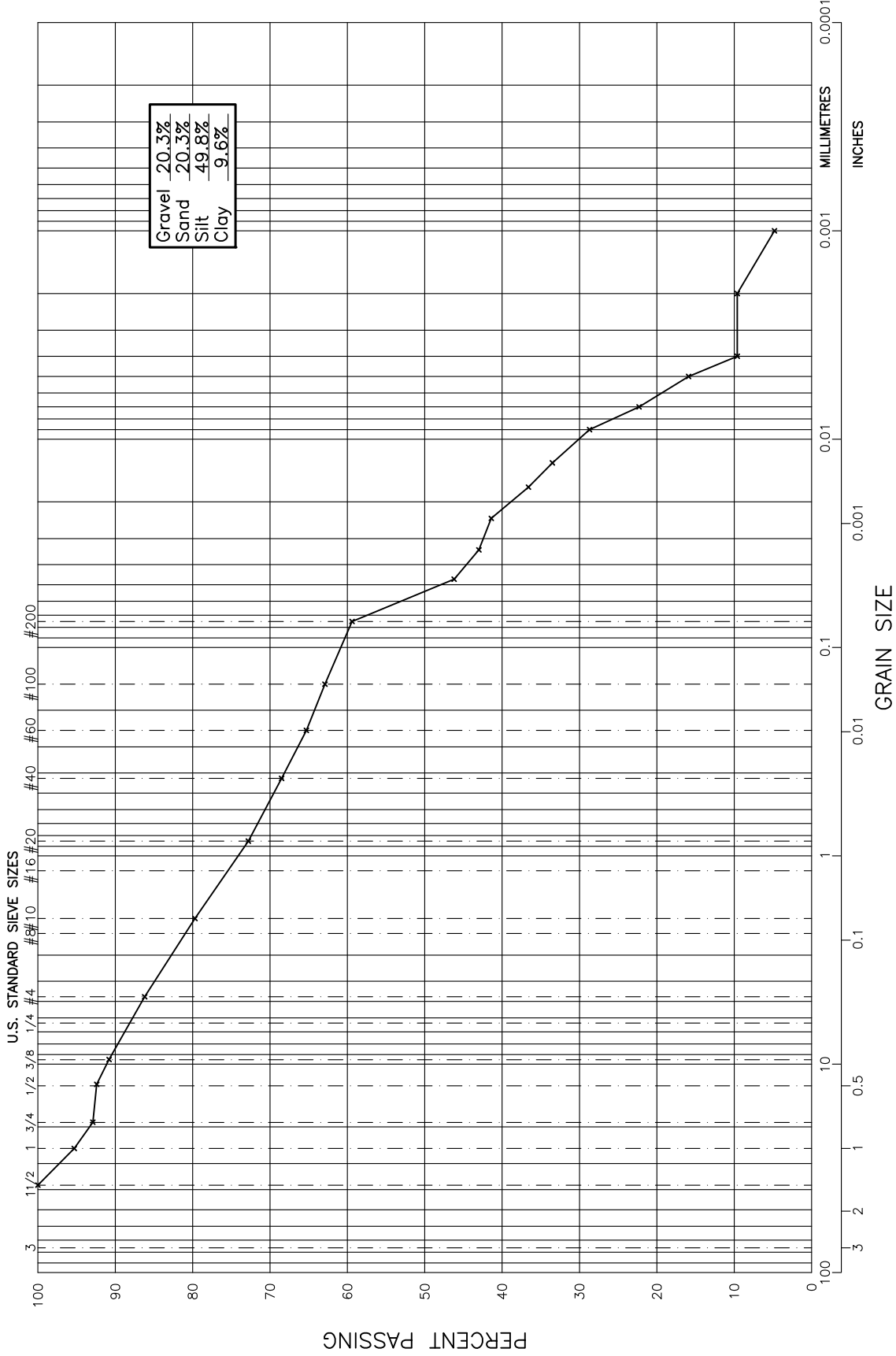
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: November 15, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt, some sand, clay and gravel				
Sample #: SPT2		Test #:		Hole #: BH-BGC11-49		Depth: 2.28 - 2.73m		Time:			
Sampled By: Client			Tested By: DJ				Checked By: DJ				
Date Sampled: Aug.5, 2011			Date Received: Sept.13, 2011				Date Tested: Nov.14, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	88.1	
Wet Wt. & Tare	855.1	25.4				20	2.7	47.3	94.6	83.4	
Dry Wt. & Tare	742.8	19.0		550.0	100.0	40	1.7	45.6	91.2	80.4	
Water Wt.	112.3	12.5	14.3	535.7	97.4	60	1.1	44.5	89.0	78.4	
Tare Wt.	192.8	9.5	6.2	529.5	96.3	100	1.0	43.5	87.0	76.7	
Wt. Of Dry Soil	550.0	4.75	15.7	513.8	93.4	200	3.5	40.0	80.0	70.5	
Moisture Content %	20.4	10	29.2	484.6	88.1	Pan	40.0				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	550.0			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.881	0.5	45.0	17.0	0.01417	36.0	10.4	4.551	0.064	72.0	63.4
50.0	0.881	1	37.0	17.0	0.01417	28.0	11.7	3.417	0.048	56.0	49.3
50.0	0.881	2	32.0	17.0	0.01417	23.0	12.5	2.500	0.035	46.0	40.5
50.0	0.881	4	29.0	17.0	0.01417	20.0	13.0	1.802	0.026	40.0	35.2
50.0	0.881	8	26.5	17.0	0.01417	17.5	13.4	1.295	0.018	35.0	30.8
50.0	0.881	15	24.0	17.0	0.01417	15.0	13.8	0.960	0.014	30.0	26.4
50.0	0.881	30	22.0	17.0	0.01417	13.0	14.2	0.687	0.010	26.0	22.9
50.0	0.881	60	20.0	17.0	0.01417	11.0	14.5	0.491	0.007	22.0	19.4
50.0	0.881	120	18.0	17.0	0.01417	9.0	14.8	0.351	0.005	18.0	15.9
50.0	0.881	240	16.0	17.0	0.01417	7.0	15.1	0.251	0.004	14.0	12.3
50.0	0.881	480	15.0	18.0	0.01399	7.0	15.1	0.178	0.002	14.0	12.3
50.0	0.881	1440	14.0	18.0	0.01399	6.0	15.3	0.103	0.001	12.0	10.6
Hydrometer #: 932452			Graduate #: 6			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL		FINE		SAND		SILT		CLAY									
COARSE	MEDIUM	1/2	3/4	1	1/2	3/8	1/4	#4	#10	#20	#40	#60	#100	#200	COARSE	MEDIUM	FINE



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-51-G6

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH51-G6

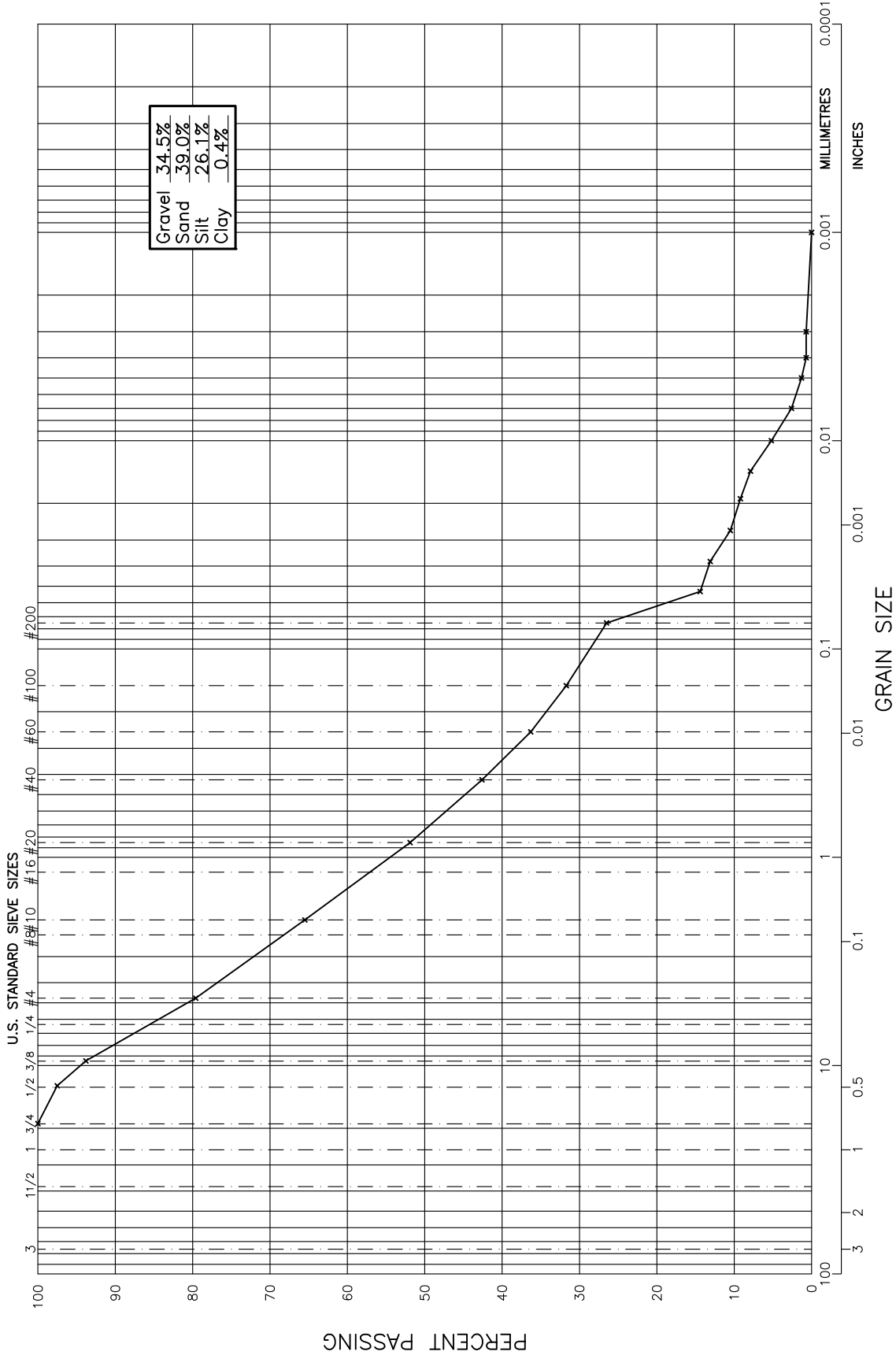
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: November 16, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt, gravelly and sandy				
Sample #: G6		Test #:		Hole #: BH-BGC11-51		Depth: 7.9 - 8.1m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: Aug.5, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.15, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		875.6	100.0	10		50.0	100.0	79.7	
Wet Wt. & Tare	1208.8	25.4	41.1	834.5	95.3	20	4.3	45.7	91.4	72.8	
Dry Wt. & Tare	1057.0	19.0	20.9	813.6	92.9	40	2.7	43.0	86.0	68.5	
Water Wt.	151.8	12.5	4.8	808.8	92.4	60	2.0	41.0	82.0	65.3	
Tare Wt.	181.4	9.5	13.7	795.1	90.8	100	1.5	39.5	79.0	62.9	
Wt. Of Dry Soil	875.6	4.75	40.2	754.9	86.2	200	2.2	37.3	74.6	59.4	
Moisture Content %	17.3	10	57.3	697.6	79.7	Pan	37.3				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	875.6			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.797	0.5	39.0	18.0	0.01399	31.0	11.2	4.729	0.066	62.0	49.4
50.0	0.797	1	37.0	18.0	0.01399	29.0	11.5	3.393	0.047	58.0	46.2
50.0	0.797	2	35.0	18.0	0.01399	27.0	11.8	2.433	0.034	54.0	43.0
50.0	0.797	4	34.0	18.0	0.01399	26.0	12.0	1.732	0.024	52.0	41.4
50.0	0.797	8	31.0	18.0	0.01399	23.0	12.5	1.250	0.017	46.0	36.6
50.0	0.797	15	29.0	18.0	0.01399	21.0	12.8	0.925	0.013	42.0	33.5
50.0	0.797	30	26.0	18.0	0.01399	18.0	13.3	0.666	0.009	36.0	28.7
50.0	0.797	60	22.0	18.0	0.01399	14.0	14.0	0.483	0.007	28.0	22.3
50.0	0.797	120	18.0	18.0	0.01399	10.0	14.6	0.349	0.005	20.0	15.9
50.0	0.797	240	14.0	18.0	0.01399	6.0	15.3	0.253	0.004	12.0	9.6
50.0	0.797	480	12.0	23.0	0.01317	6.0	15.3	0.179	0.002	12.0	9.6
50.0	0.797	1440	11.0	18.0	0.01399	3.0	15.8	0.105	0.001	6.0	4.8
Hydrometer #: 932452			Graduate #: 8			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL		SAND		SILT		CLAY	
COARSE	MEDIUM	COARSE	MEDIUM	COARSE	MEDIUM	COARSE	MEDIUM



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-51-G10

PROJECT NO.

K-3300

PLATE NO.

3300-GS-BH51-G10

GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: November 16, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand and Gravel, silty				
Sample #: G10		Test #:		Hole #: BH-BGC11-51		Depth: 13.9 - 14.1m		Time:			
Sampled By: Client			Tested By: DJ				Checked By: DJ				
Date Sampled: Aug.6, 2011			Date Received: Sept.13, 2011				Date Tested: Nov.15, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		948.8	100.0	10		50.0	100.0	65.5	
Wet Wt. & Tare	1181.4	25.4		948.8	100.0	20	10.4	39.6	79.2	51.9	
Dry Wt. & Tare	1080.3	19.0		948.8	100.0	40	7.1	32.5	65.0	42.6	
Water Wt.	101.1	12.5	23.3	925.5	97.5	60	4.8	27.7	55.4	36.3	
Tare Wt.	131.5	9.5	35.7	889.8	93.8	100	3.5	24.2	48.4	31.7	
Wt. Of Dry Soil	948.8	4.75	134.9	754.9	79.6	200	4.0	20.2	40.4	26.5	
Moisture Content %	10.7	10	133.3	621.6	65.5	Pan	20.2				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	948.8			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.655	0.5	22.0	18.0	0.01399	14.0	14.0	5.289	0.074	28.0	18.3
50.0	0.655	1	19.0	18.0	0.01399	11.0	14.5	3.805	0.053	22.0	14.4
50.0	0.655	2	18.0	18.0	0.01399	10.0	14.6	2.706	0.038	20.0	13.1
50.0	0.655	4	16.0	18.0	0.01399	8.0	15.0	1.935	0.027	16.0	10.5
50.0	0.655	8	15.0	18.0	0.01399	7.0	15.1	1.376	0.019	14.0	9.2
50.0	0.655	15	14.0	18.0	0.01399	6.0	15.3	1.010	0.014	12.0	7.9
50.0	0.655	30	12.0	18.0	0.01399	4.0	15.6	0.722	0.010	8.0	5.2
50.0	0.655	60	10.0	18.0	0.01399	2.0	16.0	0.516	0.007	4.0	2.6
50.0	0.655	120	9.0	18.0	0.01399	1.0	16.1	0.367	0.005	2.0	1.3
50.0	0.655	240	8.5	18.0	0.01399	0.5	16.2	0.260	0.004	1.0	0.7
50.0	0.655	480	7.5	20.0	0.01365	0.5	16.2	0.184	0.003	1.0	0.7
50.0	0.655	1440	7.0	20.0	0.01365	0.0	16.3	0.106	0.001	0.0	0.0
Hydrometer #: 932452			Graduate #: 7			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

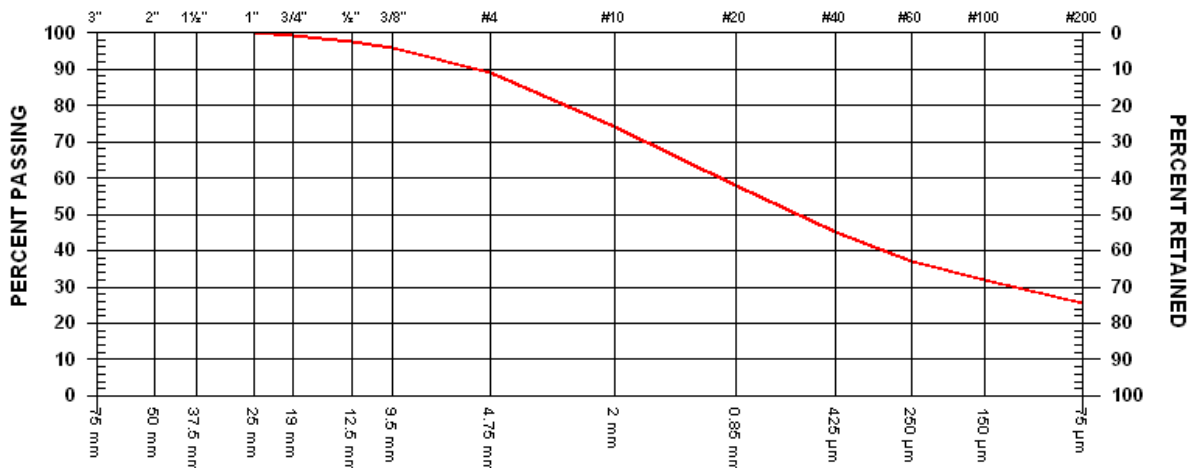
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 39 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.14 DATE SAMPLED 2011.Aug.06

SUPPLIER	BH-BGC11-51	SAMPLED BY	Client
SOURCE	G14 @ 20.1 - 20.3m	TESTED BY	DJ
SPECIFICATION		TEST METHOD	WASHED
MATERIAL TYPE	Crush		

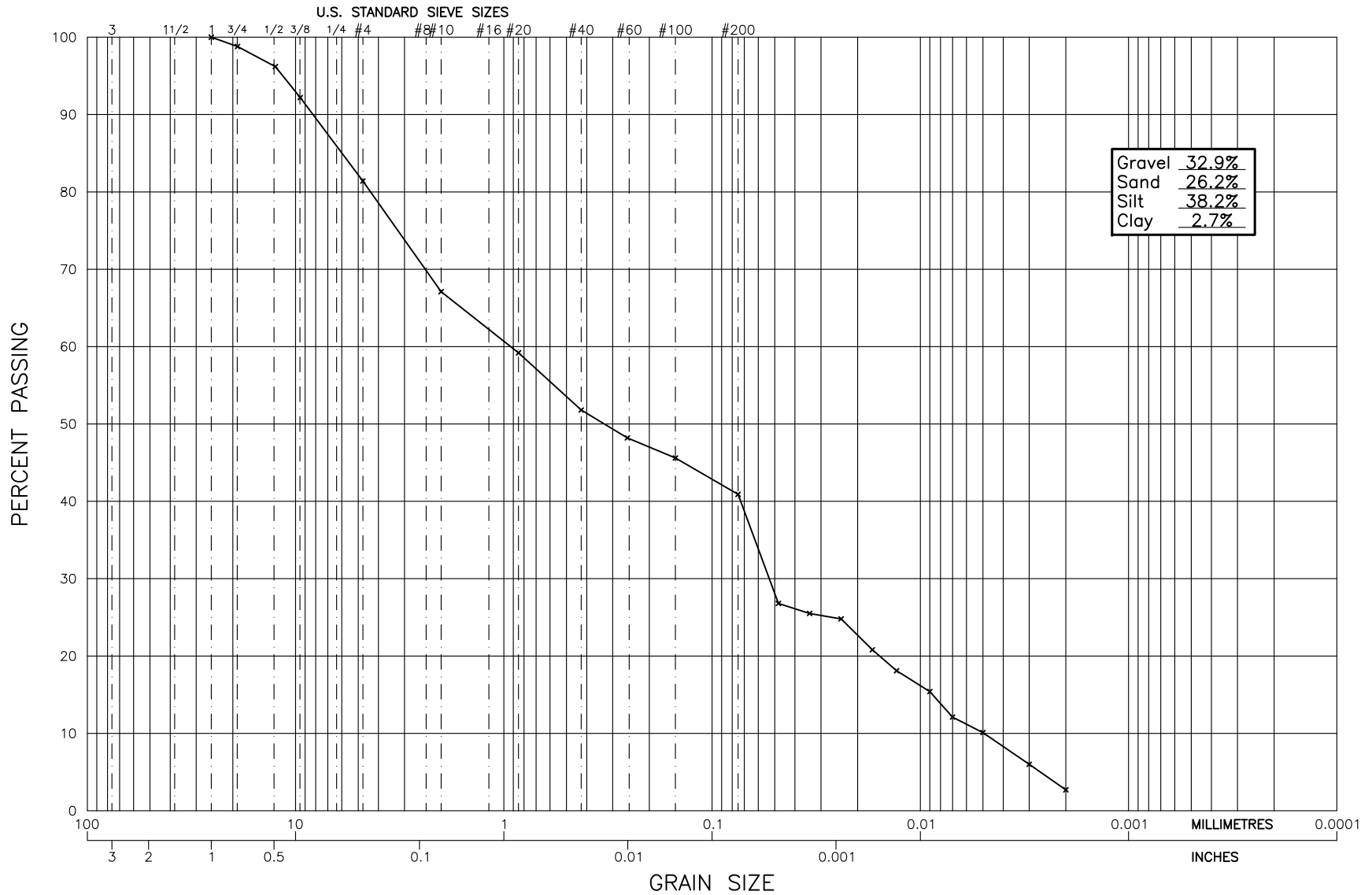


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm	100.0	
3/4" 19 mm	99.3	
1/2" 12.5 mm	97.4	
3/8" 9.5 mm	95.7	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	88.8	
No. 10 2.00 mm	74.2	
No. 20 850 µm	57.8	
No. 40 425 µm	45.0	
No. 60 250 µm	37.1	
No. 100 150 µm	31.9	
No. 200 75 µm	25.7	

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-53-G2

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH53-G2

GeoNorth Engineering

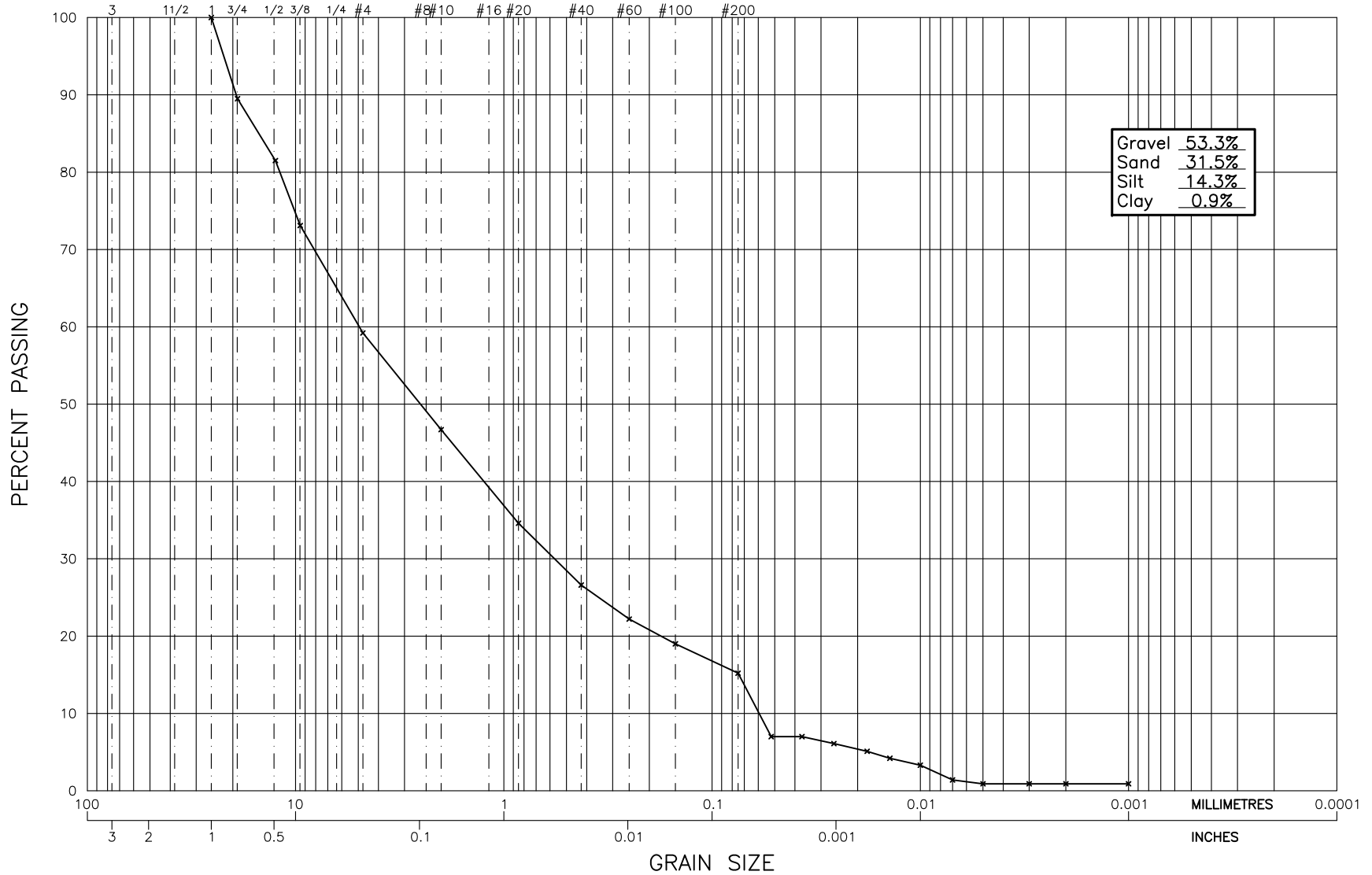
Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Sept.29, 2011					
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300					
Source/Location: Dublin Gulch, Yukon							Type: Silt/Gravel,sandy					
Sample #: G2		Test #:		Hole #: BH-BGC11-53		Depth: 1.9 - 2.1m		Time:				
Sampled By: Client				Tested By: DJ				Checked By:				
Date Sampled: August 7, 2011				Date Received: September 13, 2011				Date Tested: Sept.28, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis						
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.		
Tare No.		38.1				10		50.0	100.0	67.1		
Wet Wt. & Tare	1347.3	25.4		1038.0	100.0	20	5.9	44.1	88.2	59.2		
Dry Wt. & Tare	1229.1	19.0	12.1	1025.9	98.8	40	5.5	38.6	77.2	51.8		
Water Wt.	118.2	12.5	27.8	998.1	96.2	60	2.7	35.9	71.8	48.2		
Tare Wt.	191.1	9.5	41.0	957.1	92.2	100	1.9	34.0	68.0	45.6		
Wt. Of Dry Soil	1038.0	4.75	112.0	845.1	81.4	200	3.5	30.5	61.0	40.9		
Moisture Content %	11.4	10	148.9	696.2	67.1	Pan	30.5					
Dry Wt. Of Sample from Initial Moisture						Total	50.0					
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=						
		Total	1038.0			Tare		Wt. Passing #200 =				
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)	
50.0	0.671	0.5	30.0	22.0	0.01332	23.5	12.4	4.983	0.066	47.0	31.5	
50.0	0.671	1	26.5	22.0	0.01332	20.0	13.0	3.605	0.048	40.0	26.8	
50.0	0.671	2	25.5	22.0	0.01332	19.0	13.2	2.565	0.034	38.0	25.5	
50.0	0.671	4	25.0	22.0	0.01332	18.5	13.2	1.820	0.024	37.0	24.8	
50.0	0.671	8	22.0	22.0	0.01332	15.5	13.7	1.310	0.017	31.0	20.8	
50.0	0.671	15	20.0	22.0	0.01332	13.5	14.1	0.968	0.013	27.0	18.1	
50.0	0.671	30	18.0	22.0	0.01332	11.5	14.4	0.693	0.009	23.0	15.4	
50.0	0.671	60	15.5	22.0	0.01332	9.0	14.8	0.497	0.007	18.0	12.1	
50.0	0.671	120	14.0	22.0	0.01332	7.5	15.1	0.354	0.005	15.0	10.1	
50.0	0.671	240	10.5	24.0	0.01301	4.5	15.6	0.255	0.003	9.0	6.0	
50.0	0.671	480	8.0	24.0	0.01301	2.0	16.0	0.182	0.002	4.0	2.7	
50.0	0.671	1440	8.0	24.0	0.01301	2.0	16.0	0.105	0.001	4.0	2.7	
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml			
Density of Solids:												
Description of Sample:												

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	

U.S. STANDARD SIEVE SIZES



Gravel	53.3%
Sand	31.5%
Silt	14.3%
Clay	0.9%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-53-G4

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH53-G4

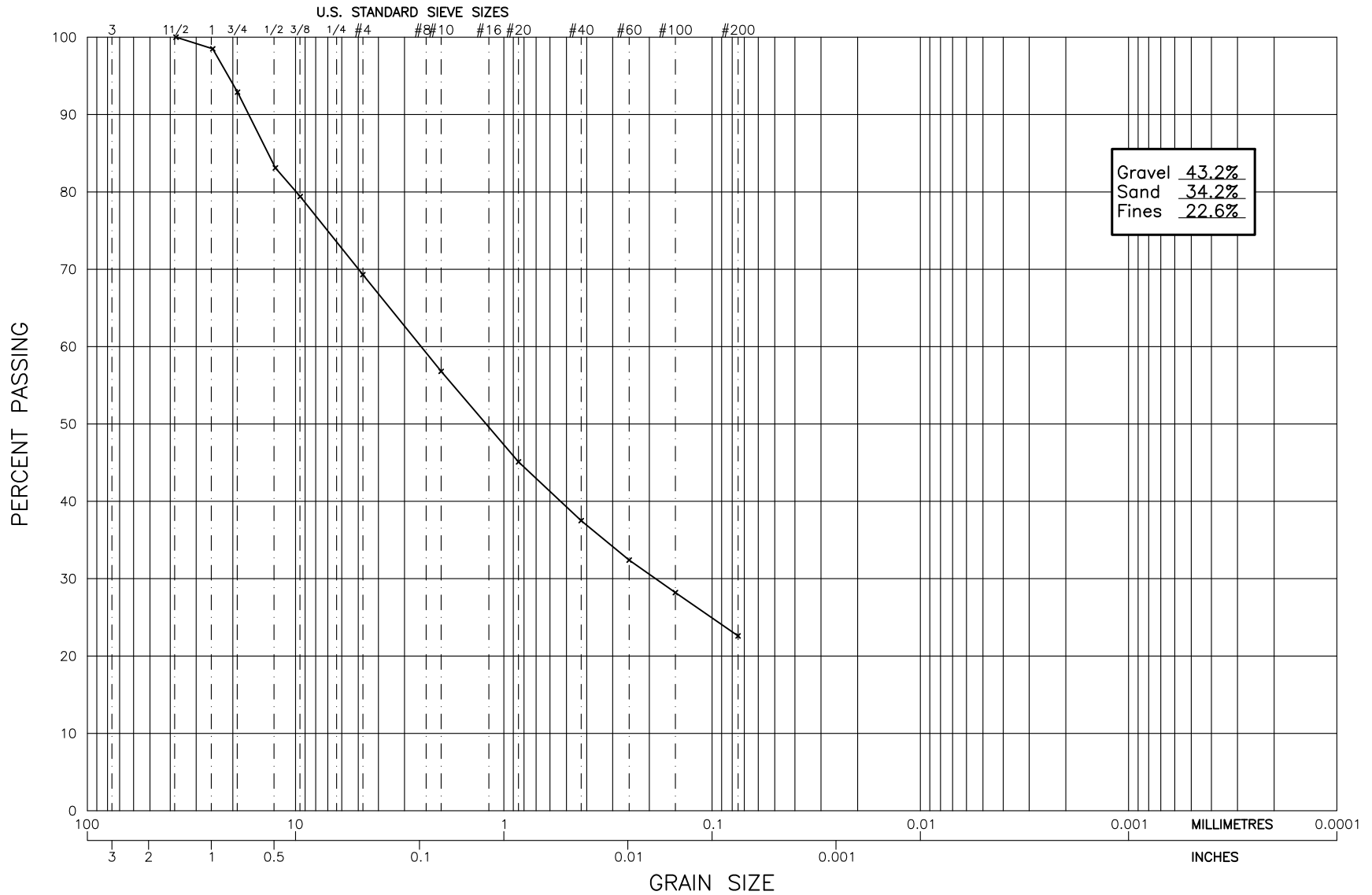
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Sept.29, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel,sandy,some silt				
Sample #: G4		Test #:		Hole #: BH-BGC11-53		Depth: 4.9 - 5.1m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 7, 2011				Date Received: September 13, 2011				Date Tested: Sept.28, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	46.7	
Wet Wt. & Tare	1239.1	25.4		1016.0	100.0	20	13.0	37.0	74.0	34.6	
Dry Wt. & Tare	1195.8	19.0	106.9	909.1	89.5	40	8.5	28.5	57.0	26.6	
Water Wt.	43.3	12.5	81.0	828.1	81.5	60	4.7	23.8	47.6	22.2	
Tare Wt.	179.8	9.5	85.6	742.5	73.1	100	3.5	20.3	40.6	19.0	
Wt. Of Dry Soil	1016.0	4.75	140.8	601.7	59.2	200	4.0	16.3	32.6	15.2	
Moisture Content %	4.3	10	127.0	474.7	46.7	Pan	16.3				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	1016.0			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.467	0.5	17.0	22.0	0.01332	10.5	14.6	5.397	0.072	21.0	9.8
50.0	0.467	1	14.0	22.0	0.01332	7.5	15.1	3.880	0.052	15.0	7.0
50.0	0.467	2	14.0	22.0	0.01332	7.5	15.1	2.744	0.037	15.0	7.0
50.0	0.467	4	13.0	22.0	0.01332	6.5	15.2	1.951	0.026	13.0	6.1
50.0	0.467	8	12.0	22.0	0.01332	5.5	15.4	1.387	0.018	11.0	5.1
50.0	0.467	15	11.0	22.0	0.01332	4.5	15.6	1.018	0.014	9.0	4.2
50.0	0.467	30	10.0	22.0	0.01332	3.5	15.7	0.724	0.010	7.0	3.3
50.0	0.467	60	8.0	22.0	0.01332	1.5	16.0	0.517	0.007	3.0	1.4
50.0	0.467	120	7.5	22.0	0.01332	1.0	16.1	0.367	0.005	2.0	0.9
50.0	0.467	240	7.0	24.0	0.01301	1.0	16.1	0.259	0.003	2.0	0.9
50.0	0.467	480	7.0	24.0	0.01301	1.0	16.1	0.183	0.002	2.0	0.9
50.0	0.467	1440	7.0	24.0	0.01301	1.0	16.1	0.106	0.001	2.0	0.9
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	43.2%
Sand	34.2%
Fines	22.6%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-53-G6

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH53-G6



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

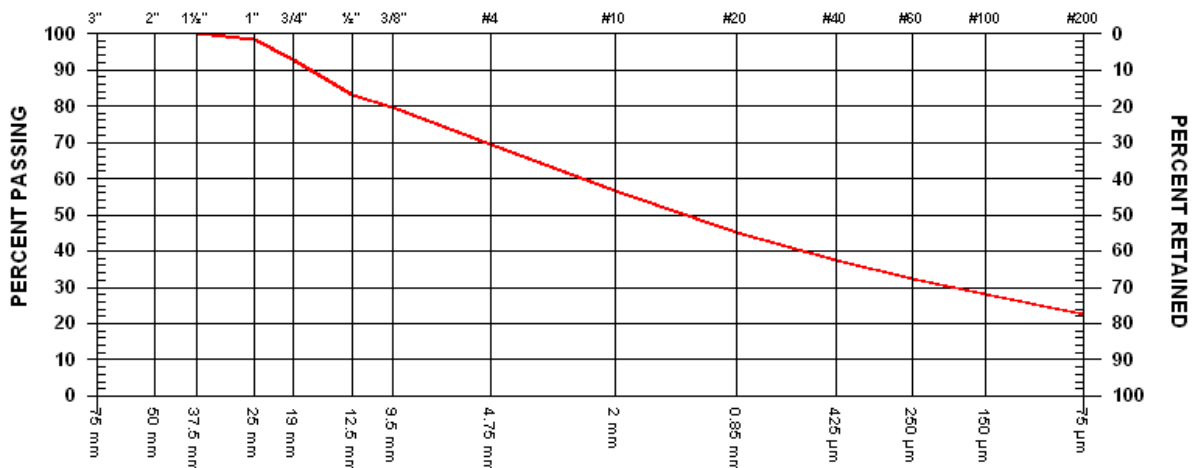
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 5 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Sep.29 DATE SAMPLED 2011.Aug.07

SUPPLIER	BH-BGC11-53	SAMPLED BY	Client
SOURCE	G6 @ 7.9 - 8.1m	TESTED BY	DJ
SPECIFICATION		TEST METHOD	WASHED
MATERIAL TYPE	Gravel		

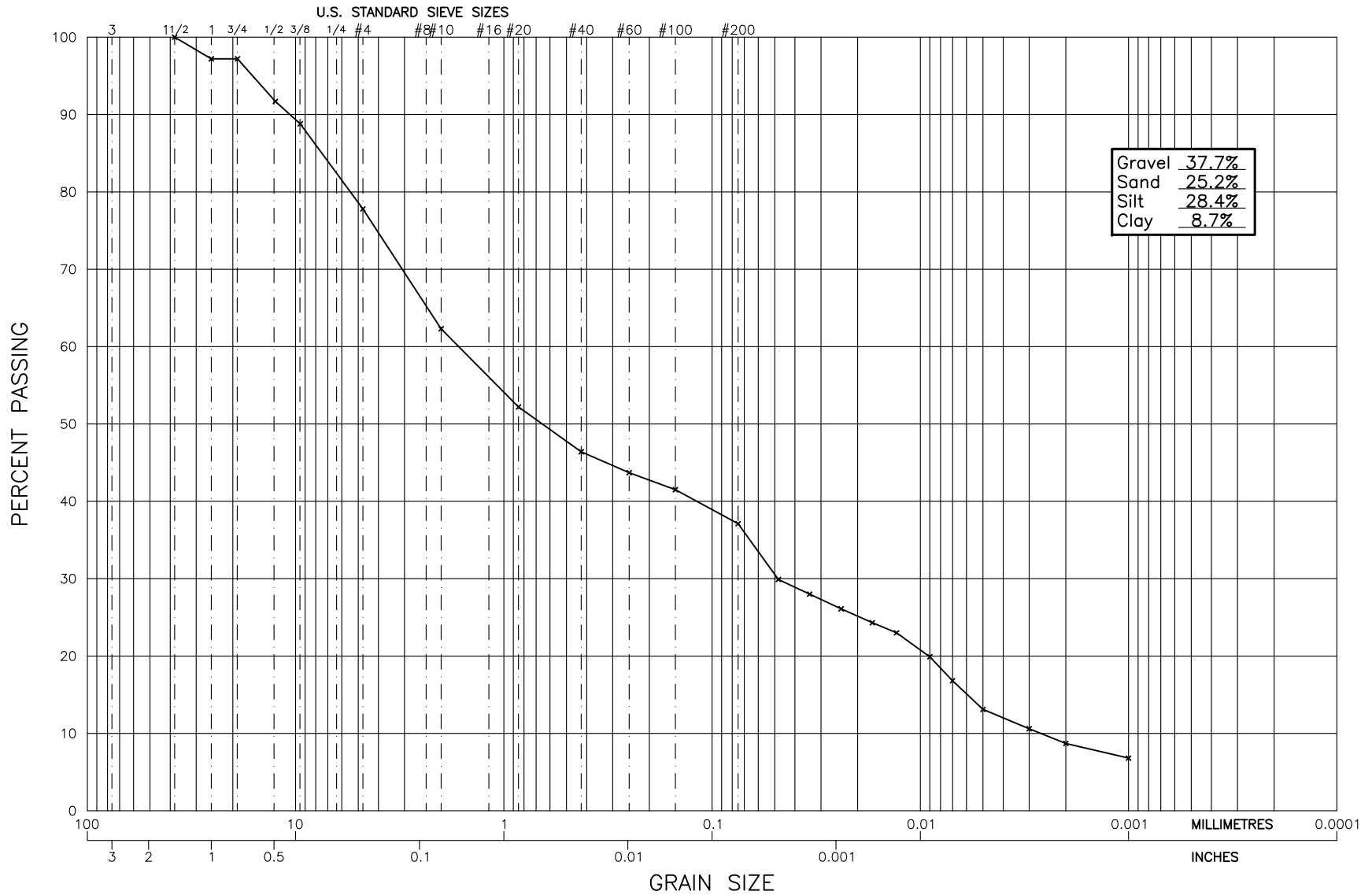


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	100.0
1"	25 mm	98.5
3/4"	19 mm	92.9
1/2"	12.5 mm	83.1
3/8"	9.5 mm	79.4

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	69.3
No. 10	2.00 mm	56.8
No. 20	850 µm	45.1
No. 40	425 µm	37.5
No. 60	250 µm	32.4
No. 100	150 µm	28.2
No. 200	75 µm	22.6

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-53-SPT2

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH53-SPT2

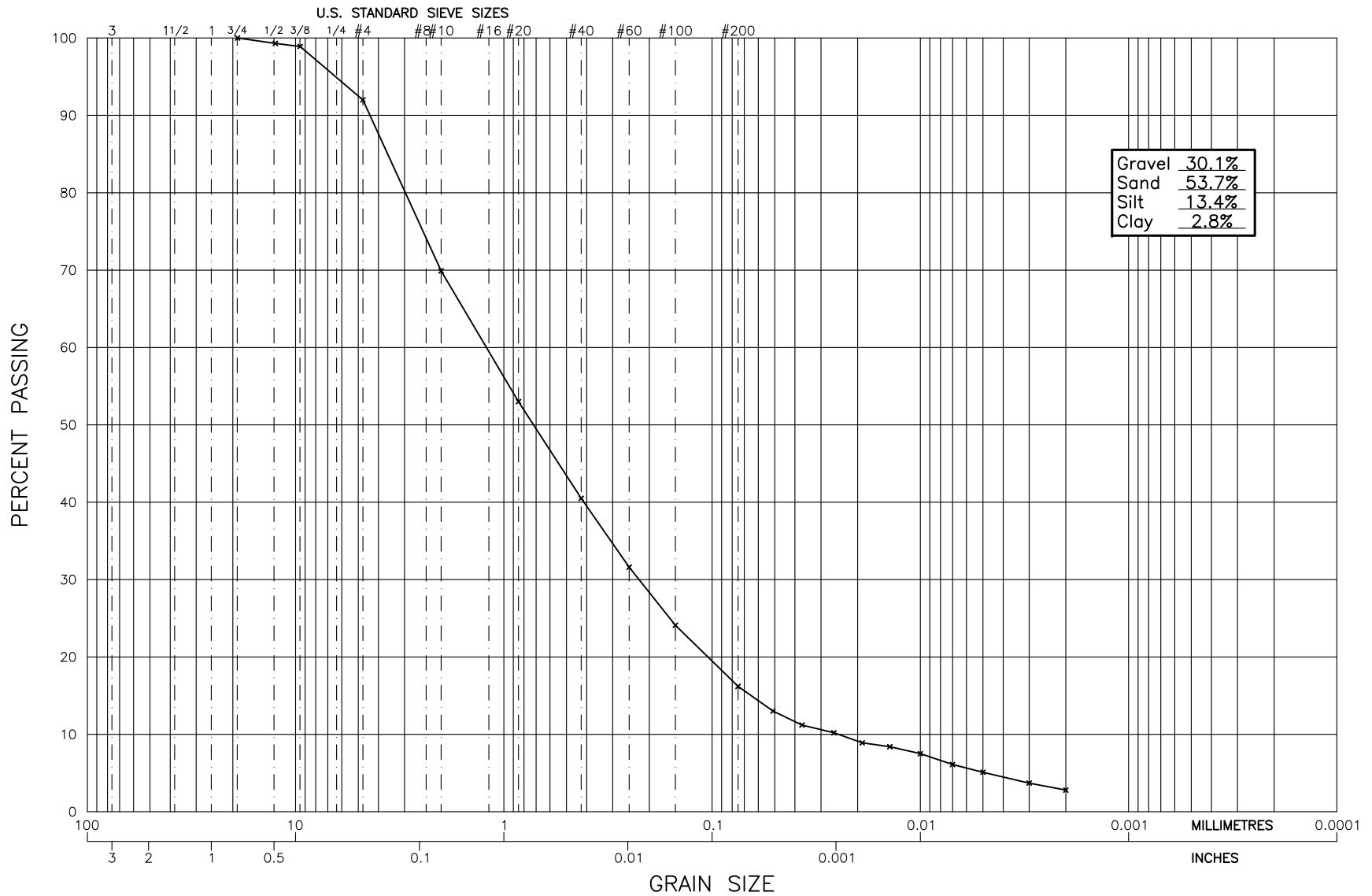
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct.6, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel,silty,sandy				
Sample #: SPT2		Test #:		Hole #: BH-BGC11-53		Depth: 2.28 - 2.73m		Time:			
Sampled By: Client				Tested By: MM				Checked By: DJ			
Date Sampled: August 7, 2011				Date Received: September 13, 2011				Date Tested: Oct.4, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		649.4	100.0	10		50.0	100.0	62.3	
Wet Wt. & Tare	896.1	25.4	18.4	631.0	97.2	20	8.1	41.9	83.8	52.2	
Dry Wt. & Tare	830.2	19.0	0.0	631.0	97.2	40	4.6	37.3	74.6	46.4	
Water Wt.	65.9	12.5	35.7	595.3	91.7	60	2.2	35.1	70.2	43.7	
Tare Wt.	180.8	9.5	18.9	576.4	88.8	100	1.8	33.3	66.6	41.5	
Wt. Of Dry Soil	649.4	4.75	71.0	505.4	77.8	200	3.5	29.8	59.6	37.1	
Moisture Content %	10.1	10	101.1	404.3	62.3	Pan	29.8				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	649.4			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.623	0.5	35.0	20.0	0.01365	28.0	11.7	4.832	0.066	56.0	34.9
50.0	0.623	1	31.0	20.0	0.01365	24.0	12.3	3.512	0.048	48.0	29.9
50.0	0.623	2	29.5	20.0	0.01365	22.5	12.6	2.508	0.034	45.0	28.0
50.0	0.623	4	28.0	20.0	0.01365	21.0	12.8	1.791	0.024	42.0	26.1
50.0	0.623	8	26.5	20.0	0.01365	19.5	13.1	1.279	0.017	39.0	24.3
50.0	0.623	15	25.5	20.0	0.01365	18.5	13.2	0.940	0.013	37.0	23.0
50.0	0.623	30	23.0	20.0	0.01365	16.0	13.7	0.675	0.009	32.0	19.9
50.0	0.623	60	20.5	20.0	0.01365	13.5	14.1	0.484	0.007	27.0	16.8
50.0	0.623	120	17.5	21.0	0.01348	10.5	14.6	0.348	0.005	21.0	13.1
50.0	0.623	240	15.5	21.0	0.01348	8.5	14.9	0.249	0.003	17.0	10.6
50.0	0.623	480	14.0	21.0	0.01348	7.0	15.1	0.178	0.002	14.0	8.7
50.0	0.623	1440	12.5	20.0	0.01365	5.5	15.4	0.103	0.001	11.0	6.8
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	30.1%
Sand	53.7%
Silt	13.4%
Clay	2.8%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-53-SPT4

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH53-SPT4

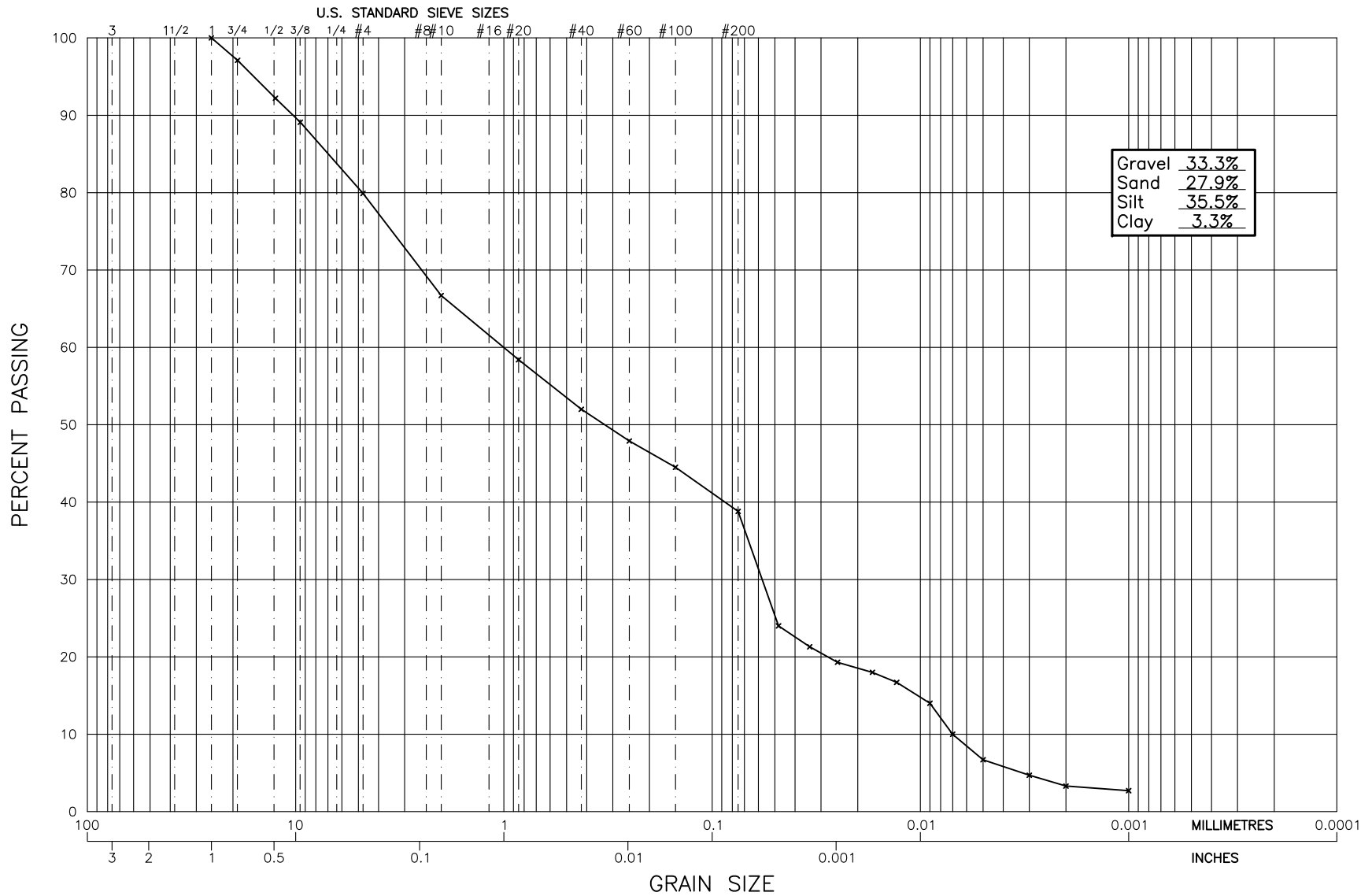
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct. 6, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand, gravelly, some silt				
Sample #: SPT4		Test #:		Hole #: BH-BGC11-53		Depth: 6.85 - 7.30m		Time:			
Sampled By: Client				Tested By: MM				Checked By:			
Date Sampled: August 7, 2011				Date Received: September 13, 2011				Date Tested: Oct. 4, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		75.0	100.0	69.9
Wet Wt. & Tare	661.4		25.4				20	18.1	56.9	75.9	53.0
Dry Wt. & Tare	646.7		19.0		464.3	100.0	40	13.4	43.5	58.0	40.5
Water Wt.	14.7		12.5	3.2	461.1	99.3	60	9.6	33.9	45.2	31.6
Tare Wt.	182.4		9.5	2.1	459.0	98.9	100	8.0	25.9	34.5	24.1
Wt. Of Dry Soil	464.3		4.75	31.9	427.1	92.0	200	8.5	17.4	23.2	16.2
Moisture Content %	3.2		10	102.7	324.4	69.9	Pan	17.4			
Dry Wt. Of Sample from Initial Moisture							Total	75.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=				
			Total	464.3			Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
75.0	0.699	0.5	24.0	20.0	0.01365	17.0	13.5	5.194	0.071	22.7	15.8
75.0	0.699	1	21.0	20.0	0.01365	14.0	14.0	3.740	0.051	18.7	13.0
75.0	0.699	2	19.0	20.0	0.01365	12.0	14.3	2.675	0.037	16.0	11.2
75.0	0.699	4	18.0	20.0	0.01365	11.0	14.5	1.903	0.026	14.7	10.2
75.0	0.699	8	16.5	20.0	0.01365	9.5	14.7	1.357	0.019	12.7	8.9
75.0	0.699	15	16.0	20.0	0.01365	9.0	14.8	0.994	0.014	12.0	8.4
75.0	0.699	30	15.0	20.0	0.01365	8.0	15.0	0.707	0.010	10.7	7.5
75.0	0.699	60	13.5	20.0	0.01365	6.5	15.2	0.504	0.007	8.7	6.1
75.0	0.699	120	12.5	21.0	0.01348	5.5	15.4	0.358	0.005	7.3	5.1
75.0	0.699	240	11.0	21.0	0.01348	4.0	15.6	0.255	0.003	5.3	3.7
75.0	0.699	480	10.0	21.0	0.01348	3.0	15.8	0.181	0.002	4.0	2.8
75.0	0.699	1440	10.0	20.0	0.01365	3.0	15.8	0.105	0.001	4.0	2.8
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-55-G4

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH55-G4

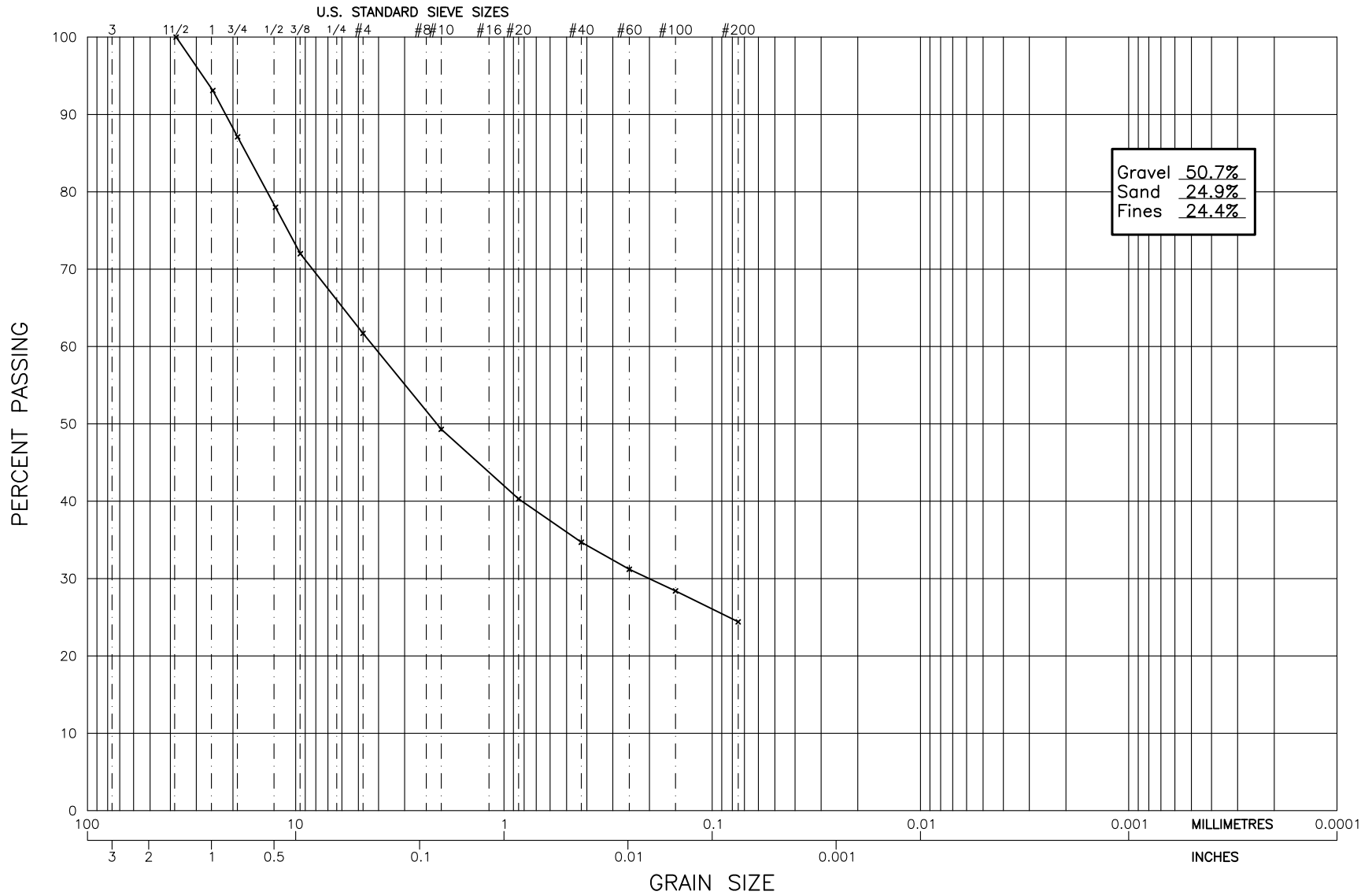
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold Soil and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt/Gravel/Sand				
Sample #: G4		Test #:		Hole #: BH-BGC11-55		Depth: 4.9 - 5.1m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 7, 2011				Date Received: September 13, 2011				Date Tested: Sept. 23, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	66.7	
Wet Wt. & Tare	1345.5	25.4		1054.1	100.0	20	6.2	43.8	87.6	58.4	
Dry Wt. & Tare	1234.1	19.0	30.1	1024.0	97.1	40	4.8	39.0	78.0	52.0	
Water Wt.	111.4	12.5	52.6	971.4	92.2	60	3.1	35.9	71.8	47.9	
Tare Wt.	180.0	9.5	32.6	938.8	89.1	100	2.5	33.4	66.8	44.5	
Wt. Of Dry Soil	1054.1	4.75	96.3	842.5	79.9	200	4.3	29.1	58.2	38.8	
Moisture Content %	10.6	10	139.6	702.9	66.7	Pan	29.1				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	1054.1			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.667	0.5	29.0	23.0	0.01317	23.0	12.5	5.000	0.066	46.0	30.7
50.0	0.667	1	24.0	23.0	0.01317	18.0	13.3	3.650	0.048	36.0	24.0
50.0	0.667	2	22.0	23.0	0.01317	16.0	13.7	2.613	0.034	32.0	21.3
50.0	0.667	4	20.5	23.0	0.01317	14.5	13.9	1.864	0.025	29.0	19.3
50.0	0.667	8	19.5	23.0	0.01317	13.5	14.1	1.326	0.017	27.0	18.0
50.0	0.667	15	18.5	23.0	0.01317	12.5	14.2	0.974	0.013	25.0	16.7
50.0	0.667	30	16.5	23.0	0.01317	10.5	14.6	0.697	0.009	21.0	14.0
50.0	0.667	60	13.5	23.0	0.01317	7.5	15.1	0.501	0.007	15.0	10.0
50.0	0.667	120	11.0	23.0	0.01317	5.0	15.5	0.359	0.005	10.0	6.7
50.0	0.667	240	9.5	23.0	0.01317	3.5	15.7	0.256	0.003	7.0	4.7
50.0	0.667	480	8.5	23.0	0.01317	2.5	15.9	0.182	0.002	5.0	3.3
50.0	0.667	1450	8.0	23.0	0.01317	2.0	16.0	0.105	0.001	4.0	2.7
Hydrometer #: 932452			23		Dispersing Agent: Sodium Hex				Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-55-SPT1

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH55-SPT1



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 630 - 1718 Argyle Street
 Halifax, Nova Scotia
 B3J 3N6

ATTN: Peter Quinn

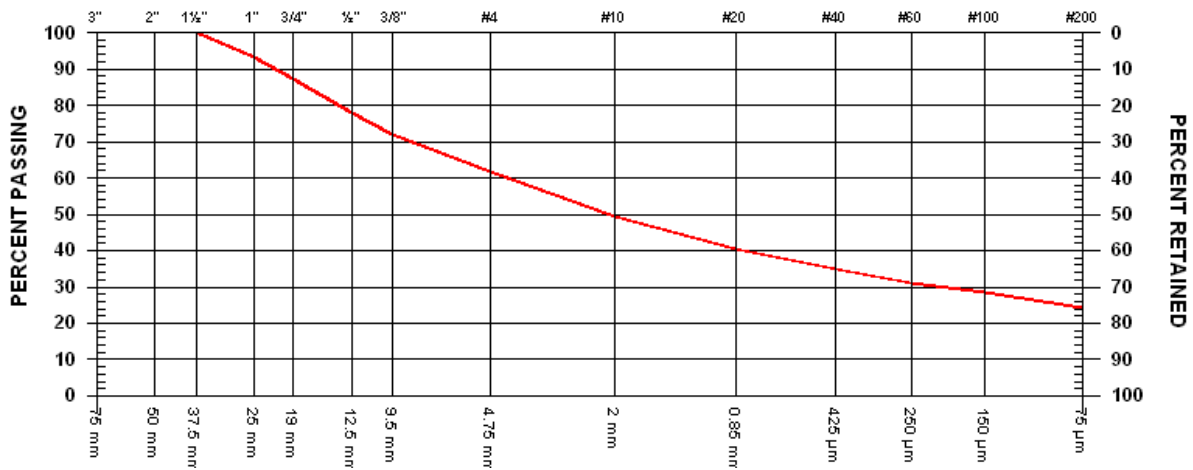
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 2 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Sep.21 DATE SAMPLED 2011.Aug.07

SUPPLIER BH-BGC11-55-SPT1 SAMPLED BY Client
 SOURCE 0.76 - 1.21m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel

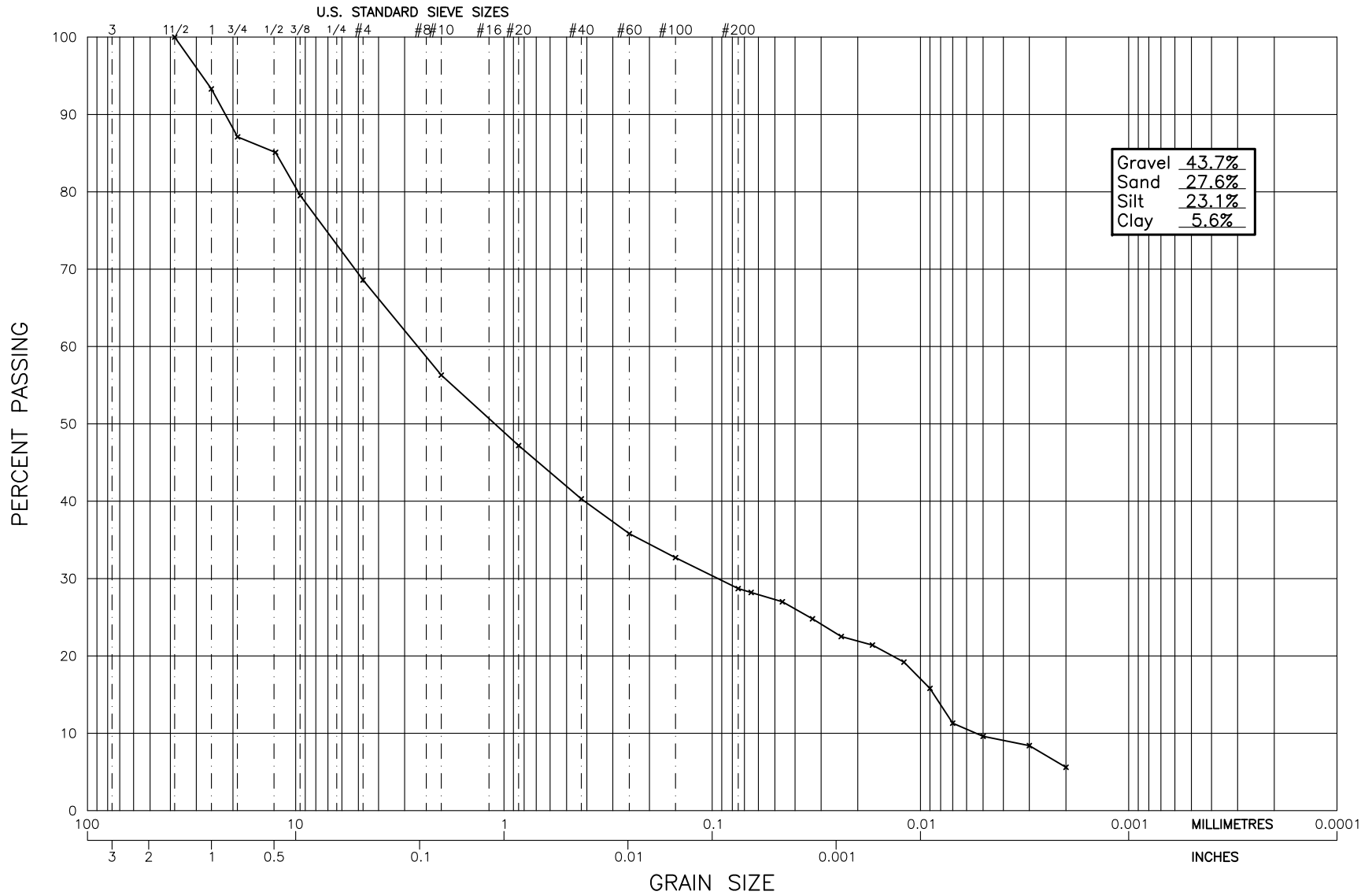


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	100.0
1"	25 mm	93.1
3/4"	19 mm	87.1
1/2"	12.5 mm	78.0
3/8"	9.5 mm	72.0

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	61.7
No. 10	2.00 mm	49.3
No. 20	850 µm	40.3
No. 40	425 µm	34.7
No. 60	250 µm	31.2
No. 100	150 µm	28.4
No. 200	75 µm	24.4

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	43.7%
Sand	27.6%
Silt	23.1%
Clay	5.6%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-55-SPT6

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH55-SPT6

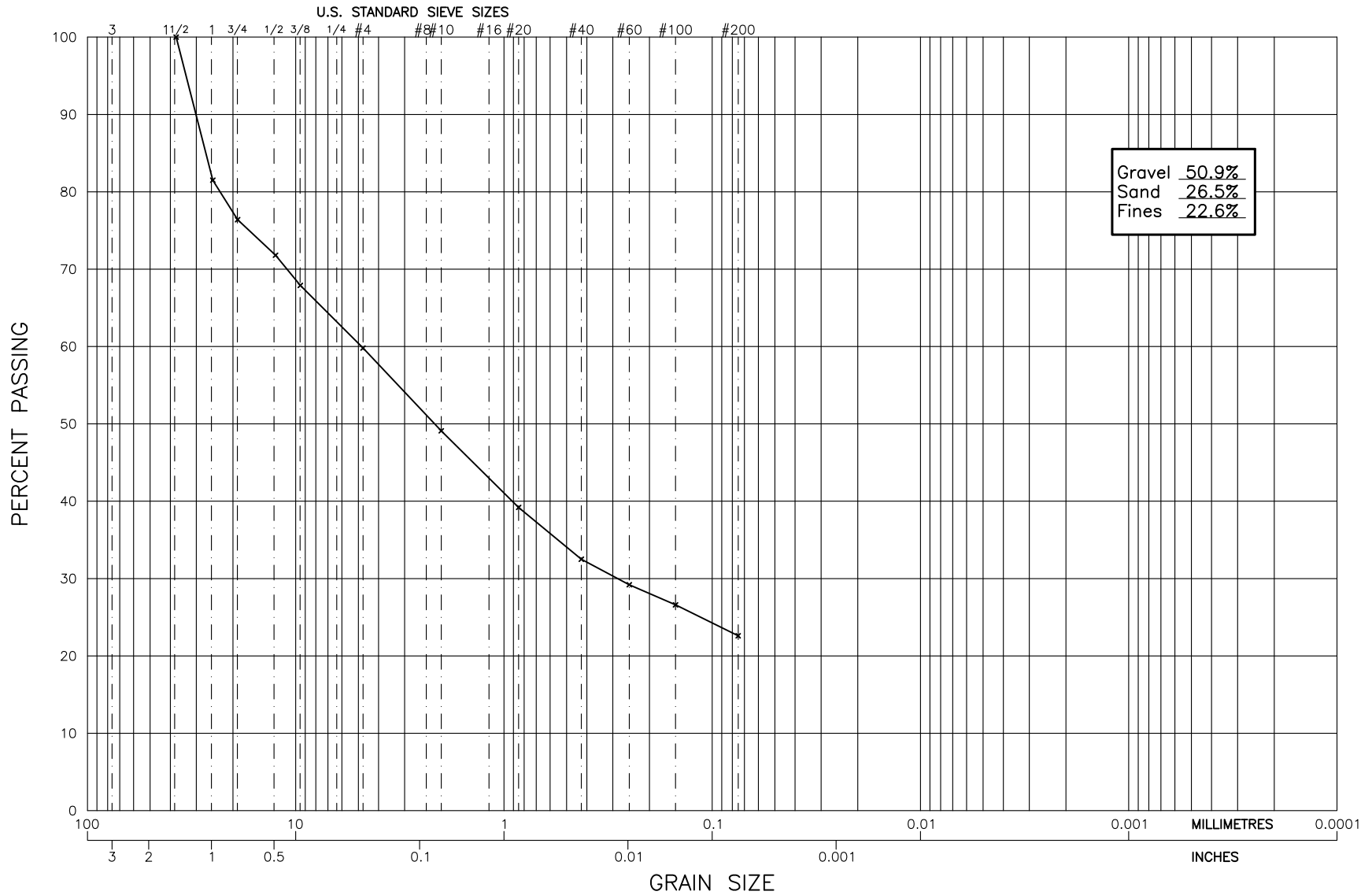
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel,sandy/silty				
Sample #: SPT 6		Test #:		Hole #: BH-BGC11-55		Depth: 8.38 - 8.83m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 7, 2011				Date Received: September 13, 2011				Date Tested: Sept. 21, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1		407.6	100.0	10		50.0	100.0	56.3
Wet Wt. & Tare	613.9		25.4	27.3	380.3	93.3	20	8.1	41.9	83.8	47.2
Dry Wt. & Tare	587.3		19.0	25.4	354.9	87.1	40	6.1	35.8	71.6	40.3
Water Wt.	26.6		12.5	8.1	346.8	85.1	60	4.0	31.8	63.6	35.8
Tare Wt.	179.7		9.5	22.7	324.1	79.5	100	2.8	29.0	58.0	32.7
Wt. Of Dry Soil	407.6		4.75	44.3	279.8	68.6	200	3.5	25.5	51.0	28.7
Moisture Content %	6.5		10	50.2	229.6	56.3	Pan	25.5			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=				
			Total	407.6			Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.563	0.5	31.0	23.0	0.01317	25.0	12.2	4.934	0.065	50.0	28.2
50.0	0.563	1	30.0	23.0	0.01317	24.0	12.3	3.512	0.046	48.0	27.0
50.0	0.563	2	28.0	23.0	0.01317	22.0	12.7	2.516	0.033	44.0	24.8
50.0	0.563	4	26.0	23.0	0.01317	20.0	13.0	1.802	0.024	40.0	22.5
50.0	0.563	8	25.0	23.0	0.01317	19.0	13.2	1.283	0.017	38.0	21.4
50.0	0.563	15	23.0	23.0	0.01317	17.0	13.5	0.948	0.012	34.0	19.2
50.0	0.563	30	20.0	23.0	0.01317	14.0	14.0	0.683	0.009	28.0	15.8
50.0	0.563	60	16.0	23.0	0.01317	10.0	14.6	0.494	0.007	20.0	11.3
50.0	0.563	120	15.0	22.0	0.01332	8.5	14.9	0.352	0.005	17.0	9.6
50.0	0.563	240	14.0	22.0	0.01332	7.5	15.1	0.250	0.003	15.0	8.4
50.0	0.563	480	12.0	21.0	0.01348	5.0	15.5	0.180	0.002	10.0	5.6
50.0	0.563	1440	11.0	23.0	0.01317	5.0	15.5	0.104	0.001	10.0	5.6
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	50.9%
Sand	26.5%
Fines	22.6%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-56-G2

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH56-G2



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

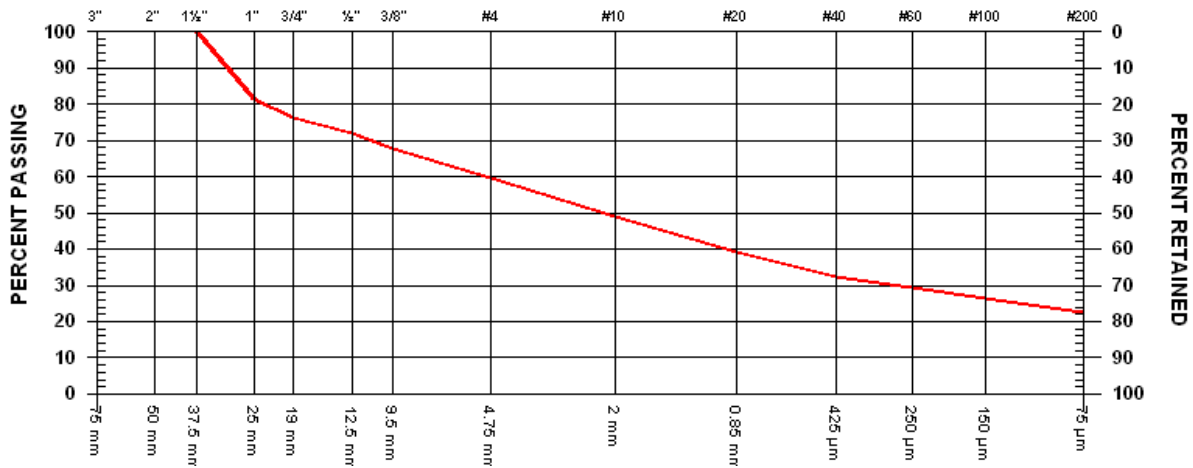
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 7 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Sep.30 DATE SAMPLED 2011.Aug.08

SUPPLIER BH-BGC11-56 SAMPLED BY Client
 SOURCE G2 @ 1.9 - 2.1m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel

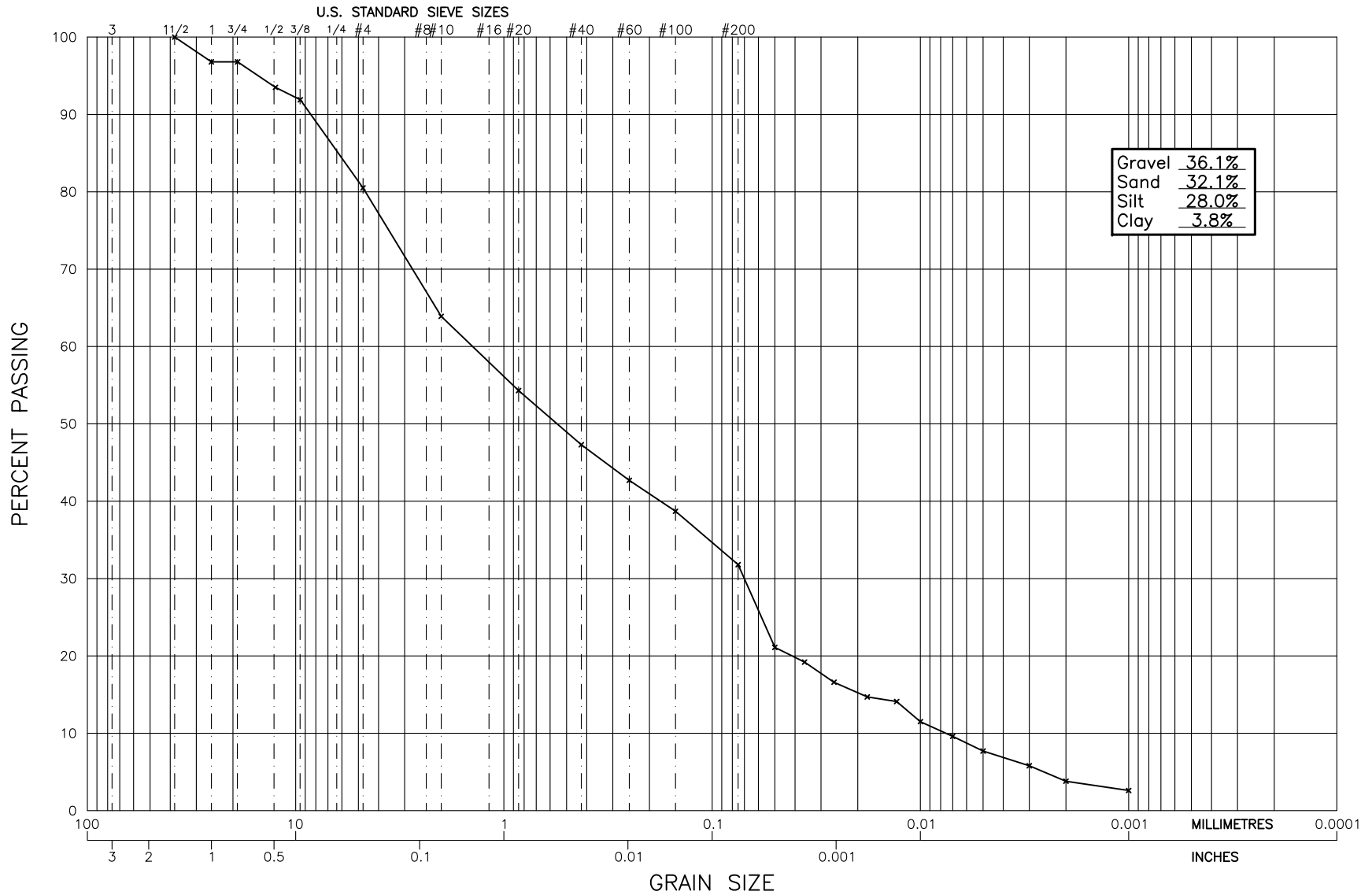


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	100.0
1"	25 mm	81.5
3/4"	19 mm	76.4
1/2"	12.5 mm	71.8
3/8"	9.5 mm	67.9

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	59.8
No. 10	2.00 mm	49.1
No. 20	850 µm	39.2
No. 40	425 µm	32.5
No. 60	250 µm	29.2
No. 100	150 µm	26.6
No. 200	75 µm	22.6

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	36.1%
Sand	32.1%
Silt	28.0%
Clay	3.8%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-57-SPT2

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH57-SPT2

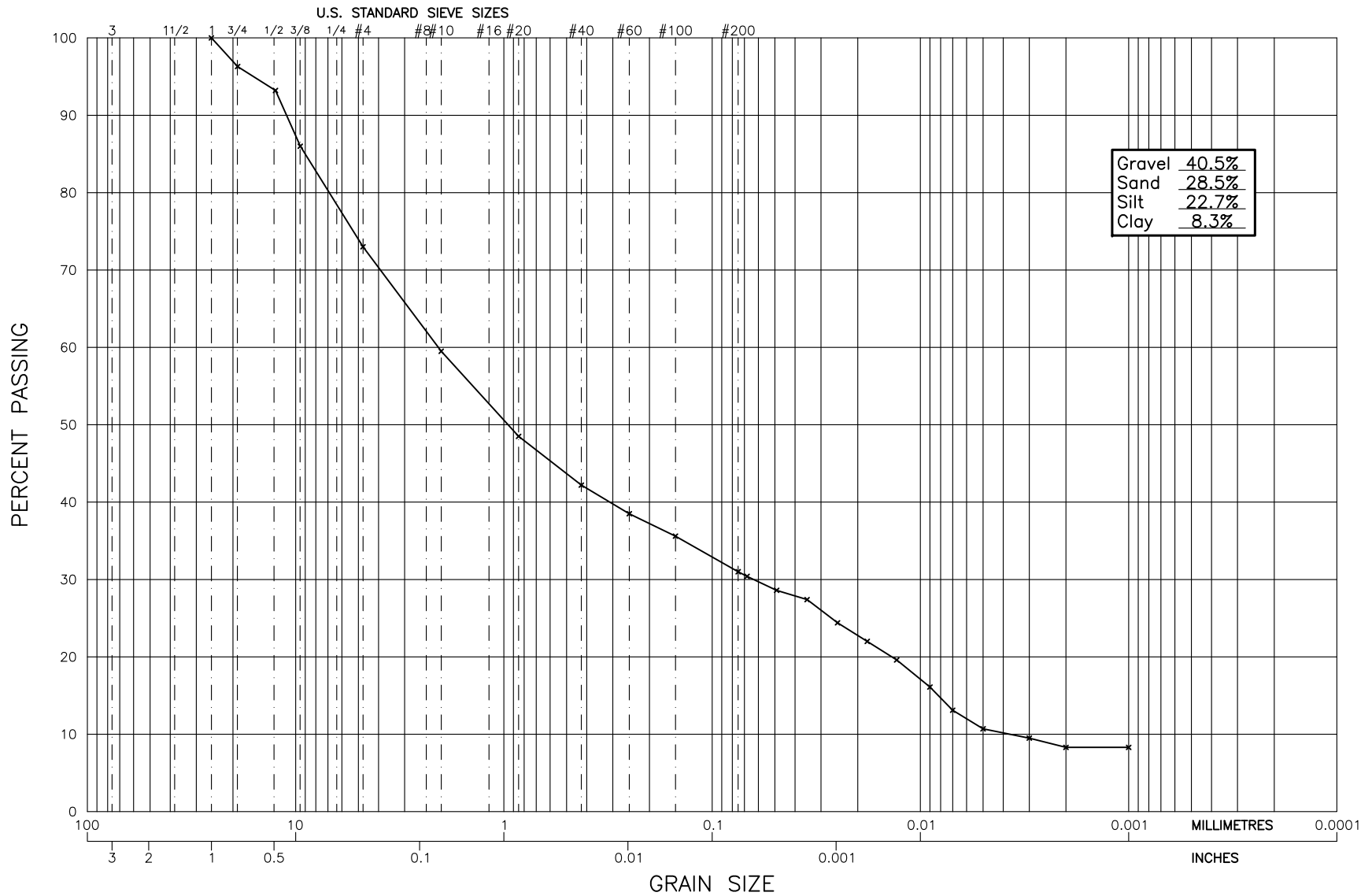
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct.11, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type:				
Sample #: SPT2		Test #:		Hole #: BH-BGC11-57		Depth: 3.8 - 4.25m		Time:			
Sampled By: Client				Tested By: MM			Checked By: DJ				
Date Sampled: August 9, 2011				Date Received: September 13, 2011			Date Tested: Oct.4, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		839.7	100.0	10		50.0	100.0	63.9	
Wet Wt. & Tare	1100.1	25.4	26.7	813.0	96.8	20	7.5	42.5	85.0	54.3	
Dry Wt. & Tare	1020.9	19.0	0.0	813.0	96.8	40	5.5	37.0	74.0	47.3	
Water Wt.	79.2	12.5	28.1	784.9	93.5	60	3.6	33.4	66.8	42.7	
Tare Wt.	181.2	9.5	13.6	771.3	91.9	100	3.1	30.3	60.6	38.7	
Wt. Of Dry Soil	839.7	4.75	95.5	675.8	80.5	200	5.4	24.9	49.8	31.8	
Moisture Content %	9.4	10	139.0	536.8	63.9	Pan	24.9				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	839.7			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.639	0.5	29.0	20.0	0.01365	22.0	12.7	5.033	0.069	44.0	28.1
50.0	0.639	1	23.5	20.0	0.01365	16.5	13.6	3.684	0.050	33.0	21.1
50.0	0.639	2	22.0	20.0	0.01365	15.0	13.8	2.629	0.036	30.0	19.2
50.0	0.639	4	20.0	20.0	0.01365	13.0	14.2	1.881	0.026	26.0	16.6
50.0	0.639	8	18.5	20.0	0.01365	11.5	14.4	1.342	0.018	23.0	14.7
50.0	0.639	15	18.0	20.0	0.01365	11.0	14.5	0.983	0.013	22.0	14.1
50.0	0.639	30	16.0	20.0	0.01365	9.0	14.8	0.703	0.010	18.0	11.5
50.0	0.639	60	14.5	20.0	0.01365	7.5	15.1	0.501	0.007	15.0	9.6
50.0	0.639	120	13.0	21.0	0.01348	6.0	15.3	0.357	0.005	12.0	7.7
50.0	0.639	240	11.5	21.0	0.01348	4.5	15.6	0.255	0.003	9.0	5.8
50.0	0.639	480	10.0	21.0	0.01348	3.0	15.8	0.181	0.002	6.0	3.8
50.0	0.639	1440	9.0	20.0	0.01365	2.0	16.0	0.105	0.001	4.0	2.6
Hydrometer #: 932452			Graduate #: 6			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-57-SPT4

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH57-SPT4

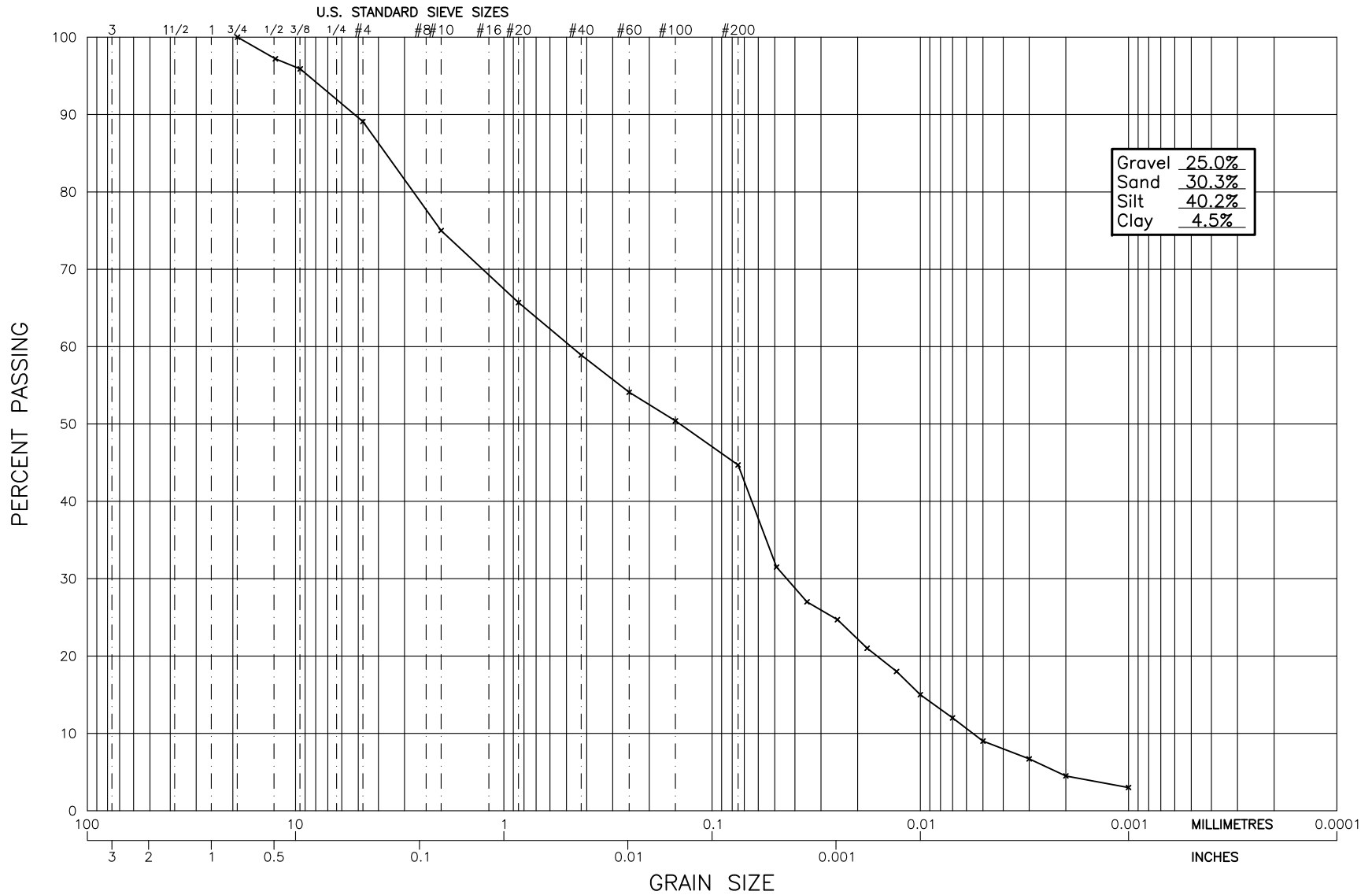
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct.12, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type:				
Sample #: SPT4		Test #:		Hole #: BH-BGC11-57		Depth: 6.85 - 7.1m		Time:			
Sampled By: Client				Tested By: DJ			Checked By: DJ				
Date Sampled: August 9, 2011				Date Received: September 13, 2011			Date Tested: Oct. 11, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	59.5	
Wet Wt. & Tare	687.8	25.4		456.5	100.0	20	9.3	40.7	81.4	48.5	
Dry Wt. & Tare	649.4	19.0	16.8	439.7	96.3	40	5.3	35.4	70.8	42.2	
Water Wt.	38.4	12.5	14.3	425.4	93.2	60	3.1	32.3	64.6	38.5	
Tare Wt.	192.9	9.5	32.8	392.6	86.0	100	2.4	29.9	59.8	35.6	
Wt. Of Dry Soil	456.5	4.75	59.2	333.4	73.0	200	3.9	26.0	52.0	31.0	
Moisture Content %	8.4	10	61.6	271.8	59.5	Pan	26.0				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	456.5			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.595	0.5	33.5	19.0	0.01382	25.5	12.1	4.917	0.068	51.0	30.4
50.0	0.595	1	32.0	19.0	0.01382	24.0	12.3	3.512	0.049	48.0	28.6
50.0	0.595	2	31.0	19.0	0.01382	23.0	12.5	2.500	0.035	46.0	27.4
50.0	0.595	4	28.5	19.0	0.01382	20.5	12.9	1.797	0.025	41.0	24.4
50.0	0.595	8	26.5	19.0	0.01382	18.5	13.2	1.287	0.018	37.0	22.0
50.0	0.595	15	24.5	19.0	0.01382	16.5	13.6	0.951	0.013	33.0	19.6
50.0	0.595	30	21.5	19.0	0.01382	13.5	14.1	0.685	0.009	27.0	16.1
50.0	0.595	60	19.0	19.0	0.01382	11.0	14.5	0.491	0.007	22.0	13.1
50.0	0.595	120	17.0	19.0	0.01382	9.0	14.8	0.351	0.005	18.0	10.7
50.0	0.595	240	15.0	20.0	0.01365	8.0	15.0	0.250	0.003	16.0	9.5
50.0	0.595	480	14.0	20.0	0.01365	7.0	15.1	0.178	0.002	14.0	8.3
50.0	0.595	1440	14.0	20.0	0.01365	7.0	15.1	0.103	0.001	14.0	8.3
Hydrometer #: 932452			Graduate #: 2			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-58-G2

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH58-G2

GeoNorth Engineering

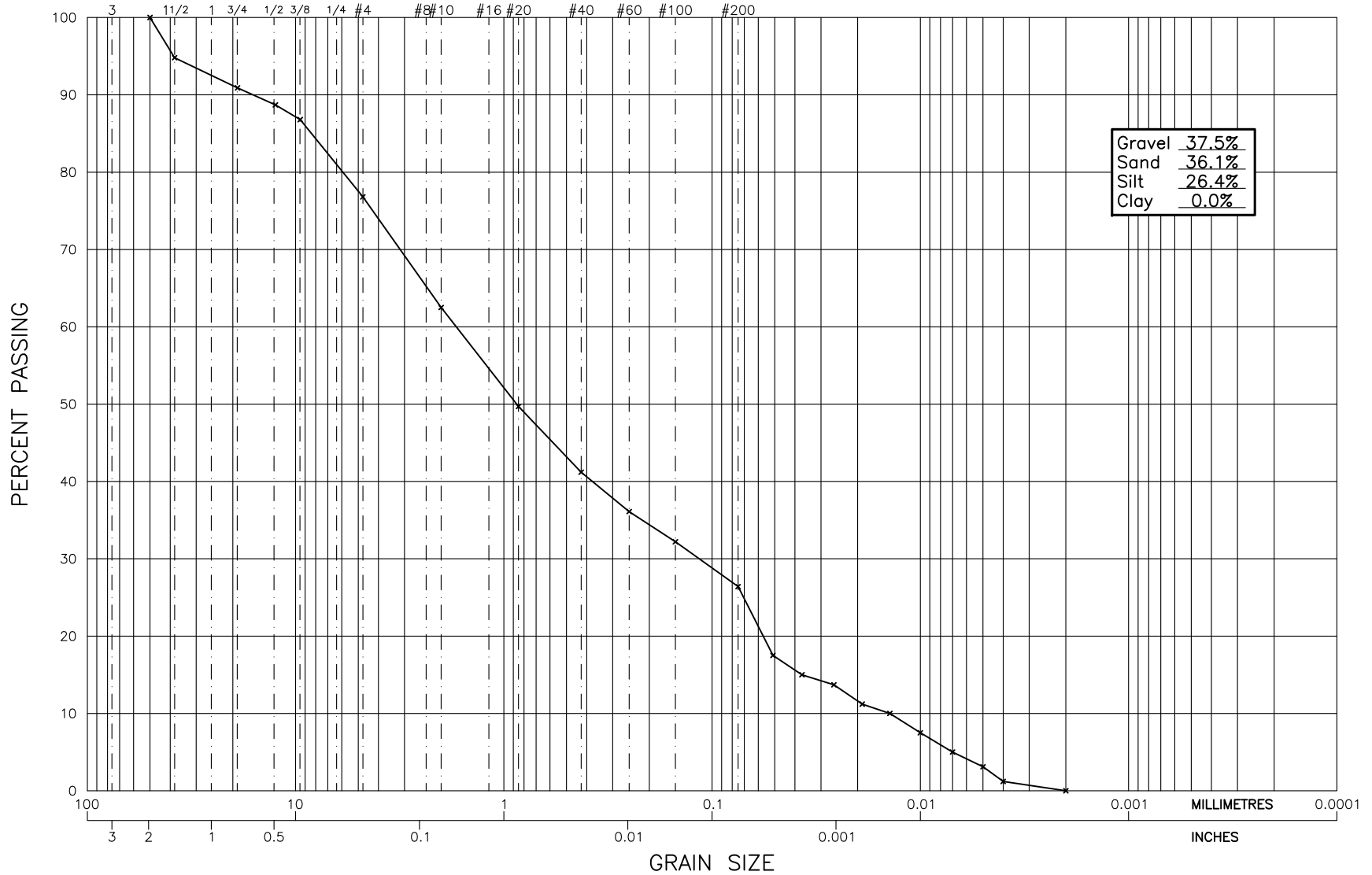
Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 10, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Silt, sandy				
Sample #: G2		Test #:		Hole #: BH-BGC11-58		Depth: 1.9-2.1m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: Aug.10, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.9, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	75.0	
Wet Wt. & Tare	1615.8	25.4				20	6.2	43.8	87.6	65.7	
Dry Wt. & Tare	1404.2	19.0		1221.7	100.0	40	4.5	39.3	78.6	58.9	
Water Wt.	211.6	12.5	34.3	1187.4	97.2	60	3.2	36.1	72.2	54.1	
Tare Wt.	182.5	9.5	15.2	1172.2	95.9	100	2.5	33.6	67.2	50.4	
Wt. Of Dry Soil	1221.7	4.75	83.3	1088.9	89.1	200	3.8	29.8	59.6	44.7	
Moisture Content %	17.3	10	173.1	915.8	75.0	Pan	29.8				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	1221.7			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.750	0.5	32.0	20.0	0.01365	25.0	12.2	4.934	0.067	50.0	37.5
50.0	0.750	1	28.0	20.0	0.01365	21.0	12.8	3.582	0.049	42.0	31.5
50.0	0.750	2	25.0	20.0	0.01365	18.0	13.3	2.581	0.035	36.0	27.0
50.0	0.750	4	23.5	20.0	0.01365	16.5	13.6	1.842	0.025	33.0	24.7
50.0	0.750	8	21.0	20.0	0.01365	14.0	14.0	1.322	0.018	28.0	21.0
50.0	0.750	15	19.0	20.0	0.01365	12.0	14.3	0.977	0.013	24.0	18.0
50.0	0.750	30	17.0	20.0	0.01365	10.0	14.6	0.699	0.010	20.0	15.0
50.0	0.750	60	15.0	20.0	0.01365	8.0	15.0	0.500	0.007	16.0	12.0
50.0	0.750	120	13.0	20.0	0.01365	6.0	15.3	0.357	0.005	12.0	9.0
50.0	0.750	240	11.5	20.0	0.01365	4.5	15.6	0.255	0.003	9.0	6.7
50.0	0.750	480	10.0	21.0	0.01348	3.0	15.8	0.181	0.002	6.0	4.5
50.0	0.750	1440	10.0	18.0	0.01399	2.0	16.0	0.105	0.001	4.0	3.0
Hydrometer #: 932452			Graduate #: 7			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	

U.S. STANDARD SIEVE SIZES



Gravel	37.5%
Sand	36.1%
Silt	26.4%
Clay	0.0%

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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-58-G4

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH58-G4

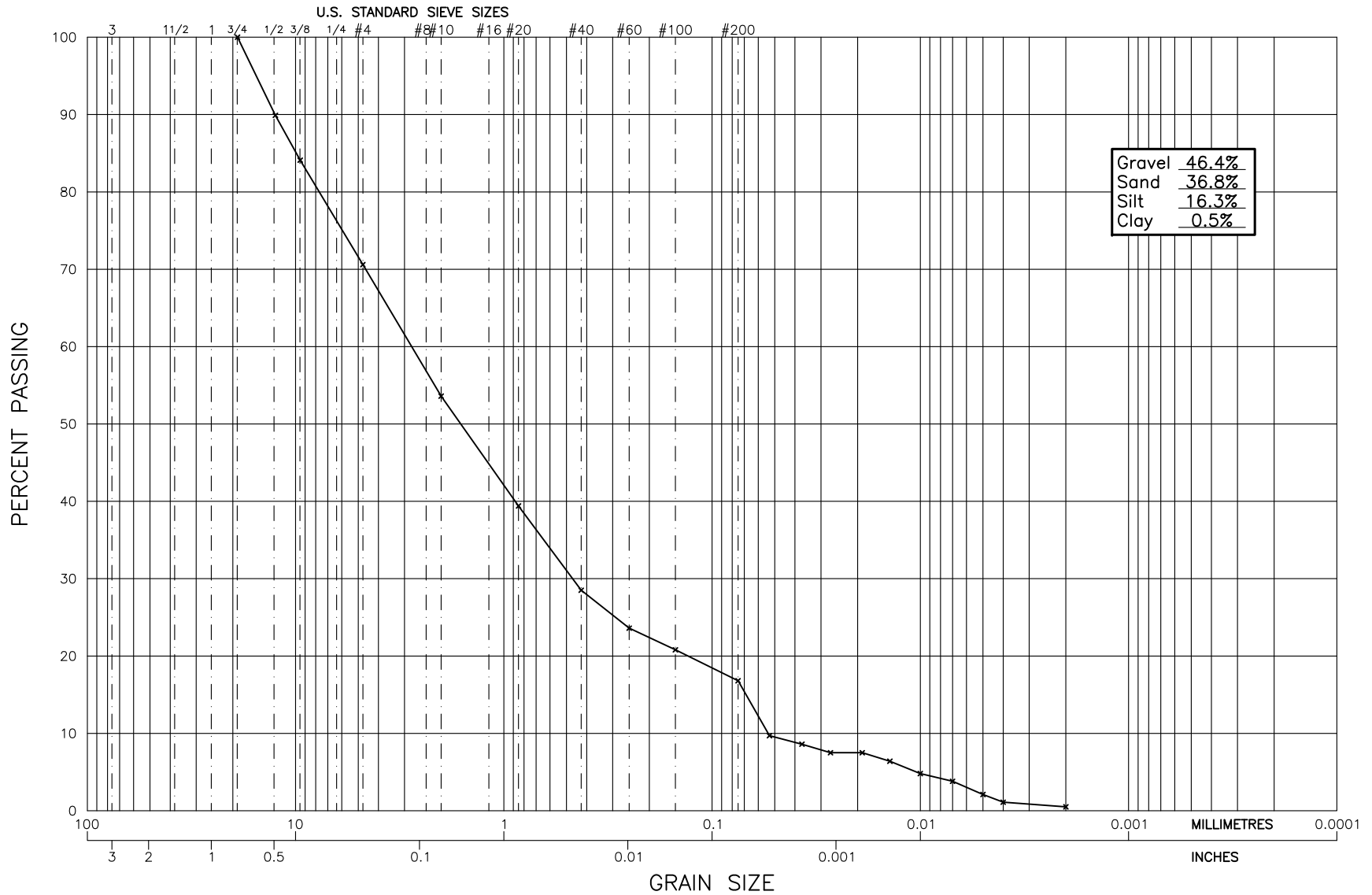
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 10, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel and Sand, silty				
Sample #: G4		Test #:		Hole #: BH-BGC11-58		Depth: 3.2-3.6m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: Aug.10, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.9, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1	78.8	1442.7	94.8	10		50.0	100.0	62.5	
Wet Wt. & Tare	1822.2	25.4	0.0	1442.7	94.8	20	10.2	39.8	79.6	49.7	
Dry Wt. & Tare	1653.0	19.0	59.7	1383.0	90.9	40	6.8	33.0	66.0	41.2	
Water Wt.	169.2	12.5	33.2	1349.8	88.7	60	4.1	28.9	57.8	36.1	
Tare Wt.	131.5	9.5	28.8	1321.0	86.8	100	3.1	25.8	51.6	32.2	
Wt. Of Dry Soil	1521.5	4.75	152.9	1168.1	76.8	200	4.7	21.1	42.2	26.4	
Moisture Content %	11.1	10	217.7	950.4	62.5	Pan	21.1				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	1521.5			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.625	0.5	23.0	20.0	0.01365	16.0	13.7	5.226	0.071	32.0	20.0
50.0	0.625	1	21.0	20.0	0.01365	14.0	14.0	3.740	0.051	28.0	17.5
50.0	0.625	2	19.0	20.0	0.01365	12.0	14.3	2.675	0.037	24.0	15.0
50.0	0.625	4	18.0	20.0	0.01365	11.0	14.5	1.903	0.026	22.0	13.7
50.0	0.625	8	16.0	20.0	0.01365	9.0	14.8	1.361	0.019	18.0	11.2
50.0	0.625	15	15.0	20.0	0.01365	8.0	15.0	0.999	0.014	16.0	10.0
50.0	0.625	30	13.0	20.0	0.01365	6.0	15.3	0.714	0.010	12.0	7.5
50.0	0.625	60	11.0	20.0	0.01365	4.0	15.6	0.510	0.007	8.0	5.0
50.0	0.625	120	9.5	20.0	0.01365	2.5	15.9	0.364	0.005	5.0	3.1
50.0	0.625	240	8.0	20.0	0.01365	1.0	16.1	0.259	0.004	2.0	1.2
50.0	0.625	480	7.0	21.0	0.01348	0.0	16.3	0.184	0.002	0.0	0.0
50.0	0.625	1440	7.0	20.0	0.01365	0.0	16.3	0.106	0.001	0.0	0.0
Hydrometer #: 932452			Graduate #: 8			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-58-G6

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH58-G6

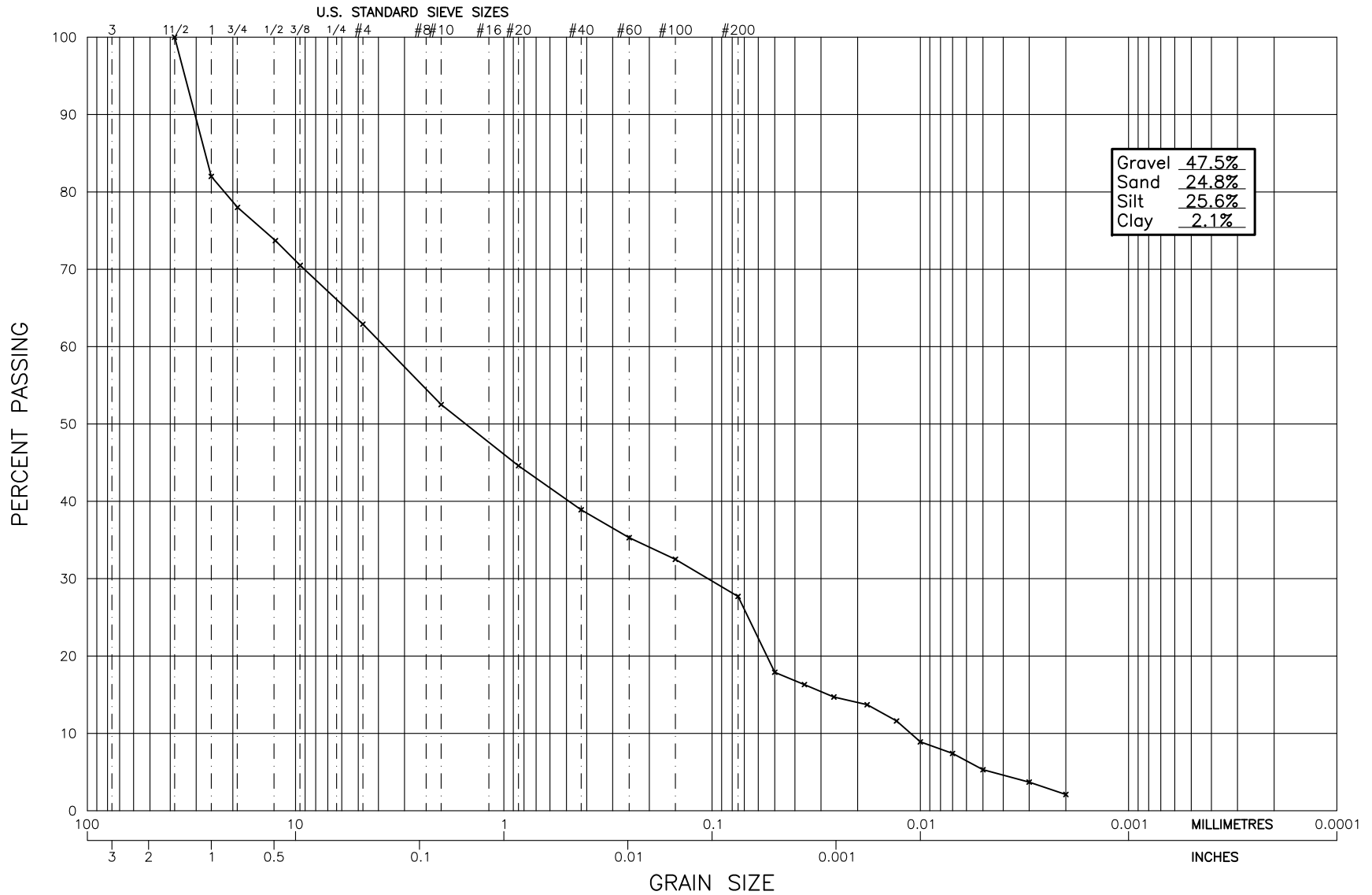
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 10, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel/Sand,some silt				
Sample #: G6		Test #:		Hole #: BH-BGC11-58		Depth: 4.4 - 4.6m		Time:			
Sampled By: Client			Tested By: DJ				Checked By: DJ				
Date Sampled: Aug.10, 2011			Date Received: Sept.13, 2011				Date Tested: Nov.9, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	53.6	
Wet Wt. & Tare	1566.8	25.4				20	13.3	36.7	73.4	39.4	
Dry Wt. & Tare	1426.7	19.0		1246.8	100.0	40	10.1	26.6	53.2	28.5	
Water Wt.	140.1	12.5	125.9	1120.9	89.9	60	4.6	22.0	44.0	23.6	
Tare Wt.	179.9	9.5	72.0	1048.9	84.1	100	2.6	19.4	38.8	20.8	
Wt. Of Dry Soil	1246.8	4.75	168.7	880.2	70.6	200	3.7	15.7	31.4	16.8	
Moisture Content %	11.2	10	211.3	668.9	53.6	Pan	15.7				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	1246.8			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.536	0.5	19.0	20.0	0.01365	12.0	14.3	5.351	0.073	24.0	12.9
50.0	0.536	1	16.0	20.0	0.01365	9.0	14.8	3.848	0.053	18.0	9.7
50.0	0.536	2	15.0	20.0	0.01365	8.0	15.0	2.736	0.037	16.0	8.6
50.0	0.536	4	14.0	20.0	0.01365	7.0	15.1	1.946	0.027	14.0	7.5
50.0	0.536	8	14.0	20.0	0.01365	7.0	15.1	1.376	0.019	14.0	7.5
50.0	0.536	15	13.0	20.0	0.01365	6.0	15.3	1.010	0.014	12.0	6.4
50.0	0.536	30	11.5	20.0	0.01365	4.5	15.6	0.720	0.010	9.0	4.8
50.0	0.536	60	10.5	20.0	0.01365	3.5	15.7	0.512	0.007	7.0	3.8
50.0	0.536	120	9.0	20.0	0.01365	2.0	16.0	0.365	0.005	4.0	2.1
50.0	0.536	240	8.0	20.0	0.01365	1.0	16.1	0.259	0.004	2.0	1.1
50.0	0.536	480	7.5	21.0	0.01348	0.5	16.2	0.184	0.002	1.0	0.5
50.0	0.536	1440	7.5	20.0	0.01365	0.5	16.2	0.106	0.001	1.0	0.5
Hydrometer #: 932452			Graduate #: 2			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-58-G8

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH58-G8

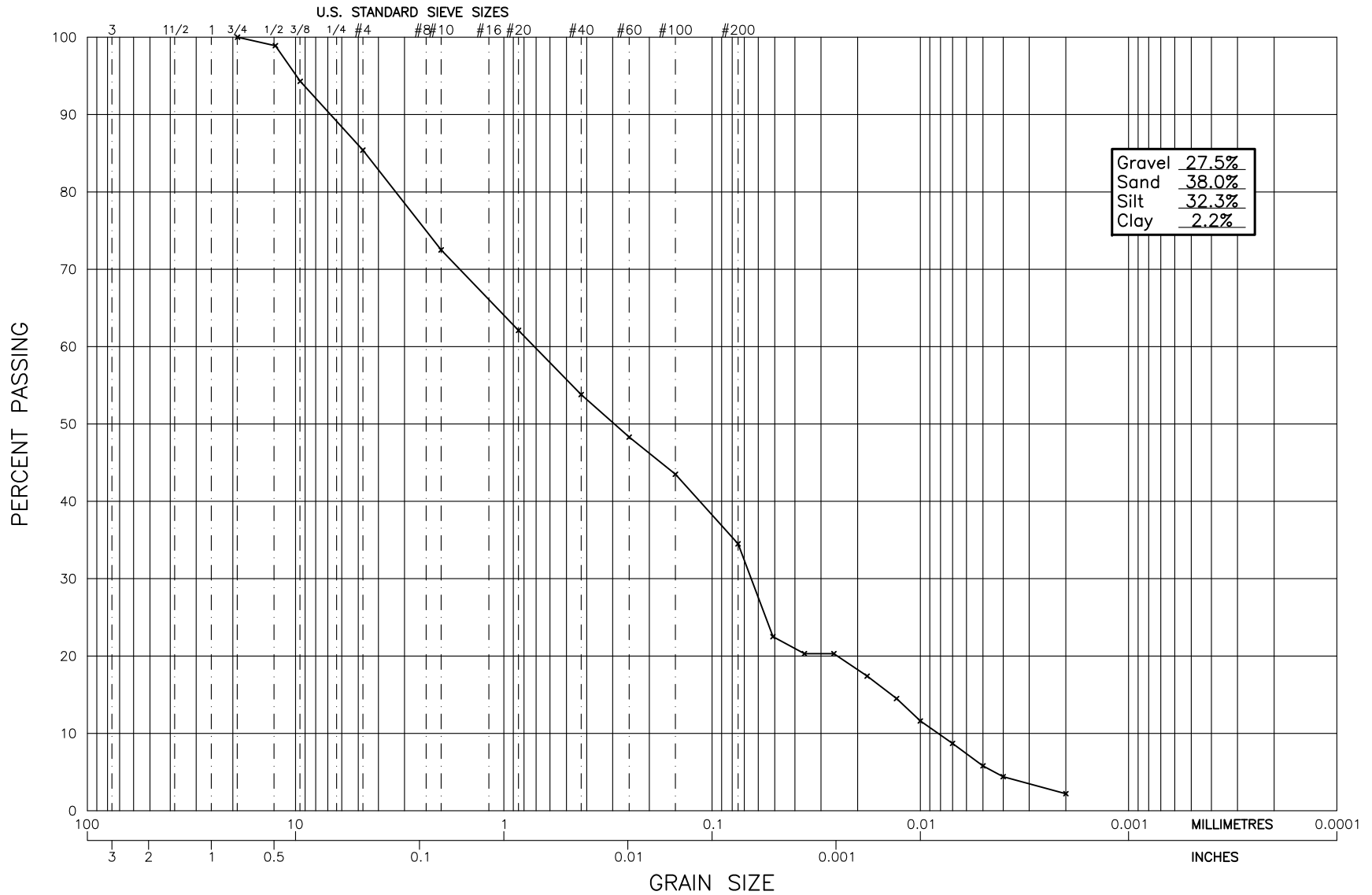
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 10, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Gravel,sandy,silty				
Sample #: G8		Test #:		Hole #: BH-BGC11-58		Depth: 5.4 - 5.5m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: Aug.10, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.9, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1		830.1	100.0	10		50.0	100.0	52.5	
Wet Wt. & Tare	1065.0	25.4	149.5	680.6	82.0	20	7.6	42.4	84.8	44.6	
Dry Wt. & Tare	1011.5	19.0	33.5	647.1	78.0	40	5.4	37.0	74.0	38.9	
Water Wt.	53.5	12.5	35.3	611.8	73.7	60	3.4	33.6	67.2	35.3	
Tare Wt.	181.4	9.5	26.5	585.3	70.5	100	2.7	30.9	61.8	32.5	
Wt. Of Dry Soil	830.1	4.75	62.9	522.4	62.9	200	4.5	26.4	52.8	27.7	
Moisture Content %	6.4	10	86.2	436.2	52.5	Pan	26.4				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	830.1			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.525	0.5	28.0	20.0	0.01365	21.0	12.8	5.066	0.069	42.0	22.1
50.0	0.525	1	24.0	20.0	0.01365	17.0	13.5	3.673	0.050	34.0	17.9
50.0	0.525	2	22.5	20.0	0.01365	15.5	13.7	2.621	0.036	31.0	16.3
50.0	0.525	4	21.0	20.0	0.01365	14.0	14.0	1.870	0.026	28.0	14.7
50.0	0.525	8	20.0	20.0	0.01365	13.0	14.2	1.330	0.018	26.0	13.7
50.0	0.525	15	18.0	20.0	0.01365	11.0	14.5	0.983	0.013	22.0	11.6
50.0	0.525	30	15.5	20.0	0.01365	8.5	14.9	0.705	0.010	17.0	8.9
50.0	0.525	60	14.0	20.0	0.01365	7.0	15.1	0.502	0.007	14.0	7.4
50.0	0.525	120	12.0	20.0	0.01365	5.0	15.5	0.359	0.005	10.0	5.3
50.0	0.525	240	10.5	20.0	0.01365	3.5	15.7	0.256	0.003	7.0	3.7
50.0	0.525	480	9.0	21.0	0.01348	2.0	16.0	0.182	0.002	4.0	2.1
50.0	0.525	1440	9.0	20.0	0.01365	2.0	16.0	0.105	0.001	4.0	2.1
Hydrometer #: 932452			Graduate #: 3			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-58-G10

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH58-G10

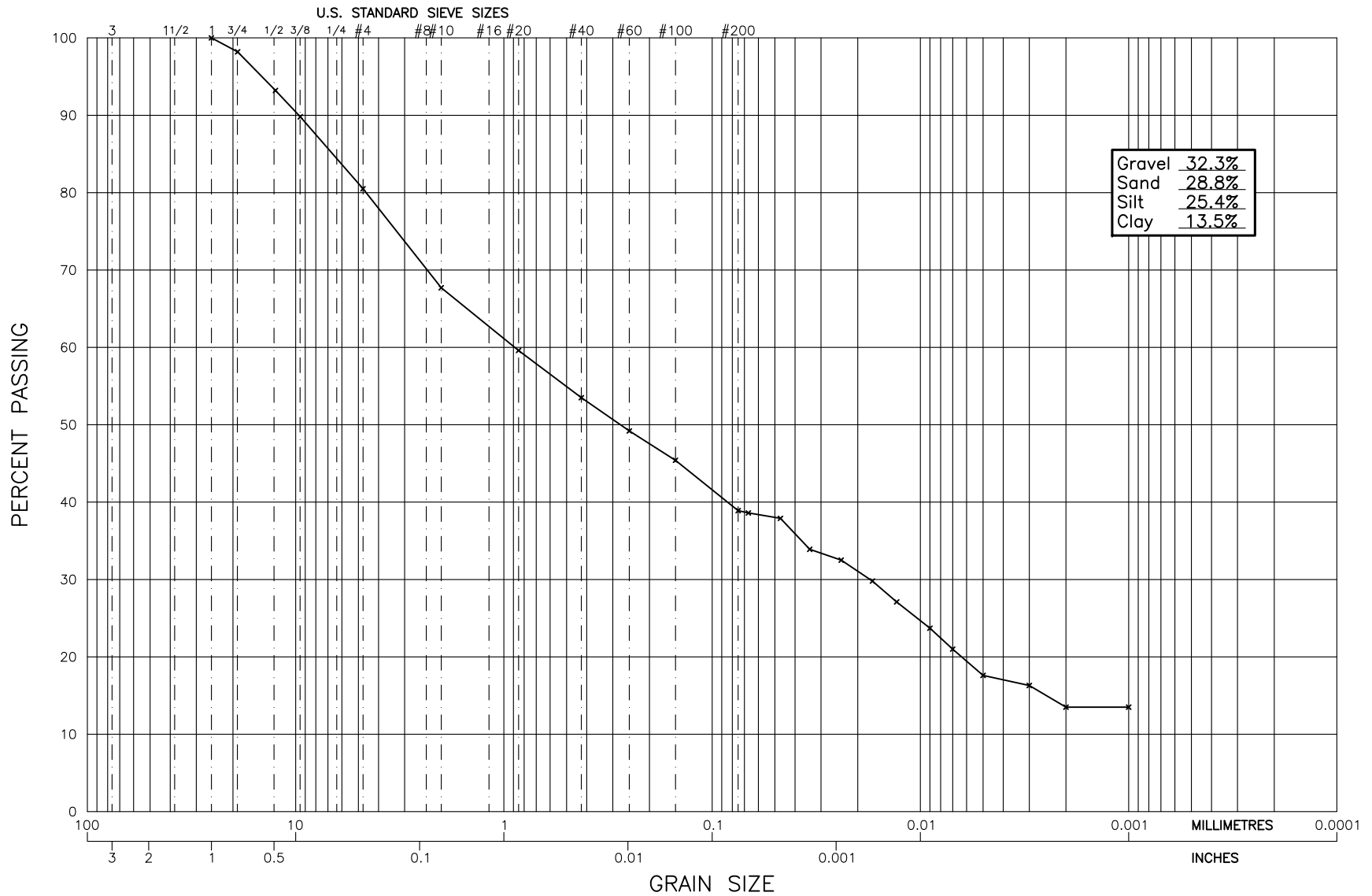
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 10, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand and Silt, gravelly				
Sample #: G10		Test #:		Hole #: BH-BGC11-58		Depth: 7.9 - 8.1m		Time:			
Sampled By: Client			Tested By: DJ				Checked By: DJ				
Date Sampled: Aug.10, 2011			Date Received: Sept.13, 2011				Date Tested: Nov.9, 2011				
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	72.5
Wet Wt. & Tare	837.6		25.4				20	7.2	42.8	85.6	62.1
Dry Wt. & Tare	785.5		19.0		605.0	100.0	40	5.7	37.1	74.2	53.8
Water Wt.	52.1		12.5	6.6	598.4	98.9	60	3.8	33.3	66.6	48.3
Tare Wt.	180.5		9.5	27.9	570.5	94.3	100	3.3	30.0	60.0	43.5
Wt. Of Dry Soil	605.0		4.75	53.7	516.8	85.4	200	6.2	23.8	47.6	34.5
Moisture Content %	8.6		10	78.0	438.8	72.5	Pan	23.8			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=				
			Total	605.0			Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.725	0.5	27.0	20.0	0.01365	20.0	13.0	5.098	0.070	40.0	29.0
50.0	0.725	1	22.5	20.0	0.01365	15.5	13.7	3.706	0.051	31.0	22.5
50.0	0.725	2	21.0	20.0	0.01365	14.0	14.0	2.644	0.036	28.0	20.3
50.0	0.725	4	21.0	20.0	0.01365	14.0	14.0	1.870	0.026	28.0	20.3
50.0	0.725	8	19.0	20.0	0.01365	12.0	14.3	1.338	0.018	24.0	17.4
50.0	0.725	15	17.0	20.0	0.01365	10.0	14.6	0.988	0.013	20.0	14.5
50.0	0.725	30	15.0	20.0	0.01365	8.0	15.0	0.707	0.010	16.0	11.6
50.0	0.725	60	13.0	20.0	0.01365	6.0	15.3	0.505	0.007	12.0	8.7
50.0	0.725	120	11.0	20.0	0.01365	4.0	15.6	0.361	0.005	8.0	5.8
50.0	0.725	240	10.0	20.0	0.01365	3.0	15.8	0.257	0.004	6.0	4.4
50.0	0.725	480	8.5	21.0	0.01348	1.5	16.0	0.183	0.002	3.0	2.2
50.0	0.725	1440	8.5	20.0	0.01365	1.5	16.0	0.106	0.001	3.0	2.2
Hydrometer #: 932452			Graduate #: 4			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-60-G3

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH60-G3

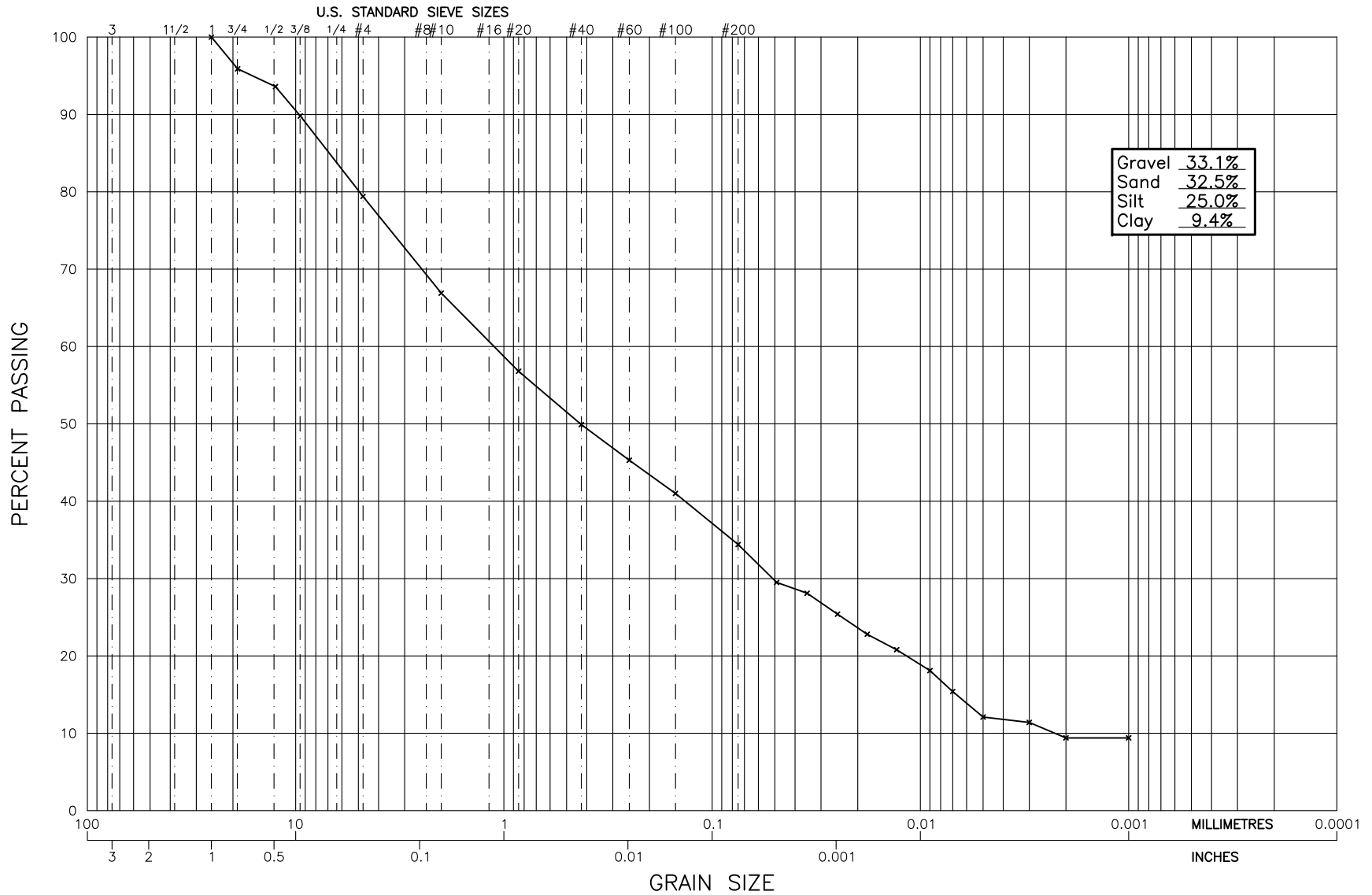
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct.12, 2011					
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300					
Source/Location: Dublin Gulch, Yukon							Type:					
Sample #: G3		Test #:		Hole #: BH-BGC11-60		Depth: 2.7 - 2.8m		Time:				
Sampled By: Client				Tested By: DJ				Checked By: DJ				
Date Sampled: August 10, 2011				Date Received: September 13, 2011				Date Tested: Oct.11, 2011				
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis					
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.			38.1				10		50.0	100.0	67.7	
Wet Wt. & Tare	897.0		25.4		654.4	100.0	20	6.0	44.0	88.0	59.6	
Dry Wt. & Tare	834.2		19.0	11.9	642.5	98.2	40	4.5	39.5	79.0	53.5	
Water Wt.	62.8		12.5	32.6	609.9	93.2	60	3.2	36.3	72.6	49.2	
Tare Wt.	179.8		9.5	22.4	587.5	89.8	100	2.8	33.5	67.0	45.4	
Wt. Of Dry Soil	654.4		4.75	60.8	526.7	80.5	200	4.8	28.7	57.4	38.9	
Moisture Content %	9.6		10	83.6	443.1	67.7	Pan	28.7				
Dry Wt. Of Sample from Initial Moisture							Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=					
			Total	654.4			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)	
50.0	0.677	0.5	36.5	19.0	0.01382	28.5	11.6	4.815	0.067	57.0	38.6	
50.0	0.677	1	36.0	19.0	0.01382	28.0	11.7	3.417	0.047	56.0	37.9	
50.0	0.677	2	33.0	19.0	0.01382	25.0	12.2	2.467	0.034	50.0	33.9	
50.0	0.677	4	32.0	19.0	0.01382	24.0	12.3	1.756	0.024	48.0	32.5	
50.0	0.677	8	30.0	19.0	0.01382	22.0	12.7	1.258	0.017	44.0	29.8	
50.0	0.677	15	28.0	19.0	0.01382	20.0	13.0	0.931	0.013	40.0	27.1	
50.0	0.677	30	25.5	19.0	0.01382	17.5	13.4	0.669	0.009	35.0	23.7	
50.0	0.677	60	23.5	19.0	0.01382	15.5	13.7	0.478	0.007	31.0	21.0	
50.0	0.677	120	21.0	19.0	0.01382	13.0	14.2	0.343	0.005	26.0	17.6	
50.0	0.677	240	19.0	20.0	0.01365	12.0	14.3	0.244	0.003	24.0	16.3	
50.0	0.677	480	17.0	20.0	0.01365	10.0	14.6	0.175	0.002	20.0	13.5	
50.0	0.677	1440	17.0	20.0	0.01365	10.0	14.6	0.101	0.001	20.0	13.5	
Hydrometer #: 932452			Graduate #: 3			Dispersing Agent: Sodium Hex			Amount: 125ml			
Density of Solids:												
Description of Sample:												

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	33.1%
Sand	32.5%
Silt	25.0%
Clay	9.4%

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 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-60-G5

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH60-G5

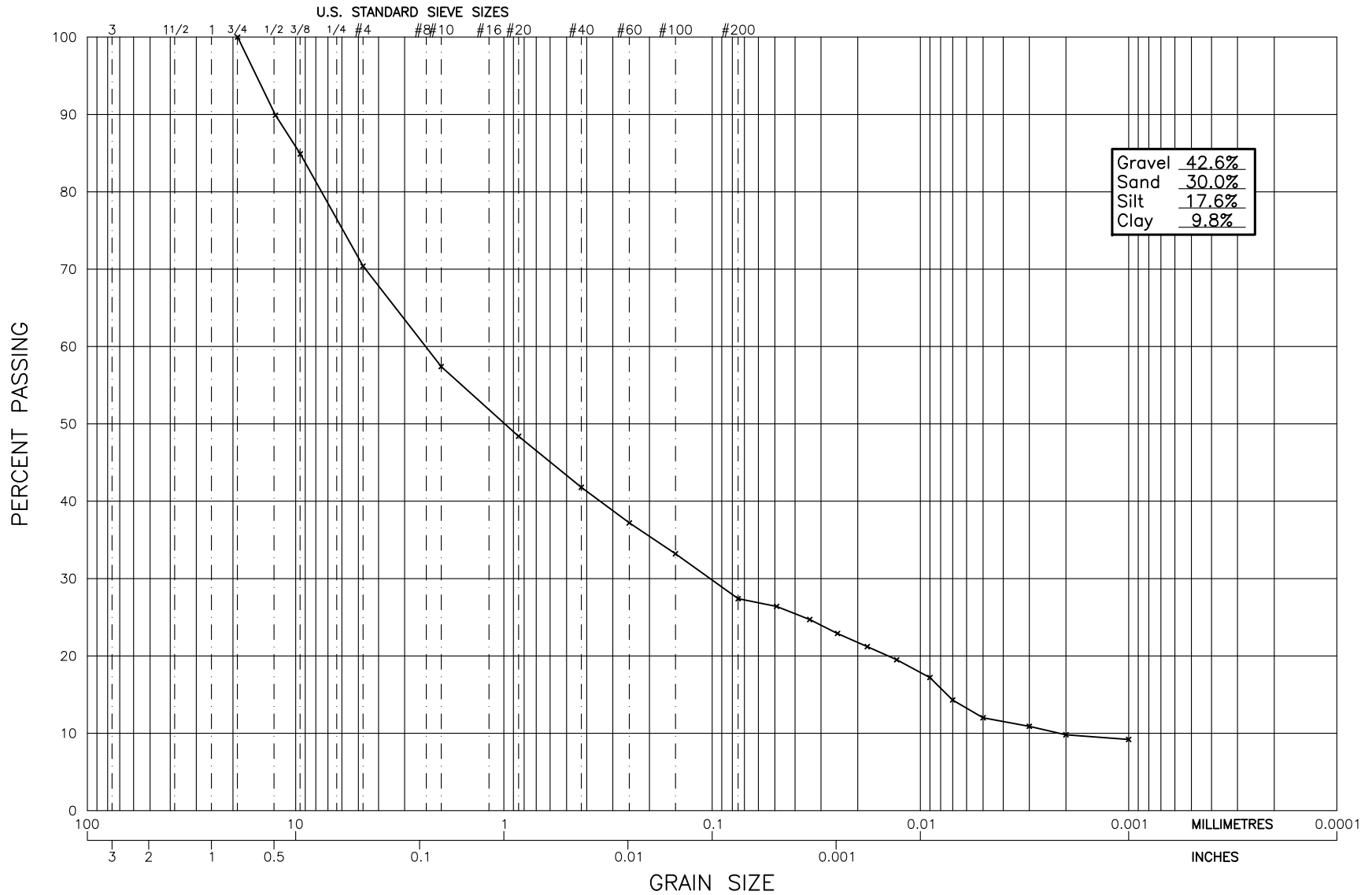
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct.12, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type:				
Sample #: G5		Test #:		Hole #: BH-BGC11-60		Depth: 3.7 - 3.8m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: August 10, 2011				Date Received: September 13, 2011				Date Tested: Oct.11, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	66.9	
Wet Wt. & Tare	1032.3	25.4		771.5	100.0	20	7.6	42.4	84.8	56.8	
Dry Wt. & Tare	963.2	19.0	31.6	739.9	95.9	40	5.1	37.3	74.6	49.9	
Water Wt.	69.1	12.5	18.0	721.9	93.6	60	3.5	33.8	67.6	45.3	
Tare Wt.	191.7	9.5	29.1	692.8	89.8	100	3.2	30.6	61.2	41.0	
Wt. Of Dry Soil	771.5	4.75	80.3	612.5	79.4	200	4.9	25.7	51.4	34.4	
Moisture Content %	9.0	10	96.0	516.5	66.9	Pan					
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	771.5			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.669	0.5	33.5	19.0	0.01382	25.5	12.1	4.917	0.068	51.0	34.1
50.0	0.669	1	30.0	19.0	0.01382	22.0	12.7	3.559	0.049	44.0	29.5
50.0	0.669	2	29.0	19.0	0.01382	21.0	12.8	2.533	0.035	42.0	28.1
50.0	0.669	4	27.0	19.0	0.01382	19.0	13.2	1.814	0.025	38.0	25.4
50.0	0.669	8	25.0	19.0	0.01382	17.0	13.5	1.299	0.018	34.0	22.8
50.0	0.669	15	23.5	19.0	0.01382	15.5	13.7	0.957	0.013	31.0	20.8
50.0	0.669	30	21.5	19.0	0.01382	13.5	14.1	0.685	0.009	27.0	18.1
50.0	0.669	60	19.5	19.0	0.01382	11.5	14.4	0.490	0.007	23.0	15.4
50.0	0.669	120	17.0	19.0	0.01382	9.0	14.8	0.351	0.005	18.0	12.1
50.0	0.669	240	15.5	20.0	0.01365	8.5	14.9	0.249	0.003	17.0	11.4
50.0	0.669	480	14.0	20.0	0.01365	7.0	15.1	0.178	0.002	14.0	9.4
50.0	0.669	1440	14.0	20.0	0.01365	7.0	15.1	0.103	0.001	14.0	9.4
Hydrometer #: 932452			Graduate #: 4			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-60-G7

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH60-G7

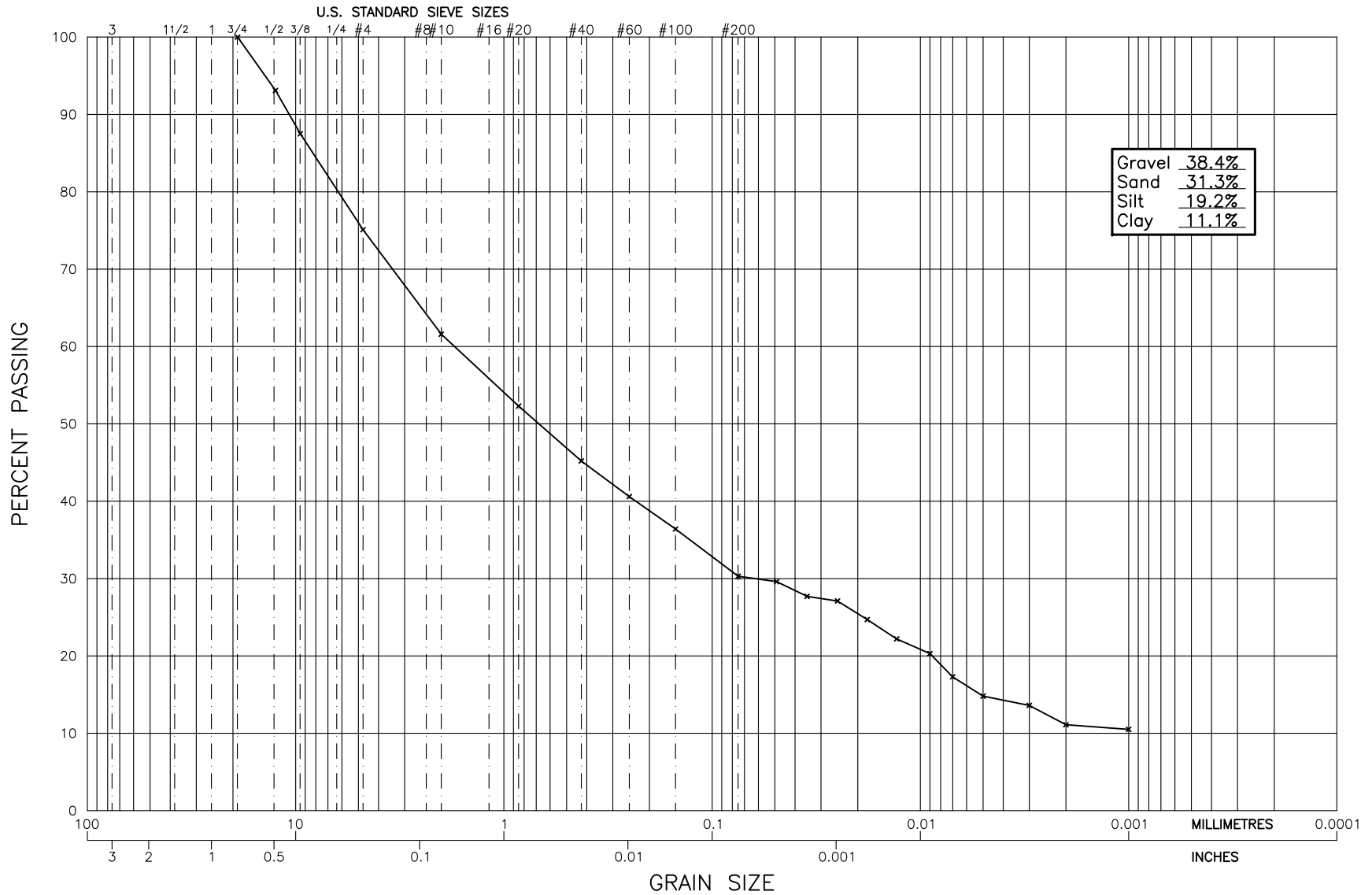
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct.12, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type:				
Sample #: G7		Test #:		Hole #: BH-BGC11-60		Depth: 6.3 - 6.6m		Time:			
Sampled By: Client				Tested By: DJ			Checked By: DJ				
Date Sampled: August 10, 2011				Date Received: September 13, 2011			Date Tested: Oct. 11, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	57.4	
Wet Wt. & Tare	941.7	25.4				20	7.8	42.2	84.4	48.4	
Dry Wt. & Tare	895.0	19.0		689.4	100.0	40	5.8	36.4	72.8	41.8	
Water Wt.	46.7	12.5	69.5	619.9	89.9	60	4.0	32.4	64.8	37.2	
Tare Wt.	205.6	9.5	34.7	585.2	84.9	100	3.5	28.9	57.8	33.2	
Wt. Of Dry Soil	689.4	4.75	99.7	485.5	70.4	200	5.0	23.9	47.8	27.4	
Moisture Content %	6.8	10	90.0	395.5	57.4	Pan	23.9				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	689.4			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.574	0.5	31.5	19.0	0.01382	23.5	12.4	4.983	0.069	47.0	27.0
50.0	0.574	1	31.0	19.0	0.01382	23.0	12.5	3.536	0.049	46.0	26.4
50.0	0.574	2	29.5	19.0	0.01382	21.5	12.7	2.525	0.035	43.0	24.7
50.0	0.574	4	28.0	19.0	0.01382	20.0	13.0	1.802	0.025	40.0	22.9
50.0	0.574	8	26.5	19.0	0.01382	18.5	13.2	1.287	0.018	37.0	21.2
50.0	0.574	15	25.0	19.0	0.01382	17.0	13.5	0.948	0.013	34.0	19.5
50.0	0.574	30	23.0	19.0	0.01382	15.0	13.8	0.679	0.009	30.0	17.2
50.0	0.574	60	20.5	19.0	0.01382	12.5	14.2	0.487	0.007	25.0	14.3
50.0	0.574	120	18.5	19.0	0.01382	10.5	14.6	0.348	0.005	21.0	12.0
50.0	0.574	240	16.5	20.0	0.01365	9.5	14.7	0.248	0.003	19.0	10.9
50.0	0.574	480	15.5	20.0	0.01365	8.5	14.9	0.176	0.002	17.0	9.8
50.0	0.574	1440	15.0	20.0	0.01365	8.0	15.0	0.102	0.001	16.0	9.2
Hydrometer #: 932452			Graduate #: 5			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-60-SPT2

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH60-SPT2

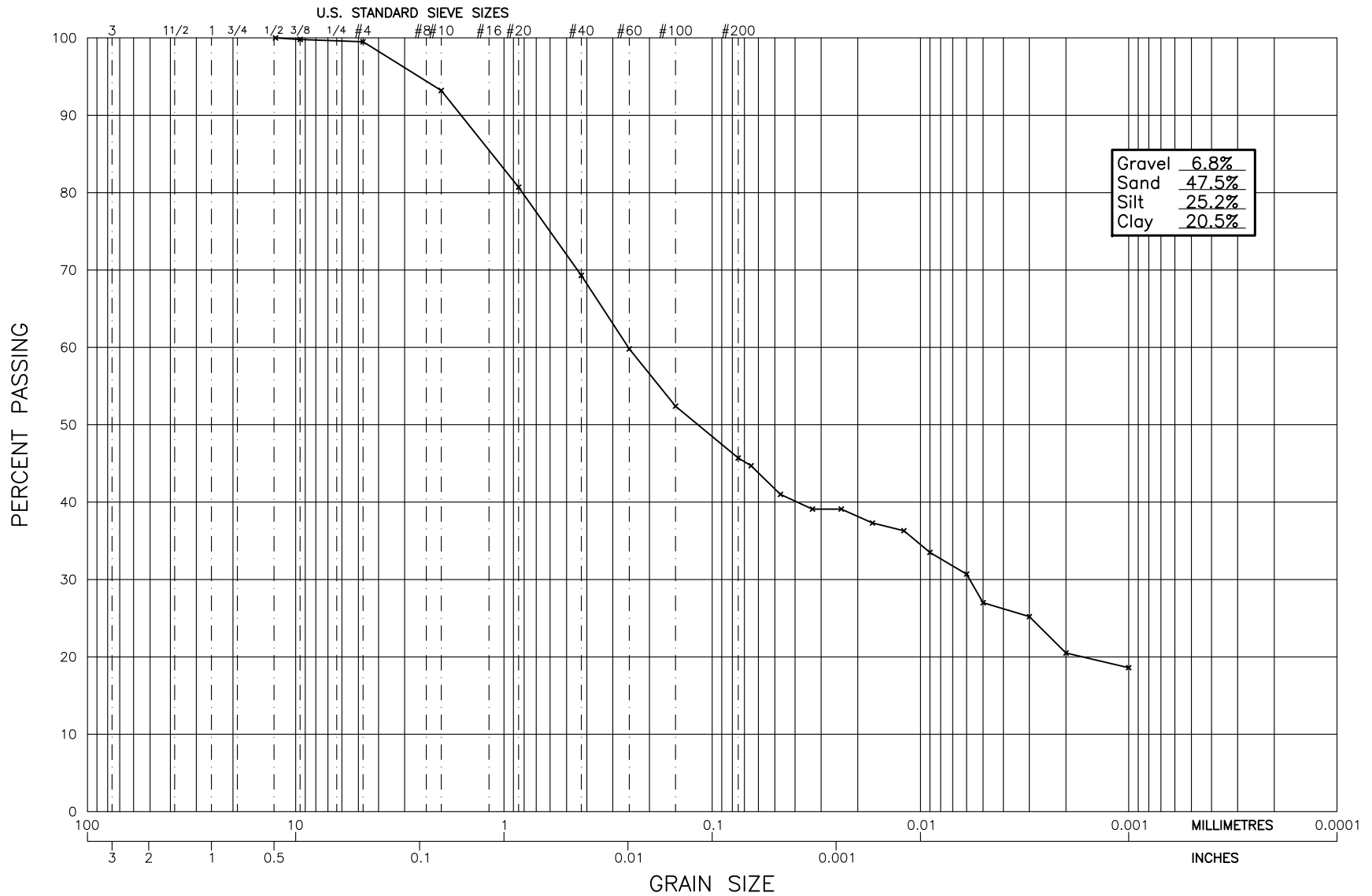
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Inc.							Date: Oct.12, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type:				
Sample #: SPT2		Test #:		Hole #: BH-BGC11-60		Depth: 6.85 - 7.0		Time:			
Sampled By: Client				Tested By: DJ			Checked By: DJ				
Date Sampled: August 10, 2011				Date Received: September 13, 2011			Date Tested: Oct.11, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	61.6	
Wet Wt. & Tare	312.9	25.4				20	7.6	42.4	84.8	52.3	
Dry Wt. & Tare	302.1	19.0		181.2	100.0	40	5.7	36.7	73.4	45.2	
Water Wt.	10.8	12.5	12.5	168.7	93.1	60	3.8	32.9	65.8	40.6	
Tare Wt.	120.9	9.5	10.2	158.5	87.5	100	3.4	29.5	59.0	36.4	
Wt. Of Dry Soil	181.2	4.75	22.5	136.0	75.1	200	4.9	24.6	49.2	30.3	
Moisture Content %	6.0	10	24.3	111.7	61.6	Pan	24.6				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	181.2			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.616	0.5	32.5	19.0	0.01382	24.5	12.3	4.950	0.068	49.0	30.2
50.0	0.616	1	32.0	19.0	0.01382	24.0	12.3	3.512	0.049	48.0	29.6
50.0	0.616	2	30.5	19.0	0.01382	22.5	12.6	2.508	0.035	45.0	27.7
50.0	0.616	4	30.0	19.0	0.01382	22.0	12.7	1.779	0.025	44.0	27.1
50.0	0.616	8	28.0	19.0	0.01382	20.0	13.0	1.275	0.018	40.0	24.7
50.0	0.616	15	26.0	19.0	0.01382	18.0	13.3	0.943	0.013	36.0	22.2
50.0	0.616	30	24.5	19.0	0.01382	16.5	13.6	0.673	0.009	33.0	20.3
50.0	0.616	60	22.0	19.0	0.01382	14.0	14.0	0.483	0.007	28.0	17.3
50.0	0.616	120	20.0	19.0	0.01382	12.0	14.3	0.345	0.005	24.0	14.8
50.0	0.616	240	18.0	20.0	0.01365	11.0	14.5	0.246	0.003	22.0	13.6
50.0	0.616	480	16.0	20.0	0.01365	9.0	14.8	0.176	0.002	18.0	11.1
50.0	0.616	1440	15.5	20.0	0.01365	8.5	14.9	0.102	0.001	17.0	10.5
Hydrometer #: 932452			Graduate #: 6			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND				SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE		



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EAGLE GOLD SOILS AND AGGREGATE TESTING
DUBLIN GULCH, YUKON
GRAIN SIZE ANALYSIS OF BH-BGC11-63-G2

PROJECT NO.
K-3300

PLATE NO.
3300-GS-BH63-G2

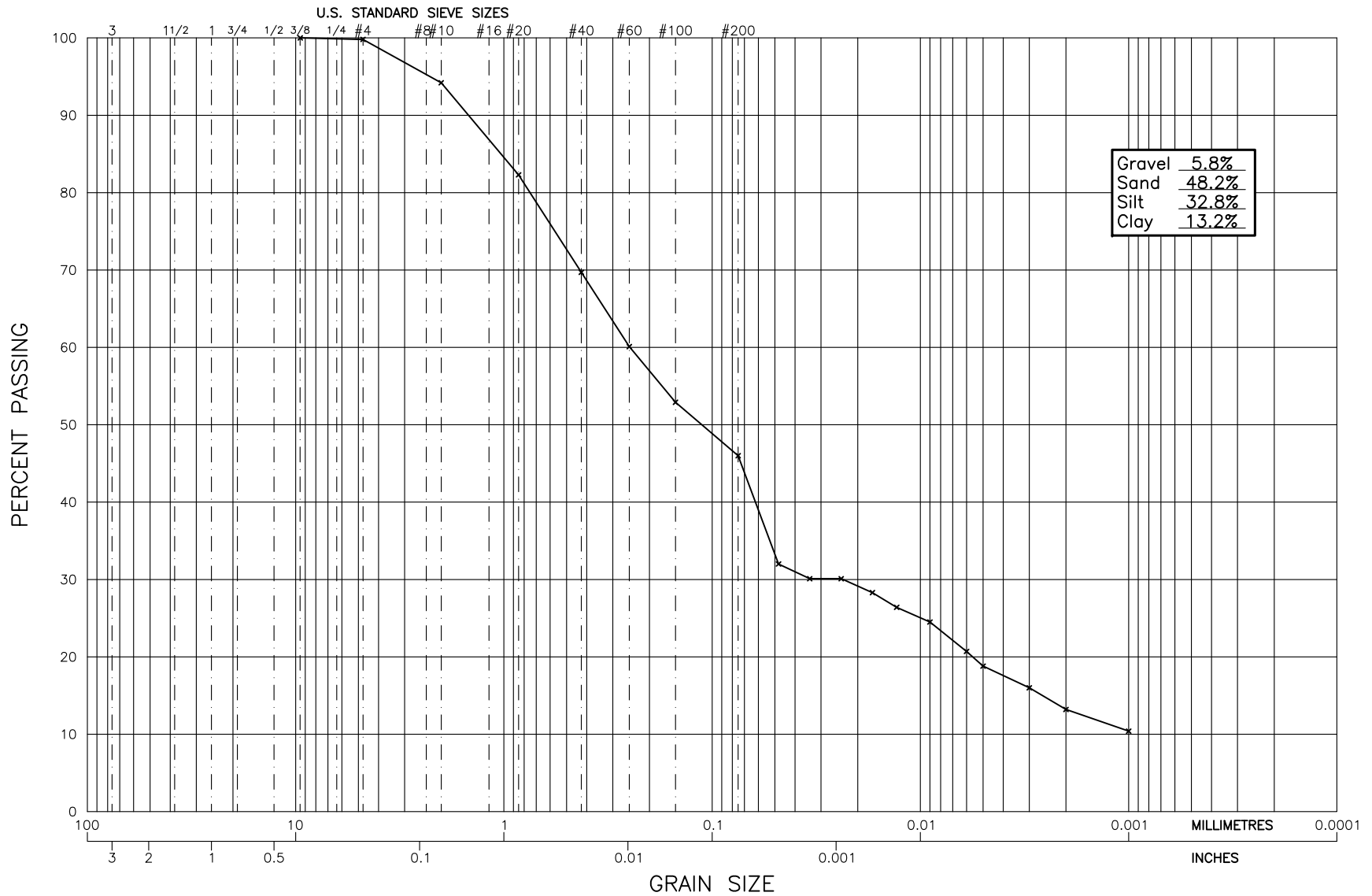
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand,silty/clayey				
Sample #: G2		Test #:		Hole #: BH-BGC11-63		Depth: 1.9 - 2.1m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 11, 2011				Date Received: September 13, 2011				Date Tested: Sept.21, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	93.2
Wet Wt. & Tare	860.5		25.4				20	6.7	43.3	86.6	80.7
Dry Wt. & Tare	725.4		19.0				40	6.1	37.2	74.4	69.3
Water Wt.	135.1		12.5		546.2	100.0	60	5.1	32.1	64.2	59.8
Tare Wt.	179.2		9.5	1.3	544.9	99.8	100	4.0	28.1	56.2	52.4
Wt. Of Dry Soil	546.2		4.75	1.5	543.4	99.5	200	3.6	24.5	49.0	45.7
Moisture Content %	24.7		10	34.5	508.9	93.2	Pan	24.5			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =			Total	546.2			Unwashed Wt.=				
							Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.932	0.5	30.0	23.0	0.01317	24.0	12.3	4.967	0.065	48.0	44.7
50.0	0.932	1	28.0	23.0	0.01317	22.0	12.7	3.559	0.047	44.0	41.0
50.0	0.932	2	27.0	23.0	0.01317	21.0	12.8	2.533	0.033	42.0	39.1
50.0	0.932	4	27.0	23.0	0.01317	21.0	12.8	1.791	0.024	42.0	39.1
50.0	0.932	8	26.0	23.0	0.01317	20.0	13.0	1.275	0.017	40.0	37.3
50.0	0.932	15	25.5	23.0	0.01317	19.5	13.1	0.934	0.012	39.0	36.3
50.0	0.932	30	24.0	23.0	0.01317	18.0	13.3	0.666	0.009	36.0	33.5
50.0	0.932	60	22.5	23.0	0.01317	16.5	13.6	0.476	0.006	33.0	30.7
50.0	0.932	120	21.0	22.0	0.01332	14.5	13.9	0.340	0.005	29.0	27.0
50.0	0.932	240	20.0	22.0	0.01332	13.5	14.1	0.242	0.003	27.0	25.2
50.0	0.932	480	18.0	21.0	0.01348	11.0	14.5	0.174	0.002	22.0	20.5
50.0	0.932	1440	16.0	23.0	0.01317	10.0	14.6	0.101	0.001	20.0	18.6
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-63-G5

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH63-G5

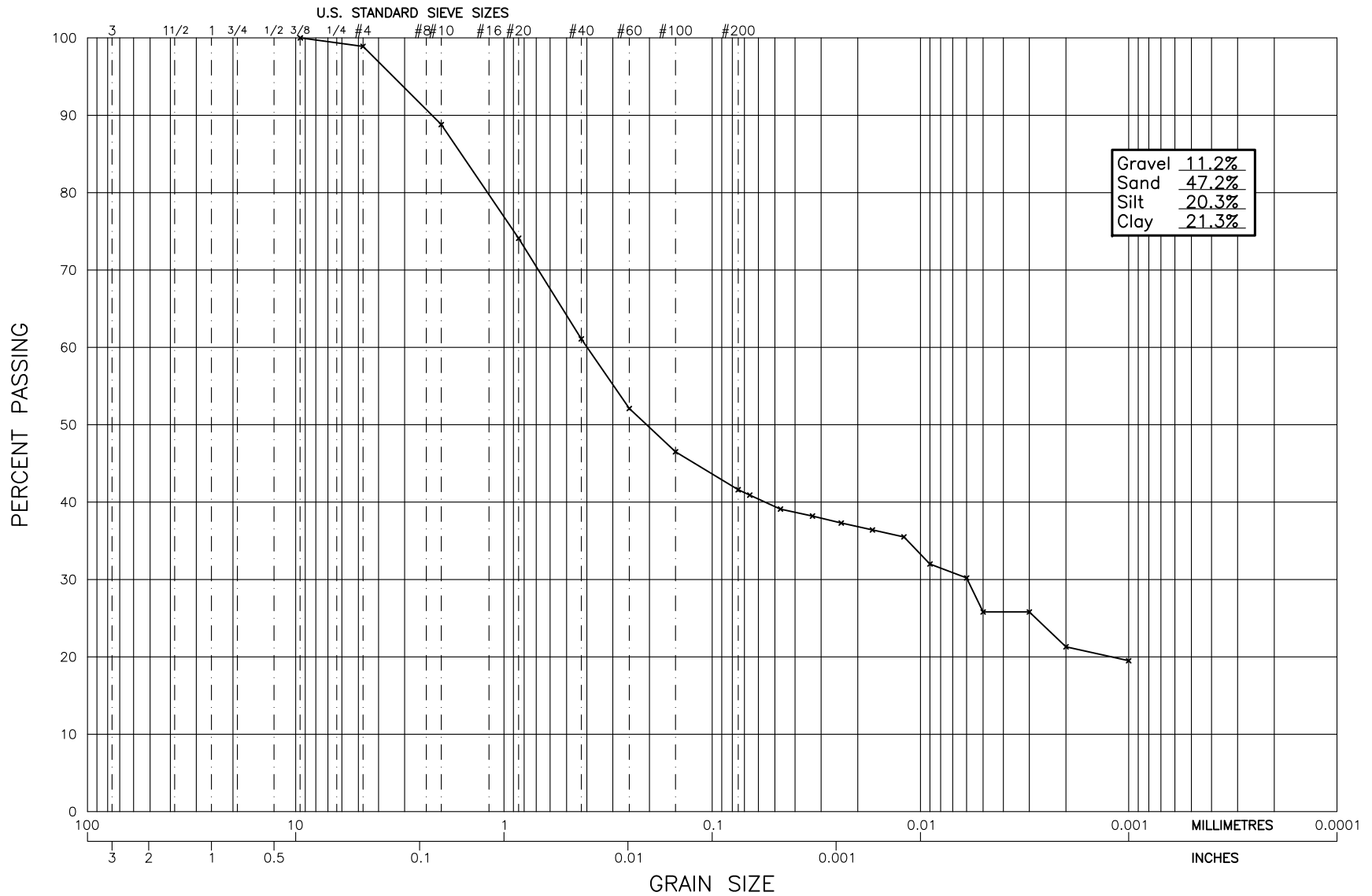
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand,silty some clay				
Sample #: G5		Test #:		Hole #: BH-BGC11-63		Depth: 3.9 - 4.0m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 11, 2011				Date Received: September 13, 2011				Date Tested: Sept.21, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	94.2
Wet Wt. & Tare	867.8		25.4				20	6.3	43.7	87.4	82.3
Dry Wt. & Tare	711.3		19.0				40	6.7	37.0	74.0	69.7
Water Wt.	156.5		12.5				60	5.1	31.9	63.8	60.1
Tare Wt.	131.6		9.5		579.7	100.0	100	3.8	28.1	56.2	52.9
Wt. Of Dry Soil	579.7		4.75	1.3	578.4	99.8	200	3.7	24.4	48.8	46.0
Moisture Content %	27.0		10	32.5	545.9	94.2	Pan	24.4			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=				
			Total	579.7			Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.942	0.5	24.0	23.0	0.01317	18.0	13.3	5.162	0.068	36.0	33.9
50.0	0.942	1	23.0	23.0	0.01317	17.0	13.5	3.673	0.048	34.0	32.0
50.0	0.942	2	22.0	23.0	0.01317	16.0	13.7	2.613	0.034	32.0	30.1
50.0	0.942	4	22.0	23.0	0.01317	16.0	13.7	1.848	0.024	32.0	30.1
50.0	0.942	8	21.0	23.0	0.01317	15.0	13.8	1.314	0.017	30.0	28.3
50.0	0.942	15	20.0	23.0	0.01317	14.0	14.0	0.966	0.013	28.0	26.4
50.0	0.942	30	19.0	23.0	0.01317	13.0	14.2	0.687	0.009	26.0	24.5
50.0	0.942	60	17.0	23.0	0.01317	11.0	14.5	0.491	0.006	22.0	20.7
50.0	0.942	120	16.5	22.0	0.01332	10.0	14.6	0.349	0.005	20.0	18.8
50.0	0.942	240	15.0	22.0	0.01332	8.5	14.9	0.249	0.003	17.0	16.0
50.0	0.942	480	14.0	21.0	0.01348	7.0	15.1	0.178	0.002	14.0	13.2
50.0	0.942	1440	11.5	23.0	0.01317	5.5	15.4	0.103	0.001	11.0	10.4
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND				SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE		



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-63-G9

PROJECT NO.
 K-3300

PLATE NO.
 3300-GS-BH63-G9

GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand, silty/clayey some gravel				
Sample #: G9		Test #:		Hole #: BH-BGC11-63		Depth: 10.15 - 10.25m		Time:			
Sampled By: Client				Tested By: DJ			Checked By:				
Date Sampled: August 11, 2011				Date Received: September 13, 2011			Date Tested: Sept. 21, 2011				
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	88.8	
Wet Wt. & Tare	688.9	25.4				20	8.3	41.7	83.4	74.1	
Dry Wt. & Tare	576.3	19.0				40	7.3	34.4	68.8	61.1	
Water Wt.	112.6	12.5				60	5.1	29.3	58.6	52.1	
Tare Wt.	182.5	9.5		393.8	100.0	100	3.1	26.2	52.4	46.5	
Wt. Of Dry Soil	393.8	4.75	4.5	389.3	98.9	200	2.8	23.4	46.8	41.6	
Moisture Content %	28.6	10	39.5	349.8	88.8	Pan	23.4				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	393.8			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.888	0.5	29.0	23.0	0.01317	23.0	12.5	5.000	0.066	46.0	40.9
50.0	0.888	1	28.0	23.0	0.01317	22.0	12.7	3.559	0.047	44.0	39.1
50.0	0.888	2	27.5	23.0	0.01317	21.5	12.7	2.525	0.033	43.0	38.2
50.0	0.888	4	27.0	23.0	0.01317	21.0	12.8	1.791	0.024	42.0	37.3
50.0	0.888	8	26.5	23.0	0.01317	20.5	12.9	1.270	0.017	41.0	36.4
50.0	0.888	15	26.0	23.0	0.01317	20.0	13.0	0.931	0.012	40.0	35.5
50.0	0.888	30	24.0	23.0	0.01317	18.0	13.3	0.666	0.009	36.0	32.0
50.0	0.888	60	23.0	23.0	0.01317	17.0	13.5	0.474	0.006	34.0	30.2
50.0	0.888	120	21.0	22.0	0.01332	14.5	13.9	0.340	0.005	29.0	25.8
50.0	0.888	240	21.0	22.0	0.01332	14.5	13.9	0.241	0.003	29.0	25.8
50.0	0.888	480	19.0	21.0	0.01348	12.0	14.3	0.173	0.002	24.0	21.3
50.0	0.888	1440	17.0	23.0	0.01317	11.0	14.5	0.100	0.001	22.0	19.5
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

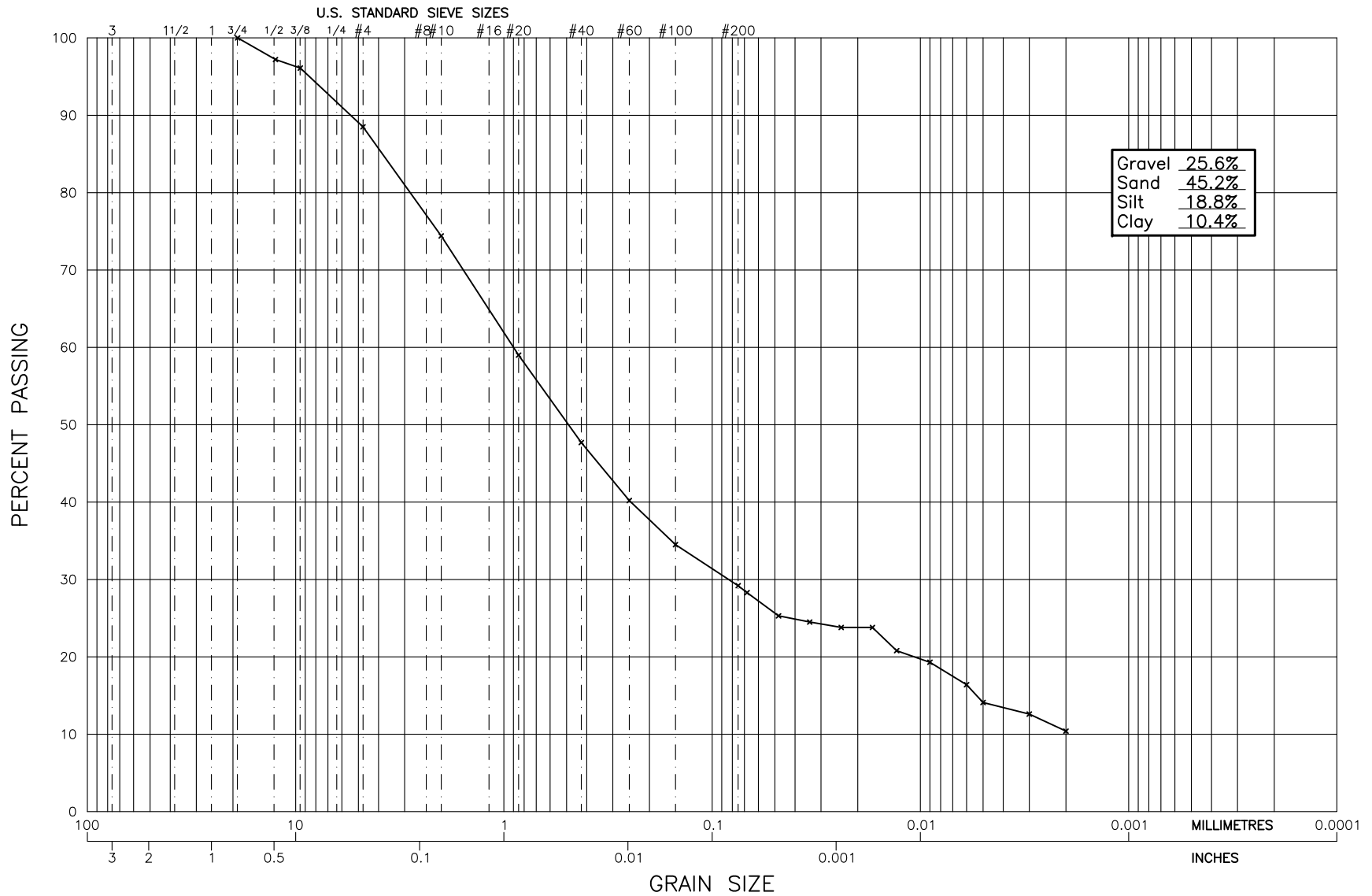
GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept. 26, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #:K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Sand, gravelly some silt and clay				
Sample #: G12		Test #:		Hole #: BH-BGC11-63		Depth: 16.2 - 16.3m		Time:			
Sampled By: Client				Tested By: DJ				Checked By:			
Date Sampled: August 11, 2011				Date Received: September 13, 2011				Date Tested: Sept. 21, 2011			
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis				
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.
Tare No.			38.1				10		50.0	100.0	74.4
Wet Wt. & Tare	985.9		25.4				20	10.3	39.7	79.4	59.0
Dry Wt. & Tare	825.8		19.0		646.1	100.0	40	7.6	32.1	64.2	47.7
Water Wt.	160.1		12.5	17.9	628.2	97.2	60	5.1	27.0	54.0	40.2
Tare Wt.	179.7		9.5	7.0	621.2	96.1	100	3.8	23.2	46.4	34.5
Wt. Of Dry Soil	646.1		4.75	49.6	571.6	88.5	200	3.6	19.6	39.2	29.2
Moisture Content %	24.8		10	91.1	480.5	74.4	Pan	19.6			
Dry Wt. Of Sample from Initial Moisture							Total	50.0			
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=				
			Total	646.1			Tare		Wt. Passing #200 =		
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R'	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.744	0.5	25.0	23.0	0.01317	19.0	13.2	5.130	0.068	38.0	28.3
50.0	0.744	1	23.0	23.0	0.01317	17.0	13.5	3.673	0.048	34.0	25.3
50.0	0.744	2	22.5	23.0	0.01317	16.5	13.6	2.605	0.034	33.0	24.5
50.0	0.744	4	22.0	23.0	0.01317	16.0	13.7	1.848	0.024	32.0	23.8
50.0	0.744	8	22.0	23.0	0.01317	16.0	13.7	1.306	0.017	32.0	23.8
50.0	0.744	15	20.0	23.0	0.01317	14.0	14.0	0.966	0.013	28.0	20.8
50.0	0.744	30	19.0	23.0	0.01317	13.0	14.2	0.687	0.009	26.0	19.3
50.0	0.744	60	17.0	23.0	0.01317	11.0	14.5	0.491	0.006	22.0	16.4
50.0	0.744	120	16.0	22.0	0.01332	9.5	14.7	0.350	0.005	19.0	14.1
50.0	0.744	240	15.0	22.0	0.01332	8.5	14.9	0.249	0.003	17.0	12.6
50.0	0.744	480	14.0	21.0	0.01348	7.0	15.1	0.178	0.002	14.0	10.4
50.0	0.744	1440	13.0	23.0	0.01317	7.0	15.1	0.103	0.001	14.0	10.4
Hydrometer #: 932452			Graduate #:			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



Gravel	25.6%
Sand	45.2%
Silt	18.8%
Clay	10.4%

GEO NORTH ENGINEERING LTD.

3975 18th Avenue
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 Tel. 250-564-4304 Fax 250-564-9323

BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-63-G12

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH63-G12

GeoNorth Engineering Ltd.

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Phone (250)564-4304; Fax (250)564-9323



SIEVE ANALYSIS REPORT

10 20 40 60 SERIES

PROJECT NO. K-3300

CLIENT BGC Engineering Inc.

C.C.

TO

BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC
V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

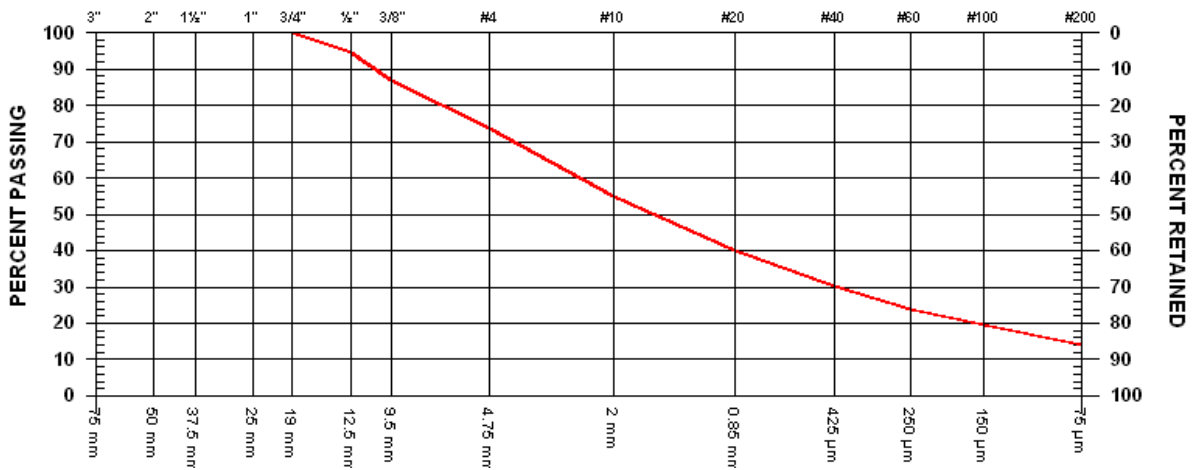
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 11 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65
SOURCE SPT 1 @ 0.76 - 1.21m
SPECIFICATION
MATERIAL TYPE Gravel

SAMPLED BY Client
TESTED BY DJ
TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	
1"	25 mm	
3/4"	19 mm	100.0
1/2"	12.5 mm	94.4
3/8"	9.5 mm	86.9

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	73.7
No. 10	2.00 mm	55.0
No. 20	850 µm	40.0
No. 40	425 µm	30.1
No. 60	250 µm	23.8
No. 100	150 µm	19.4
No. 200	75 µm	14.1

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
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 V6Z 2A9

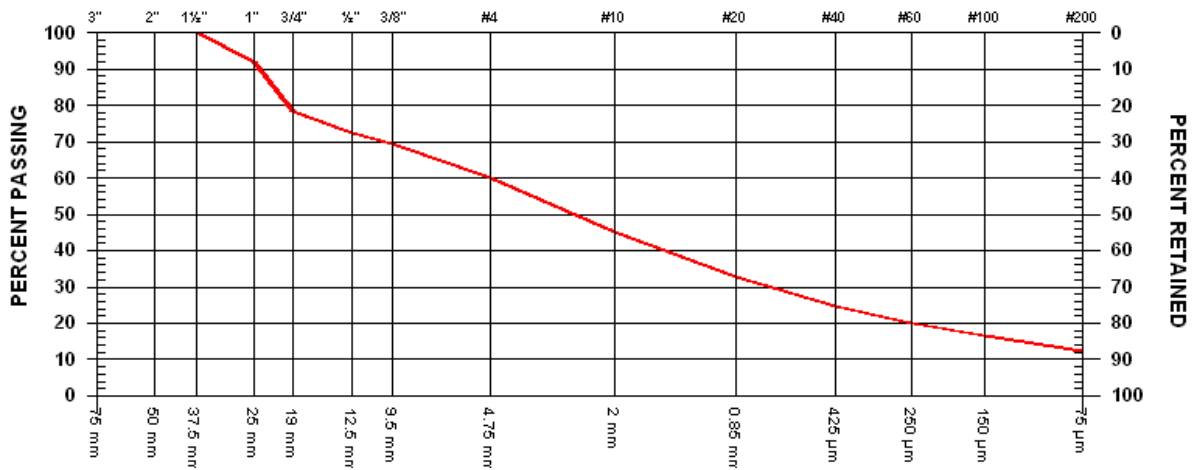
ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing
 Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 12 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65 SAMPLED BY Client
 SOURCE SPT 2 @ 1.52 - 1.97m TESTED BY DJ
 SPECIFICATION MATERIAL TYPE Gravel TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	92.1	
3/4" 19 mm	78.2	
1/2" 12.5 mm	72.5	
3/8" 9.5 mm	69.4	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	60.2	
No. 10 2.00 mm	45.0	
No. 20 850 µm	32.6	
No. 40 425 µm	24.8	
No. 60 250 µm	19.9	
No. 100 150 µm	16.4	
No. 200 75 µm	12.2	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

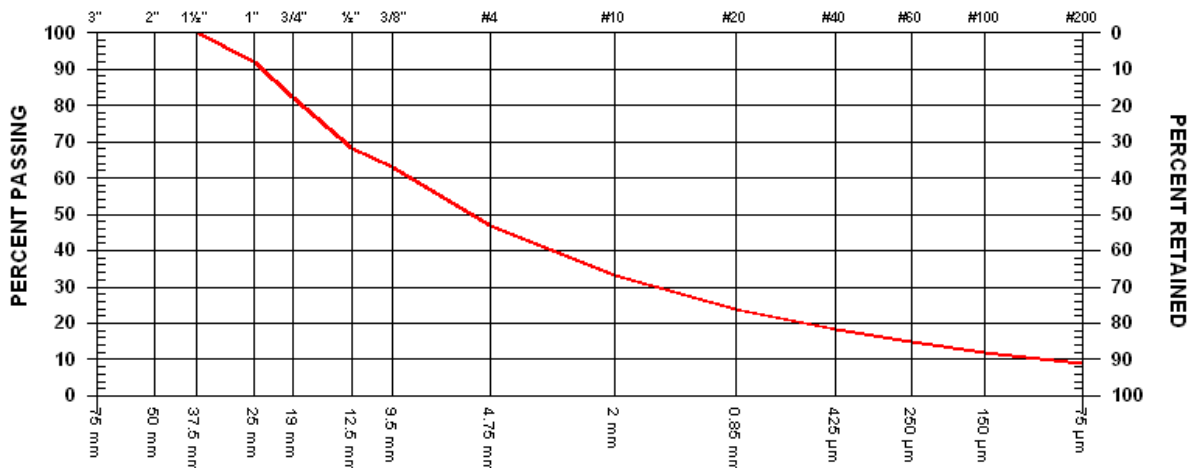
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 13 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65 SAMPLED BY Client
 SOURCE SPT 3 @ 2.28 - 2.73m TESTED BY DJ
 SPECIFICATION MATERIAL TYPE Gravel TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	92.1	
3/4" 19 mm	82.2	
1/2" 12.5 mm	68.0	
3/8" 9.5 mm	63.1	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	47.0	
No. 10 2.00 mm	33.1	
No. 20 850 µm	24.0	
No. 40 425 µm	18.3	
No. 60 250 µm	14.7	
No. 100 150 µm	12.0	
No. 200 75 µm	8.9	

COMMENTS

GeoNorth Engineering Ltd.

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SIEVE ANALYSIS REPORT

10 20 40 60 SERIES

PROJECT NO. K-3300

CLIENT BGC Engineering Inc.

C.C.

TO

BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC
V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

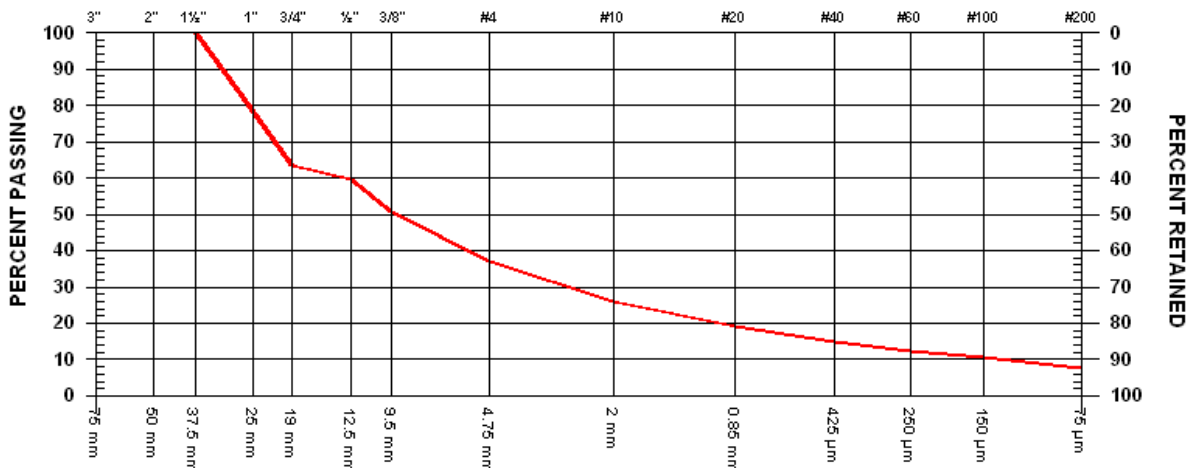
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 14 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65
SOURCE SPT 4 @ 3.04 - 3.49m
SPECIFICATION
MATERIAL TYPE Gravel

SAMPLED BY Client
TESTED BY DJ
TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	78.5	
3/4" 19 mm	63.3	
1/2" 12.5 mm	59.6	
3/8" 9.5 mm	50.7	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	36.9	
No. 10 2.00 mm	26.1	
No. 20 850 µm	19.2	
No. 40 425 µm	15.1	
No. 60 250 µm	12.5	
No. 100 150 µm	10.5	
No. 200 75 µm	7.8	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
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 Vancouver, BC
 V6Z 2A9

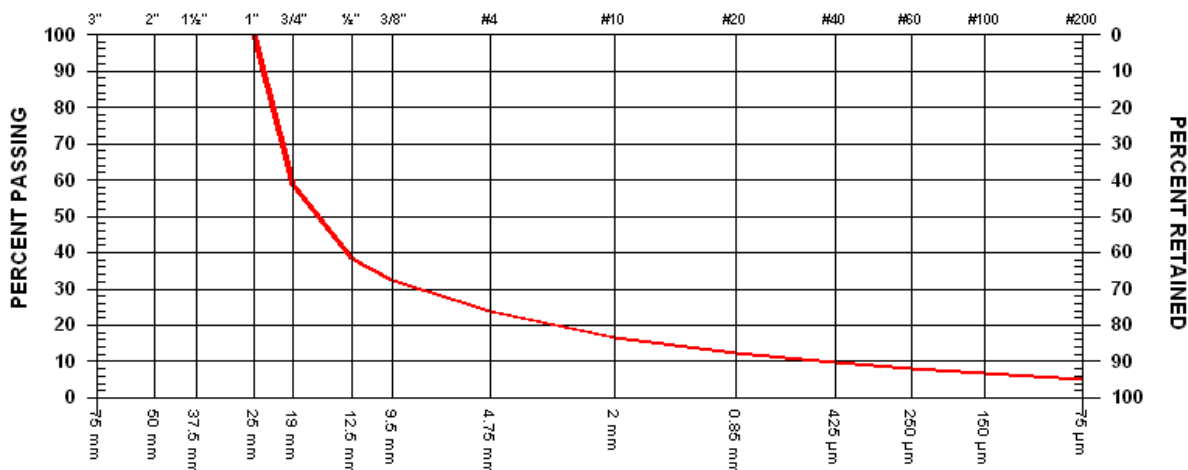
ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing
 Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 15 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65 SAMPLED BY Client
 SOURCE SPT 5 @ 3.80 - 4.25m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm	100.0	
3/4" 19 mm	58.9	
1/2" 12.5 mm	38.5	
3/8" 9.5 mm	32.5	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	24.0	
No. 10 2.00 mm	16.7	
No. 20 850 µm	12.2	
No. 40 425 µm	9.6	
No. 60 250 µm	8.0	
No. 100 150 µm	6.7	
No. 200 75 µm	5.2	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
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 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

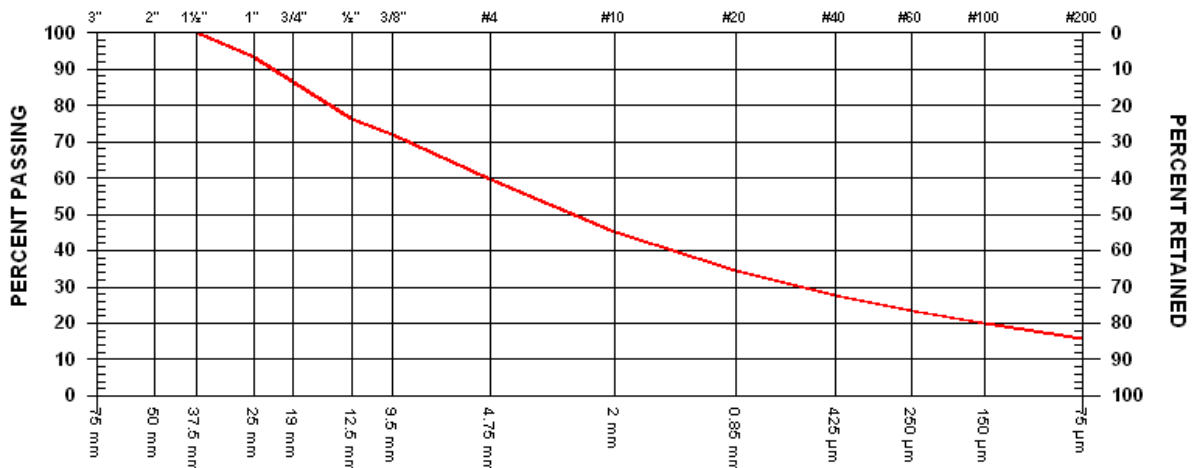
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 16 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65 SAMPLED BY Client
 SOURCE SPT 6 @ 4.56 - 5.01m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	93.3	
3/4" 19 mm	86.4	
1/2" 12.5 mm	76.3	
3/8" 9.5 mm	72.0	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	59.5	
No. 10 2.00 mm	45.1	
No. 20 850 µm	34.3	
No. 40 425 µm	27.6	
No. 60 250 µm	23.3	
No. 100 150 µm	20.2	
No. 200 75 µm	15.9	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
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ATTN: Peter Quinn

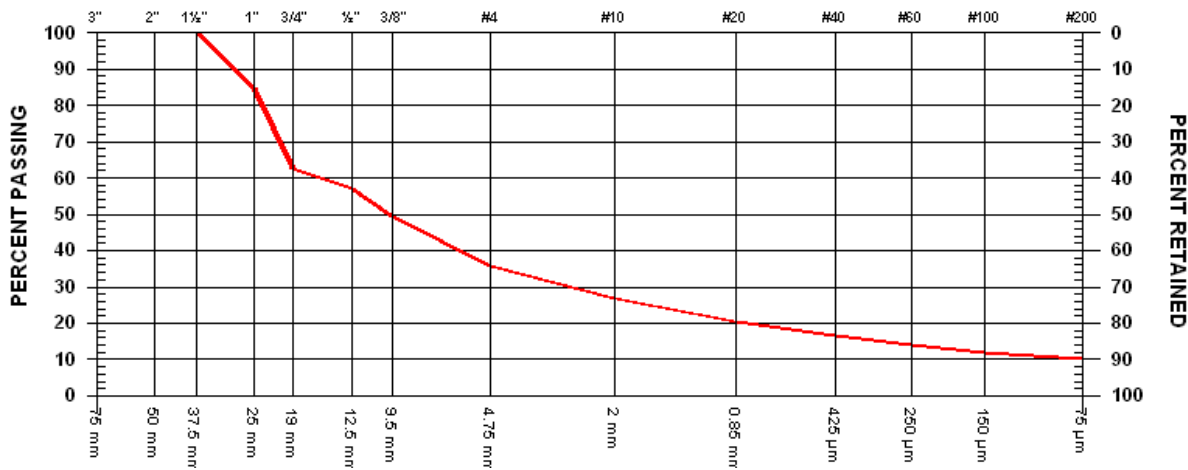
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 17 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65 SAMPLED BY Client
 SOURCE SPT 7 @ 5.33 - 5.78m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	84.7	
3/4" 19 mm	62.4	
1/2" 12.5 mm	57.2	
3/8" 9.5 mm	49.6	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	35.9	
No. 10 2.00 mm	26.9	
No. 20 850 µm	20.6	
No. 40 425 µm	16.5	
No. 60 250 µm	13.9	
No. 100 150 µm	12.1	
No. 200 75 µm	10.0	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
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TO
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 Suite 500 - 1045 Howe Street
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 V6Z 2A9

ATTN: Peter Quinn

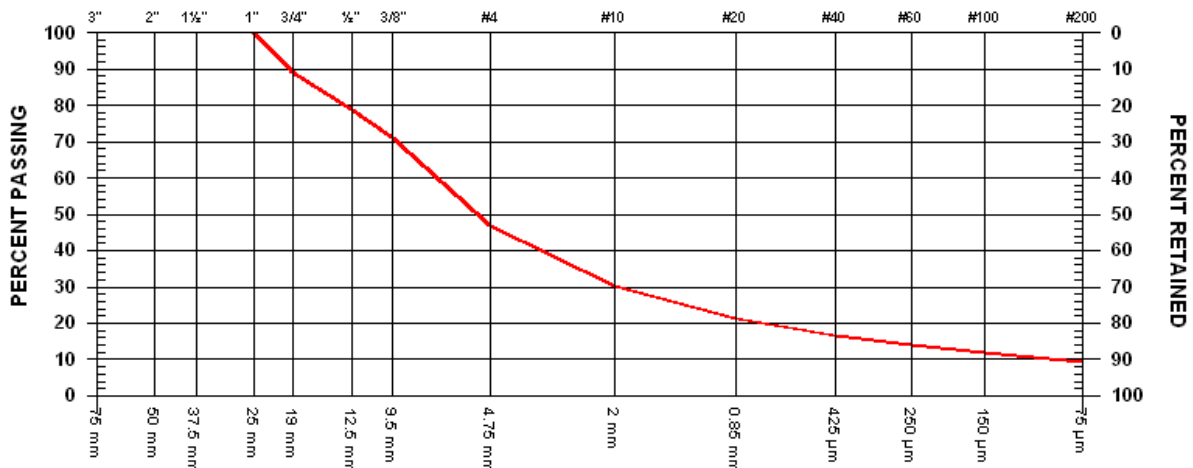
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 18 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.04 DATE SAMPLED 2011.Aug.14

SUPPLIER BH-BGC11-65 SAMPLED BY Client
 SOURCE SPT 8 @ 6.09 - 6.54m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm		
1" 25 mm	100.0	
3/4" 19 mm	89.0	
1/2" 12.5 mm	78.6	
3/8" 9.5 mm	70.9	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	47.0	
No. 10 2.00 mm	30.4	
No. 20 850 µm	21.4	
No. 40 425 µm	16.7	
No. 60 250 µm	14.0	
No. 100 150 µm	12.0	
No. 200 75 µm	9.5	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
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TO
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 Suite 500 - 1045 Howe Street
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ATTN: Peter Quinn

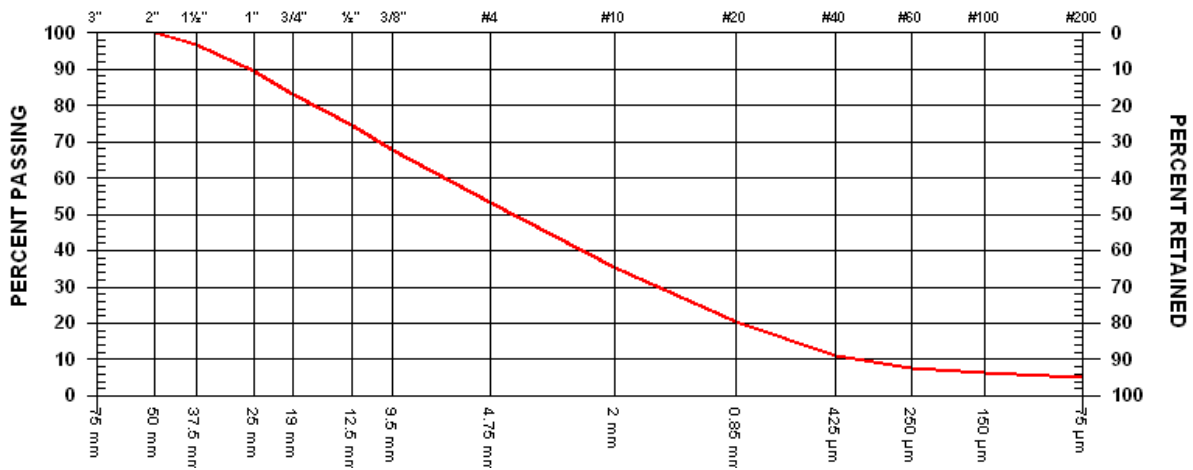
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 37 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.10 DATE SAMPLED 2011.Aug.05

SUPPLIER	TP-BGC11-103	SAMPLED BY	Client
SOURCE	G1 @ 4.5m	TESTED BY	DJ
SPECIFICATION		TEST METHOD	WASHED
MATERIAL TYPE	Gravel		



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	100.0
1 1/2"	37.5 mm	96.6
1"	25 mm	89.6
3/4"	19 mm	83.1
1/2"	12.5 mm	74.7
3/8"	9.5 mm	67.5

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	53.2
No. 10	2.00 mm	35.4
No. 20	850 µm	20.6
No. 40	425 µm	11.0
No. 60	250 µm	7.7
No. 100	150 µm	6.4
No. 200	75 µm	5.3

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
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 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

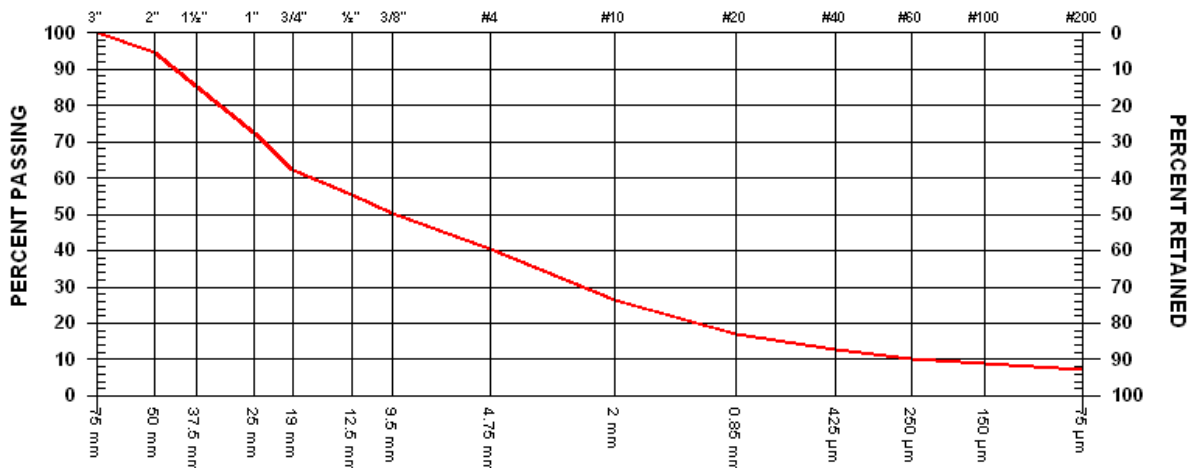
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 21 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.08 DATE SAMPLED 2011.Aug.23

SUPPLIER TP-BGC11-104
 SOURCE G1 @ 1.2-1.6m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED

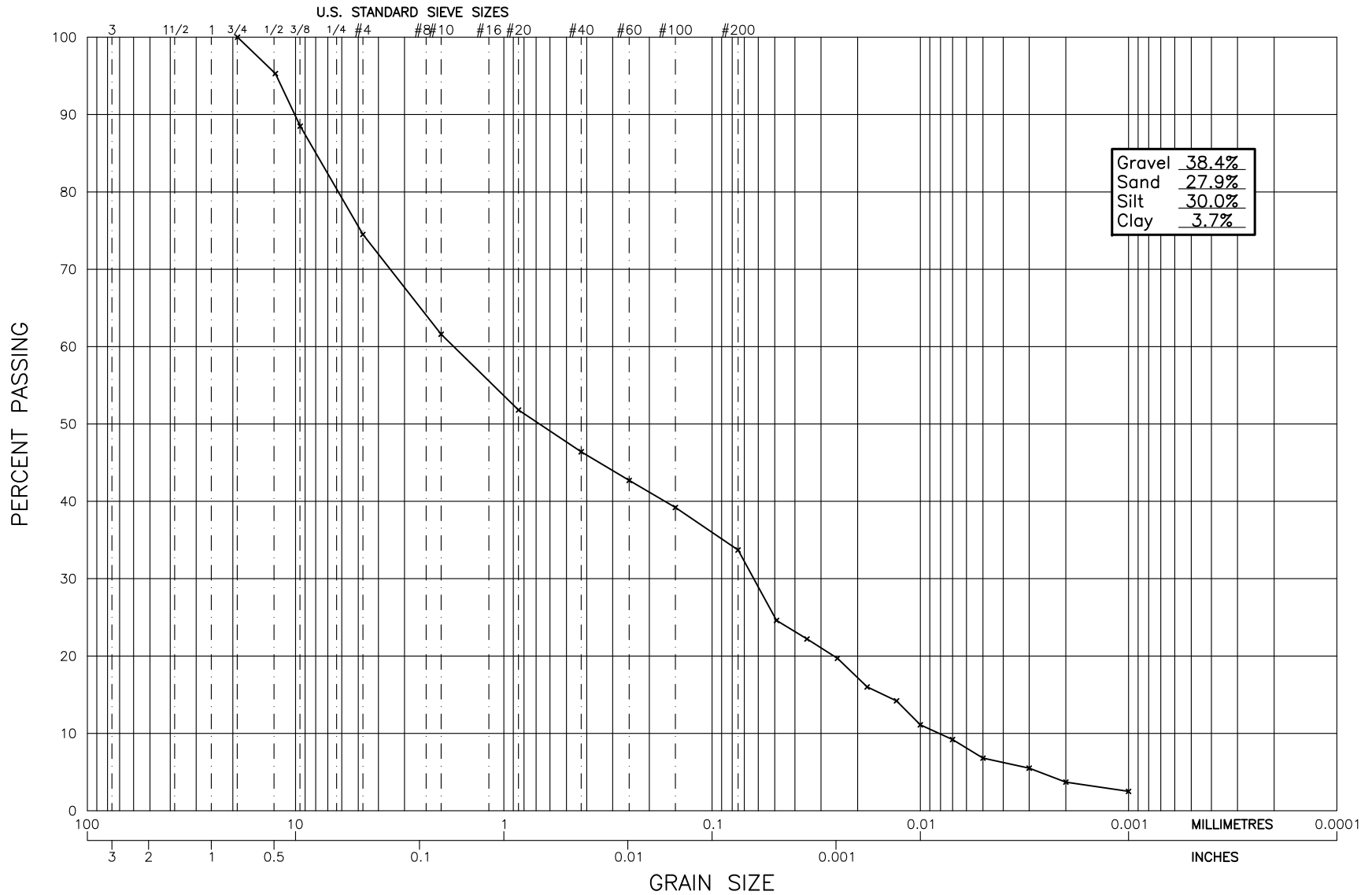


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm	100.0	
2" 50 mm	94.7	
1 1/2" 37.5 mm	85.2	
1" 25 mm	72.2	
3/4" 19 mm	62.2	
1/2" 12.5 mm	55.4	
3/8" 9.5 mm	50.3	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	40.4	
No. 10 2.00 mm	26.5	
No. 20 850 µm	17.1	
No. 40 425 µm	12.9	
No. 60 250 µm	10.4	
No. 100 150 µm	8.9	
No. 200 75 µm	7.3	

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF TP-BGC11-107-G1

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-TP107-G1

GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: November 10, 2011				
Project Name: Eagle Gold Soils and Aggregate Testing							Project #: K-3300				
Source/Location: Dublin Gulch, Yukon							Type: Gravel, silty and sandy				
Sample #: G1		Test #:		Hole #: TP-BGC11-107		Depth: 2.8 - 3.0m		Time:			
Sampled By: Client				Tested By: DJ				Checked By: DJ			
Date Sampled: Aug.16, 2011				Date Received: Sept.13, 2011				Date Tested: Nov.9, 2011			
Initial Moisture Content		Sieve Analysis				Hydrometer Sieve Analysis					
		Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.		38.1				10		50.0	100.0	61.6	
Wet Wt. & Tare	1009.0	25.4				20	7.9	42.1	84.2	51.8	
Dry Wt. & Tare	889.5	19.0		709.8	100.0	40	4.4	37.7	75.4	46.4	
Water Wt.	119.5	12.5	33.6	676.2	95.3	60	3.0	34.7	69.4	42.7	
Tare Wt.	179.7	9.5	48.0	628.2	88.5	100	2.9	31.8	63.6	39.2	
Wt. Of Dry Soil	709.8	4.75	99.7	528.5	74.5	200	4.4	27.4	54.8	33.7	
Moisture Content %	16.8	10	91.5	437.0	61.6	Pan	27.4				
Dry Wt. Of Sample from Initial Moisture						Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =						Unwashed Wt.=					
		Total	709.8			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)
50.0	0.616	0.5	28.5	20.0	0.01365	21.5	12.7	5.049	0.069	43.0	26.5
50.0	0.616	1	27.0	20.0	0.01365	20.0	13.0	3.605	0.049	40.0	24.6
50.0	0.616	2	25.0	20.0	0.01365	18.0	13.3	2.581	0.035	36.0	22.2
50.0	0.616	4	23.0	20.0	0.01365	16.0	13.7	1.848	0.025	32.0	19.7
50.0	0.616	8	20.0	20.0	0.01365	13.0	14.2	1.330	0.018	26.0	16.0
50.0	0.616	15	18.5	20.0	0.01365	11.5	14.4	0.980	0.013	23.0	14.2
50.0	0.616	30	16.0	20.0	0.01365	9.0	14.8	0.703	0.010	18.0	11.1
50.0	0.616	60	14.5	20.0	0.01365	7.5	15.1	0.501	0.007	15.0	9.2
50.0	0.616	120	12.5	20.0	0.01365	5.5	15.4	0.358	0.005	11.0	6.8
50.0	0.616	240	11.5	20.0	0.01365	4.5	15.6	0.255	0.003	9.0	5.5
50.0	0.616	480	10.0	21.0	0.01348	3.0	15.8	0.181	0.002	6.0	3.7
50.0	0.616	1440	10.0	19.0	0.01382	2.0	16.0	0.105	0.001	4.0	2.5
Hydrometer #: 932452			Graduate #: 6			Dispersing Agent: Sodium Hex			Amount: 125ml		
Density of Solids:											
Description of Sample:											



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

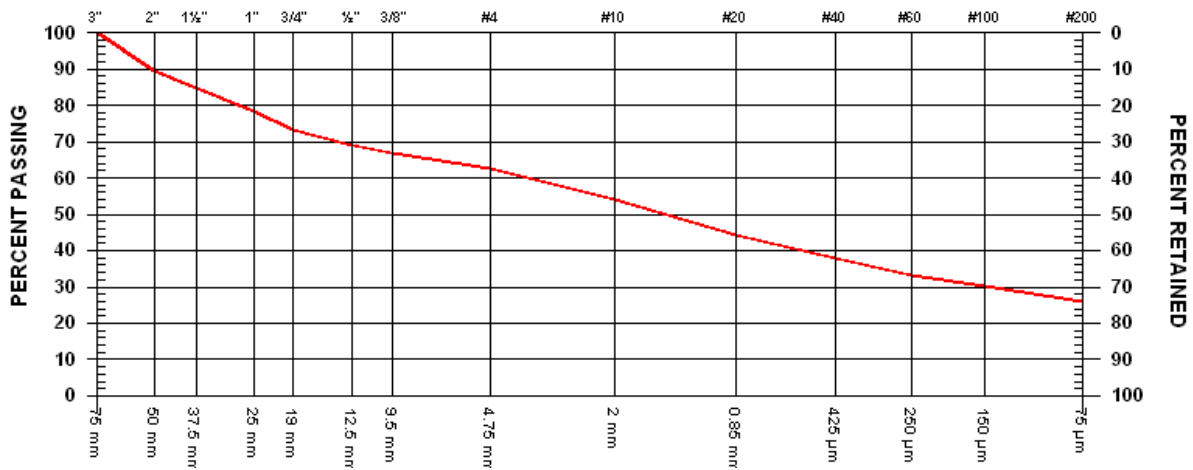
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 22 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.08 DATE SAMPLED 2011.Sep.08

SUPPLIER TP-BGC11-108
 SOURCE G1 @ 0.9-1.15m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	100.0
2"	50 mm	89.3
1 1/2"	37.5 mm	84.9
1"	25 mm	78.2
3/4"	19 mm	73.1
1/2"	12.5 mm	69.1
3/8"	9.5 mm	67.0

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	62.6
No. 10	2.00 mm	54.2
No. 20	850 µm	44.2
No. 40	425 µm	37.8
No. 60	250 µm	33.4
No. 100	150 µm	30.1
No. 200	75 µm	25.9

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

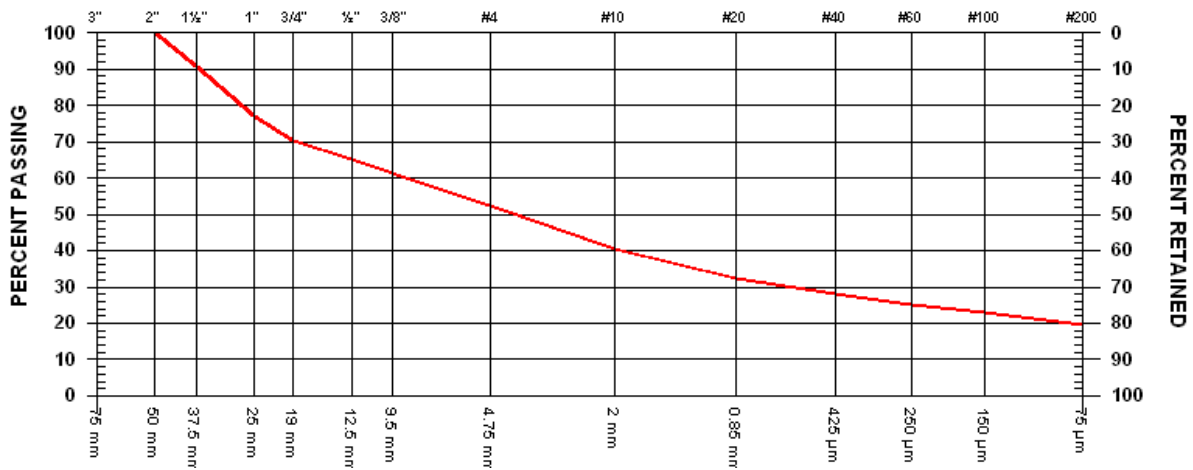
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 23 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.08 DATE SAMPLED 2011.Aug.17

SUPPLIER TP-BGC11-112
 SOURCE G1 @ 1.3-1.4m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED

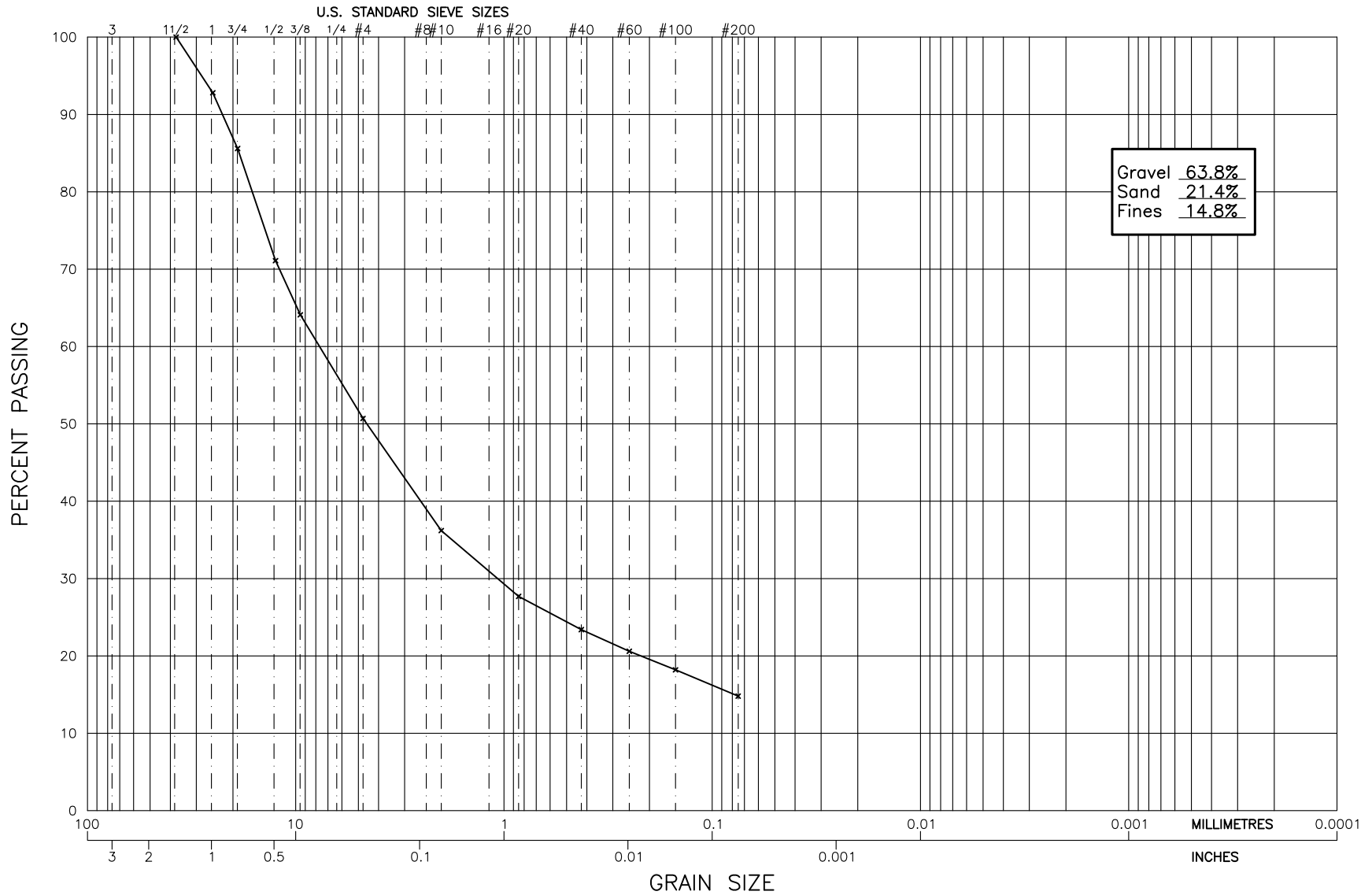


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	100.0
1 1/2"	37.5 mm	90.7
1"	25 mm	77.1
3/4"	19 mm	70.3
1/2"	12.5 mm	65.3
3/8"	9.5 mm	61.5

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	52.2
No. 10	2.00 mm	40.5
No. 20	850 µm	32.3
No. 40	425 µm	27.9
No. 60	250 µm	25.1
No. 100	150 µm	22.8
No. 200	75 µm	19.5

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



GEO NORTH ENGINEERING LTD.

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF BH-BGC11-116-G2

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-BH116-G2



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

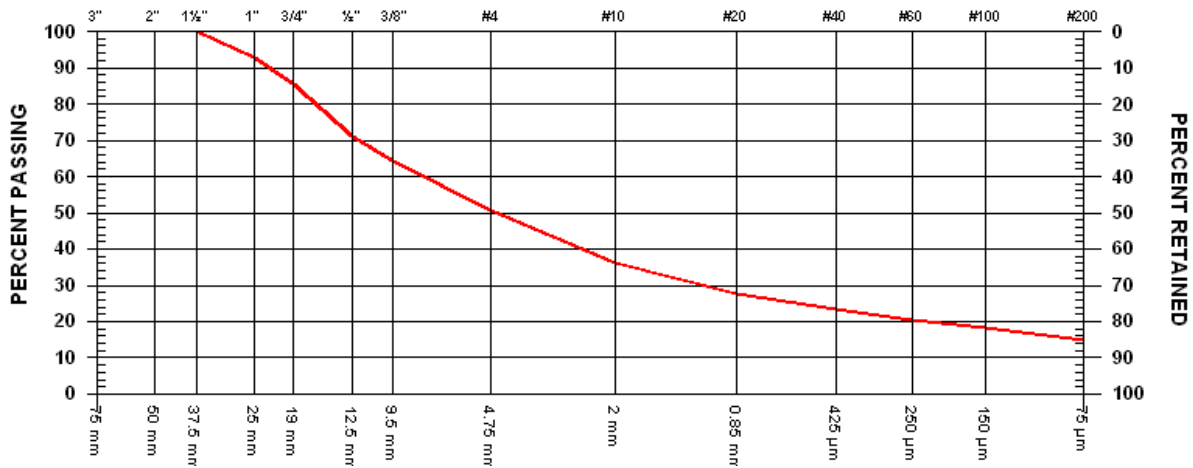
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 6 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Sep.30 DATE SAMPLED 2011.Aug.18

SUPPLIER TP-BGC11-116
 SOURCE G2 @ 2.2 - 2.3m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	
1 1/2"	37.5 mm	100.0
1"	25 mm	92.8
3/4"	19 mm	85.6
1/2"	12.5 mm	71.1
3/8"	9.5 mm	64.1

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	50.7
No. 10	2.00 mm	36.2
No. 20	850 µm	27.7
No. 40	425 µm	23.4
No. 60	250 µm	20.6
No. 100	150 µm	18.2
No. 200	75 µm	14.8

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

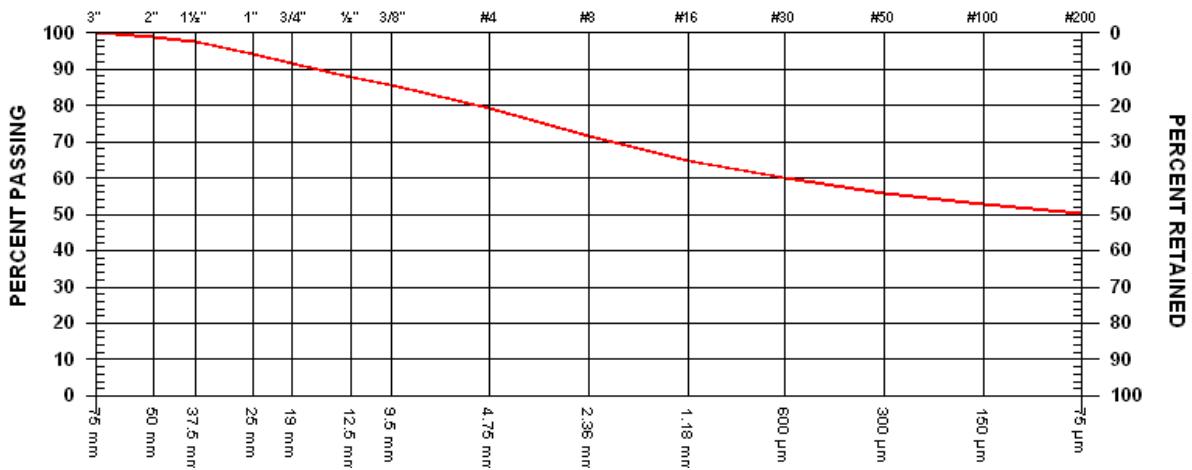
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 41 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.14 DATE SAMPLED 2011.Aug.19

SUPPLIER TP-BGC11-124
 SOURCE G1 @ 6.2 - 6.4m
 SPECIFICATION
 MATERIAL TYPE Silt, sandy

SAMPLED BY Client
 TESTED BY RO
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	100.0
2"	50 mm	98.8
1 1/2"	37.5 mm	97.4
1"	25 mm	94.1
3/4"	19 mm	91.5
1/2"	12.5 mm	87.5
3/8"	9.5 mm	85.6

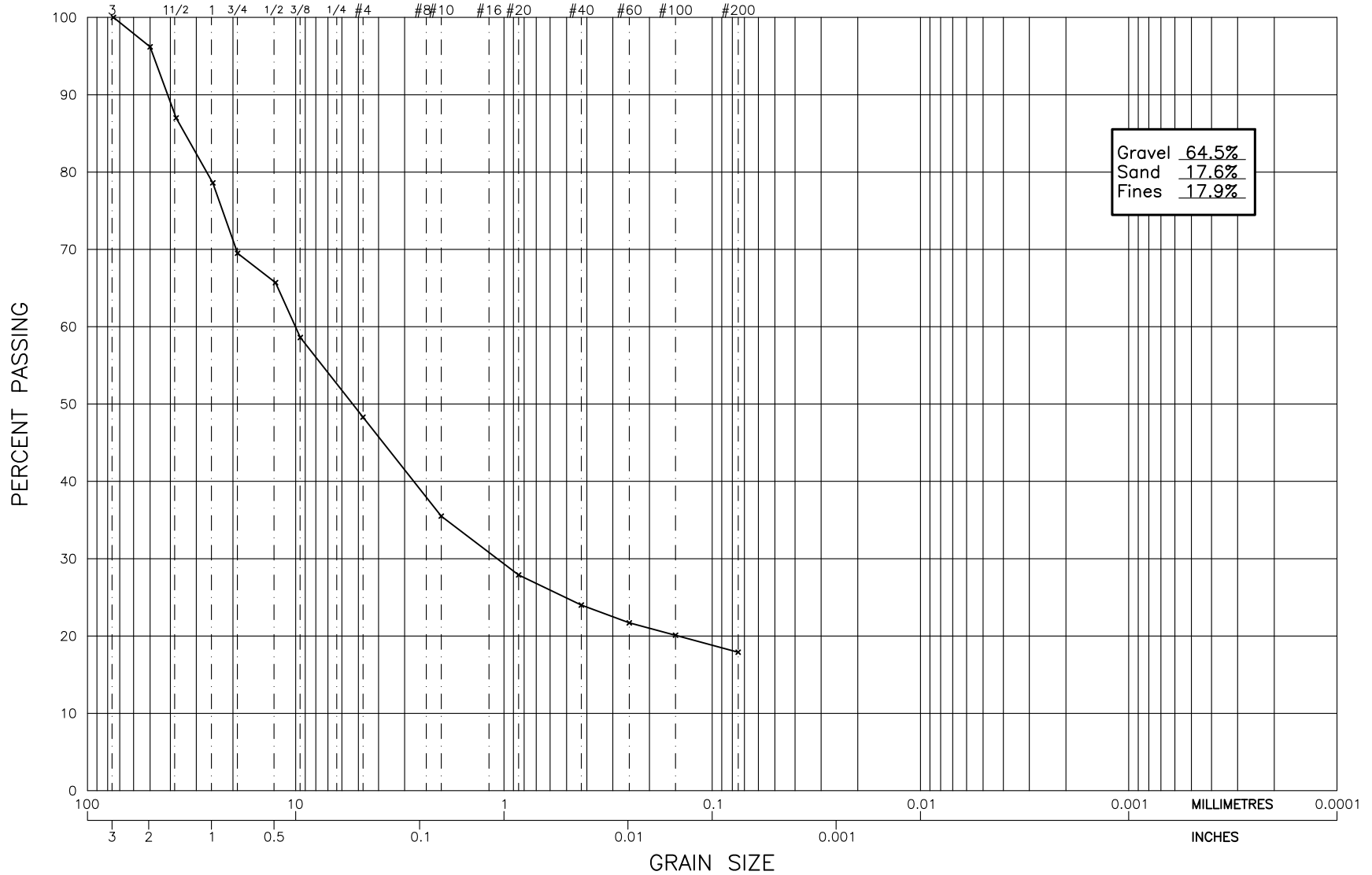
SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	79.0
No. 8	2.36 mm	71.5
No. 16	1.18 mm	64.8
No. 30	600 µm	59.9
No. 50	300 µm	55.8
No. 100	150 µm	52.9
No. 200	75 µm	50.3

MOISTURE CONTENT 23.7%

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	

U.S. STANDARD SIEVE SIZES



Gravel	64.5%
Sand	17.6%
Fines	17.9%

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF TP-BGC11-126

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-TP126



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 630 - 1718 Argyle Street
 Halifax, Nova Scotia
 B3J 3N6

ATTN: Peter Quinn

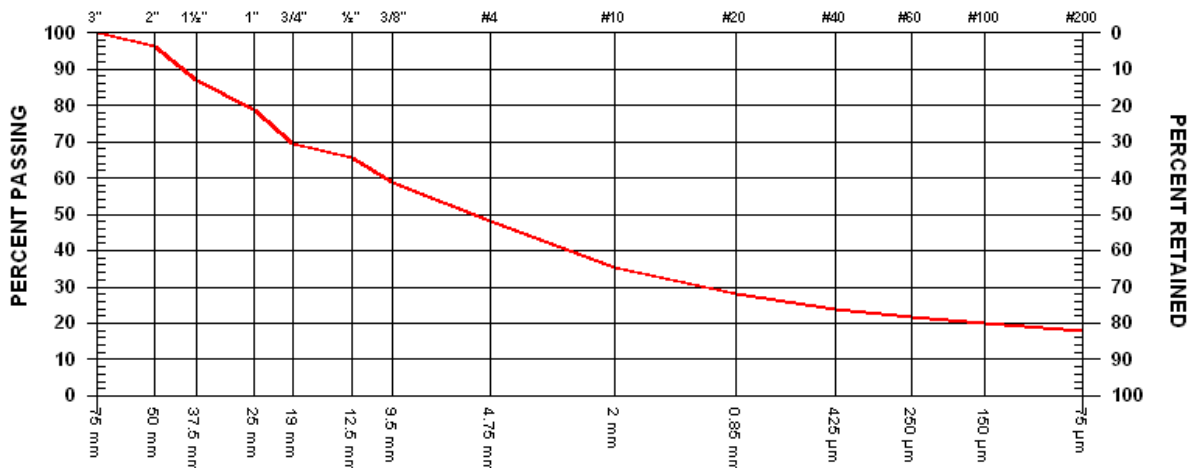
PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 1 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Sep.21 DATE SAMPLED 2011.Aug.19

SUPPLIER TP-BGC11-126 SAMPLED BY Client
 SOURCE 1.7m - 2.2m TESTED BY DJ
 SPECIFICATION TEST METHOD WASHED
 MATERIAL TYPE Gravel

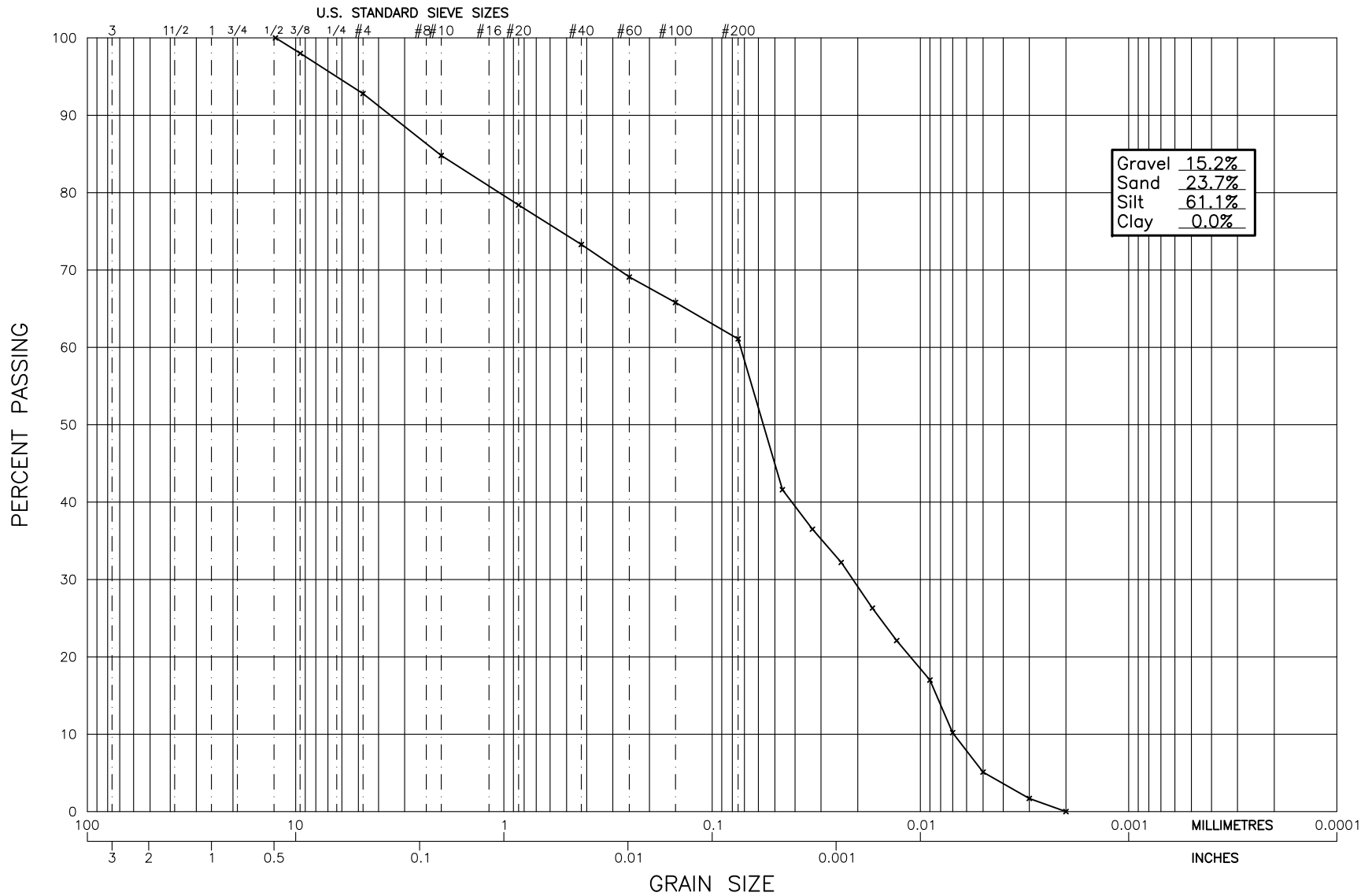


GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm	100.0	
2" 50 mm	96.2	
1 1/2" 37.5 mm	87.0	
1" 25 mm	78.6	
3/4" 19 mm	69.5	
1/2" 12.5 mm	65.7	
3/8" 9.5 mm	58.6	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	48.3	
No. 10 2.00 mm	35.5	
No. 20 850 µm	27.9	
No. 40 425 µm	24.0	
No. 60 250 µm	21.7	
No. 100 150 µm	20.1	
No. 200 75 µm	17.9	

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	



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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF TP-BGC11-128-G1

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-TP128-G1

GeoNorth Engineering

Test Designation: ASTM D-422

Hydrometer Analysis

Client: BGC Engineering Ltd.							Date: Sept.26, 2011					
Project Name: Eagle Gold Soil and Aggregate Testing							Project #: K-3300					
Source/Location: Dublin Gulch, Yukon							Type: Silt,sandy					
Sample #: G1		Test #:		Hole #: TP-BGC11-128		Depth: 2.5 - 3.0m		Time:				
Sampled By: Client				Tested By: DJ				Checked By:				
Date Sampled: August 19, 2011				Date Received: September 13, 2011				Date Tested: Sept. 23, 2011				
Initial Moisture Content			Sieve Analysis				Hydrometer Sieve Analysis					
			Sieve No.	Weight Retained	Total Wt. Passing	% Finer Than Orig. Samp.	Sieve No.	Weight Retained	Total Wt. Finer Than	% Finer Than	% Finer Than Orig Samp.	
Tare No.			38.1				10		50.0	100.0	84.8	
Wet Wt. & Tare	879.4		25.4				20	3.8	46.2	92.4	78.4	
Dry Wt. & Tare	726.5		19.0				40	3.0	43.2	86.4	73.3	
Water Wt.	152.9		12.5		545.8	100.0	60	2.5	40.7	81.4	69.1	
Tare Wt.	180.7		9.5	11.0	534.8	98.0	100	1.9	38.8	77.6	65.8	
Wt. Of Dry Soil	545.8		4.75	28.2	506.6	92.8	200	2.8	36.0	72.0	61.1	
Moisture Content %	28.0		10	43.5	463.1	84.8	Pan	36.0				
Dry Wt. Of Sample from Initial Moisture							Total	50.0				
=(100xWet Soil Wt.)/(100 + Initial Moisture) =							Unwashed Wt.=					
			Total	545.8			Tare		Wt. Passing #200 =			
Starting Wt. (g)	% - #10	Elapsed Time (min)	Reading R	Temp (OC)	K	Corr. Reading R`	Zr (cm)	SQRT(Zr)/T (min)	D (mm)	N (%)	N*(%#10)	
50.0	0.848	0.5	36.0	23.0	0.01317	30.0	11.3	4.763	0.063	60.0	50.9	
50.0	0.848	1	30.5	23.0	0.01317	24.5	12.3	3.500	0.046	49.0	41.6	
50.0	0.848	2	27.5	23.0	0.01317	21.5	12.7	2.525	0.033	43.0	36.5	
50.0	0.848	4	25.0	23.0	0.01317	19.0	13.2	1.814	0.024	38.0	32.2	
50.0	0.848	8	21.5	23.0	0.01317	15.5	13.7	1.310	0.017	31.0	26.3	
50.0	0.848	15	19.0	23.0	0.01317	13.0	14.2	0.971	0.013	26.0	22.1	
50.0	0.848	30	16.0	23.0	0.01317	10.0	14.6	0.699	0.009	20.0	17.0	
50.0	0.848	60	12.0	23.0	0.01317	6.0	15.3	0.505	0.007	12.0	10.2	
50.0	0.848	120	9.0	23.0	0.01317	3.0	15.8	0.363	0.005	6.0	5.1	
50.0	0.848	240	7.0	23.0	0.01317	1.0	16.1	0.259	0.003	2.0	1.7	
50.0	0.848	480	6.0	23.0	0.01317	0.0	16.3	0.184	0.002	0.0	0.0	
50.0	0.848	1446	6.0	23.0	0.01317	0.0	16.3	0.106	0.001	0.0	0.0	
Hydrometer #: 932452			Dispersing Agent: Sodium Hex				Amount: 125ml					
Density of Solids:												
Description of Sample:												



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

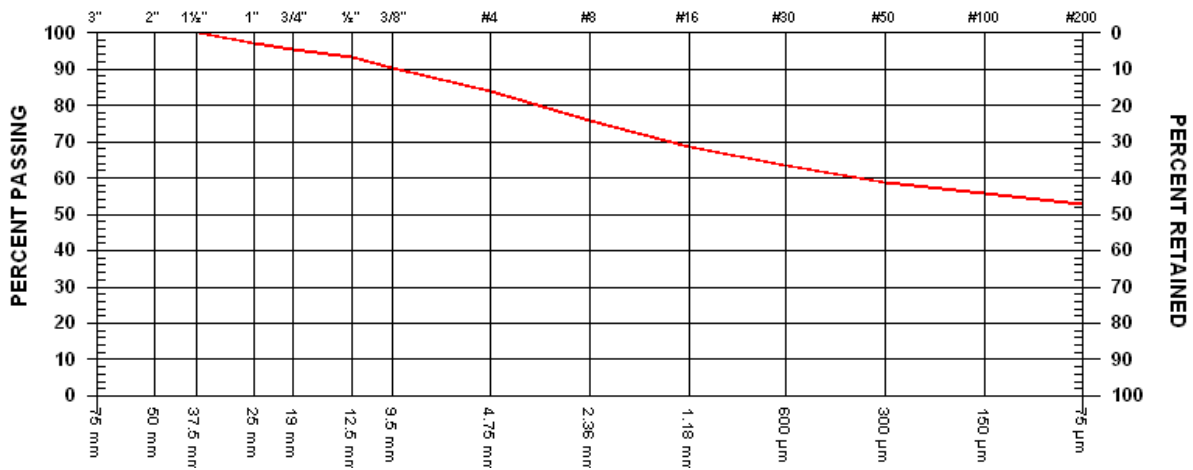
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 40 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.14 DATE SAMPLED 2011.Aug.20

SUPPLIER TP-BGC11-129
 SOURCE G1 @ 6.6 - 6.8m
 SPECIFICATION
 MATERIAL TYPE Silt some sand

SAMPLED BY Client
 TESTED BY RO
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	97.1	
3/4" 19 mm	95.4	
1/2" 12.5 mm	93.2	
3/8" 9.5 mm	90.2	

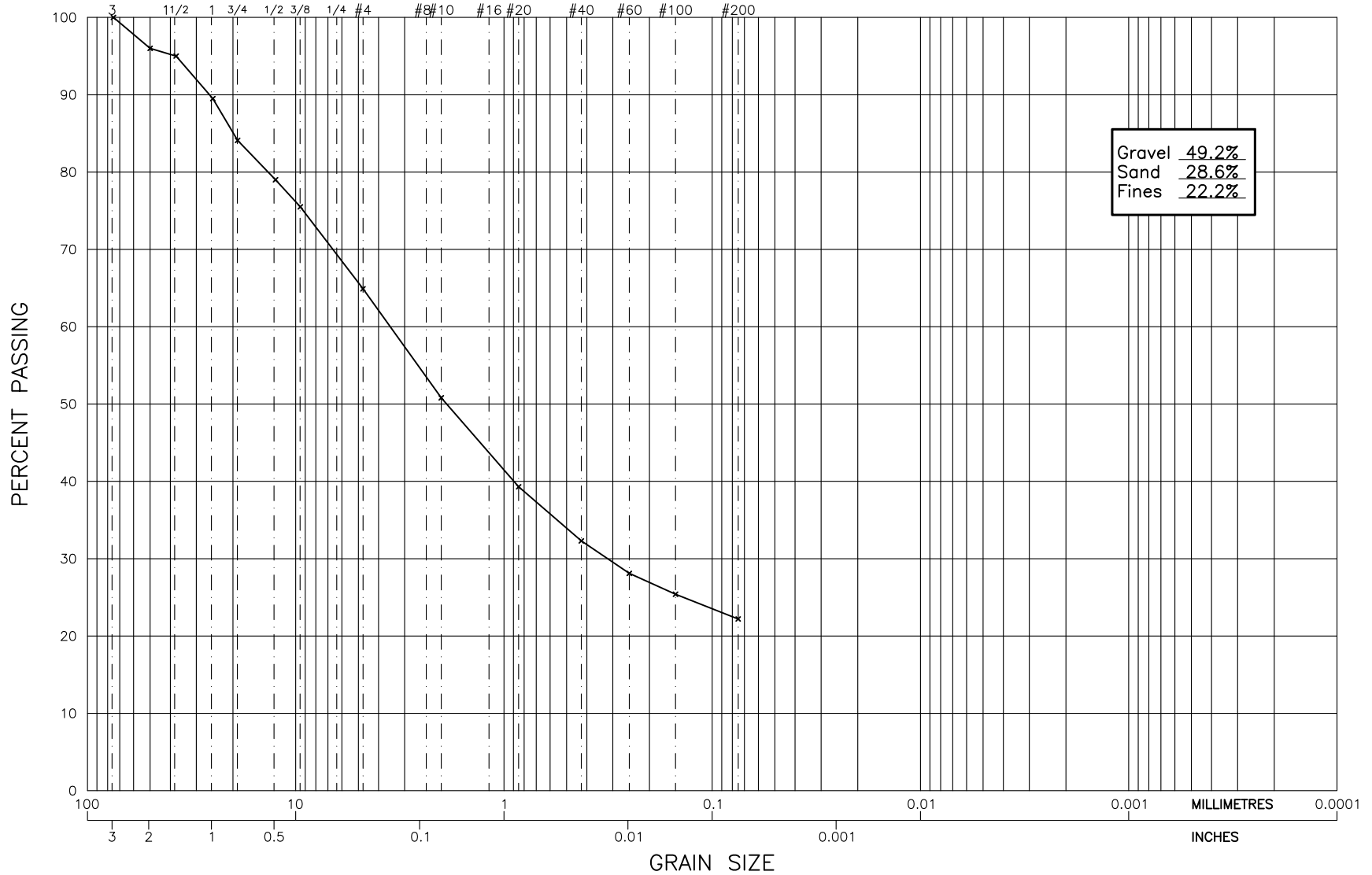
SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	83.9	
No. 8 2.36 mm	75.9	
No. 16 1.18 mm	68.6	
No. 30 600 µm	63.4	
No. 50 300 µm	58.8	
No. 100 150 µm	55.7	
No. 200 75 µm	52.7	

MOISTURE CONTENT 18.2%

COMMENTS

GRAVEL			SAND			SILT			CLAY
COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	COARSE	MEDIUM	FINE	

U.S. STANDARD SIEVE SIZES



Gravel	49.2%
Sand	28.6%
Fines	22.2%

GEO NORTH ENGINEERING LTD.

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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 GRAIN SIZE ANALYSIS OF TP-BGC11-130-G1

PROJECT NO.
 K-3300
 PLATE NO.
 3300-GS-TP130-G1



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 630 - 1718 Argyle Street
 Halifax, Nova Scotia
 B3J 3N6

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

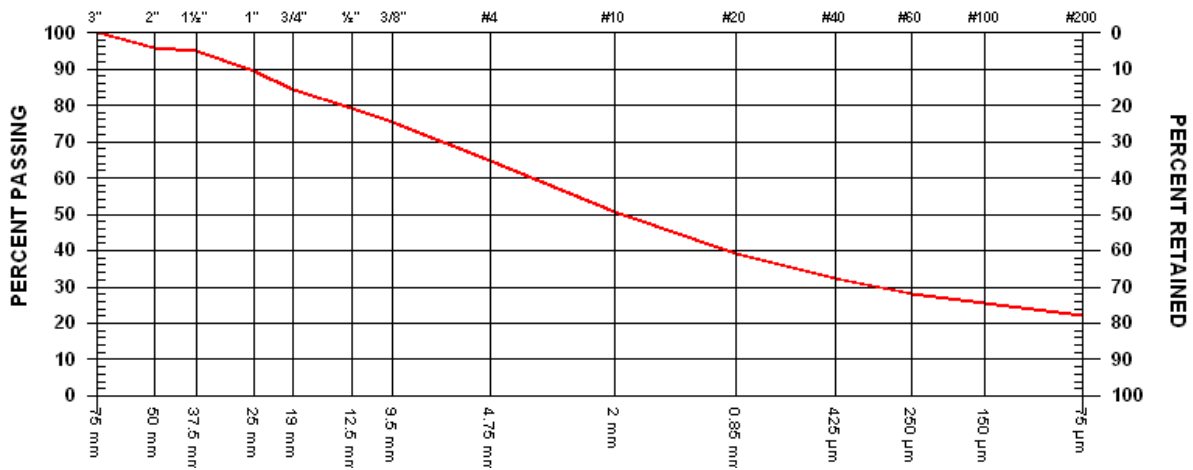
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 4 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Sep.21 DATE SAMPLED 2011.Aug.20

SUPPLIER TP-BGC11-130-G1
 SOURCE 5.8 - 6.0m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY MM
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm	100.0	
2" 50 mm	96.0	
1 1/2" 37.5 mm	95.0	
1" 25 mm	89.5	
3/4" 19 mm	84.1	
1/2" 12.5 mm	79.0	
3/8" 9.5 mm	75.5	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	64.9	
No. 10 2.00 mm	50.8	
No. 20 850 µm	39.3	
No. 40 425 µm	32.3	
No. 60 250 µm	28.1	
No. 100 150 µm	25.4	
No. 200 75 µm	22.2	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

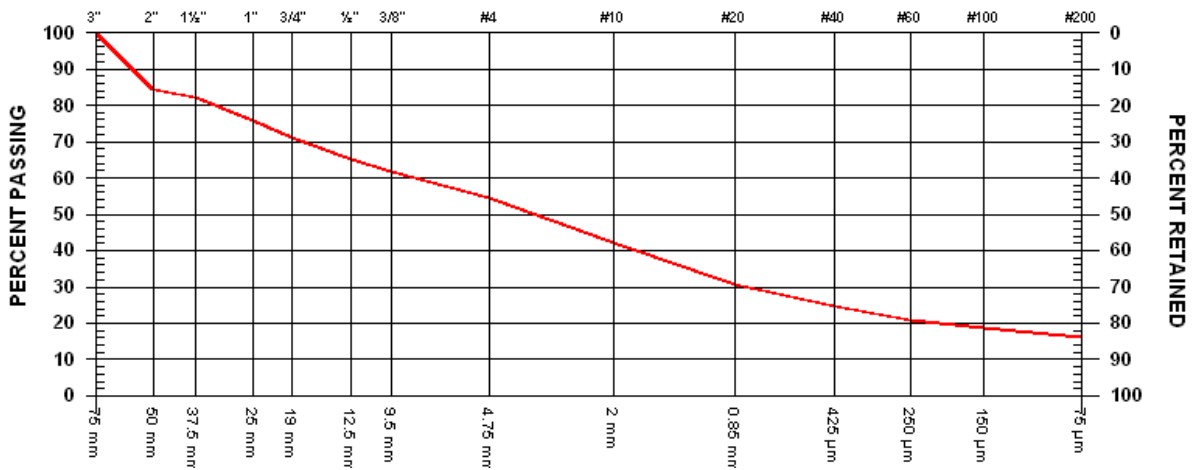
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 24 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.08 DATE SAMPLED 2011.Aug.20

SUPPLIER TP-BGC11-131
 SOURCE G1 @ 3.2-3.5m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm	100.0	
2" 50 mm	84.1	
1 1/2" 37.5 mm	82.0	
1" 25 mm	76.0	
3/4" 19 mm	70.9	
1/2" 12.5 mm	65.2	
3/8" 9.5 mm	61.8	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	54.5	
No. 10 2.00 mm	42.2	
No. 20 850 µm	30.8	
No. 40 425 µm	24.6	
No. 60 250 µm	21.0	
No. 100 150 µm	18.7	
No. 200 75 µm	16.0	

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

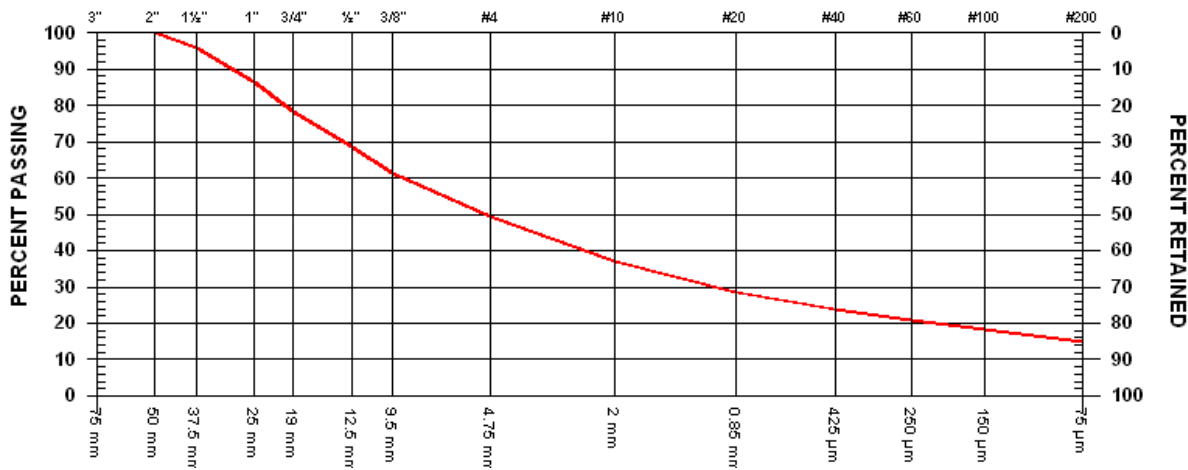
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 25 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.08 DATE SAMPLED 2011.Aug.21

SUPPLIER TP-BGC11-133
 SOURCE G1 @ 4.3-4.4m
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY DJ
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	100.0
1 1/2"	37.5 mm	95.6
1"	25 mm	86.6
3/4"	19 mm	78.5
1/2"	12.5 mm	68.4
3/8"	9.5 mm	61.2

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	49.3
No. 10	2.00 mm	36.9
No. 20	850 µm	28.6
No. 40	425 µm	23.8
No. 60	250 µm	20.8
No. 100	150 µm	18.5
No. 200	75 µm	15.1

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO
 BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

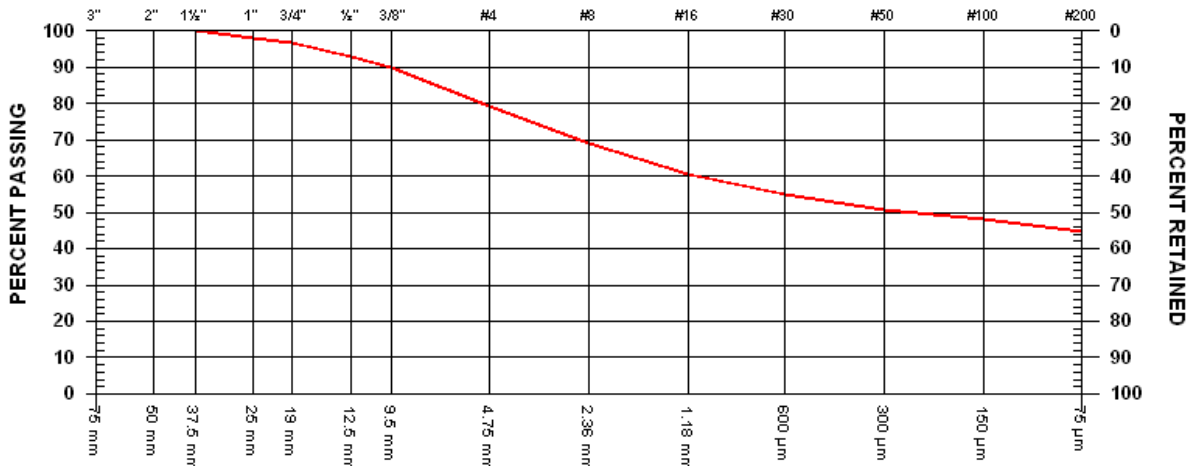
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 42 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.14 DATE SAMPLED 2011.Aug.24

SUPPLIER TP-BGC11-140
 SOURCE G1 @ 1 - 1.2m
 SPECIFICATION
 MATERIAL TYPE Sand,silty

SAMPLED BY Client
 TESTED BY RO
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3" 75 mm		
2" 50 mm		
1 1/2" 37.5 mm	100.0	
1" 25 mm	98.1	
3/4" 19 mm	96.5	
1/2" 12.5 mm	92.8	
3/8" 9.5 mm	89.7	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4 4.75 mm	79.2	
No. 8 2.36 mm	68.8	
No. 16 1.18 mm	60.4	
No. 30 600 µm	54.9	
No. 50 300 µm	50.8	
No. 100 150 µm	48.2	
No. 200 75 µm	44.8	

MOISTURE CONTENT 67.3%

COMMENTS



PROJECT NO. K-3300
 CLIENT BGC Engineering Inc.
 C.C.

TO BGC Engineering Inc.
 Suite 500 - 1045 Howe Street
 Vancouver, BC
 V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

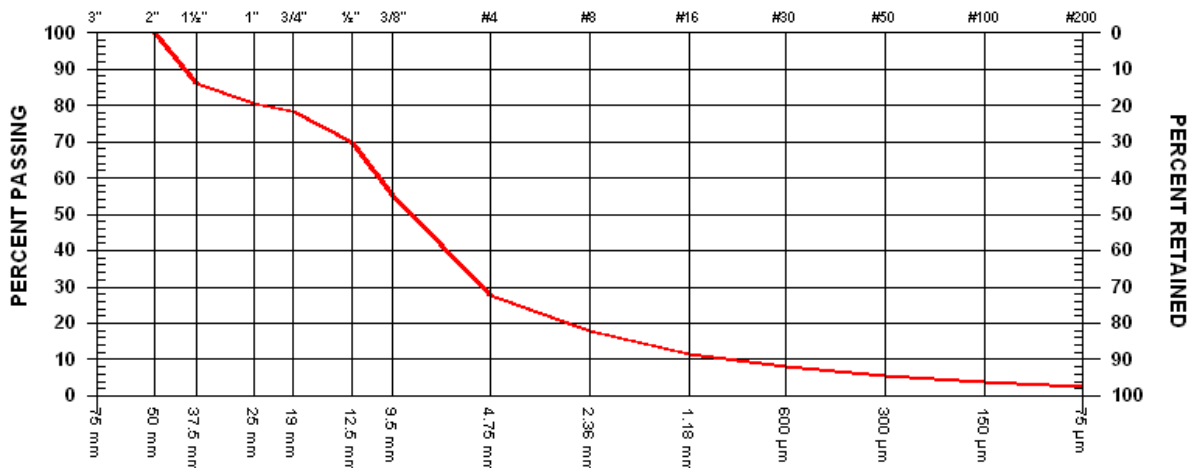
Dublin Gulch, Yukon

CONTRACTOR

SIEVE TEST NO. 38 DATE RECEIVED 2011.Sep.13 DATE TESTED 2011.Nov.11 DATE SAMPLED 2011.Jul.22

SUPPLIER BGC11-Stiener-01
 SOURCE Stiener Outcrop
 SPECIFICATION
 MATERIAL TYPE Gravel

SAMPLED BY Client
 TESTED BY LT
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
3"	75 mm	
2"	50 mm	100.0
1 1/2"	37.5 mm	86.0
1"	25 mm	80.3
3/4"	19 mm	78.2
1/2"	12.5 mm	70.0
3/8"	9.5 mm	55.2

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
No. 4	4.75 mm	27.7
No. 8	2.36 mm	17.7
No. 16	1.18 mm	11.5
No. 30	600 µm	7.9
No. 50	300 µm	5.4
No. 100	150 µm	3.8
No. 200	75 µm	2.6

COMMENTS

WATER CONTENT DETERMINATION

Reference(s)
ASTM D 4959

Client: BGC (Proj 0792-006-04)	Project No.: 11-1415-0029 Phase: 7000
Project: Eagle Gold Mine Site Infra FS SI	Lab Schedule No.: 141
Location: Dublin Gulch, Yukon	

Sample Location	Sample No.	Sample Interval		Water Content (%)
		Depth (m)	Bottom (m)	
TP-BGC11-50	1	2.80	3.00	6.1
TP-BGC11-50	2	3.30	3.60	7.3
TP-BGC11-51	1	0.20	1.44	12.8
TP-BGC11-51	2	2.40	2.80	7.3
TP-BGC11-51	3	4.20	4.50	4.2
TP-BGC11-52	1	3.30	3.50	3.2
TP-BGC11-53	1	2.00	2.20	5.8
TP-BGC11-54	1	0.40	0.60	9.5
TP-BGC11-55	1	0.40	0.60	7.2
TP-BGC11-55	2	1.90	2.00	5.7
TP-BGC11-57	1	0.90	1.10	8.5
TP-BGC11-58	1	0.70	0.70	9.5
TP-BGC11-58	2	1.90	1.90	6.5
TP-BGC11-59	1	0.40	0.60	7.3
TP-BGC11-60	1	0.70	0.90	24.8
TP-BGC11-60	2	1.70	1.90	20.5
TP-BGC11-61	1	0.80	0.90	11.8
TP-BGC11-62	1	0.90	1.00	10.5
TP-BGC11-62	2	3.50	3.70	8.8
TP-BGC11-63	1	0.40	0.60	7.4
TP-BGC11-64	1	1.50	1.60	4.7
TP-BGC11-65	1	0.40	0.50	6.5
TP-BGC11-66	1	1.30	1.30	11.5
TP-BGC11-66	2	1.80	1.80	16.7
TP-BGC11-67	1	0.70	0.90	6.5
TP-BGC11-68	1	1.60	1.80	8.5
TP-BGC11-69	1	1.50	1.70	30.5
TP-BGC11-69	2	2.80	2.90	6.8
TP-BGC11-71	1	1.30	1.60	13.8
TP-BGC11-72	2	0.80	1.00	13.7
TP-BGC11-72	1	2.40	2.60	5.4
TP-BGC11-73	1	0.80	1.00	8.5
TP-BGC11-74	1	1.20	1.40	11.8
TP-BGC11-74	2	3.00	3.00	35.9

LP	18/10/2011
Checked	Date

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WATER CONTENT DETERMINATION

Reference(s)
ASTM D 4959

Client: BGC (Proj 0792-006-04)	Project No.: 11-1415-0029 Phase: 7000
Project: Eagle Gold Mine Site Infra FS SI	Lab Schedule No.: 141
Location: Dublin Gulch, Yukon	

Sample Location	Sample No.	Sample Interval		Water Content (%)
		Depth (m)	Bottom (m)	
TP-BGC11-74	3	4.50	4.50	15.0
TP-BGC11-76	1	0.40	0.60	16.1
TP-BGC11-77	1	0.40	0.50	26.6
TP-BGC11-79	1	0.40	0.60	10.1
TP-BGC11-79	2	1.20	1.30	15.7
TP-BGC11-81	1	0.70	1.00	16.6
TP-BGC11-82	M4	0.70	1.10	28.6
TP-BGC11-82	M5	1.90	2.10	27.3
TP-BGC11-82	M2	3.00	3.30	10.3
TP-BGC11-82	M1	4.70	4.90	12.4
TP-BGC11-82	M3	6.70	7.00	40.1
TP-BGC11-83	1	0.60	0.80	20.1
TP-BGC11-84	1	0.40	0.60	17.9
TP-BGC11-84	2	2.50	2.50	28.5
TP-BGC11-85	1	0.60	0.80	16.6
TP-BGC11-87	1	0.60	0.80	10.5
TP-BGC11-88	1	1.50	1.80	12.7
TP-BGC11-89	1	0.40	0.60	2.0
TP-BGC11-89	2	1.30	1.50	14.3
TP-BGC11-90	1	0.80	1.00	13.2
TP-BGC11-90	2	3.50	3.60	23.3
TP-BGC11-91	1	0.80	1.00	49.5
TP-BGC11-91	2	1.60	1.80	6.1
TP-BGC11-92	1	0.60	0.80	63.6
TP-BGC11-94	1	1.20	1.30	16.4
TP-BGC11-94	2	2.30	2.50	11.4

LP	18/10/2011
Checked	Date

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WATER CONTENT DETERMINATION

Reference(s)
ASTM D 4959

Client: BGC (Proj 0792-006-04)	Project No.: 11-1415-0029 Phase: 7000
Project: Eagle Gold Mine Site Infra FS SI	Lab Schedule No.: 141
Location: Dublin Gulch, Yukon	

Sample Location	Sample No.	Sample Interval		Water Content (%)
		Depth (m)	Bottom (m)	
TP-BGC11-95	1	0.90	1.10	13.1
TP-BGC11-95	2	2.00	3.00	22.4
TP-BGC11-96	1	0.70	0.90	7.9
TP-BGC11-96	2	1.30	1.50	19.8
TP-BGC11-96	3	1.80	2.00	9.3

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	LP	04/11/2011	
	Checked	Date	

GEONORTH ENGINEERING LTD.

MOISTURE / % PASSING WORKSHEET

Client: BGC Engineering Ltd.
Project Name: Eagle Gold Soils and Aggregate Testing

File No. K-3300
Date: 11-Nov-11

Hole Number	BBGC11-39								
Depth	1.15-1.60m	1.60-2.05m	2.28-2.73m	2.75-3.20m	4.6-5.05m	5.53-5.98m	6.22-6.67m	7.16-7.61m	7.75-8.20m
Sample No.	SPT1	SPT2	SPT3	SPT4	SPT5	SPT6	SPT7	SPT8	SPT9
Wet Weight and Tare	894.2	711.6	812.3	559.7	531.8	638.3	789.9	763.8	783.1
Dry Weight and Tare	714.6	591.0	723.9	544.5	524.6	622.4	752.4	723.2	760.3
Washed Weight and Tare									
Weight of Tare	183.1	196.7	191.0	179.0	179.6	179.8	180.5	131.5	181.4
Weight of Water	179.6	120.6	88.4	15.2	7.2	15.9	37.5	40.6	22.8
Weight of minus #200									
Dry Weight	531.5	394.3	532.9	365.5	345.0	442.6	571.9	591.7	578.9
Percent Moisture	33.8	30.6	16.6	4.2	2.1	3.6	6.6	6.9	3.9
Percent minus #200									
Comments									

Hole Number	BH-BGC11-39		TP-103	BH-BGC11-49			
Depth	8.71-9.16m	9.25-9.70m	4.5m	3.4-3.6m	6.4-6.6m	9.4-9.6m	12.4-12.6m
Sample No.	SPT10	SPT11	G1	G4	G6	G8	G10
Wet Weight and Tare	1008.0	1104.0	2709.5	1798.8	1241.4	1302.8	1240.8
Dry Weight and Tare	898.4	972.3	2637.0	1630.5	1118.7	1072.7	1133.3
Washed Weight and Tare							
Weight of Tare	179.6	181.1	365.3	181.5	180.4	180.8	205.2
Weight of Water	109.6	131.7	72.5	168.3	122.7	230.1	107.5
Weight of minus #200							
Dry Weight	718.8	791.2	2271.7	1449.0	938.3	891.9	928.1
Percent Moisture	15.2	16.6	3.2	11.6	13.1	25.8	11.6
Percent minus #200							
Comments							

Hole Number	BH-BGC11-49	
Depth	0.76-1.21	2.28-2.73m
Sample No.	SPT1	SPT2
Wet Weight and Tare	353.0	855.1
Dry Weight and Tare	339.2	742.8
Washed Weight and Tare		
Weight of Tare	211.9	192.8
Weight of Water	13.8	112.3
Weight of minus #200		
Dry Weight	127.3	550.0
Percent Moisture	10.8	20.4
Percent minus #200		
Comments		

Hole Number
Depth
Tare Number
Wet Weight and Tare
Dry Weight and Tare
Washed Weight and Tare
Weight of Tare
Weight of Water
Weight of minus #200
Dry Weight
Percent Moisture
Percent minus #200
Comments

GEONORTH ENGINEERING LTD.

MOISTURE / % PASSING WORKSHEET

Client: BGC Engineering Inc. **File No.** K-3300
Project Name: Eagle Gold Soils and Aggregate Testing, Dublin Gulch, Yukon **Date:** 11-Oct-11

Hole Number	BH-40A	BH-54	TP-111	TP-116	BH-57			
Depth	18.78-18.83m	29.97-30.04m	.9-1.1m	2.2-2.3m	2.28-2.65m	3.8-4.25m	5.33-5.78m	6.85-7.1m
Sample Number	1	G1	G1	G2	SPT1	SPT2	SPT3	SPT4
Wet Weight and Tare	250.0	556.9	384.9	1098.2	353.0	1100.1	340.3	687.8
Dry Weight and Tare	238.5	528.3	351.3	1027.6	334.3	1020.9	322.8	649.4
Washed Weight and Tare								
Weight of Tare	147.9	181.5	151.9	180.7	148.1	181.2	151.9	192.9
Weight of Water	11.5	28.6	33.6	70.6	18.7	79.2	17.5	38.4
Weight of minus #200								
Dry Weight	90.6	346.8	199.4	846.9	186.2	839.7	170.9	456.5
Percent Moisture	12.7	8.2	16.9	8.3	10.0	9.4	10.2	8.4
Percent minus #200								
Comments								

Hole Number	BH-56				
Depth	0.76-1.21m	1.9-2.1m	3.8-4.25m	5.33-5.53m	6.85-7.15m
Sample Number	SPT1	G2	SPT3	SPT4	SPT5
Wet Weight and Tare	307.7	2069.1	495.9	536.3	365.0
Dry Weight and Tare	299.6	1988.8	469.0	515.0	354.3
Washed Weight and Tare					
Weight of Tare	151.9	679.6	179.8	191.1	147.9
Weight of Water	8.1	80.3	26.9	21.3	10.7
Weight of minus #200					
Dry Weight	147.7	1309.2	289.2	323.9	206.4
Percent Moisture	5.5	6.1	9.3	6.6	5.2
Percent minus #200					
Comments					

Hole Number	BH-60				
Depth	2.7-2.8m	3.7-3.8m	4.9-5.1m	6.3-6.6m	6.85-7.0m
Sample Number	G3	G5	G6	G7	SPT2
Wet Weight and Tare	897.0	1032.3	352.3	941.7	312.9
Dry Weight and Tare	834.2	963.2	339.5	895.0	302.1
Washed Weight and Tare					
Weight of Tare	179.8	191.7	148.6	205.6	120.9
Weight of Water	62.8	69.1	12.8	46.7	10.8
Weight of minus #200					
Dry Weight	654.4	771.5	190.9	689.4	181.2
Percent Moisture	9.6	9.0	6.7	6.8	6.0
Percent minus #200					
Comments					

Hole Number
Depth
Tare Number
Wet Weight and Tare
Dry Weight and Tare
Washed Weight and Tare
Weight of Tare
Weight of Water
Weight of minus #200
Dry Weight
Percent Moisture
Percent minus #200
Comments

GEONORTH ENGINEERING LTD.

MOISTURE / % PASSING WORKSHEET

Client: BGC Engineering Ltd.
Project Name: Eagle Gold Soils and Aggregate Testing

File No. K-3300
Date: 8-Nov-11

Hole Number	BH-BGC11-42								
Depth	1.8 - 2.27m	5.0 - 5.32m	6.4 - 6.84m	9.4 - 9.88m	10.6-11.0m	13.3-13.65m	15.25-15.4m	15.6-15.8m	19.7-20.0m
Sample No.	SA#2	SA#4	SA#5	SA#7	SA#8	SA#12	SA#15	SA#16	SA#20
Wet Weight and Tare	1350.5	304.4	1265.1	1568.2	243.4	2585.1	246.4	259.9	597.3
Dry Weight and Tare	1180.8	283.4	1136.5	1436.3	228.7	2355.6	239.4	249.8	532.7
Washed Weight and Tare									
Weight of Tare	196.8	137.1	191.1	367.0	136.9	366.4	136.9	136.9	179.2
Weight of Water	169.7	21.0	128.6	131.9	14.7	229.5	7.0	10.1	64.6
Weight of minus #200									
Dry Weight	984.0	146.3	945.4	1069.3	91.8	1989.2	102.5	112.9	353.5
Percent Moisture	17.2	14.4	13.6	12.3	16.0	11.5	6.8	8.9	18.3
Percent minus #200									
Comments									

Hole Number	BH-BGC11-42			BH-BGC11-65				
Depth	21.55-21.9m	23.0-23.4m	27.6-28.0m	0.76-1.21m	1.52-1.97m	2.28-2.73m	3.04-3.49m	3.8-4.25m
Sample No.	SA#22	SA#23	SA#26	SPT 1	SPT 2	SPT 3	SPT 4	SPT 5
Wet Weight and Tare	653.2	292.3	2312.3	970.9	805.7	456.4	548.9	355.7
Dry Weight and Tare	576.8	271.7	2067.5	951.3	792.4	450.4	539.8	354.5
Washed Weight and Tare								
Weight of Tare	181.2	138.3	380.4	365.0	368.0	179.7	131.6	180.6
Weight of Water	76.4	20.6	244.8	19.6	13.3	6.0	9.1	1.2
Weight of minus #200								
Dry Weight	395.6	133.4	1687.1	586.3	424.4	270.7	408.2	173.9
Percent Moisture	19.3	15.4	14.5	3.3	3.1	2.2	2.2	0.7
Percent minus #200								
Comments								

Hole Number	BH-BGC11-65		
Depth	4.56-5.01m	5.33-5.78m	6.09-6.54m
Sample No.	SPT 6	SPT 7	SPT 8
Wet Weight and Tare	908.0	433.8	378.6
Dry Weight and Tare	882.9	421.4	363.1
Washed Weight and Tare			
Weight of Tare	361.5	179.7	181.5
Weight of Water	25.1	12.4	15.5
Weight of minus #200			
Dry Weight	521.4	241.7	181.6
Percent Moisture	4.8	5.1	8.5
Percent minus #200			
Comments			

Hole Number
 Depth
 Sample No.
 Wet Weight and Tare
 Dry Weight and Tare
 Washed Weight and Tare
 Weight of Tare
 Weight of Water
 Weight of minus #200
 Dry Weight
 Percent Moisture
 Percent minus #200
 Comments

GEONORTH ENGINEERING LTD.

MOISTURE / % PASSING WORKSHEET

Client: BGC Engineering Inc. **File No.** K-3300
Project Name: Eagle Gold Soils and Aggregate Testing, Dublin Gulch, Yukon **Date:** 29-Sep-11

Hole Number	BH-BGC11-44								
Depth	0.2 - 0.4m	1.6 - 2.0m	4.8 - 5.1m	6.3 - 6.6m	9.6 - 9.9m	2.27-2.72m	3.79-4.24m	8.35-8.85m	12.97-13.42m
Sample Number	S1	S3	S5	S7	S11	SPT1	SPT2	SPT5	SPT10
Wet Weight and Tare	976.4	1044.3	1634.4	1543.7	1079.5	266.0	887.5	339.5	962.3
Dry Weight and Tare	876.8	885.5	1461.3	1280.7	950.9	211.7	748.9	312.1	878.3
Washed Weight and Tare									
Weight of Tare	180.5	205.1	192.9	191.5	179.7	147.9	196.8	151.9	179.2
Weight of Water	99.6	158.8	173.1	263.0	128.6	54.3	138.6	27.4	84.0
Weight of minus #200									
Dry Weight	696.3	680.4	1268.4	1089.2	771.2	63.8	552.1	160.2	699.1
Percent Moisture	14.3	23.3	13.6	24.1	16.7	85.1	25.1	17.1	12.0
Percent minus #200									
Comments									

Hole Number	BH-BGC11-44			BH-BGC11-53						
Depth	14.49-14.79m			0.76-1.21m	2.28-2.73m	3.8-4.25m	6.85-7.3m	9.9-10.35m	1.9-2.1m	3.2-3.5m
Sample Number	SPT11			SPT1	SPT2	SPT3	SPT4	SPT5	G2	G3
Wet Weight and Tare	591.0			264.1	896.1	242.8	661.4	304.5	1347.3	295.7
Dry Weight and Tare	534.6			252.2	830.1	238.4	646.7	297.4	1229.1	282.1
Washed Weight and Tare										
Weight of Tare	131.7			151.8	180.8	136.6	182.4	152.1	191.1	118.2
Weight of Water	56.4			11.9	66.0	4.4	14.7	7.1	118.2	13.6
Weight of minus #200										
Dry Weight	402.9			100.4	649.3	101.8	464.3	145.3	1038.0	163.9
Percent Moisture	14.0			11.9	10.2	4.3	3.2	4.9	11.4	8.3
Percent minus #200										
Comments										

Hole Number	BH-BGC11-53		
Depth	4.9-5.1m	7.9-8.1m	10.9-11.1m
Sample Number	G4	G6	G8
Wet Weight and Tare	1239.1	1948.0	389.3
Dry Weight and Tare	1195.8	1896.1	382.1
Washed Weight and Tare			
Weight of Tare	179.8	667.3	151.9
Weight of Water	43.3	51.9	7.2
Weight of minus #200			
Dry Weight	1016.0	1228.8	230.2
Percent Moisture	4.3	4.2	3.1
Percent minus #200			
Comments			

Hole Number
 Depth
 Tare Number
 Wet Weight and Tare
 Dry Weight and Tare
 Washed Weight and Tare
 Weight of Tare
 Weight of Water
 Weight of minus #200
 Dry Weight
 Percent Moisture
 Percent minus #200
 Comments

GEONORTH ENGINEERING LTD.

MOISTURE / % PASSING WORKSHEET

Client: BGC Engineering Inc.
Project Name: Eagle Gold Soilsand Aggregate Testing

File No. K-3300
Date: 15-Nov-11

Hole Number	BH-BGC11-51					
Depth	7.9 - 8.1m	10.9-11.1m	13.9-14.1m	17.0-17.2m	20.1-20.3m	24.7-24.9m
Sample No.	G6	G8	G10	G12	G14	G17
Wet Weight and Tare	1208.8	254.9	1181.4	241.6	1977.7	223.2
Dry Weight and Tare	1057.0	240.3	1080.3	231.4	1845.3	217.6
Washed Weight and Tare						
Weight of Tare	181.4	137.1	131.5	136.6	595.8	136.7
Weight of Water	151.8	14.6	101.1	10.2	132.4	5.6
Weight of minus #200						
Dry Weight	875.6	103.2	948.8	94.8	1249.5	80.9
Percent Moisture	17.3	14.1	10.7	10.8	10.6	6.9
Percent minus #200						
Comments						

Hole Number	BH-BGC11-51				
Depth	0.76-1.21m	2.28-2.73m	3.8-4.25m	5.33-5.78m	9.9-10.25m
Sample No.	SPT1	SPT2	SPT3	SPT4	SPT5
Wet Weight and Tare	630.5	225.8	831.1	253.7	257.1
Dry Weight and Tare	580.8	217.8	683.6	244.6	236.8
Washed Weight and Tare					
Weight of Tare	179.6	138.4	180.4	137.2	136.9
Weight of Water	49.7	8.0	147.5	9.1	20.3
Weight of minus #200					
Dry Weight	401.2	79.4	503.2	107.4	99.9
Percent Moisture	12.4	10.1	29.3	8.5	20.3
Percent minus #200					
Comments					

Hole Number	TP124	TP129	TP140
Depth	6.2-6.4m	6.6-6.8m	1.0-1.2m
Sample No.	G1	G1	G1
Wet Weight and Tare	1315.7	1148.1	2131.5
Dry Weight and Tare	1116.8	969.8	1865.3
Washed Weight and Tare			
Weight of Tare	179.7	179.2	179.9
Weight of Water	198.9	178.3	266.2
Weight of minus #200			
Dry Weight	937.1	790.6	1685.4
Percent Moisture	21.2	22.6	15.8
Percent minus #200			
Comments			

Hole Number
Depth
Tare Number
Wet Weight and Tare
Dry Weight and Tare
Washed Weight and Tare
Weight of Tare
Weight of Water
Weight of minus #200
Dry Weight
Percent Moisture
Percent minus #200
Comments

GEONORTH ENGINEERING LTD.

MOISTURE / % PASSING WORKSHEET

Client: BGC Engineering Ltd.
Project Name: Eagle Gold Soils and Aggregate Testing

File No. K-3300
Date: 9-Nov-11

Hole Number	BH-BGC11-58				
	1.9-2.1m	3.2-3.6m	4.4-4.6m	5.4-5.5m	7.9-8.1m
Depth					
Sample No.	G2	G4	G6	G8	G10
Wet Weight and Tare	1615.8	1822.2	1566.8	1065.0	837.6
Dry Weight and Tare	1404.2	1653.0	1426.7	1011.5	785.5
Washed Weight and Tare					
Weight of Tare	182.5	131.5	179.9	181.4	180.5
Weight of Water	211.6	169.2	140.1	53.5	52.1
Weight of minus #200					
Dry Weight	1221.7	1521.5	1246.8	830.1	605.0
Percent Moisture	17.3	11.1	11.2	6.4	8.6
Percent minus #200					
Comments					

Hole Number	TP-104	TP-107	TP-108	TP-112	TP-119
Depth	1.2-1.6m	2.8-3.0m	0.9-1.15m	1.3-1.4m	0.9-1.1m
Sample No.	G1	G1	G1	G1	G1
Wet Weight and Tare	2228.9	1009.0	2015.0	2883.2	942.7
Dry Weight and Tare	2112.5	889.5	1865.1	2607.7	770.5
Washed Weight and Tare					
Weight of Tare	358.8	179.7	366.1	679.3	180.5
Weight of Water	116.4	119.5	149.9	275.5	172.2
Weight of minus #200					
Dry Weight	1753.7	709.8	1499.0	1928.4	590.0
Percent Moisture	6.6	16.8	10.0	14.3	29.2
Percent minus #200					
Comments	Sieve		Sieve	Sieve	

Hole Number	TP-120	TP-121	TP-122	TP-131	TP-133
Depth	3.8-4.0m	2.9-3.1m	5.4-5.8m	3.2-3.5m	4.3-4.4m
Sample No.	G1	G1	G1	G1	G1
Wet Weight and Tare	1840.4	1154.5	985.7	8663.5	5033.5
Dry Weight and Tare	1509.9	964.4	856.8	8105.4	4644.0
Washed Weight and Tare					
Weight of Tare	361.5	191.8	180.2	1263.6	365.0
Weight of Water	330.5	190.1	128.9	558.1	389.5
Weight of minus #200					
Dry Weight	1148.4	772.6	676.6	6841.8	4279.0
Percent Moisture	28.8	24.6	19.1	8.2	9.1
Percent minus #200					
Comments				Sieve	Sieve

Hole Number
Depth
Sample No.
Wet Weight and Tare
Dry Weight and Tare
Washed Weight and Tare
Weight of Tare
Weight of Water
Weight of minus #200
Dry Weight
Percent Moisture
Percent minus #200
Comments

GEONORTH ENGINEERING LTD.

MOISTURE / % PASSING WORKSHEET

Client: BGC Engineering Ltd. **File No.** K-3300
Project Name: Eagle Gold Soils and Aggregate Testing **Date:** 26-Sep-11

Hole Number	TP-126	TP-106	TP-128	TP-130	BH-55	BH-55	BH-55	BH-55	BH-55
Depth	1.7 - 2.2m	0.8 - 1.5m	2.5 - 3.0m	5.8 - 6.0m	0.76-1.21m	2.28-2.73m	4.9-5.1m	6.3-6.5m	8.38-8.83m
Tare Number									
Wet Weight and Tare	365.6	379.9	349.1	1492.8	680.7	294.5	1345.5	278.8	613.9
Dry Weight and Tare	342.6	356.5	307.4	1404.7	641.0	280.0	1234.1	263.9	587.3
Washed Weight and Tare									
Weight of Tare	148.0	151.9	152.2	181.5	191.6	136.9	180.0	136.6	179.7
Weight of Water	23.0	23.4	41.7	88.1	39.7	14.5	111.4	14.9	26.6
Weight of minus #200									
Dry Weight	194.6	204.6	155.2	1223.2	449.4	143.1	1054.1	127.3	407.6
Percent Moisture	11.8	11.4	26.9	7.2	8.8	10.1	10.6	11.7	6.5
Percent minus #200									
Comments									

Hole Number	BH-63	BH-63	BH-63	BH-63	BH-63	BH-63	BH-63
Depth	1.9-2.1m	2.9-3.0m	3.9-4.0m	5.9-6.1m	10.15-10.25m	11.6-11.7m	16.2-16.3m
Tare Number							
Wet Weight and Tare	860.5	291.6	867.8	294.4	688.9	333.5	985.9
Dry Weight and Tare	725.4	267.0	711.3	261.3	576.3	307.2	825.8
Washed Weight and Tare							
Weight of Tare	179.2	138.2	131.6	138.3	182.5	148.5	179.7
Weight of Water	135.1	24.6	156.5	33.1	112.6	26.3	160.1
Weight of minus #200							
Dry Weight	546.2	128.8	579.7	123.0	393.8	158.7	646.1
Percent Moisture	24.7	19.1	27.0	26.9	28.6	16.6	24.8
Percent minus #200							
Comments							

Hole Number	BH-47	BH-47	BH-47	BH-47	BH-47	BH-47	BH-47
Depth	0.75-1.2m	2.0-2.27m	2.8-2.95m	5.33-5.88m	6.85-7.3m	8.38-8.58m	11.8-12.1m
Tare Number							
Wet Weight and Tare	894.6	924.7	306.4	712.3	259.8	583.9	1089.1
Dry Weight and Tare	747.3	764.9	298.3	663.4	245.4	542.7	995.9
Washed Weight and Tare							
Weight of Tare	180.4	181.1	138.3	180.5	136.8	192.9	181.5
Weight of Water	147.3	159.8	8.1	48.9	14.4	41.2	93.2
Weight of minus #200							
Dry Weight	566.9	583.8	160.0	482.9	108.6	349.8	814.4
Percent Moisture	26.0	27.4	5.1	10.1	13.3	11.8	11.4
Percent minus #200							
Comments							

Hole Number
Depth
Tare Number
Wet Weight and Tare
Dry Weight and Tare
Washed Weight and Tare
Weight of Tare
Weight of Water
Weight of minus #200
Dry Weight
Percent Moisture
Percent minus #200
Comments

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-51
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.20 to 1.44
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	52
Liquid Limit	27
Plastic Limit	22
Plasticity Index	5
Natural Water Content (%)	12.8
Liquidity Index	-1.8



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

TM	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKIN 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

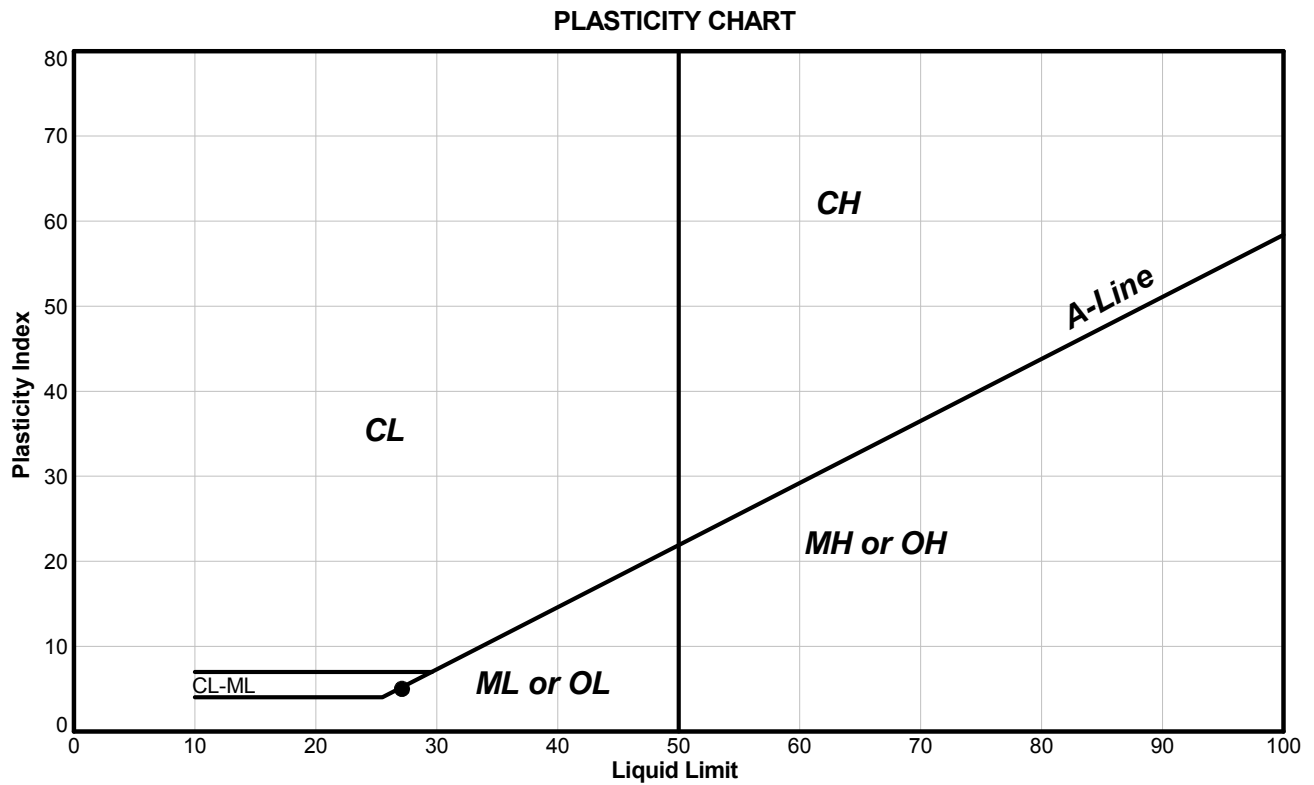
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-51
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.20 to 1.44
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-51	1	0.20	1.44	52	27	22	5	12.8	-1.8

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

TM	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-54
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	32
Liquid Limit	21
Plastic Limit	20
Plasticity Index	1
Natural Water Content (%)	9.5
Liquidity Index	-10.5



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_Graphics\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKin 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

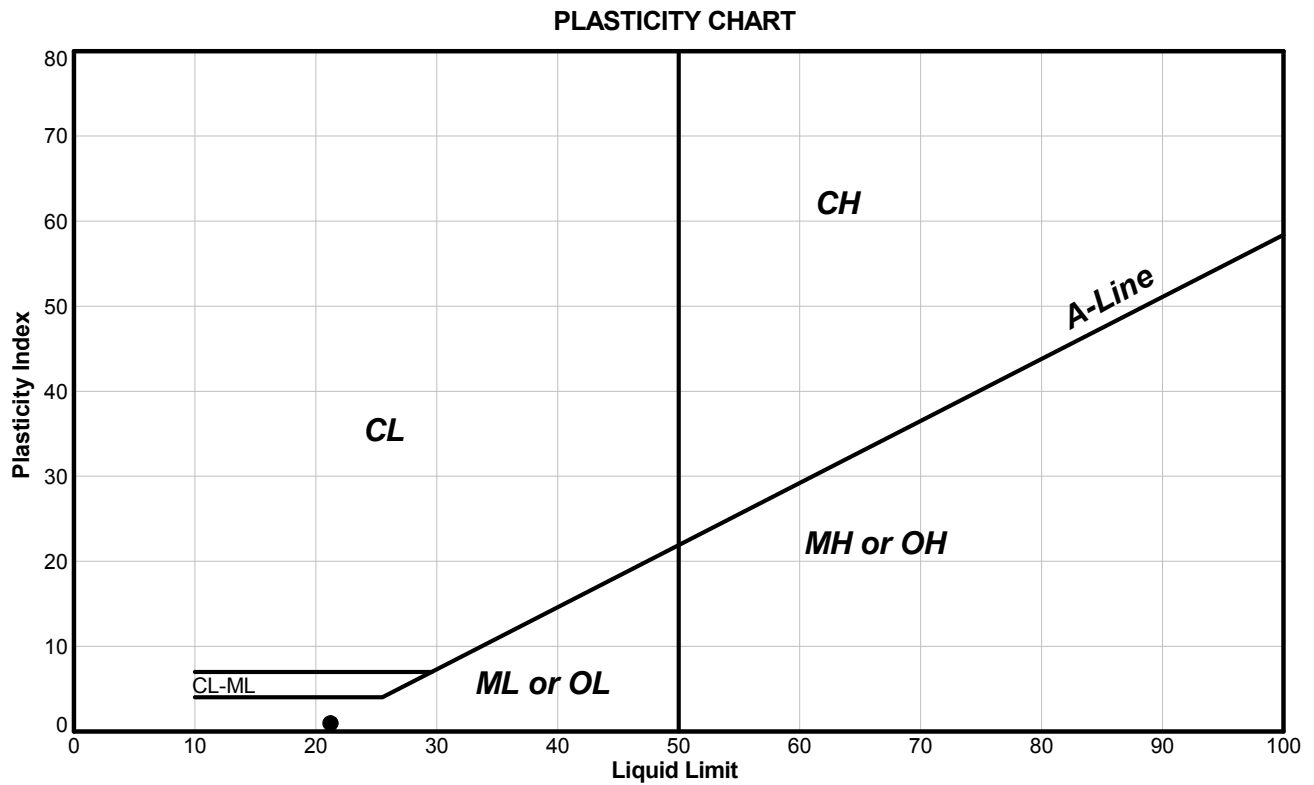
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-54
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-54	1	0.40	0.60	32	21	20	1	9.5	-10.5

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

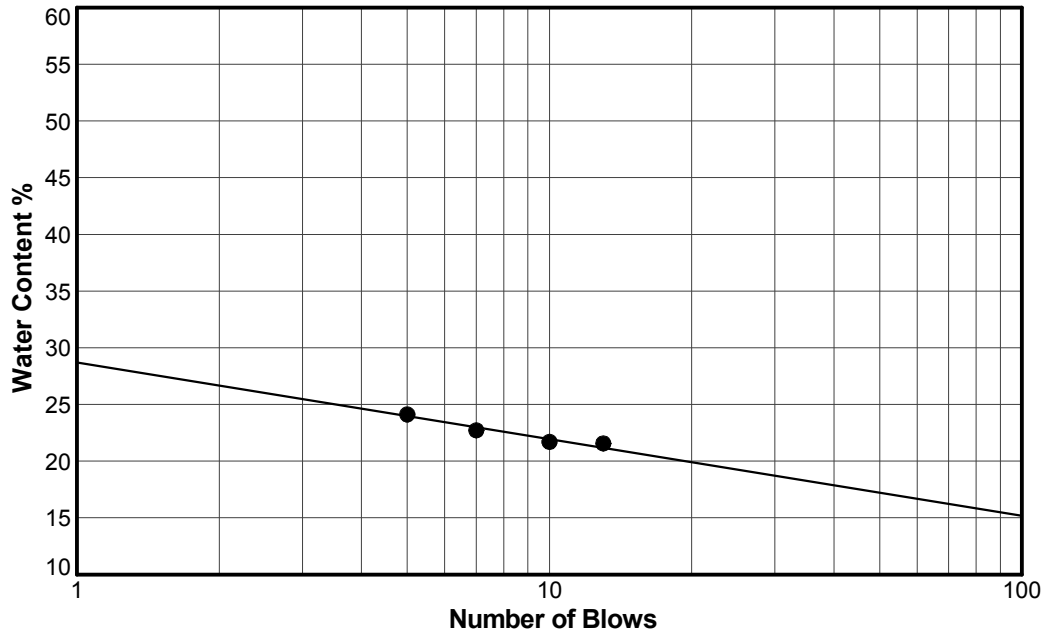
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-55
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).

Other Remarks: N/A

Test Method: A-Multi Point **Preparation Method:** Wet

SUMMARY	
Percent Passing #40 Sieve (%)	28
Liquid Limit	NP
Plastic Limit	NP
Plasticity Index	NP
Natural Water Content (%)	7.2
Liquidity Index	NP



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	23/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

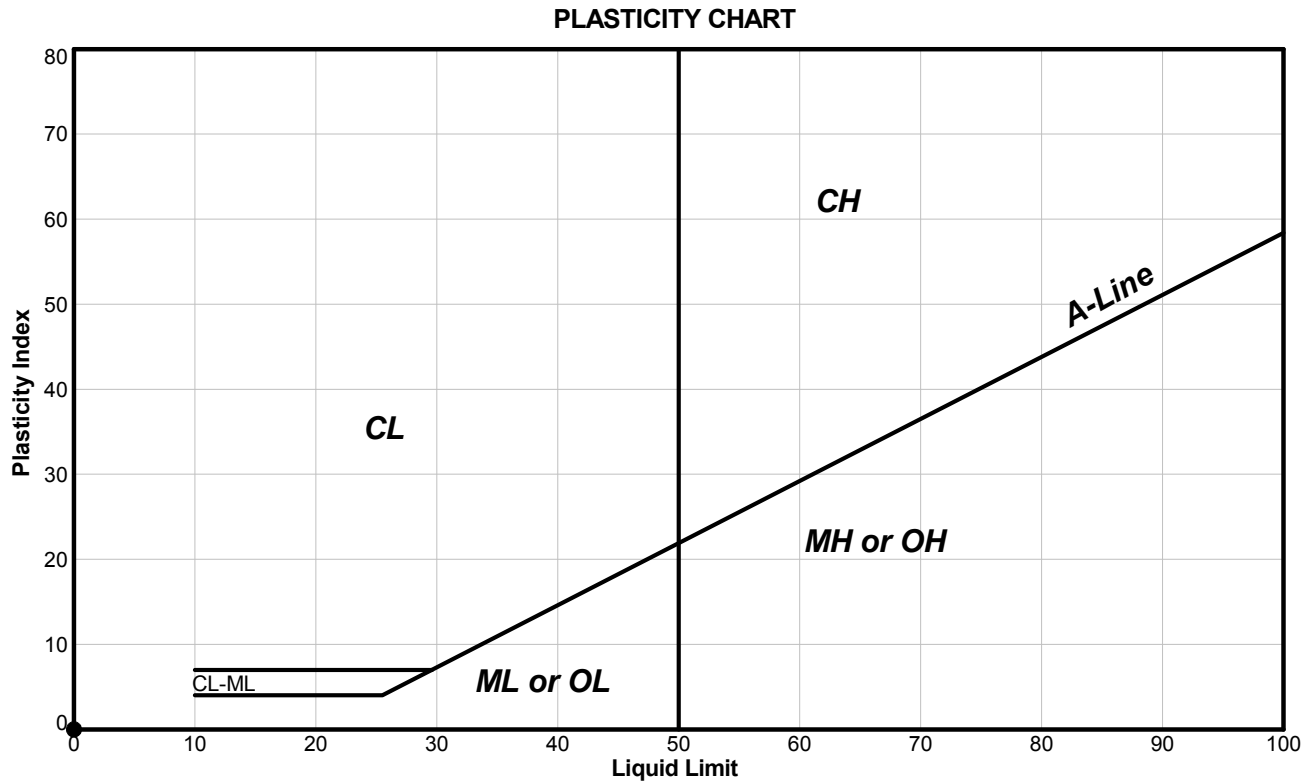
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-55
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-55	1	0.40	0.60	28	NP	NP	NP	7.2	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	23/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-57
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.90 to 1.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	38
Liquid Limit	28
Plastic Limit	23
Plasticity Index	5
Natural Water Content (%)	8.5
Liquidity Index	-2.9



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

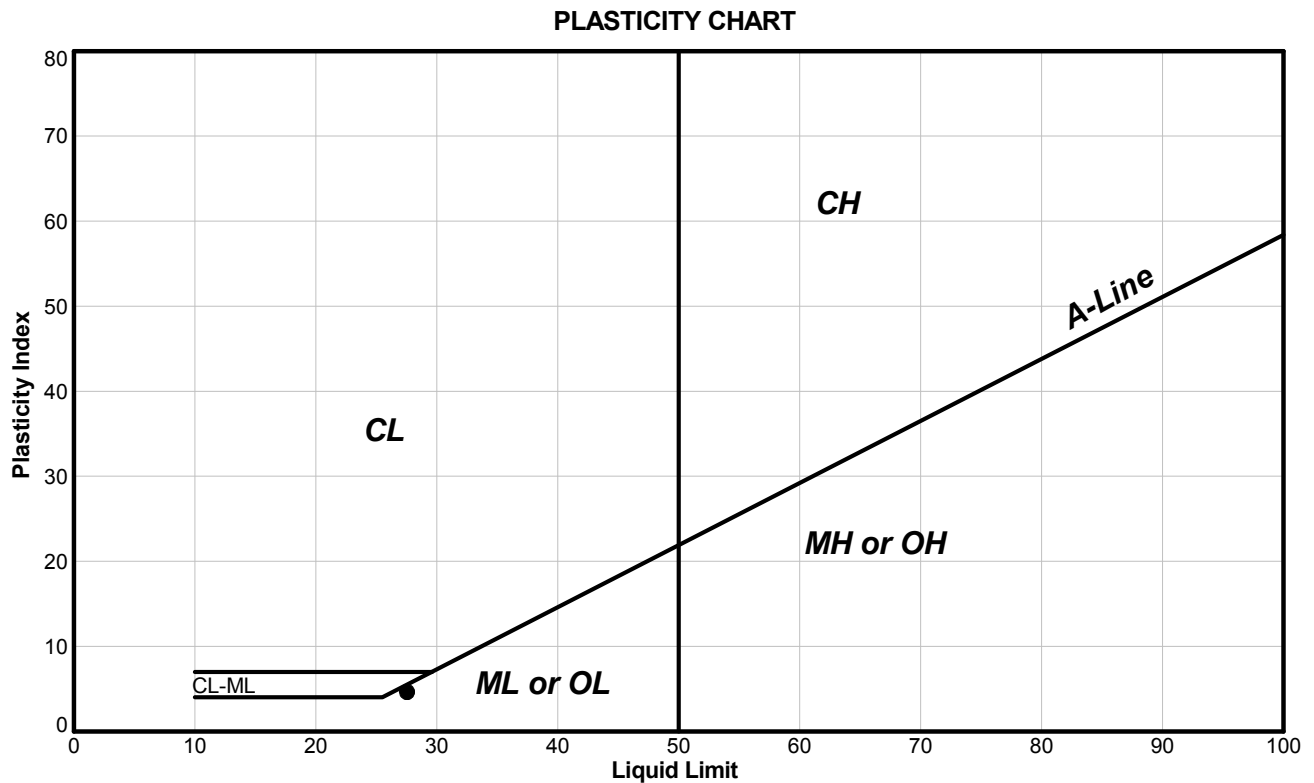
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-57
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.90 to 1.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-57	1	0.90	1.10	38	28	23	5	8.5	-2.9

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-58
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

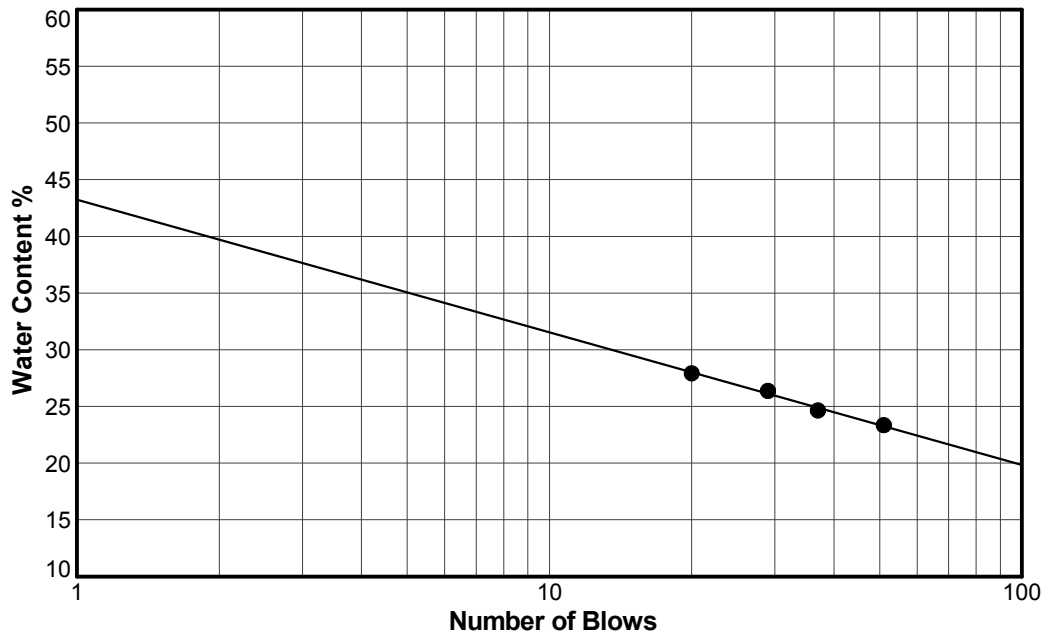
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	35
Liquid Limit	27
Plastic Limit	23
Plasticity Index	4
Natural Water Content (%)	9.5
Liquidity Index	-3.4



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 21/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

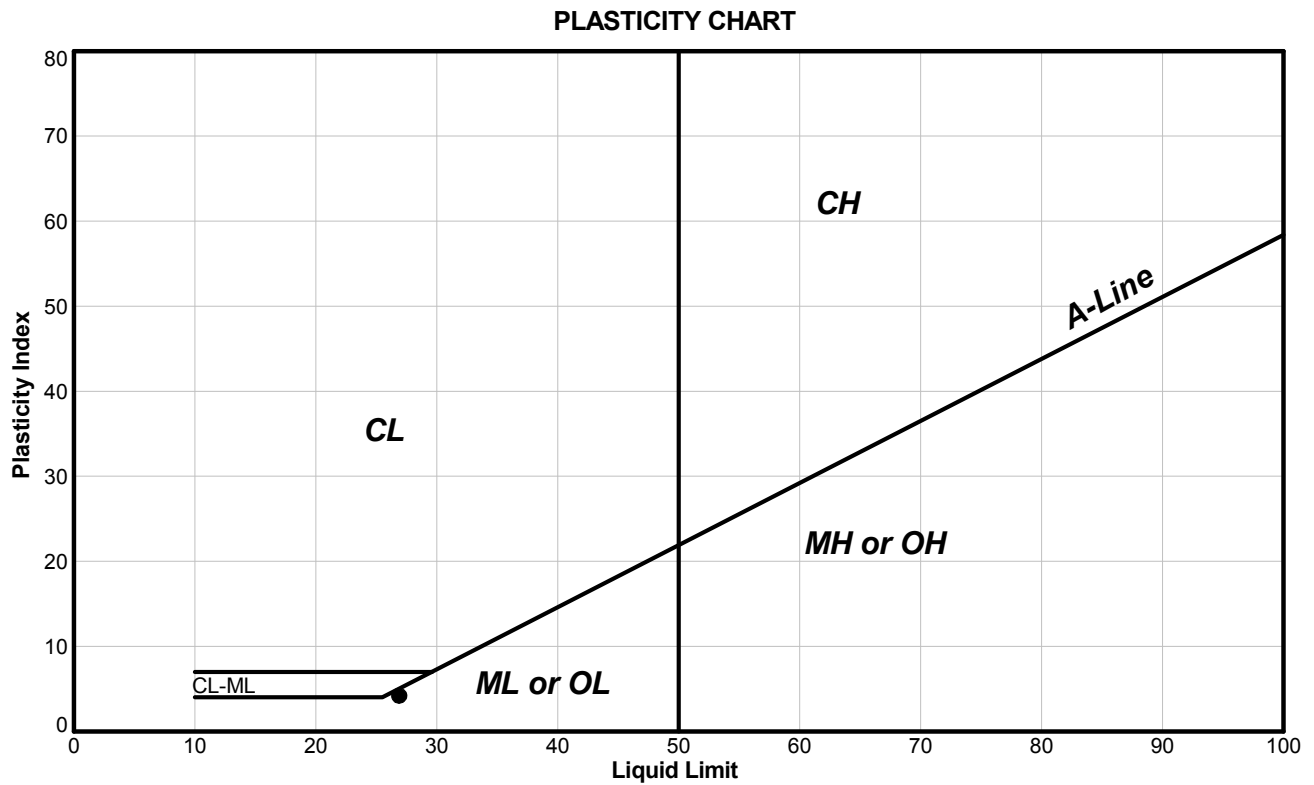
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-58
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-58	1	0.70	0.70	35	27	23	4	9.5	-3.4

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-59
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

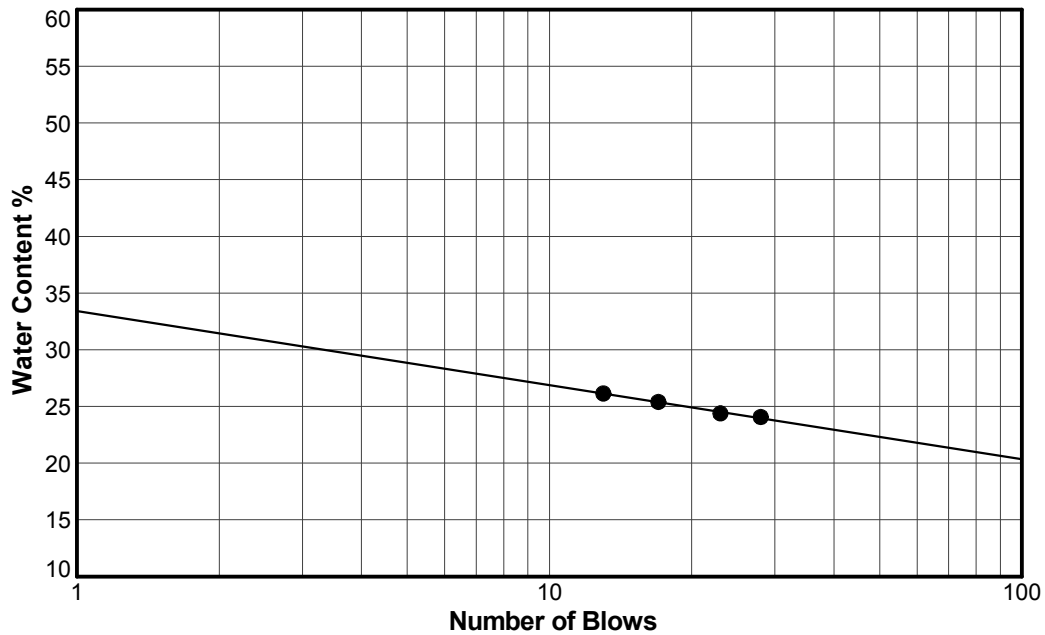
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	17
Liquid Limit	24
Plastic Limit	23
Plasticity Index	1
Natural Water Content (%)	7.3
Liquidity Index	-15.7



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

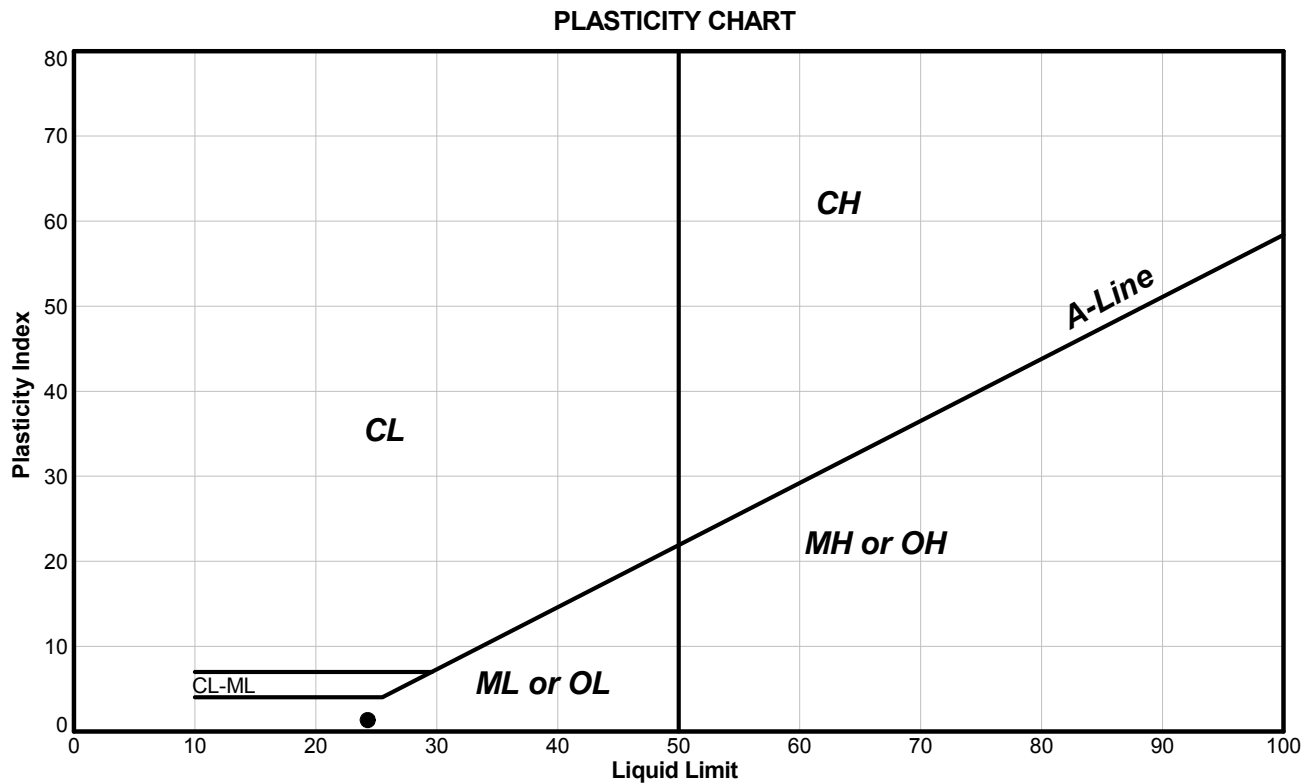
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-59
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-59	1	0.40	0.60	17	24	23	1	7.3	-15.7

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-60
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	66
Liquid Limit	27
Plastic Limit	26
Plasticity Index	1
Natural Water Content (%)	24.8
Liquidity Index	-1.2



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	07/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

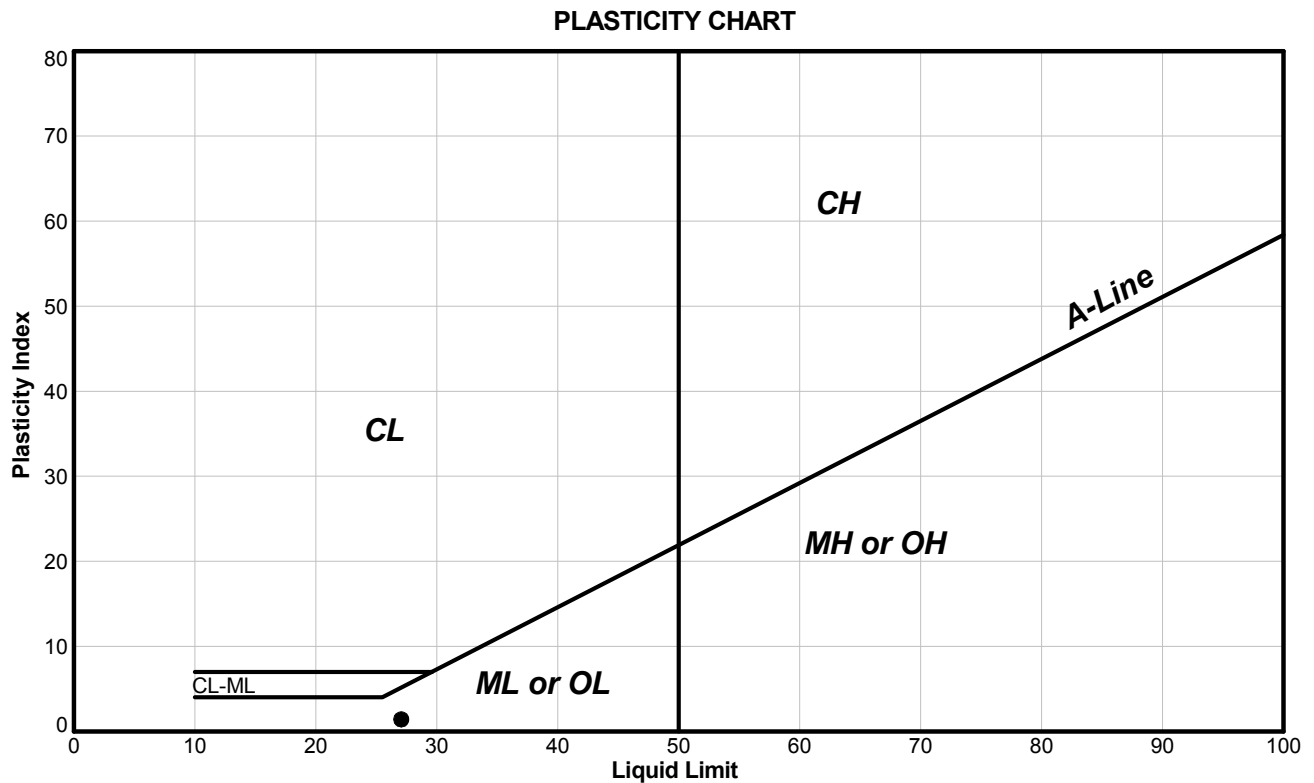
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-60
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-60	1	0.70	0.90	66	27	26	1	24.8	-1.2

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	07/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-61
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	49
Liquid Limit	24
Plastic Limit	21
Plasticity Index	3
Natural Water Content (%)	11.8
Liquidity Index	-3.1



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

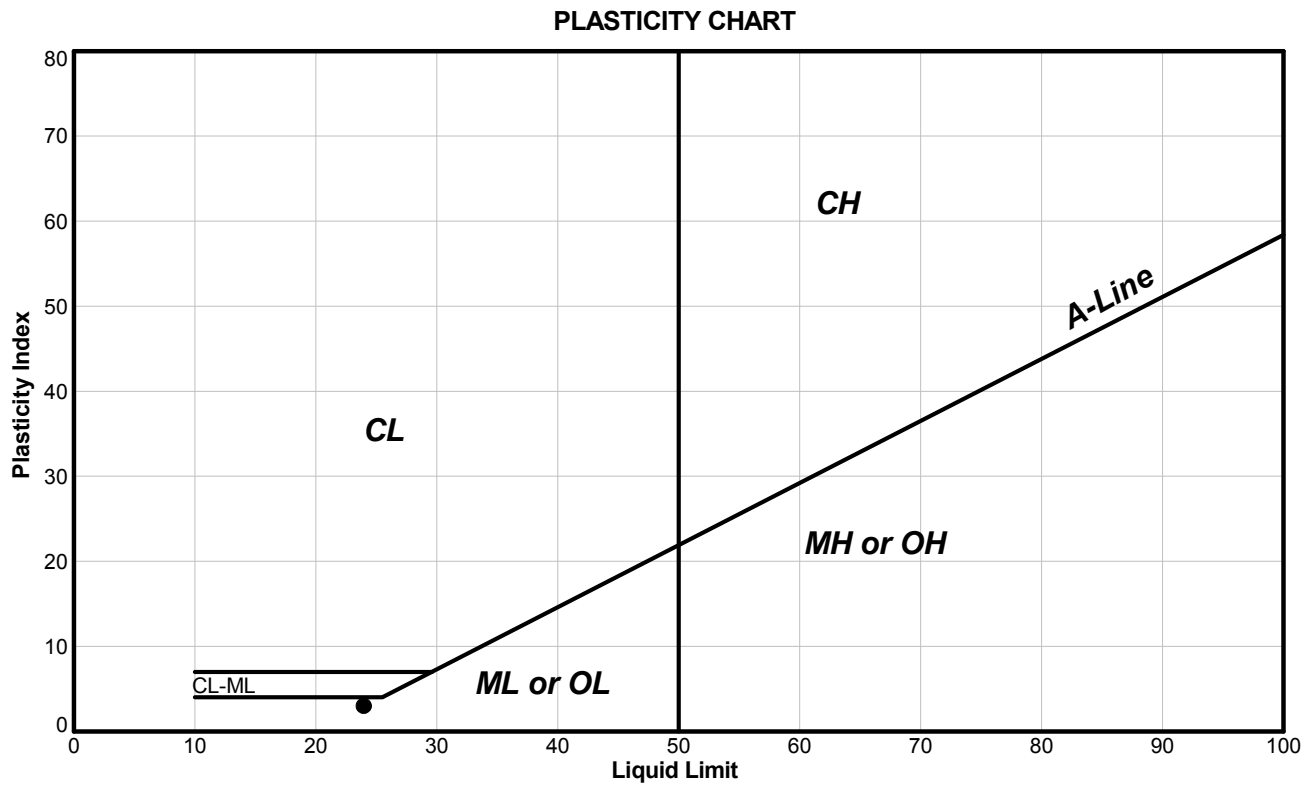
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-61
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 0.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-61	1	0.80	0.90	49	24	21	3	11.8	-3.1

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

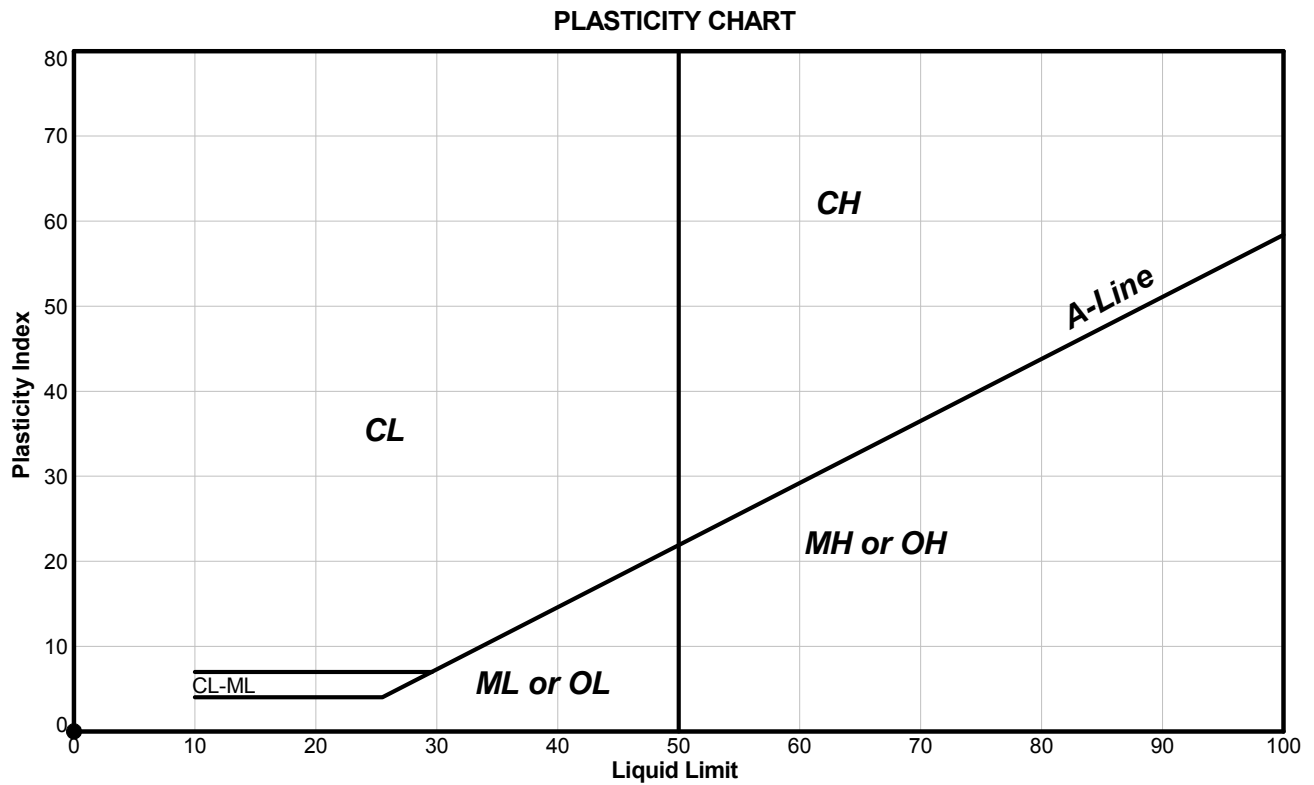
SK	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-62
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.90 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-62	1	0.90	1.00	49	NP	NP	NP	10.5	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	14/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-62
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.50 to 3.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

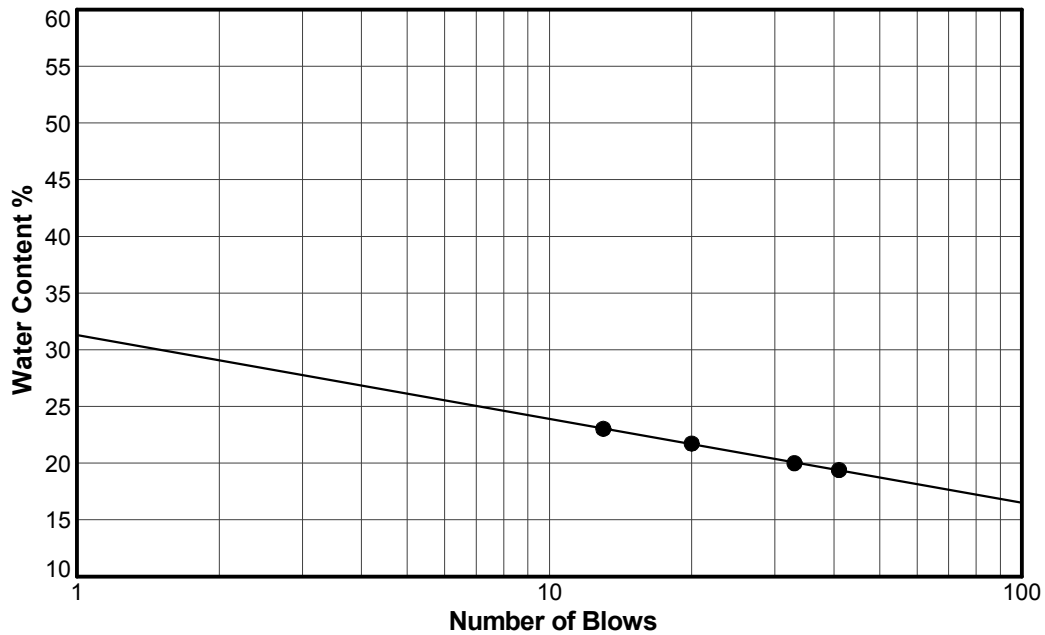
Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	41
Liquid Limit	21
Plastic Limit	16
Plasticity Index	5
Natural Water Content (%)	8.8
Liquidity Index	-1.4



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	14/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

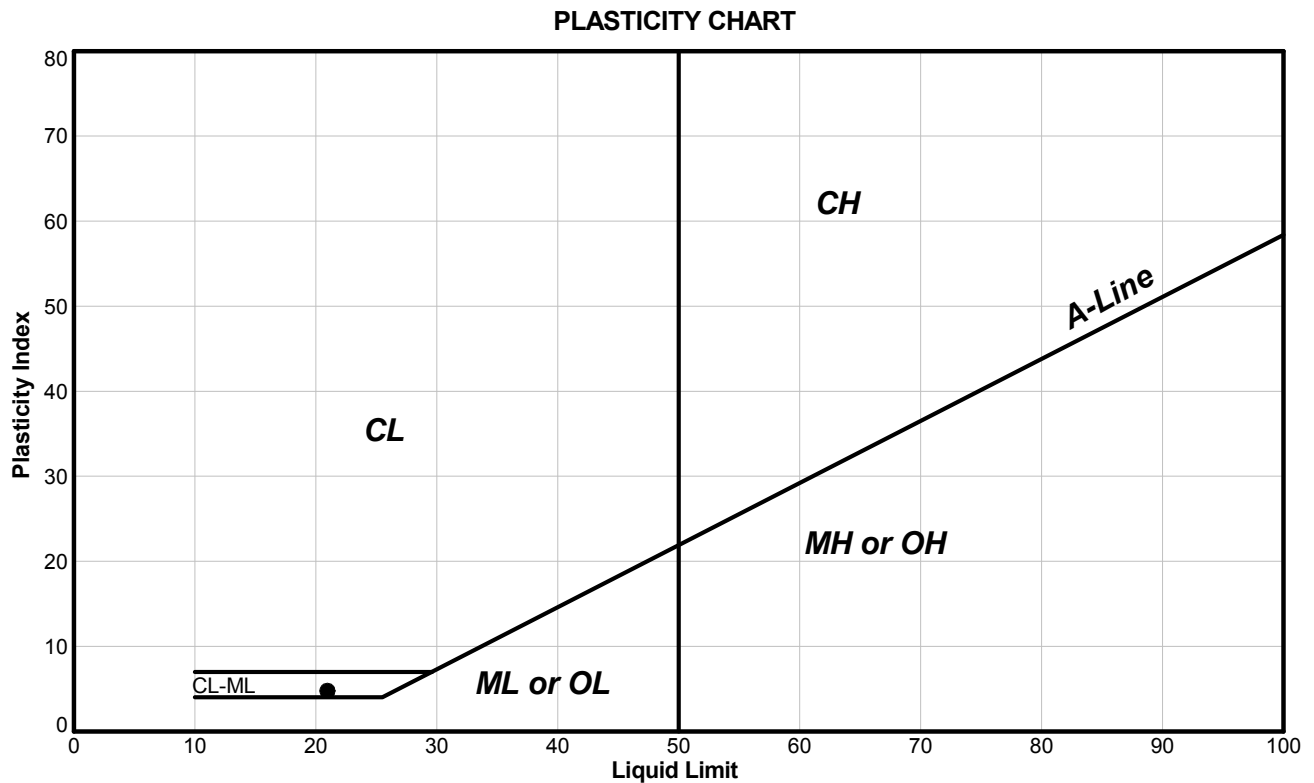
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-62
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.50 to 3.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-62	2	3.50	3.70	41	21	16	5	8.8	-1.4

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

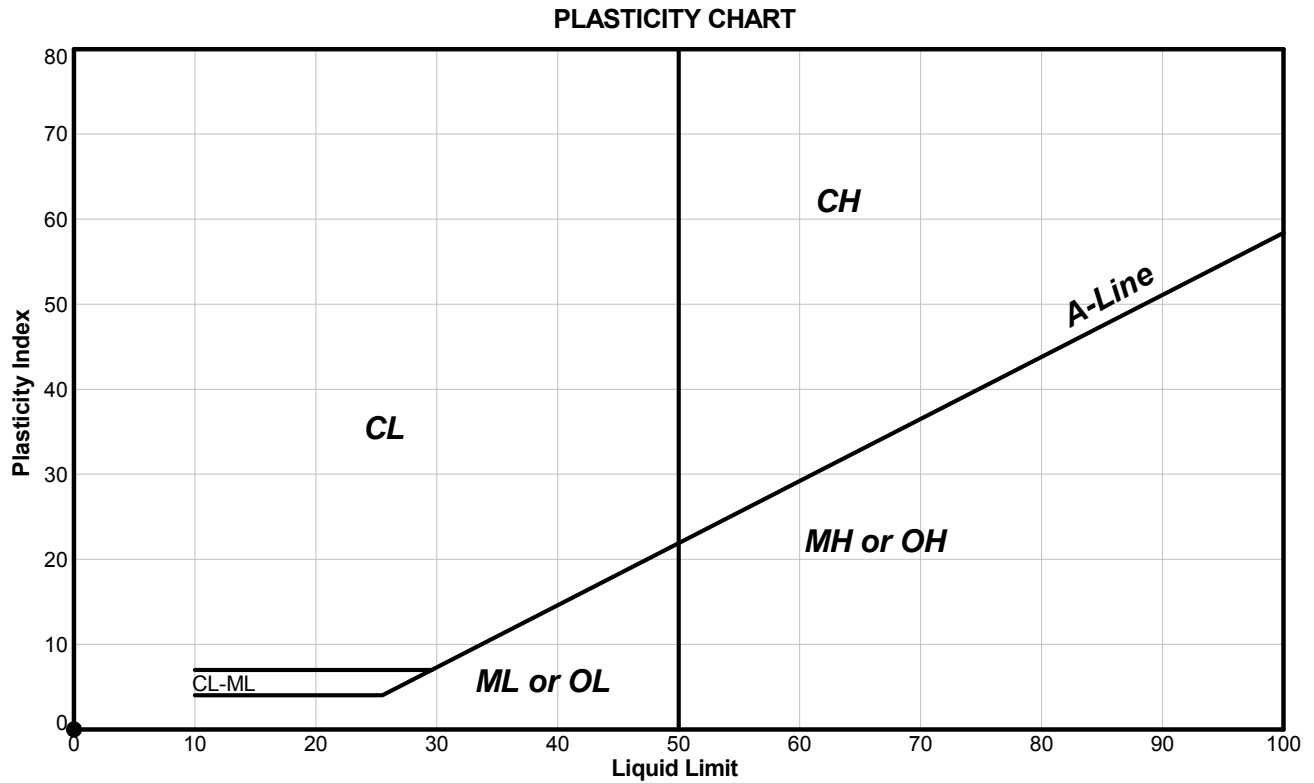
SK	14/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-63
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-63	1	0.40	0.60	31	NP	NP	NP	7.4	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

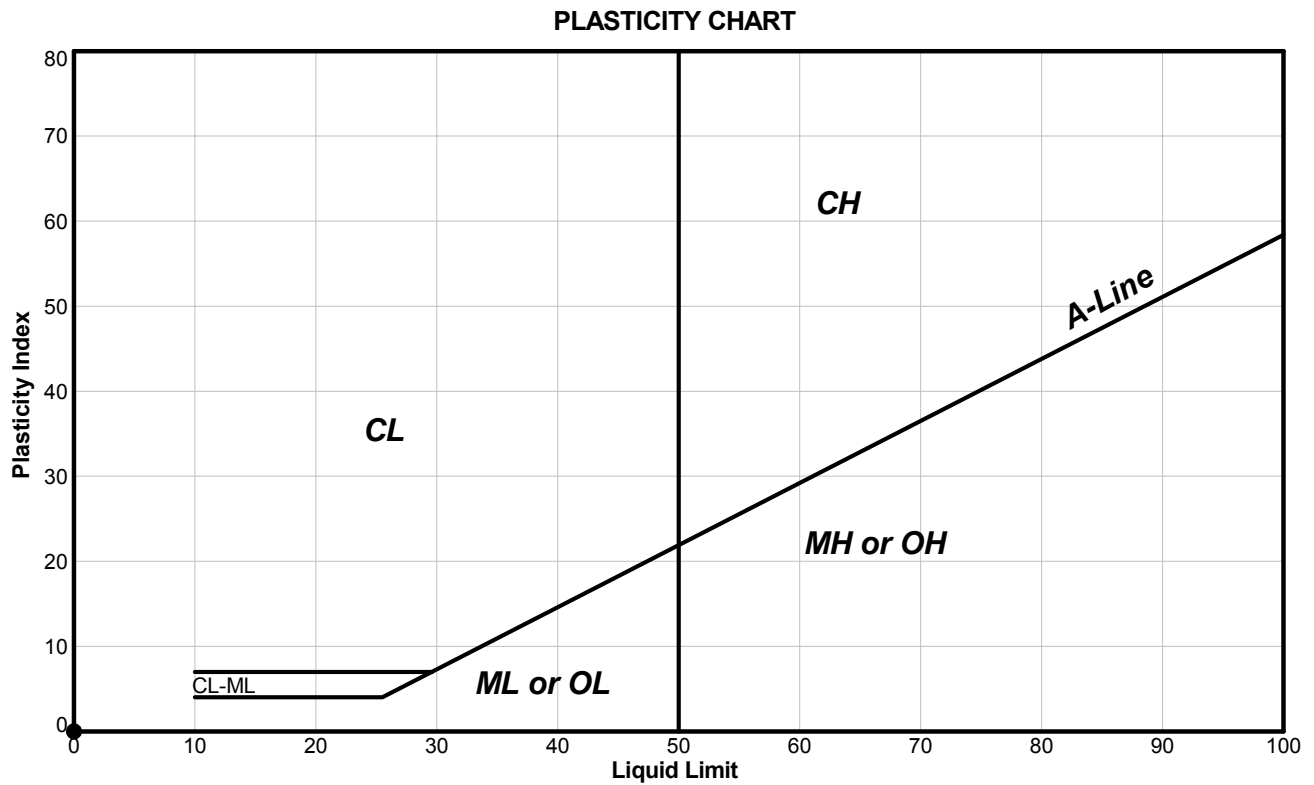
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-64
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-64	1	1.50	1.60	41	NP	NP	NP	4.7	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	02/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

File: N:\BUR_G\RAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 04/11/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-66
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.80 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL - Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.

Other Remarks: N/A

Test Method: A-Multi Point **Preparation Method:** Wet

SUMMARY	
Percent Passing #40 Sieve (%)	84
Liquid Limit	29
Plastic Limit	19
Plasticity Index	10
Natural Water Content (%)	16.7
Liquidity Index	-0.2



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 21/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

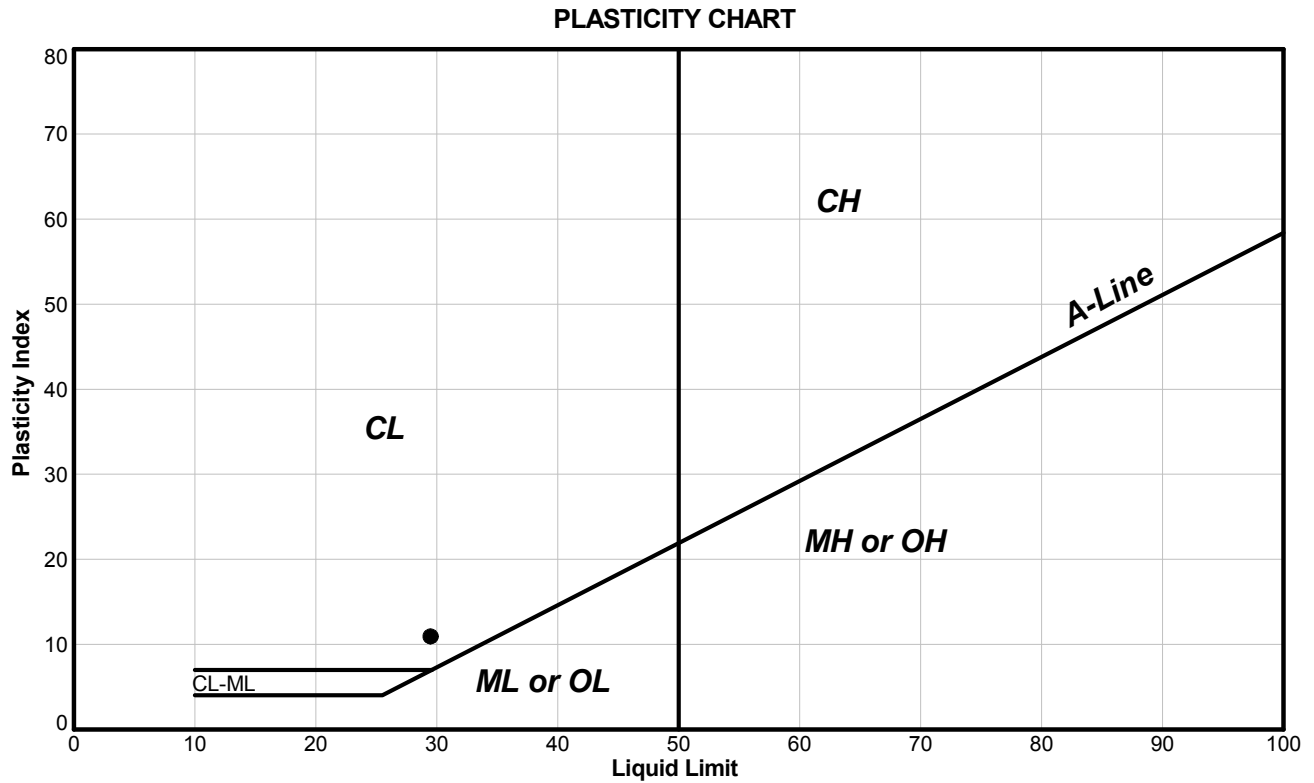
Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-66
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.80 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL - Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.

Other Remarks: N/A

Test Method: A-Multi Point **Preparation Method:** Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-66	2	1.80	1.80	84	29	19	10	16.7	-0.2

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	21/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 21/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-69
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	97
Liquid Limit	NP
Plastic Limit	NP
Plasticity Index	NP
Natural Water Content (%)	30.5
Liquidity Index	NP



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	07/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

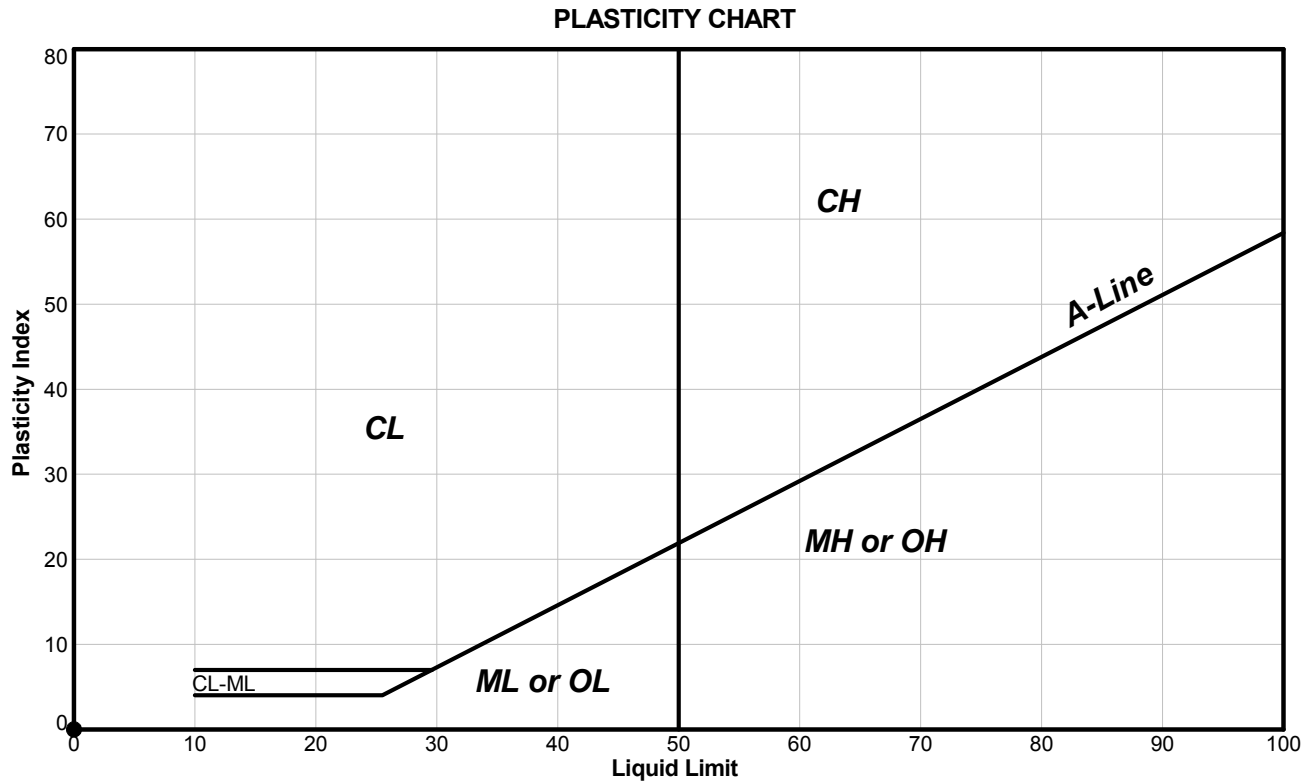
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-69
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.70
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-69	1	1.50	1.70	97	NP	NP	NP	30.5	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	07/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_G\RA\PHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-69
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.80 to 2.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	31
Liquid Limit	23
Plastic Limit	17
Plasticity Index	6
Natural Water Content (%)	6.8
Liquidity Index	-1.7



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	14/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

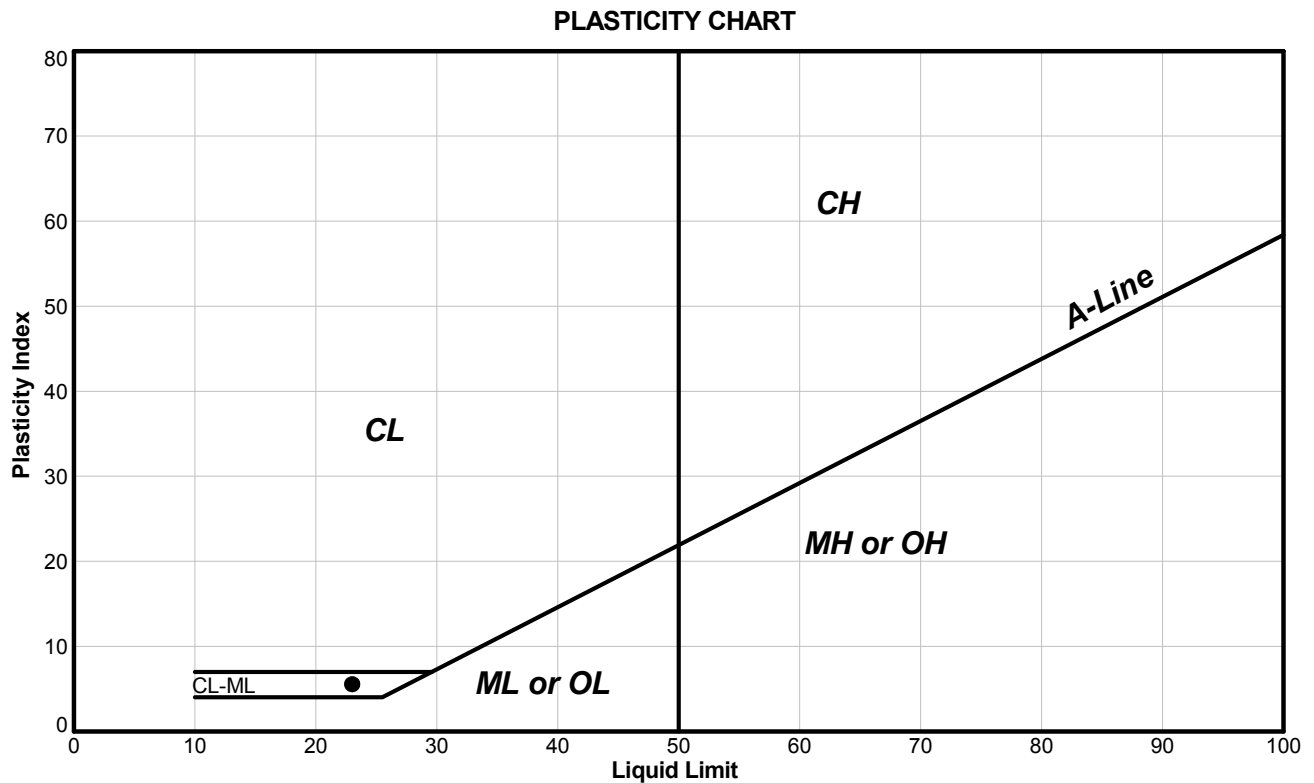
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-69
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.80 to 2.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-69	2	2.80	2.90	31	23	17	6	6.8	-1.7

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	14/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-71
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

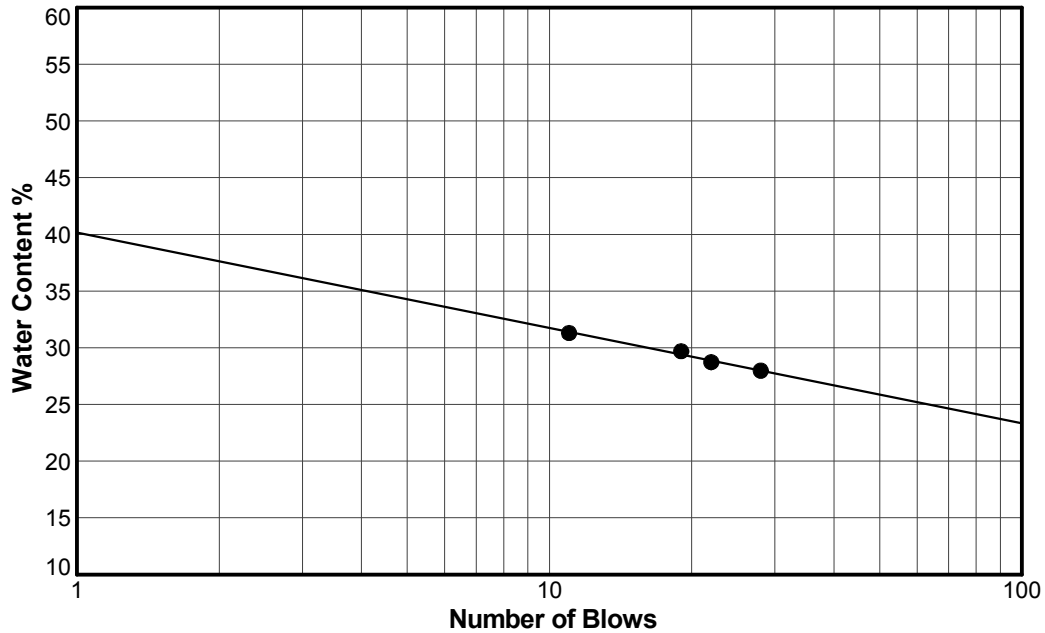
Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	42
Liquid Limit	28
Plastic Limit	22
Plasticity Index	6
Natural Water Content (%)	13.8
Liquidity Index	-1.4



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKIN 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

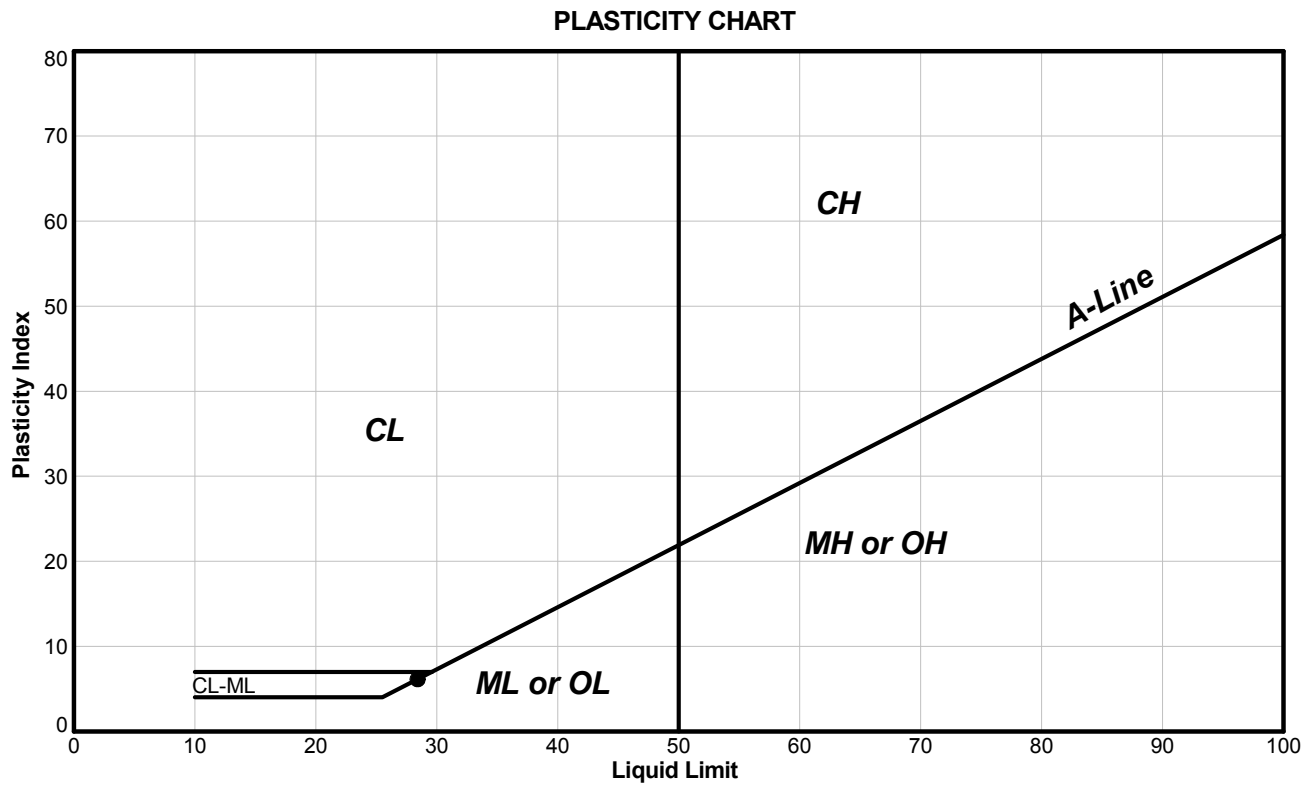
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-71
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-71	1	1.30	1.60	42	28	22	6	13.8	-1.4

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-72
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

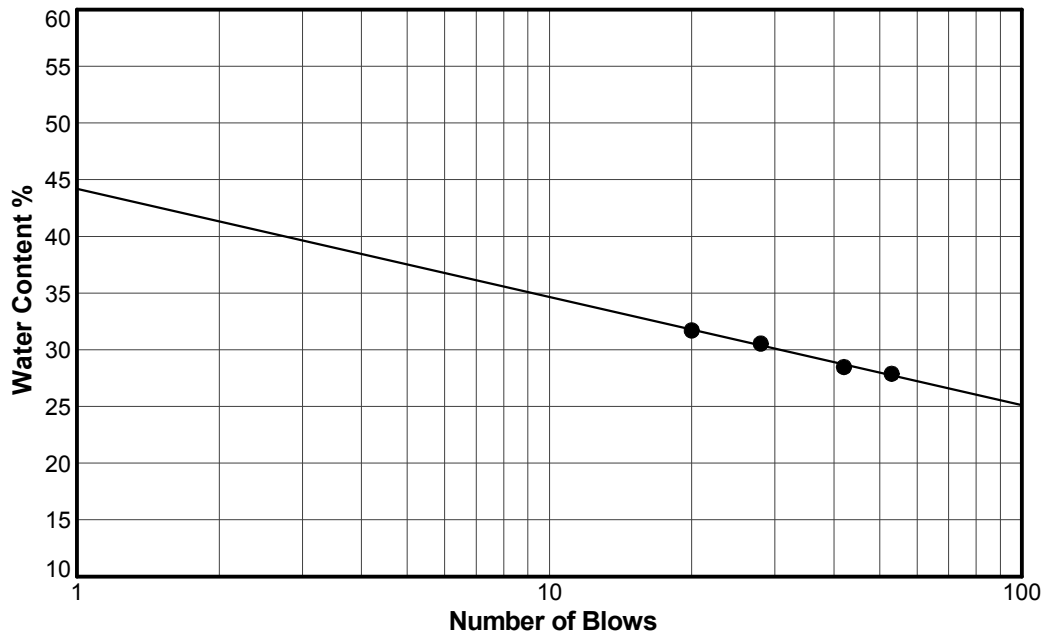
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	26
Liquid Limit	31
Plastic Limit	24
Plasticity Index	7
Natural Water Content (%)	13.7
Liquidity Index	-1.5



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

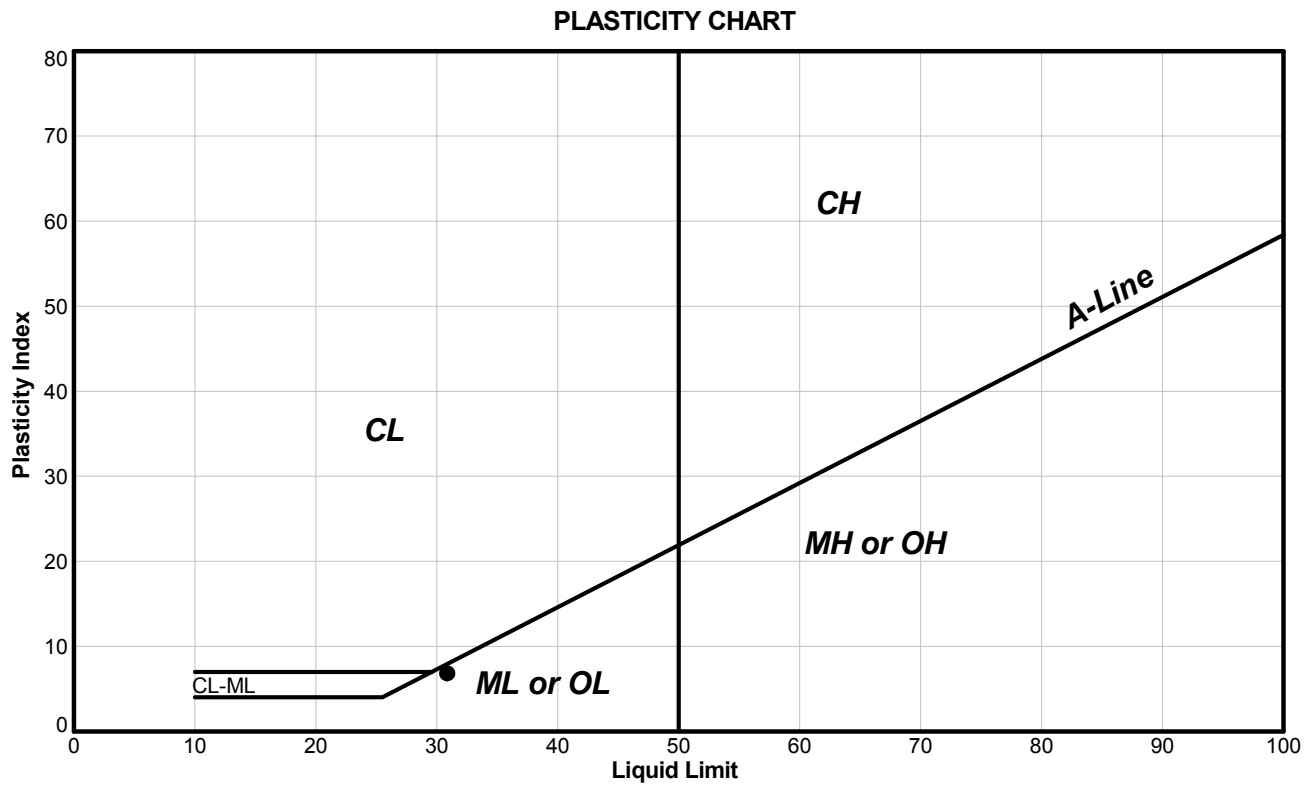
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-72
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-72	2	0.80	1.00	26	31	24	7	13.7	-1.5

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	16/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

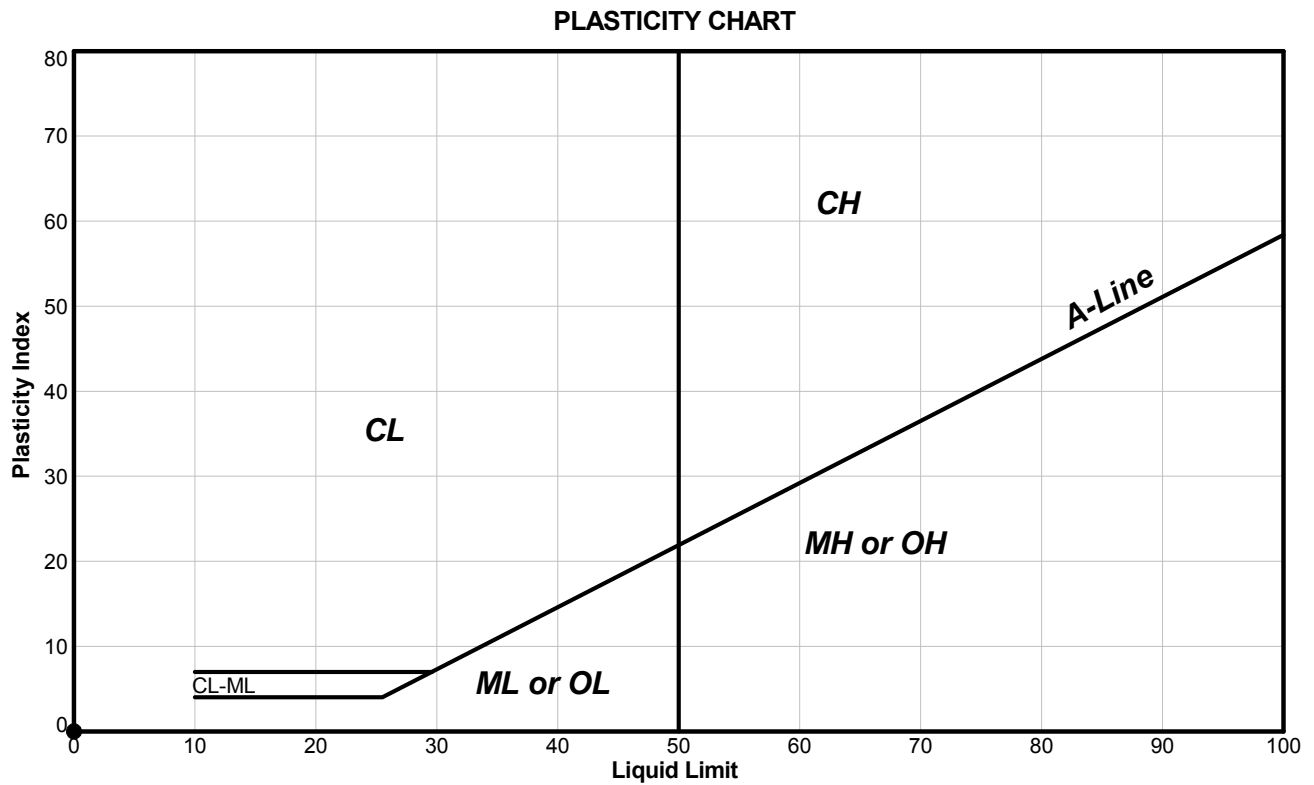
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-73
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-73	1	0.80	1.00	45	NP	NP	NP	8.5	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	19/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.20 to 1.40
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

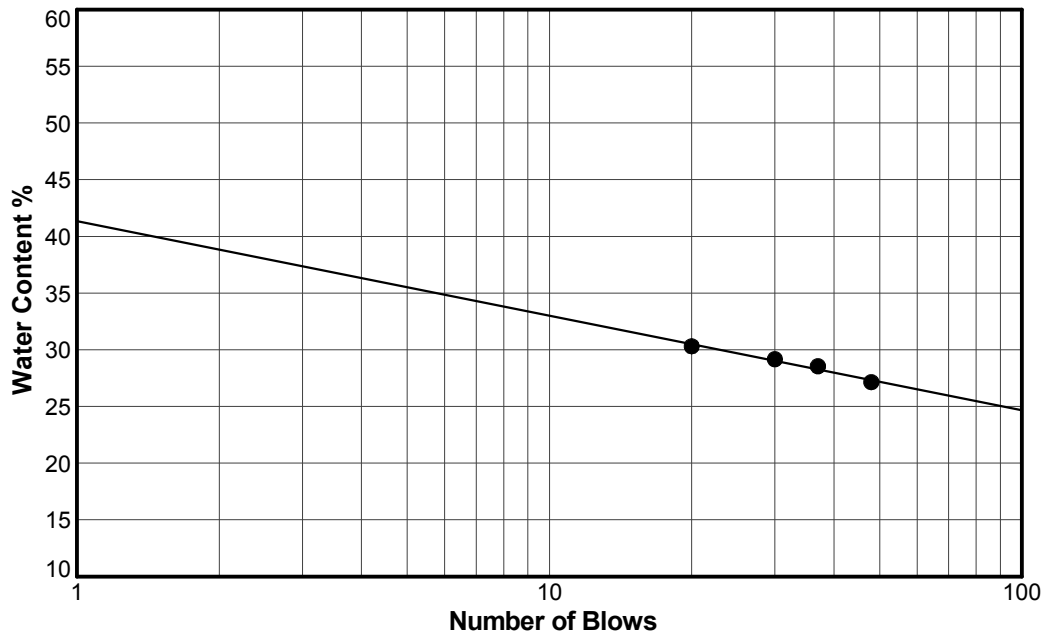
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	44
Liquid Limit	30
Plastic Limit	25
Plasticity Index	5
Natural Water Content (%)	11.8
Liquidity Index	-2.6



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

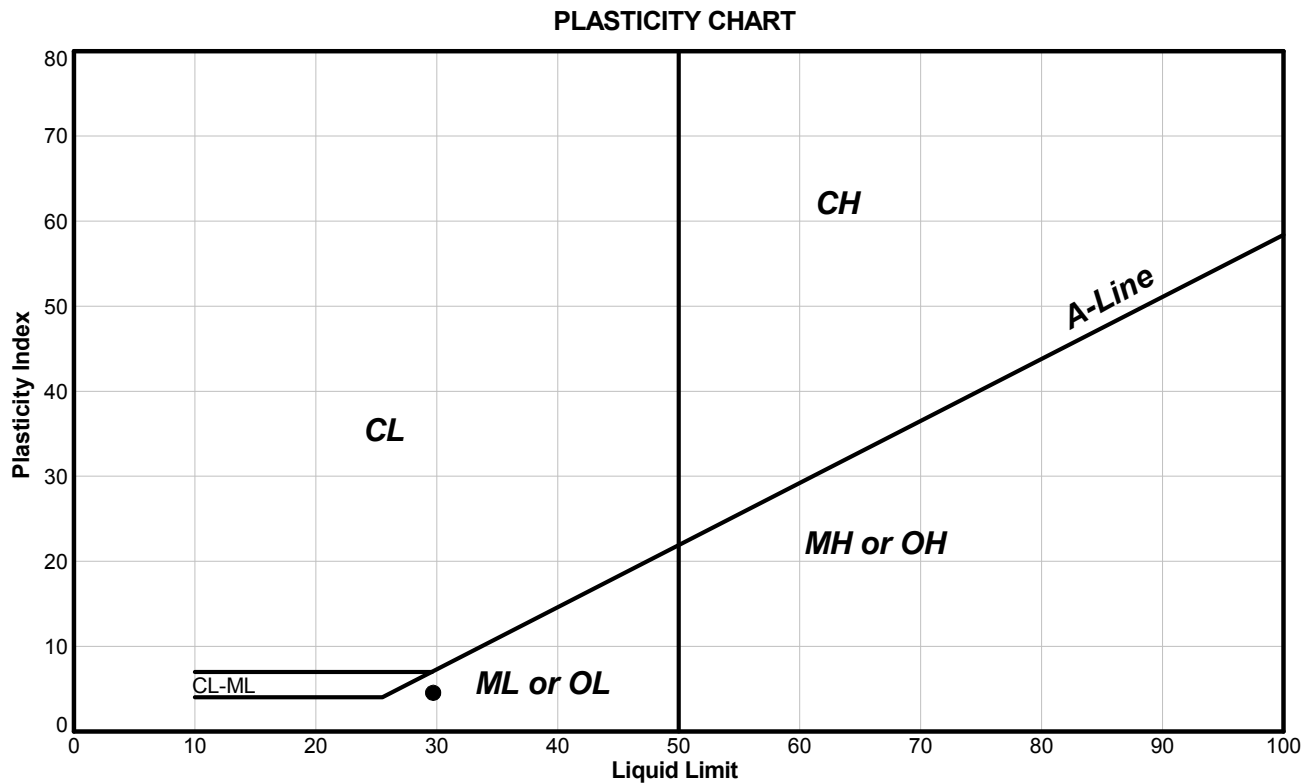
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.20 to 1.40
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-74	1	1.20	1.40	44	30	25	5	11.8	-2.6

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	13/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template:BC REGION TEMPLATE.BETA.1.GDT Library:BC REGION LIBRARY.GLB_Skin 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.00 to 3.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

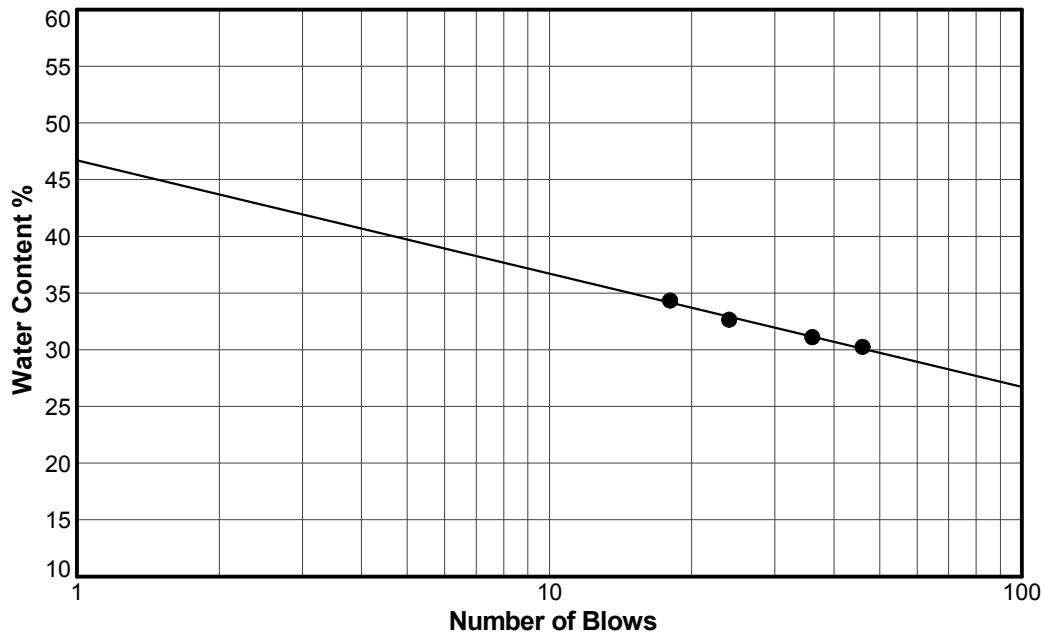
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	99
Liquid Limit	33
Plastic Limit	30
Plasticity Index	3
Natural Water Content (%)	35.9
Liquidity Index	2.0



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

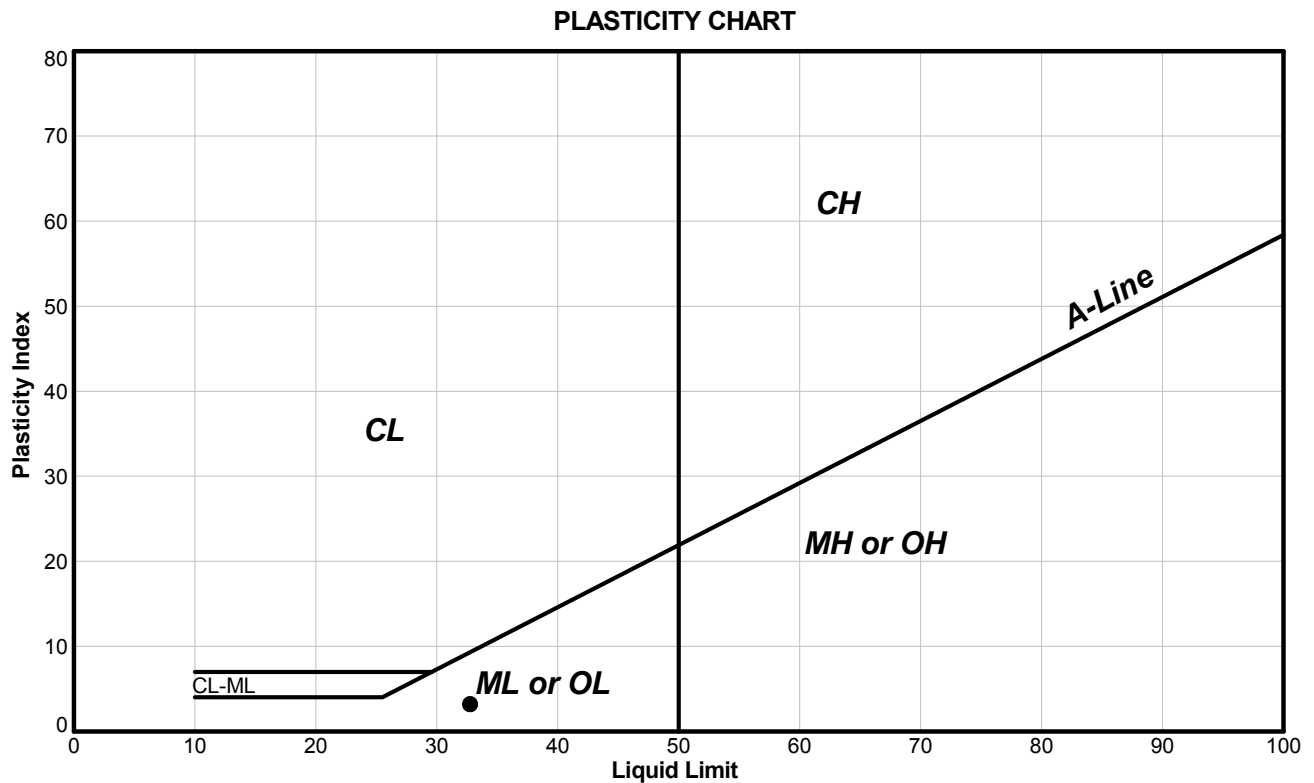
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.00 to 3.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-74	2	3.00	3.00	99	33	30	3	35.9	2.0

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	12/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 3
Location: Dublin Gulch, Yukon	Depth Interval (m): 4.50 to 4.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

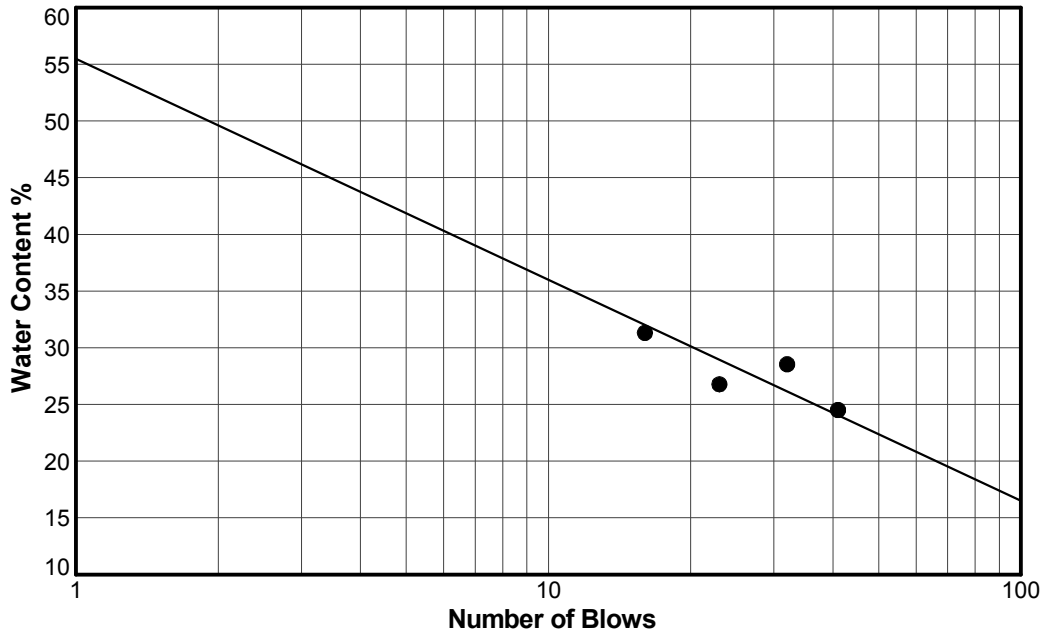
Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	57
Liquid Limit	28
Plastic Limit	22
Plasticity Index	6
Natural Water Content (%)	15.0
Liquidity Index	-1.2



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	19/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

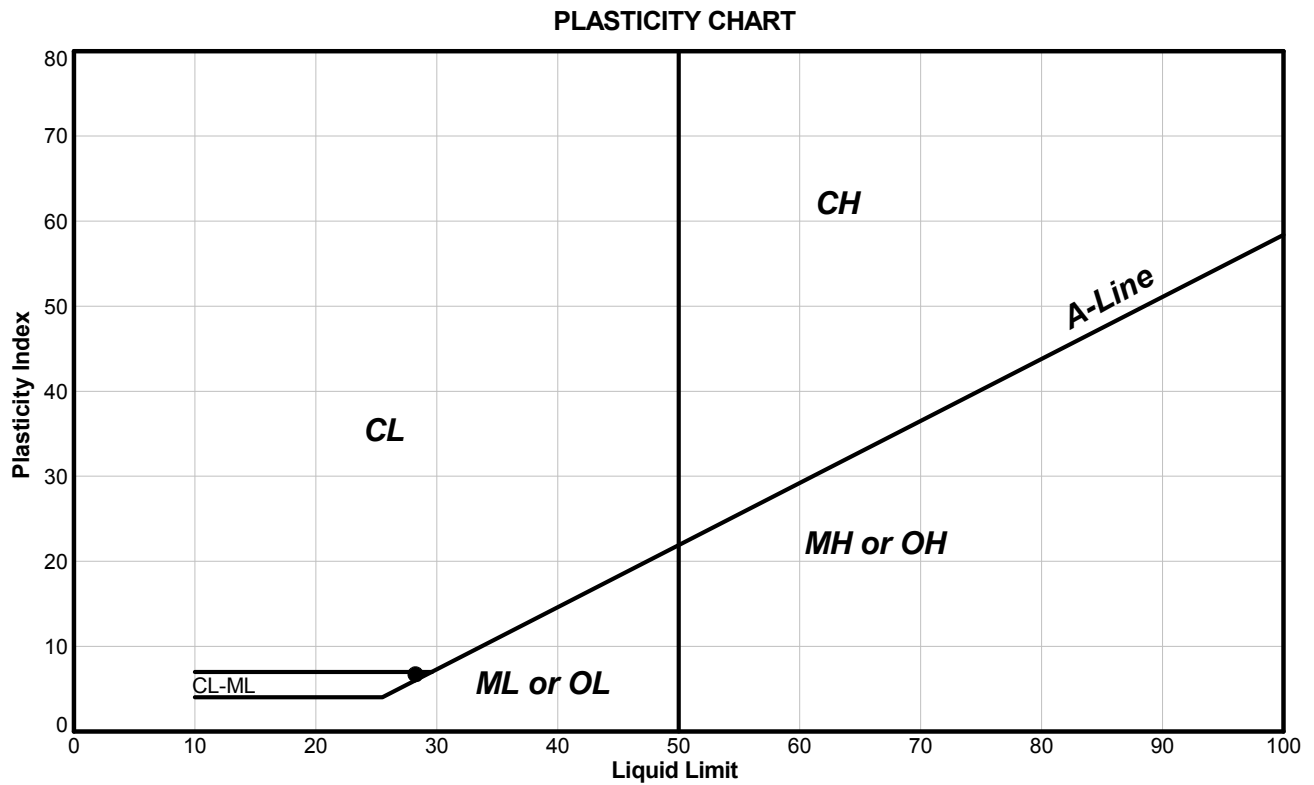
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-74
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 3
Location: Dublin Gulch, Yukon	Depth Interval (m): 4.50 to 4.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-74	3	4.50	4.50	57	28	22	6	15.0	-1.2

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	19/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

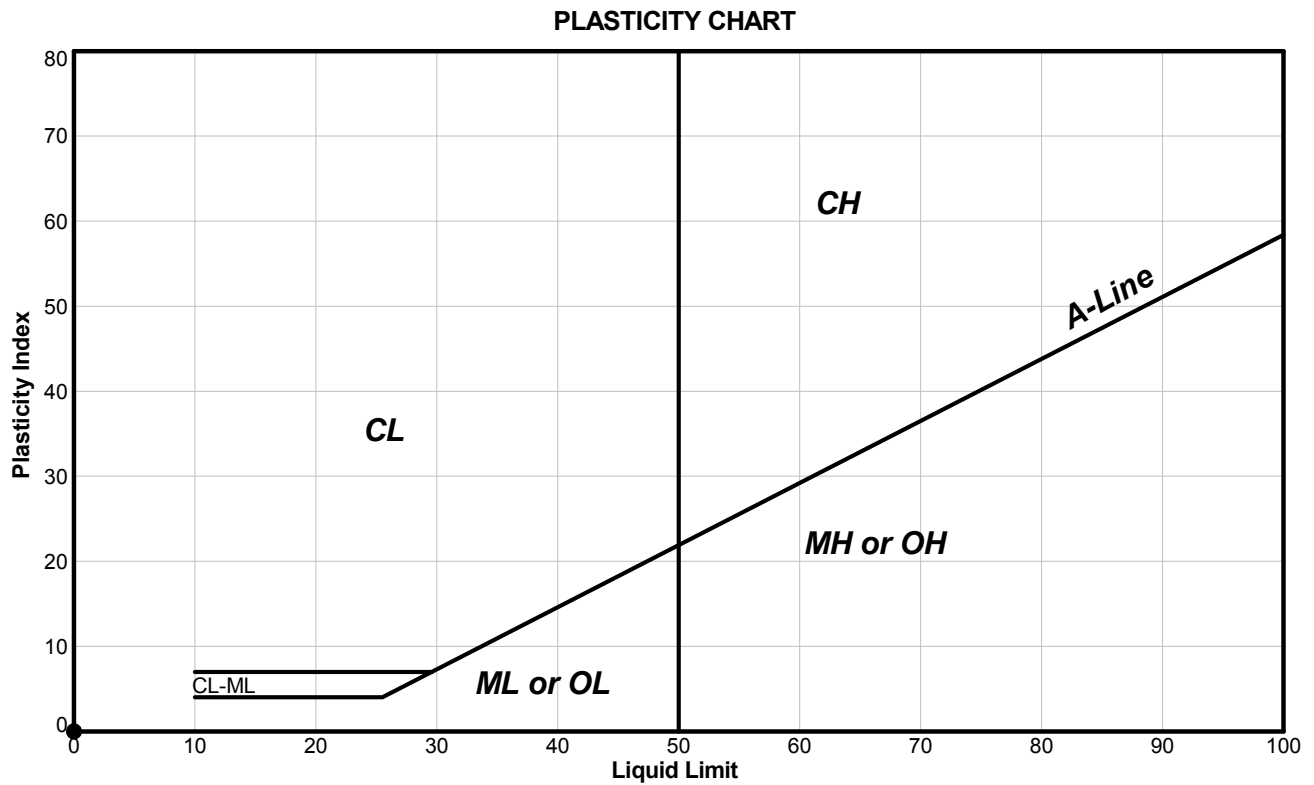
File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB_Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-76
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-76	1	0.40	0.60	68	NP	NP	NP	16.1	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	22/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-77
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

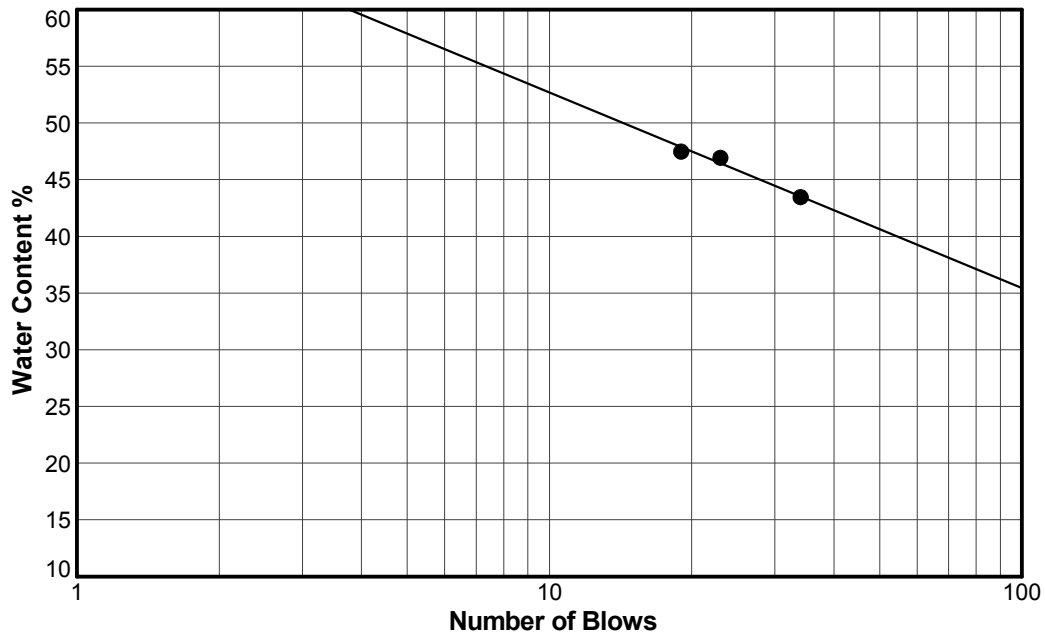
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	57
Liquid Limit	46
Plastic Limit	29
Plasticity Index	17
Natural Water Content (%)	26.6
Liquidity Index	-0.1



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

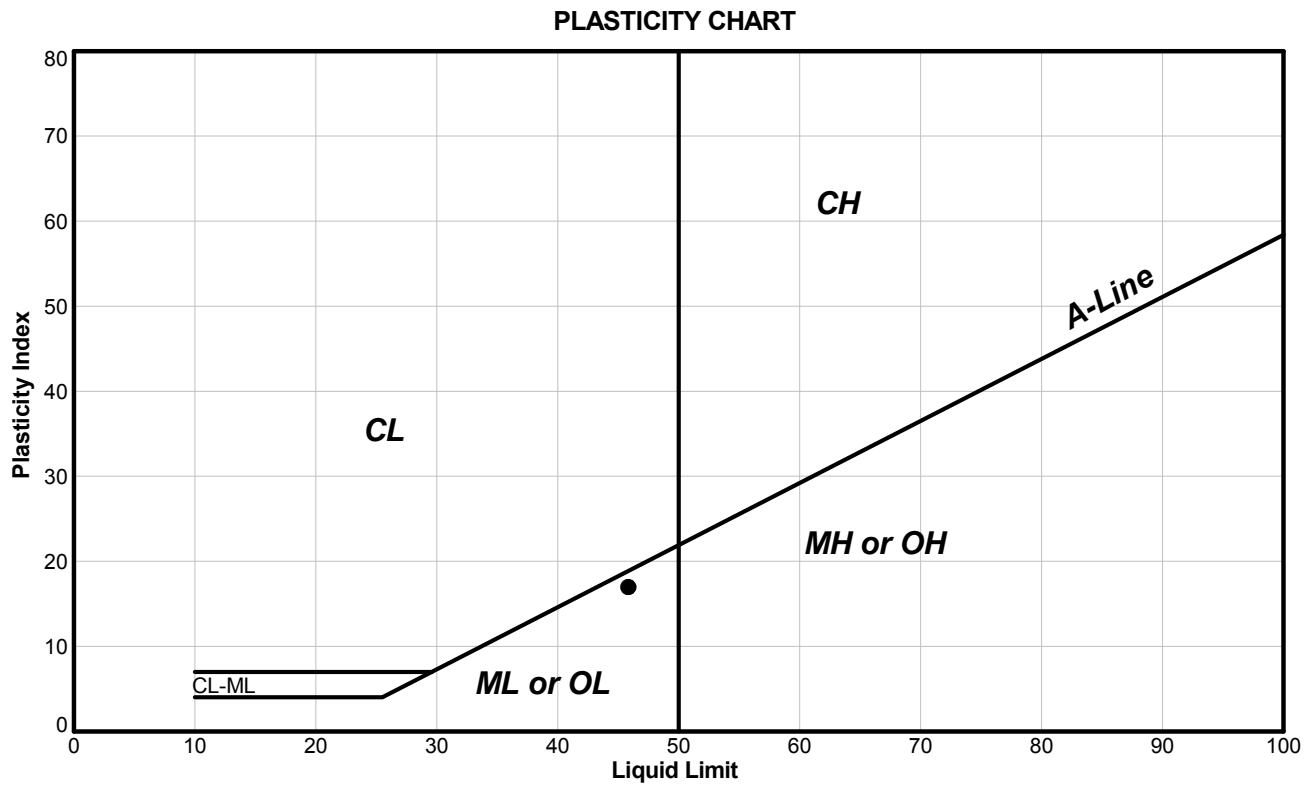
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-77
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-77	1	0.40	0.50	57	46	29	17	26.6	-0.1

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-79
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	18
Liquid Limit	26
Plastic Limit	25
Plasticity Index	1
Natural Water Content (%)	10.1
Liquidity Index	-14.9



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	27/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 21/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

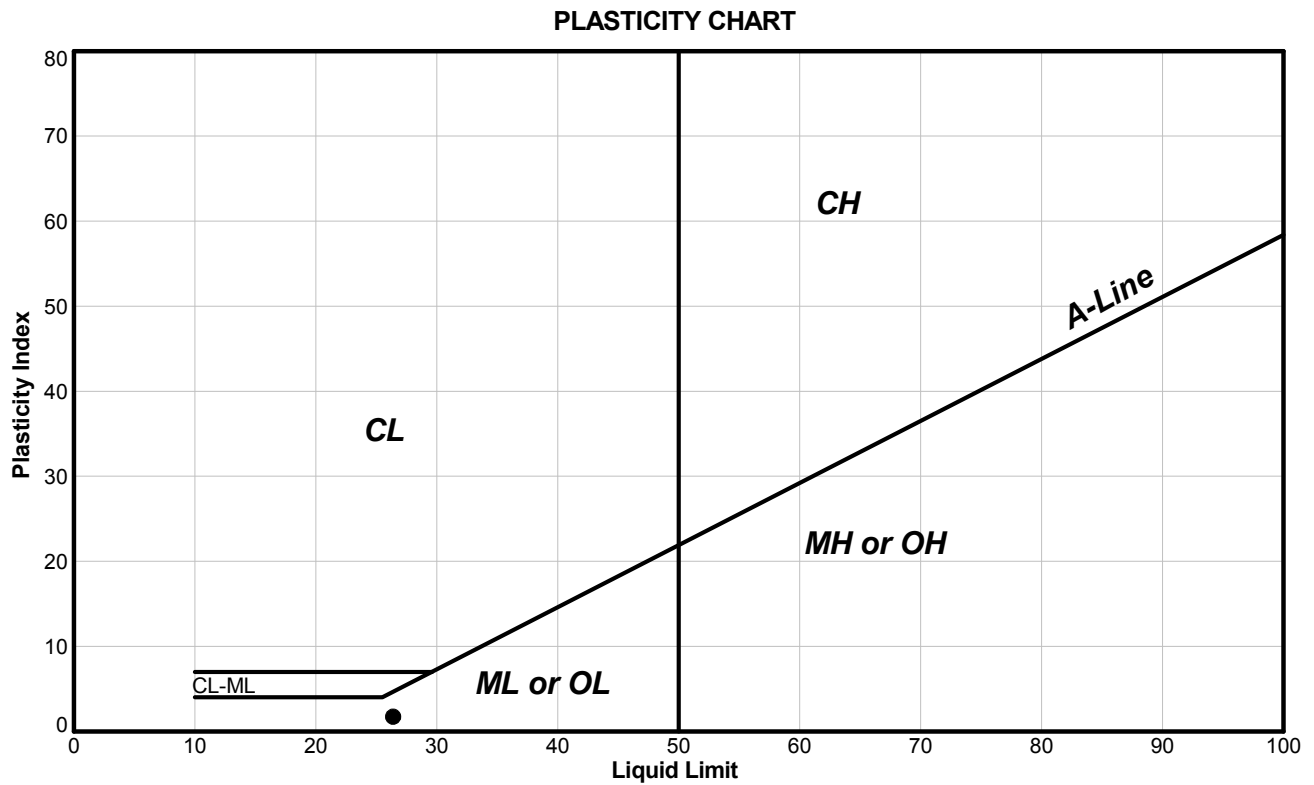
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-79
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.40 to 0.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-79	1	0.40	0.60	18	26	25	1	10.1	-14.9

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

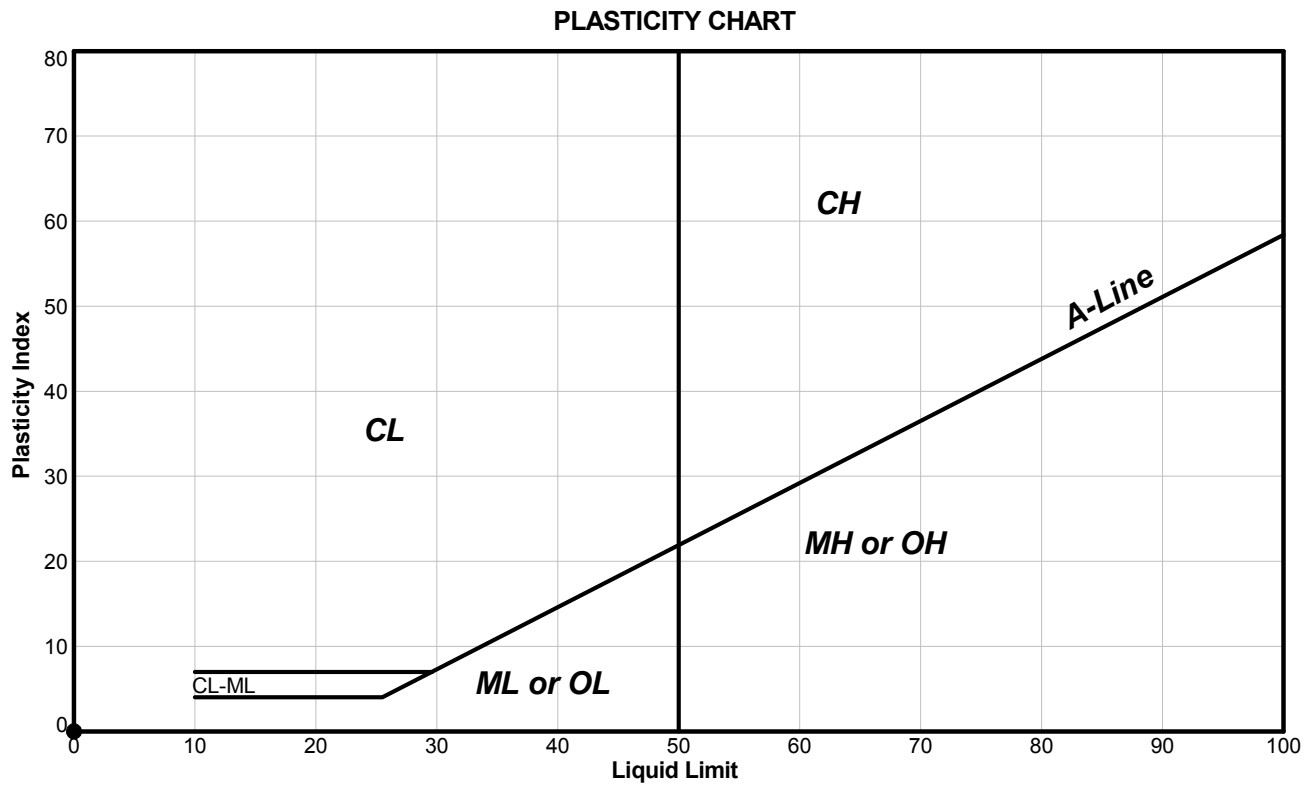
SK	27/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-81
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-81	1	0.70	1.00	17	NP	NP	NP	16.6	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	27/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

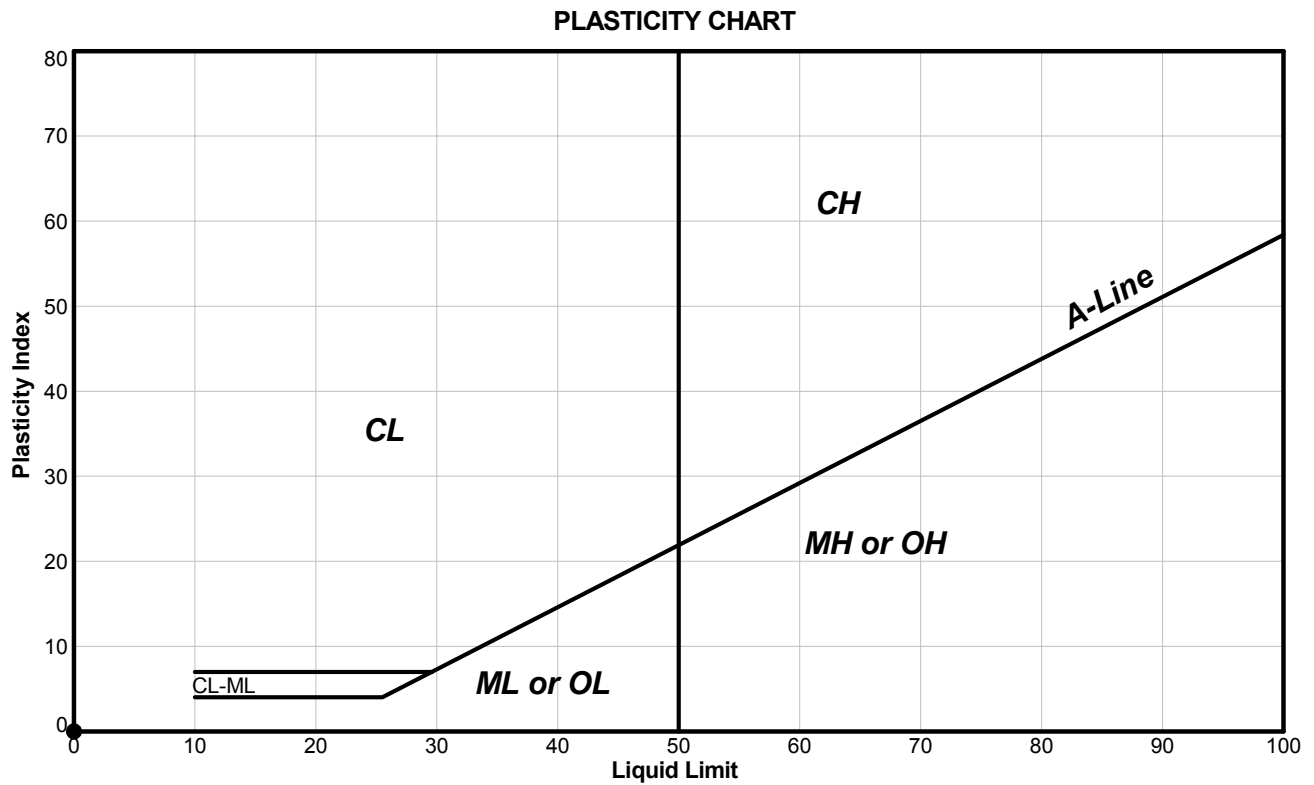
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M1
Location: Dublin Gulch, Yukon	Depth Interval (m): 4.70 to 4.90
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-82	M1	4.70	4.90	100	NP	NP	NP	12.4	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	28/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.00 to 3.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	84
Liquid Limit	30
Plastic Limit	27
Plasticity Index	3
Natural Water Content (%)	10.3
Liquidity Index	-5.6



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	23/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

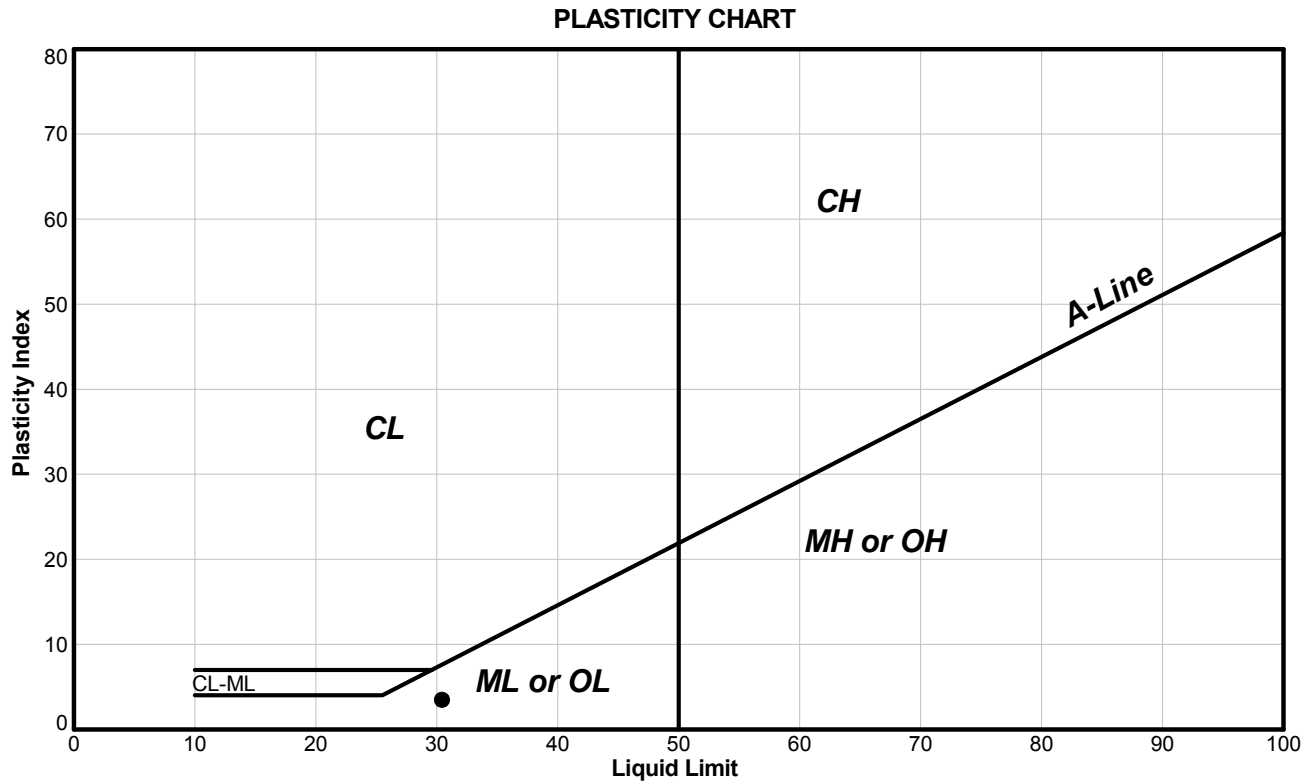
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.00 to 3.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-82	M2	3.00	3.30	84	30	27	3	10.3	-5.6

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

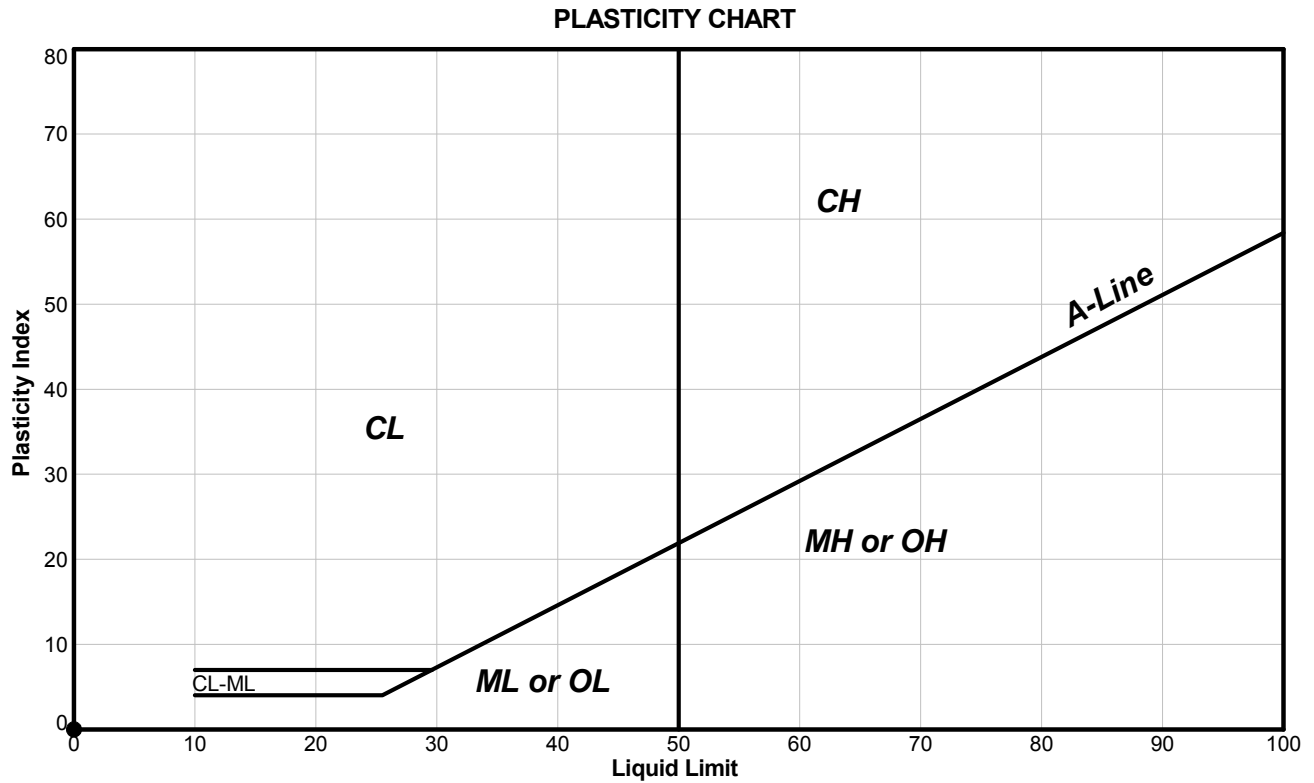
SK	23/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M3
Location: Dublin Gulch, Yukon	Depth Interval (m): 6.70 to 7.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-82	M3	6.70	7.00	89	NP	NP	NP	40.1	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	23/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M4
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 1.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

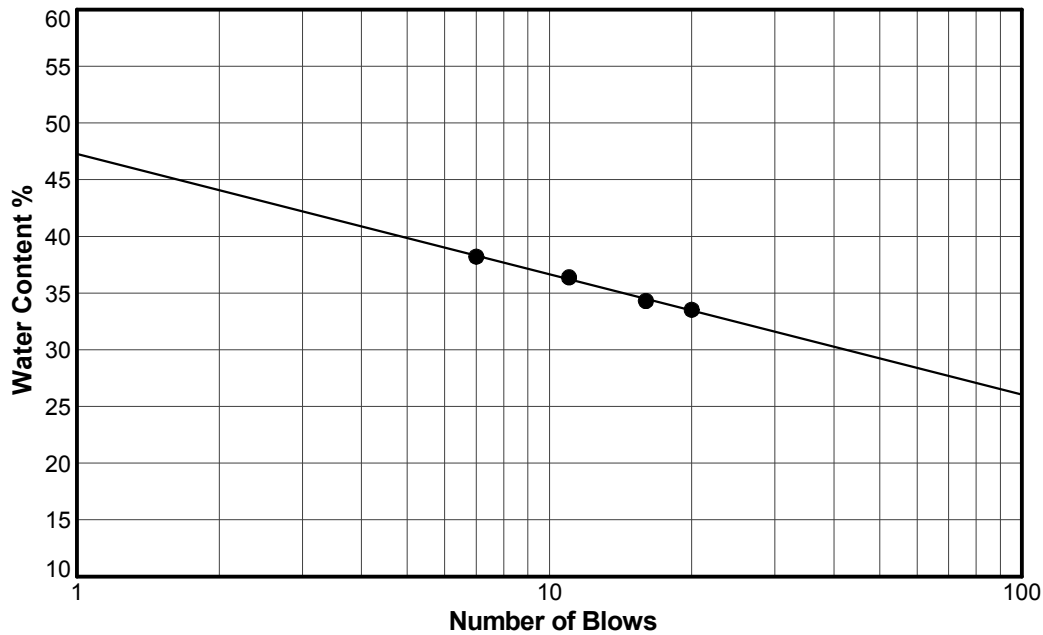
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	94
Liquid Limit	32
Plastic Limit	28
Plasticity Index	4
Natural Water Content (%)	28.6
Liquidity Index	0.2



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

File: N:\BUR_GRAPHICS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 2/10/11

SK	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M4
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.70 to 1.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

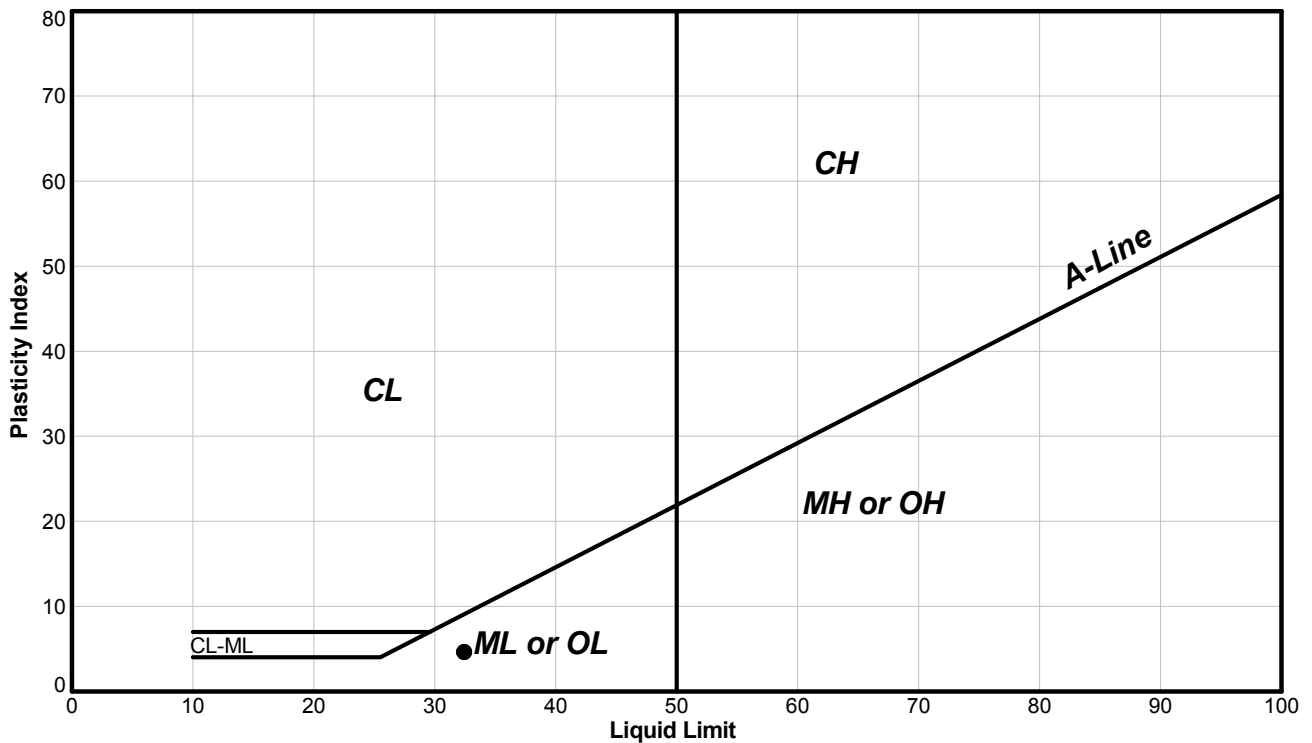
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

PLASTICITY CHART



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-82	M4	0.70	1.10	94	32	28	4	28.6	0.2

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M5
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.90 to 2.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

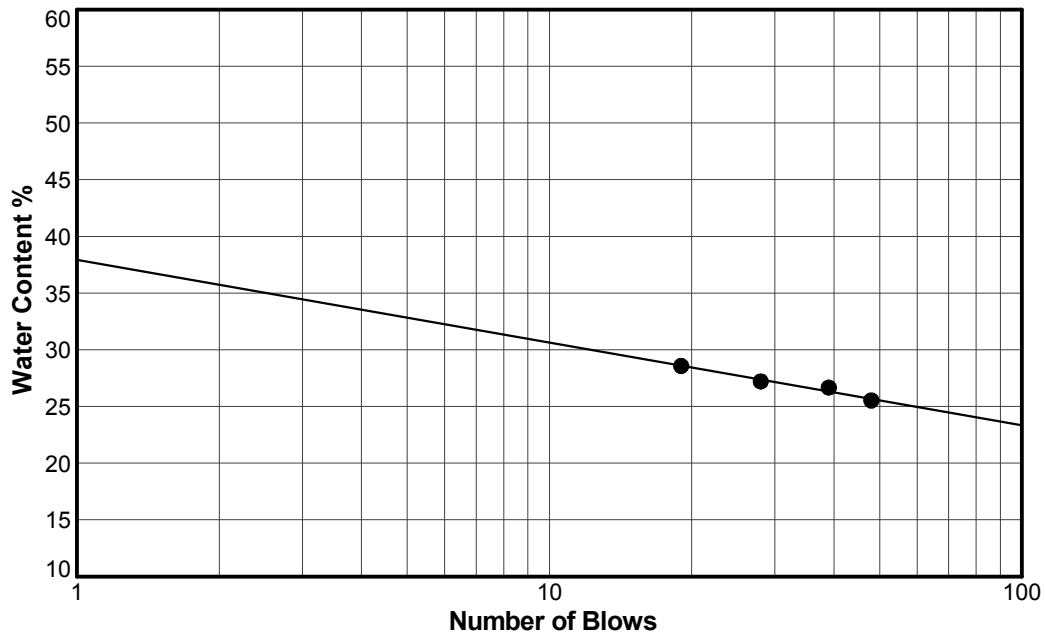
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	87
Liquid Limit	28
Plastic Limit	23
Plasticity Index	5
Natural Water Content (%)	27.3
Liquidity Index	0.9



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	23/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-82
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: M5
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.90 to 2.10
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

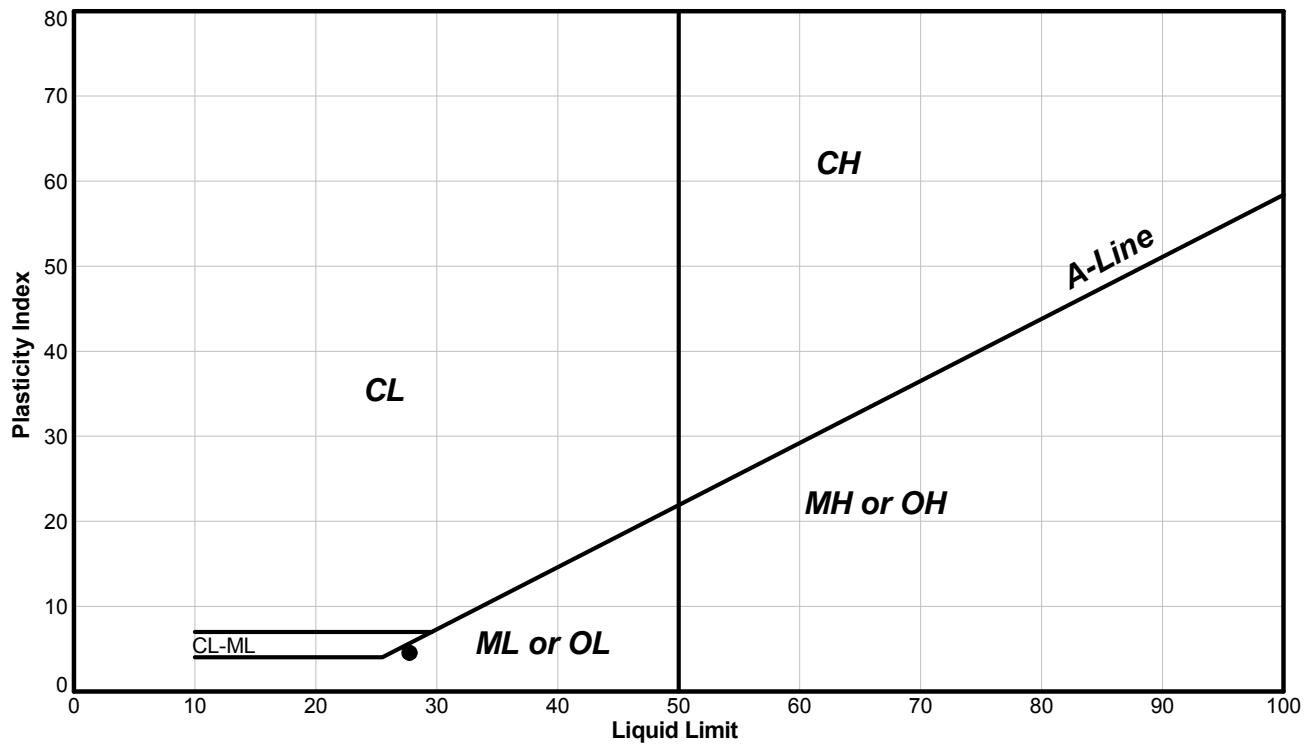
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

PLASTICITY CHART



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-82	M5	1.90	2.10	87	28	23	5	27.3	0.9

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	23/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_G\RA\PHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

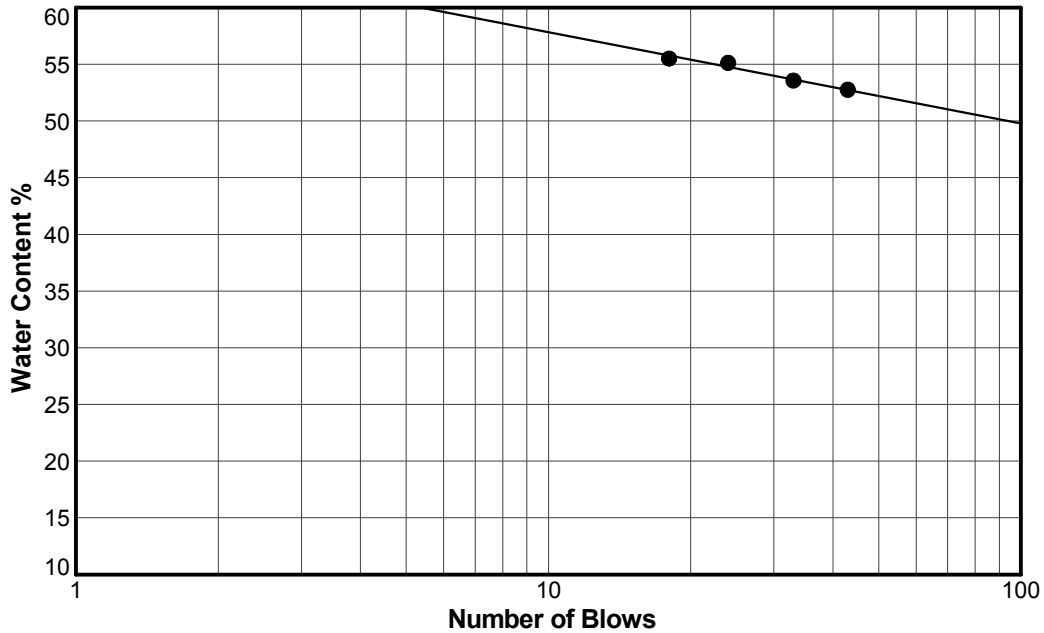
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-83
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: MH - Inorganic silts, micaeous or diatomaceous fine sandy or silty soils, elastic silts.

Other Remarks: N/A

Test Method: A-Multi Point **Preparation Method:** Wet

SUMMARY	
Percent Passing #40 Sieve (%)	45
Liquid Limit	55
Plastic Limit	44
Plasticity Index	11
Natural Water Content (%)	20.1
Liquidity Index	-2.2



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	08/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 21/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

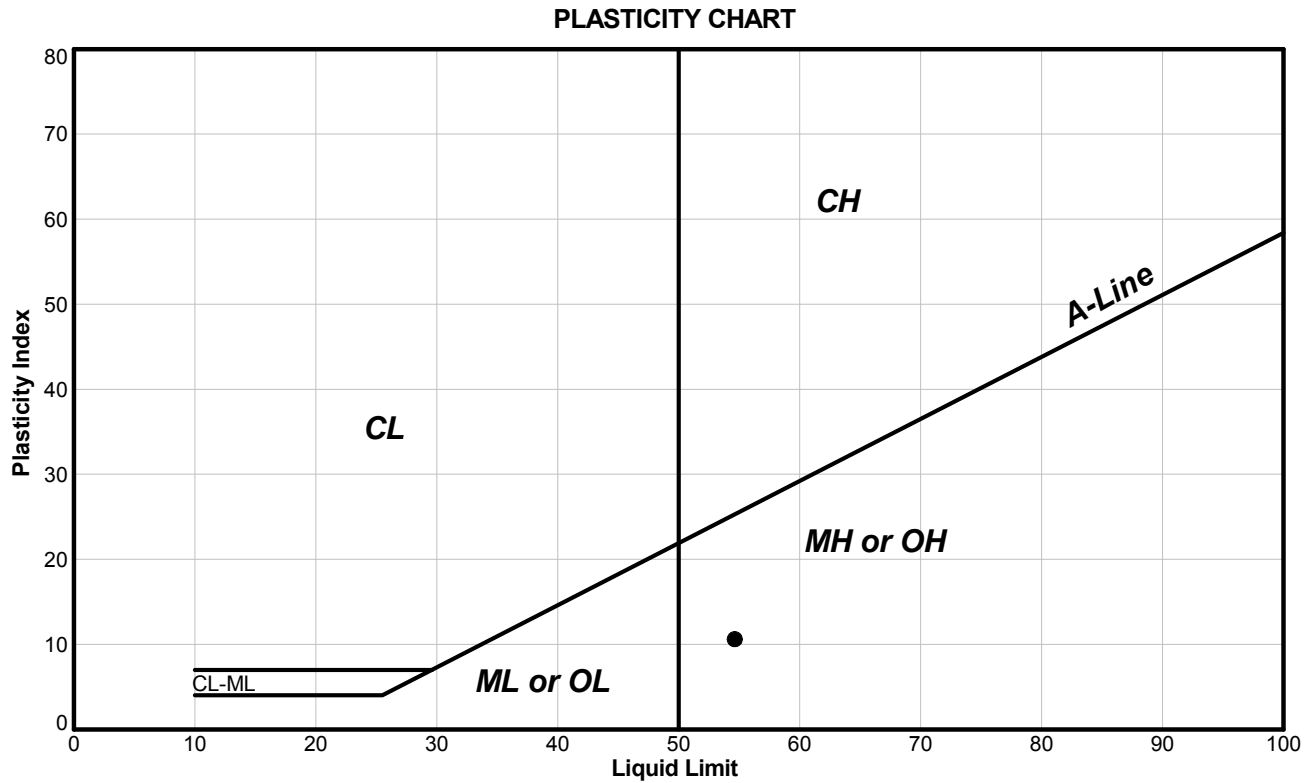
Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-83
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: MH - Inorganic silts, micaeous or diatomaceous fine sandy or silty soils, elastic silts.

Other Remarks: N/A

Test Method: A-Multi Point **Preparation Method:** Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-83	1	0.60	0.80	45	55	44	11	20.1	-2.2

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	08/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template:BC REGION TEMPLATE.BETA.1.GDT Library:BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-84
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.50 to 2.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	95
Liquid Limit	29
Plastic Limit	26
Plasticity Index	3
Natural Water Content (%)	28.5
Liquidity Index	0.8



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-84
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 2.50 to 2.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

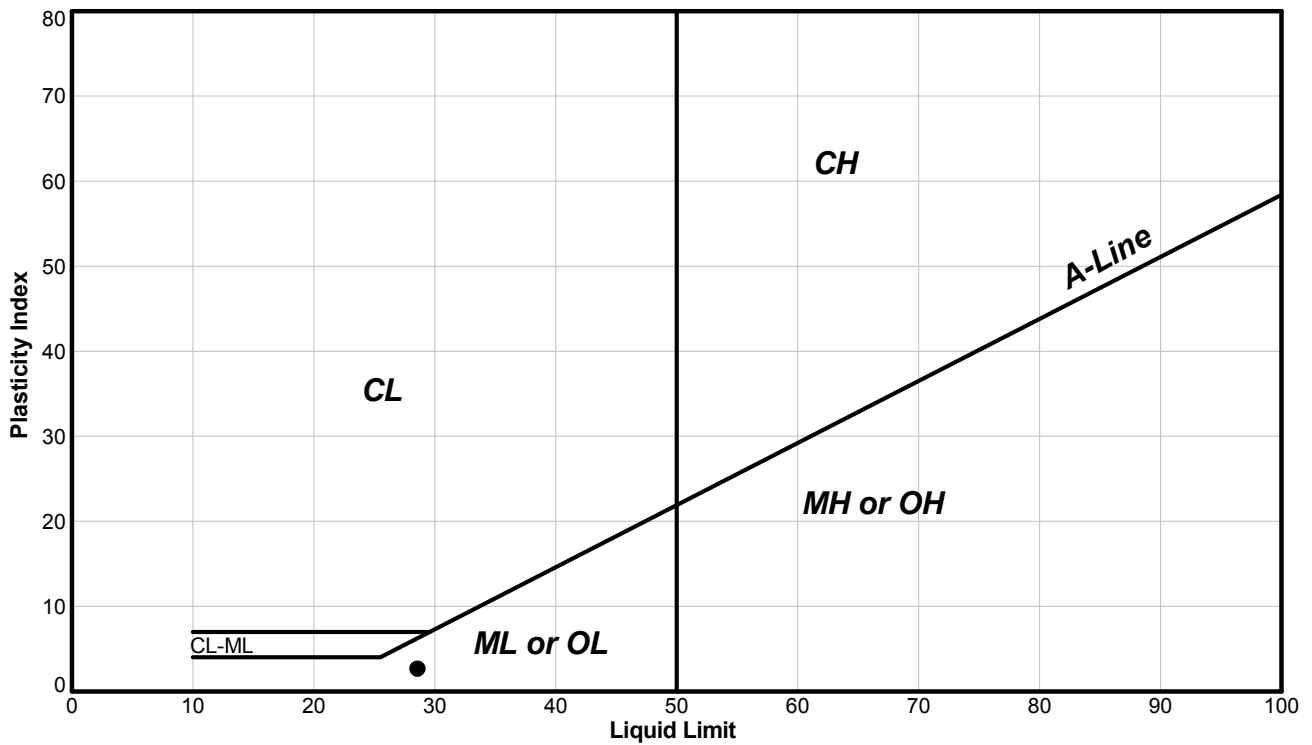
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

PLASTICITY CHART



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-84	2	2.50	2.50	95	29	26	3	28.5	0.8

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-85
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	64
Liquid Limit	23
Plastic Limit	22
Plasticity Index	1
Natural Water Content (%)	16.6
Liquidity Index	-5.4



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	28/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_Graphics\PROJECTS\2011\1415\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKim 21/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

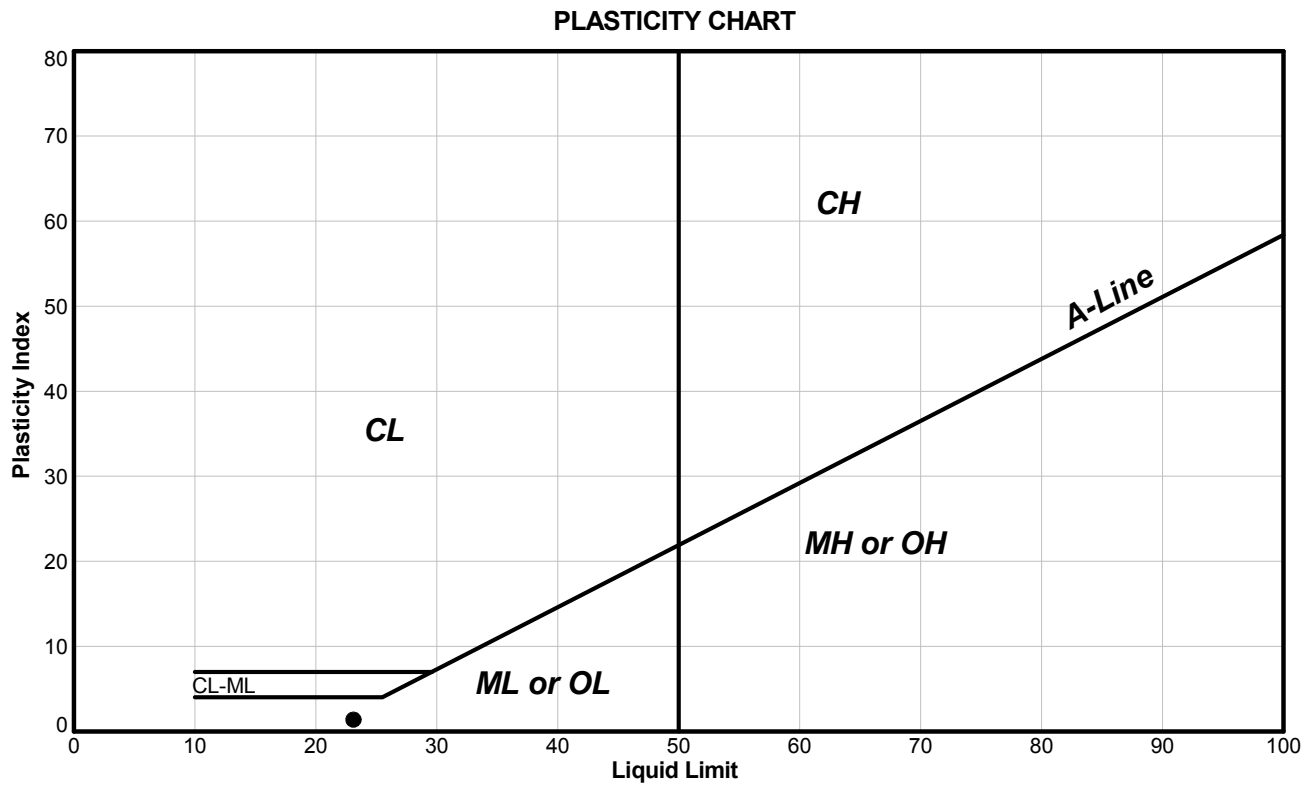
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-85
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-85	1	0.60	0.80	64	23	22	1	16.6	-5.4

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	28/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-88
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	44
Liquid Limit	26
Plastic Limit	19
Plasticity Index	7
Natural Water Content (%)	12.7
Liquidity Index	-0.9



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	05/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

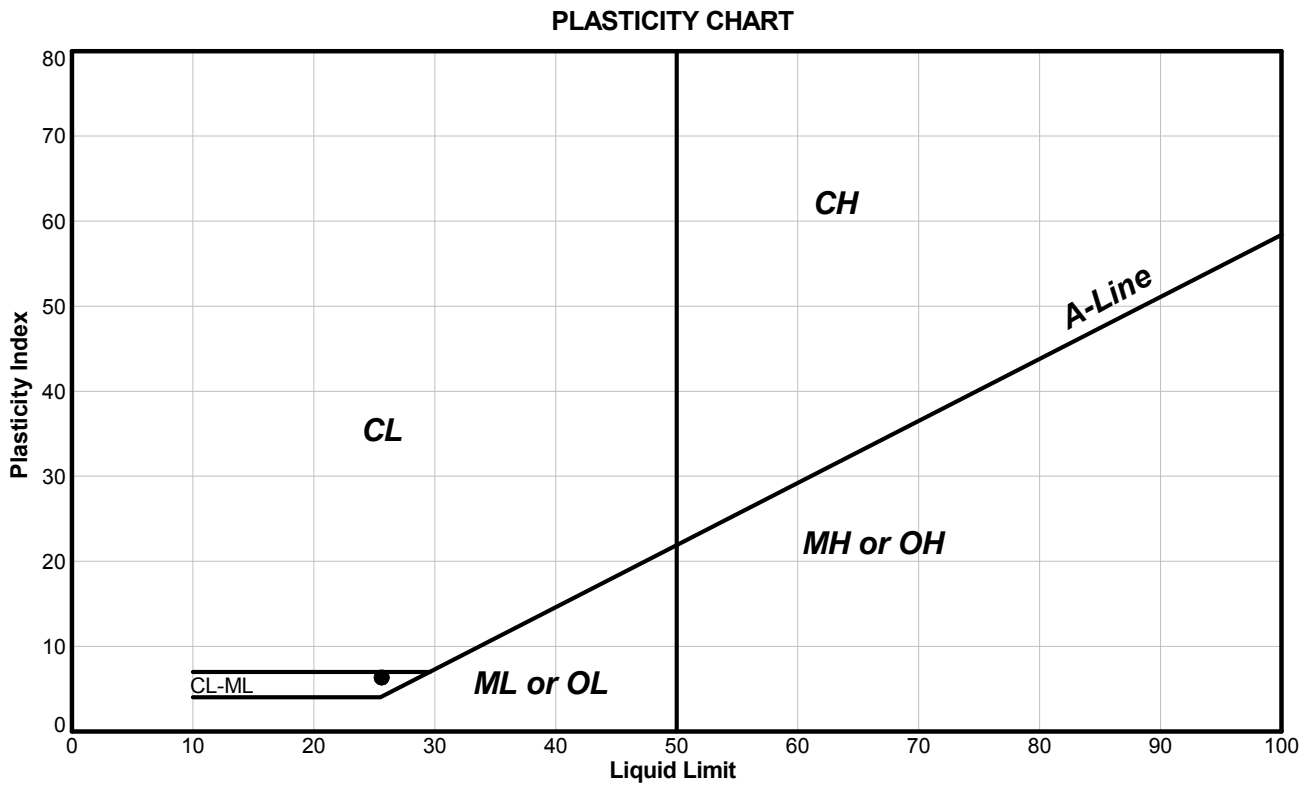
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-88
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.50 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: CL-ML - Inorganic clays of low to medium plasticity --- Inorganic silts and very fine sands, clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-88	1	1.50	1.80	44	26	19	7	12.7	-0.9

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	05/10/2011	LP	15/10/2011
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-89
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

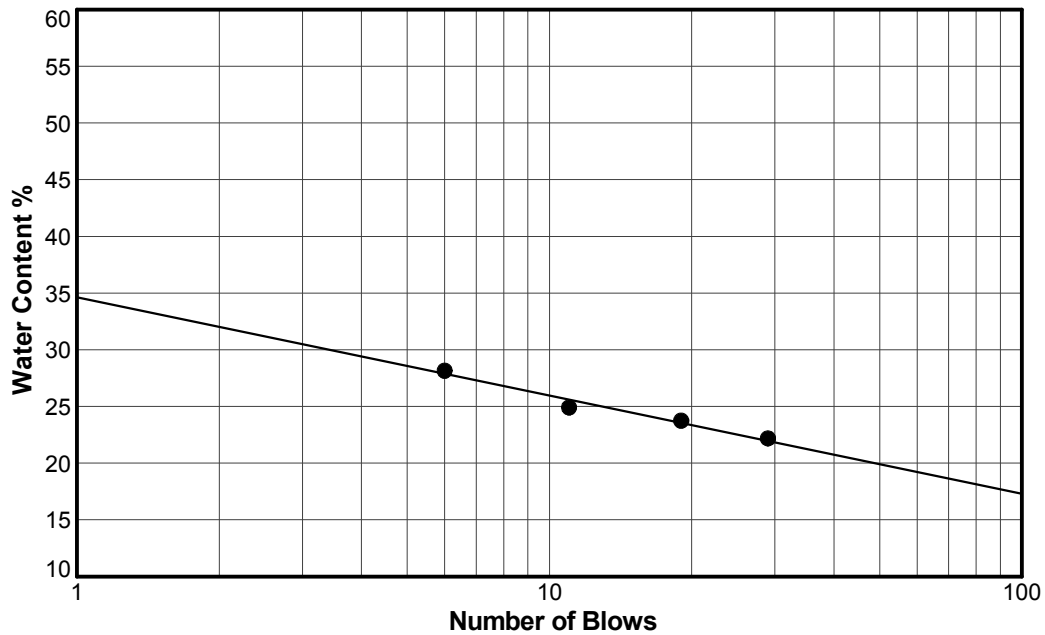
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	55
Liquid Limit	23
Plastic Limit	21
Plasticity Index	2
Natural Water Content (%)	14.3
Liquidity Index	-3.3



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	26/09/2011	LP	15/10/2011
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

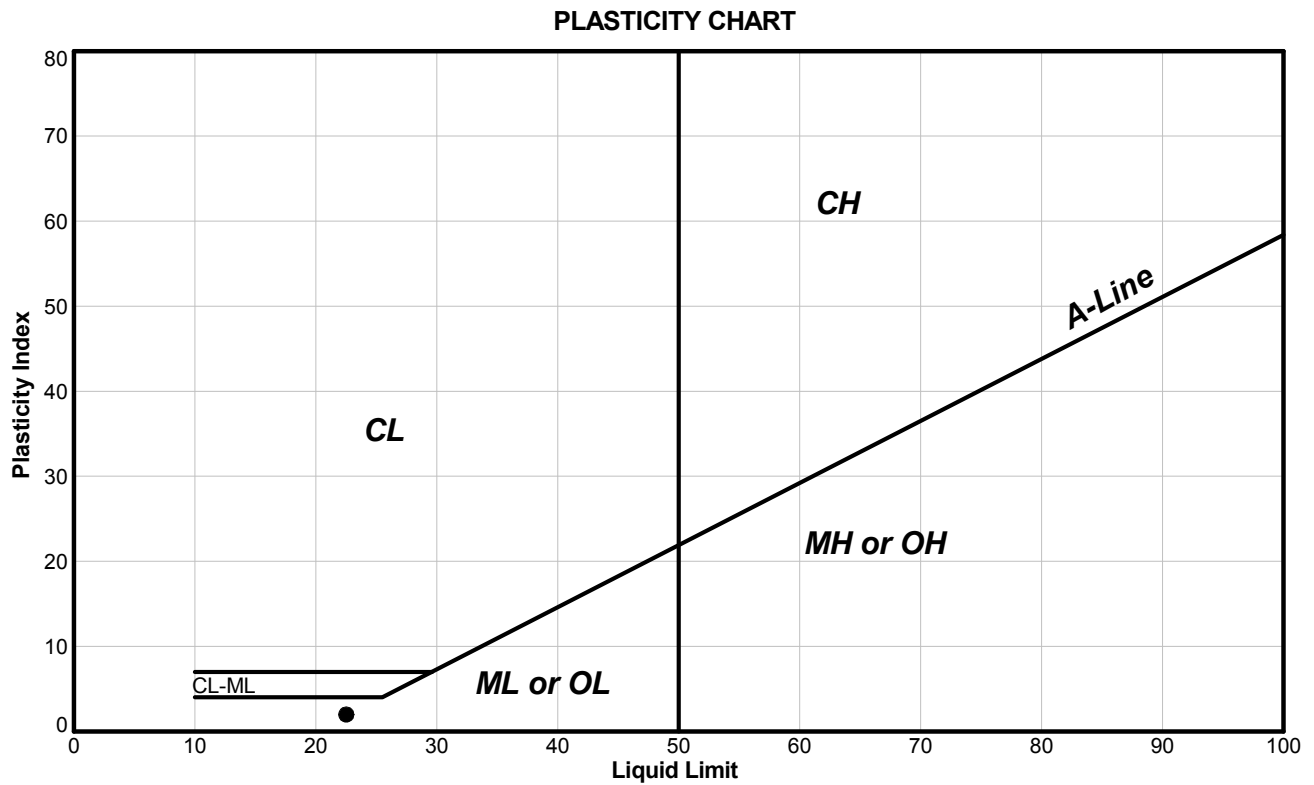
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-89
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-89	2	1.30	1.50	55	23	21	2	14.3	-3.3

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	26/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-90
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.50 to 3.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	99
Liquid Limit	24
Plastic Limit	22
Plasticity Index	2
Natural Water Content (%)	23.3
Liquidity Index	0.7



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	08/09/2011	LP	15/10/2011
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-90
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 3.50 to 3.60
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

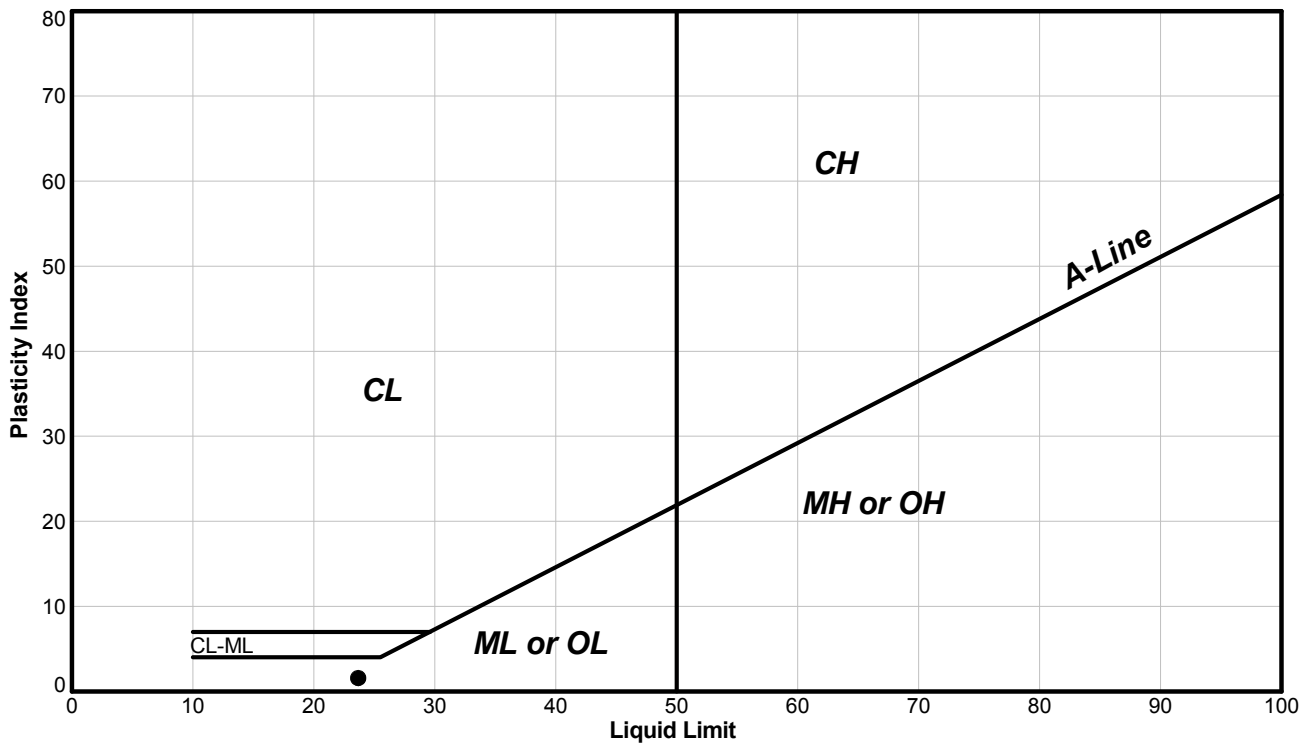
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

PLASTICITY CHART



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-90	2	3.50	3.60	99	24	22	2	23.3	0.7

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	08/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-91
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

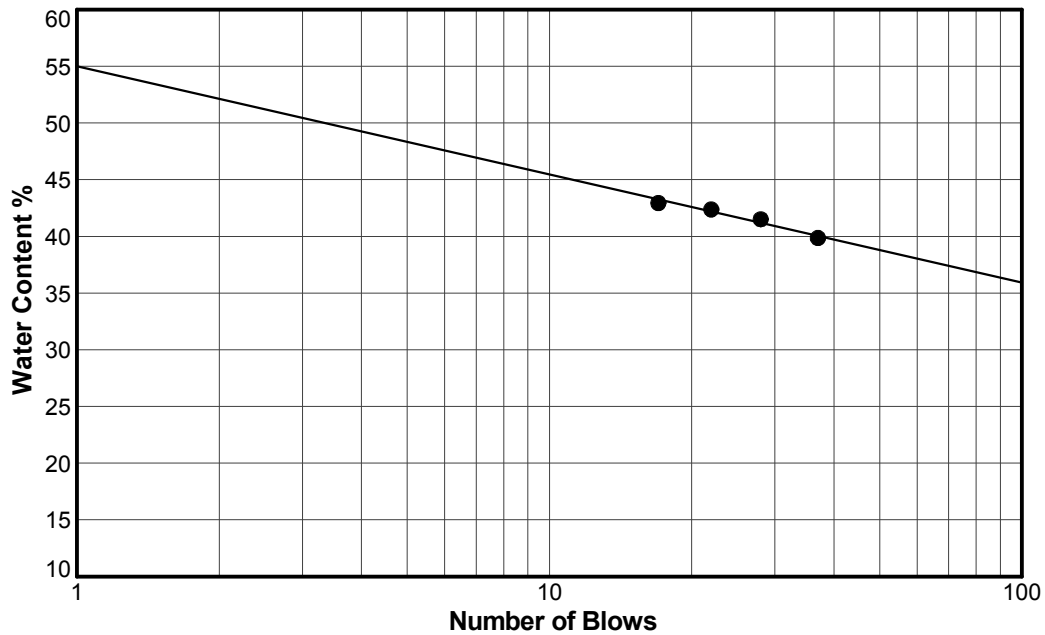
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	79
Liquid Limit	42
Plastic Limit	35
Plasticity Index	7
Natural Water Content (%)	49.5
Liquidity Index	2.1



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_G\RA\CHS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG LIMITS (REPORT) Template: BC REGION LIBRARY.GLB SKIN 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-91
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.80 to 1.00
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

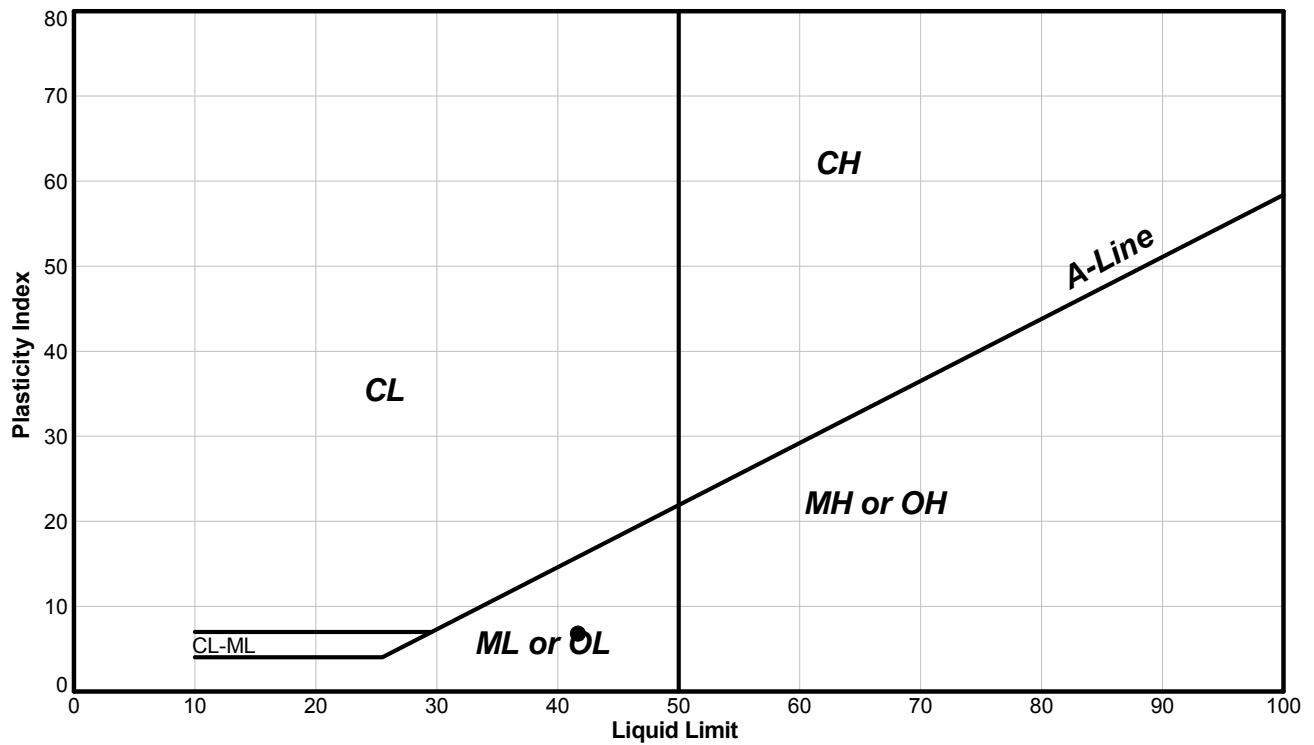
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

PLASTICITY CHART



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-91	1	0.80	1.00	79	42	35	7	49.5	2.1

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

		LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-91
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.60 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).

Other Remarks: N/A

Test Method: A-Multi Point **Preparation Method:** Wet

SUMMARY	
Percent Passing #40 Sieve (%)	23
Liquid Limit	NP
Plastic Limit	NP
Plasticity Index	NP
Natural Water Content (%)	6.1
Liquidity Index	NP



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	28/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

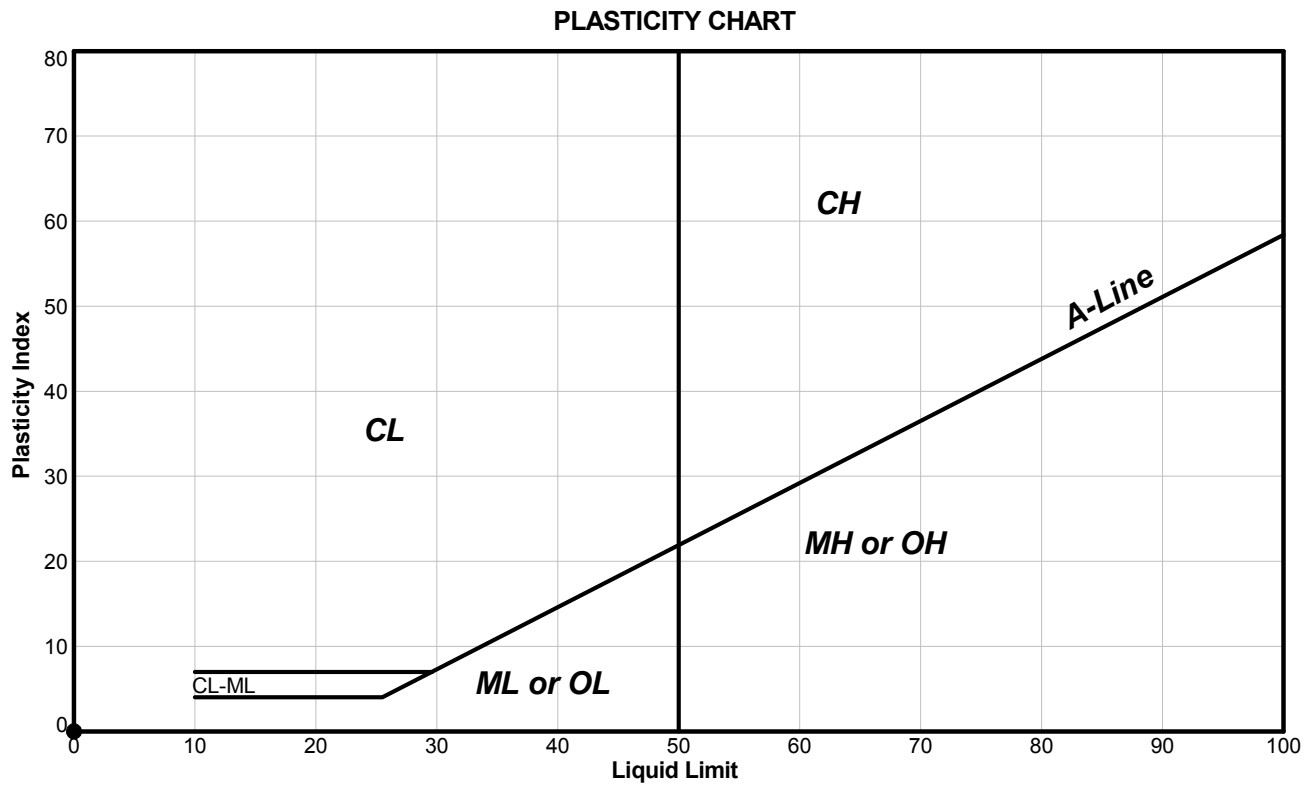
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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-91
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.60 to 1.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: Non-Plastic Soil (NP).	
Other Remarks: N/A	
Test Method: A-Multi Point	Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-91	2	1.60	1.80	23	NP	NP	NP	6.1	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	28/09/2011	LP	15/10/2011
Tech	Date	Checked	Date

File: N:\BUR_GRAFHCS\PROJECTS\2011\1415\0029\PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-92
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	97
Liquid Limit	45
Plastic Limit	42
Plasticity Index	3
Natural Water Content (%)	63.6
Liquidity Index	7.2



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	05/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

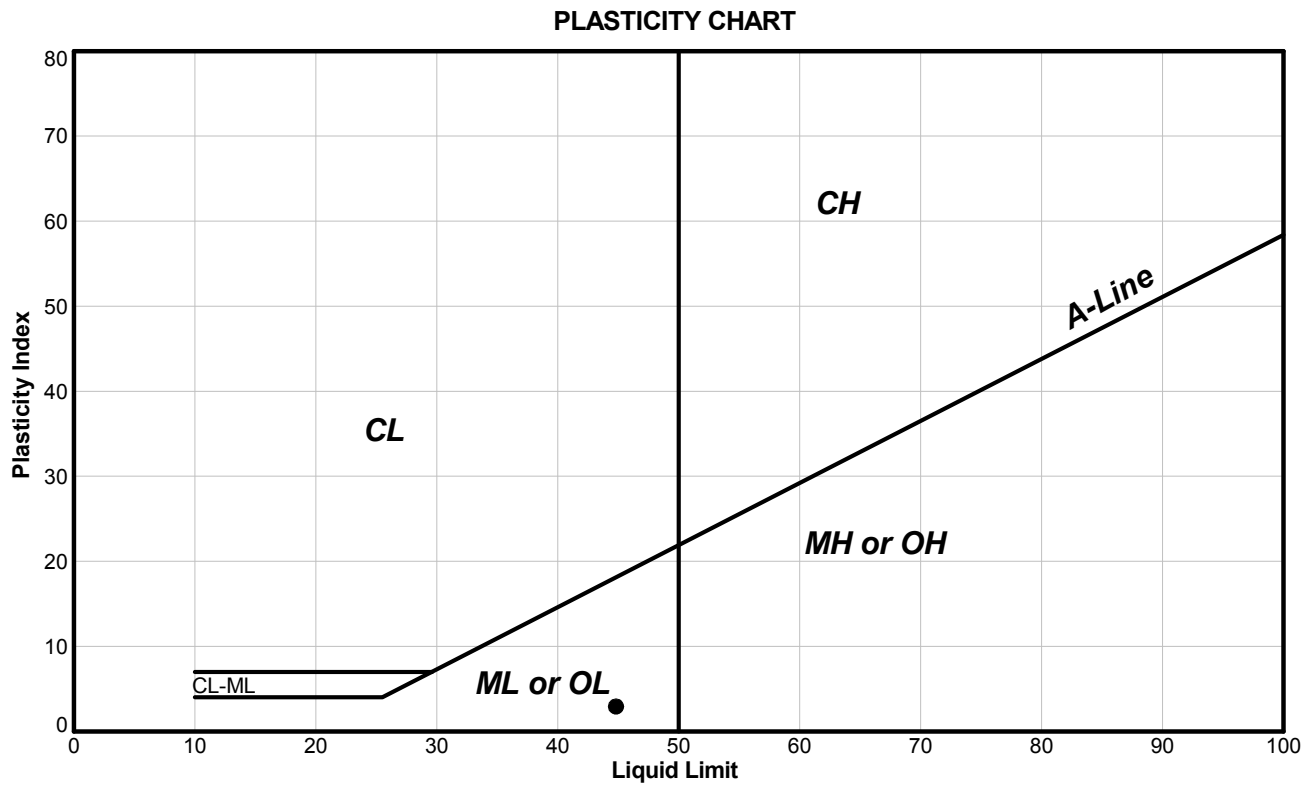
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-92
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 0.60 to 0.80
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-92	1	0.60	0.80	97	45	42	3	63.6	7.2

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	05/10/2011	LP	15/10/2011
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File N:\BUR_GRAFHCS\PROJECTS\2011\1415\11-1415-0029 PHASE 7000\11-1415-0029 EAGLE GOLD.GPJ Output Form: LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE.BETA.1.GDT Library: BC REGION LIBRARY.GLB Skin: 2/10/11

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-94
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.20 to 1.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	52
Liquid Limit	27
Plastic Limit	24
Plasticity Index	3
Natural Water Content (%)	16.4
Liquidity Index	-2.5



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

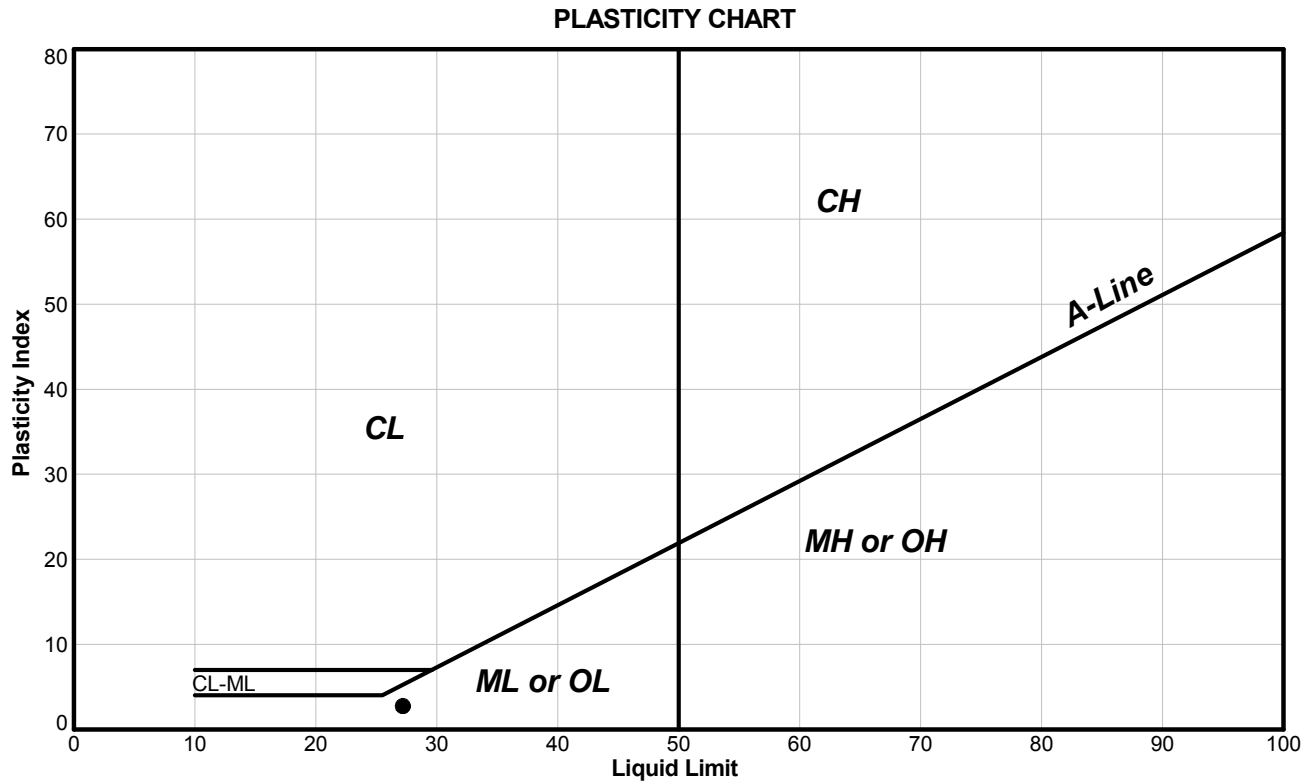
Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-94
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 1
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.20 to 1.30
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-94	1	1.20	1.30	52	27	24	3	16.4	-2.5

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	04/10/2011	LP	15/10/2011
Tech	Date	Checked	Date

LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
ASTM D 4318-10

Client: BGC (Proj 0792-006-04)	ID: TP-BGC11-96
Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

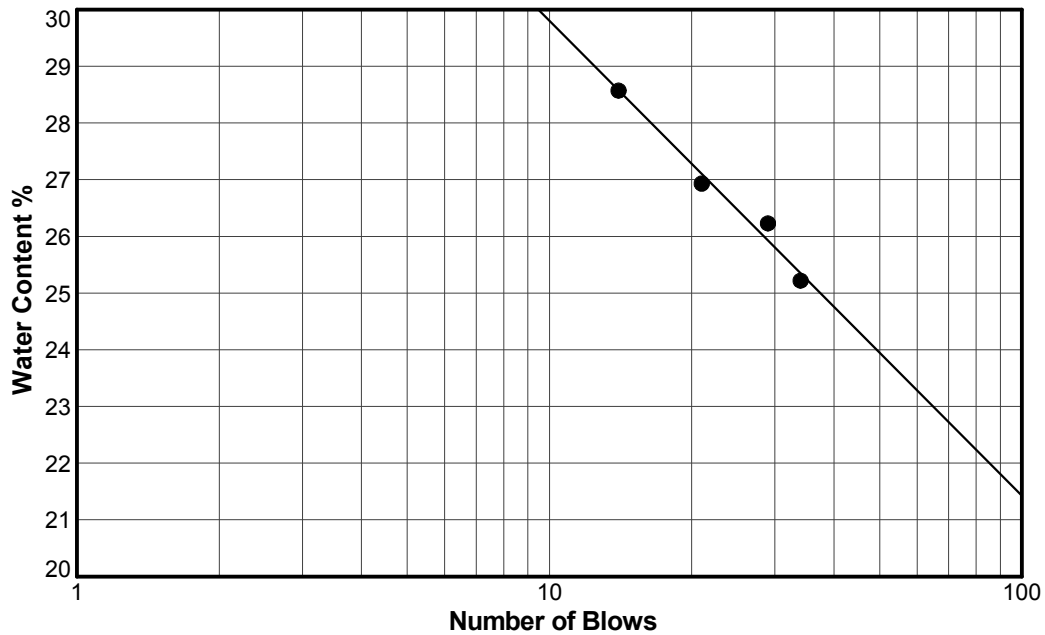
Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

SUMMARY	
Percent Passing #40 Sieve (%)	72
Liquid Limit	26
Plastic Limit	22
Plasticity Index	4
Natural Water Content (%)	19.8
Liquidity Index	-0.6



Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	03/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

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LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS

Reference(s)
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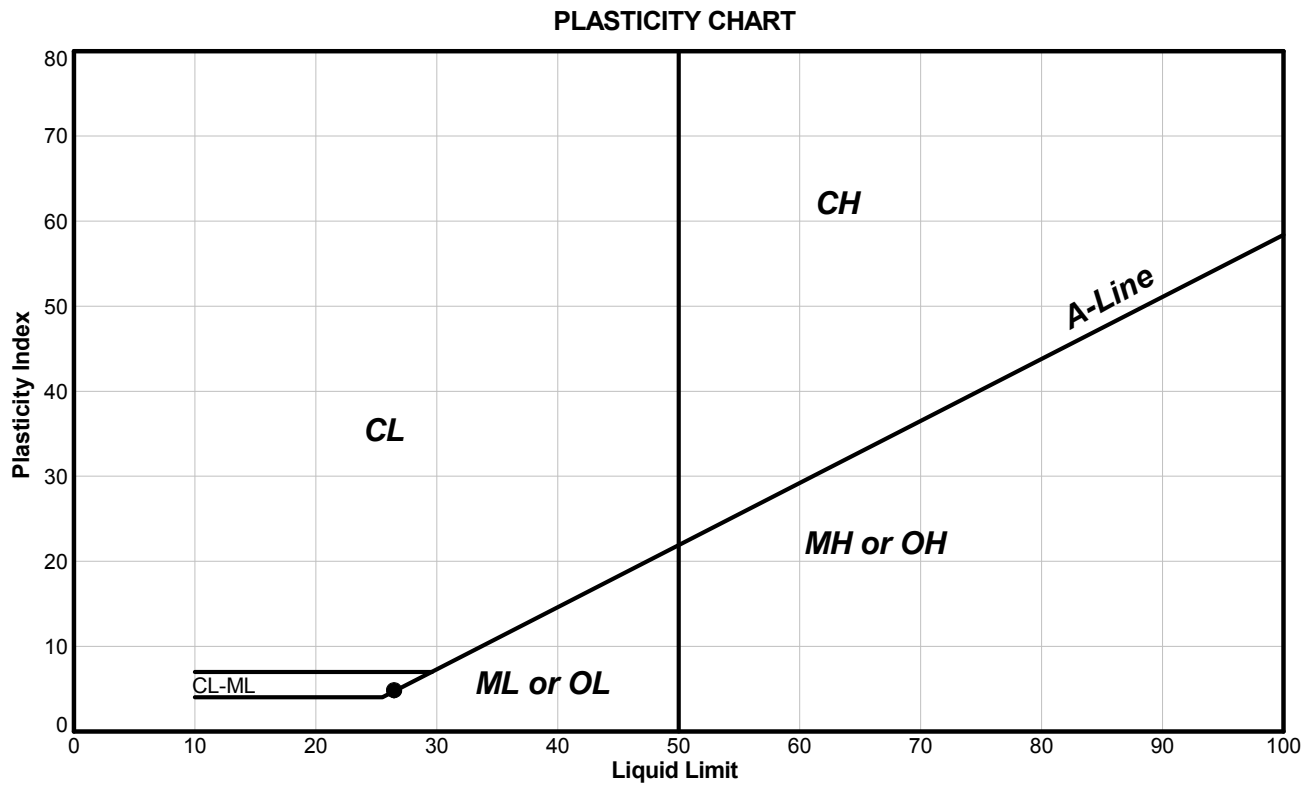
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Project: Eagle Gold Mine Site Infra FS SI	Sample No.: 2
Location: Dublin Gulch, Yukon	Depth Interval (m): 1.30 to 1.50
Project No.: 11-1415-0029 Phase: 7000	Lab Schedule No.: 141

Classification and Definition: ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.

Other Remarks: N/A

Test Method: A-Multi Point

Preparation Method: Wet

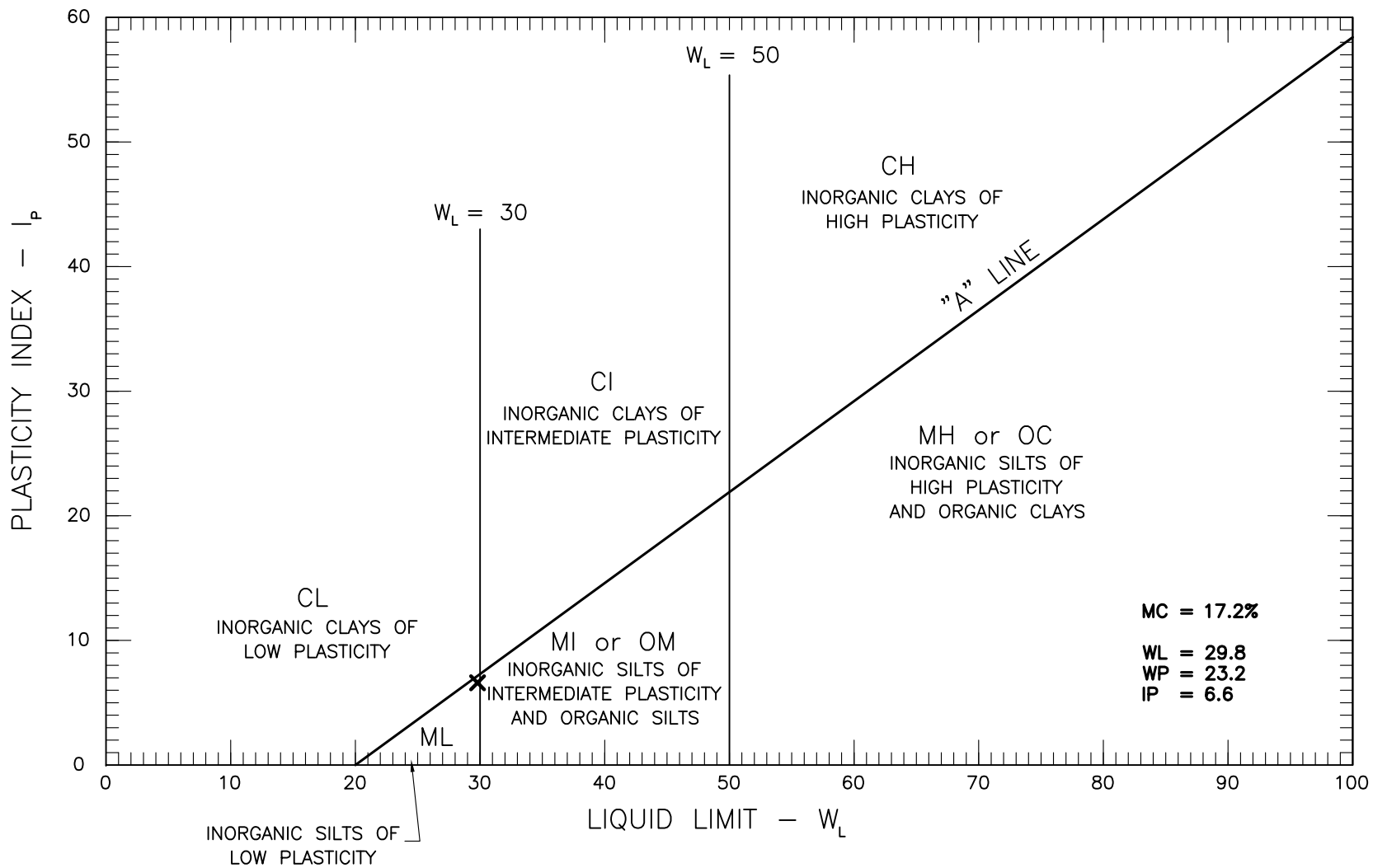


Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	TP-BGC11-96	2	1.30	1.50	72	26	22	4	19.8	-0.6

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	03/11/2011	LP	03/11/2011
Tech	Date	Checked	Date

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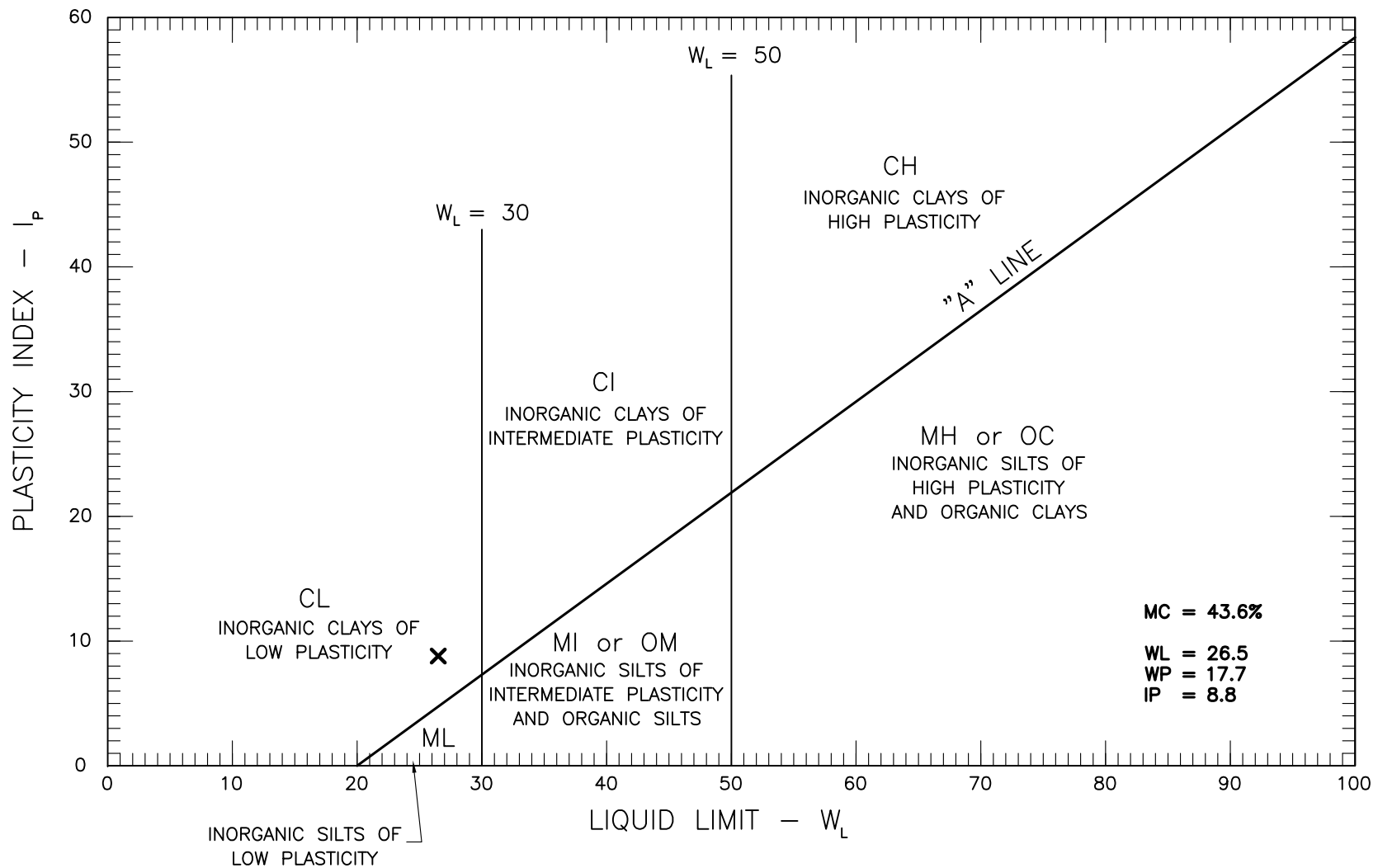
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-42-S2

PROJECT NO.

K-3300

PLATE NO.

3300-AL-BH42-S2



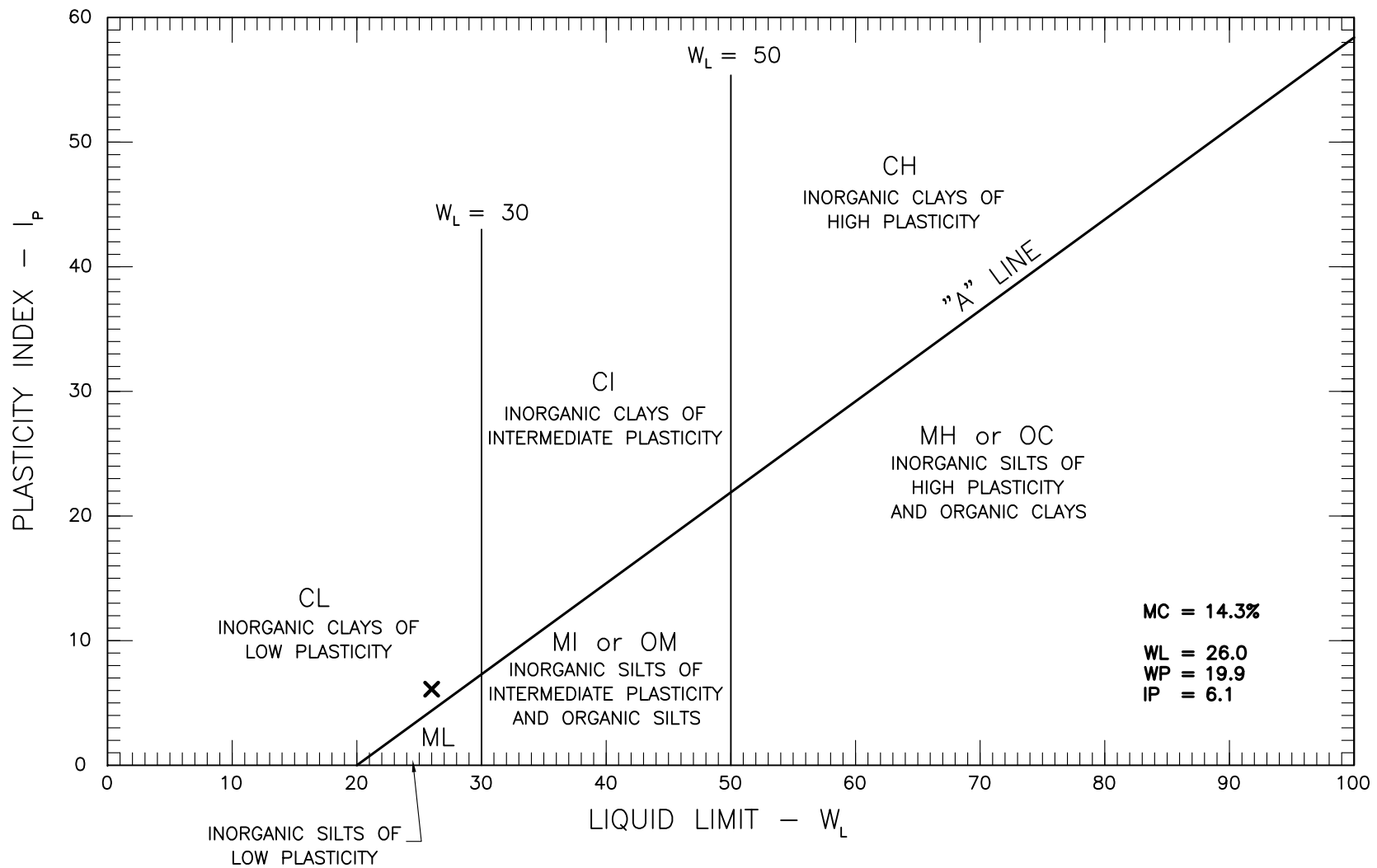
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PROJECT NO.
 K-3300

PLATE NO.
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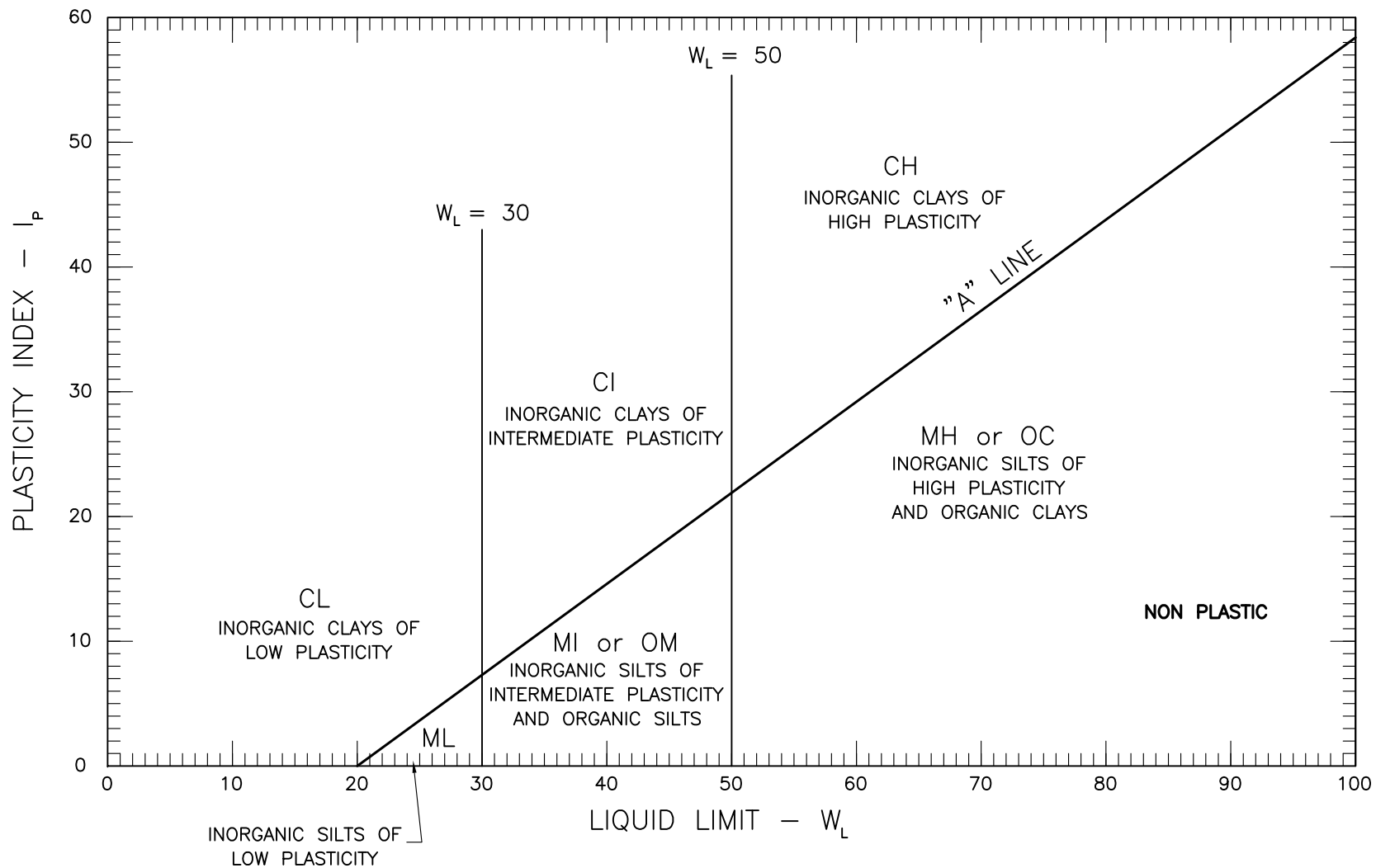
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PROJECT NO.
 K-3300

PLATE NO.
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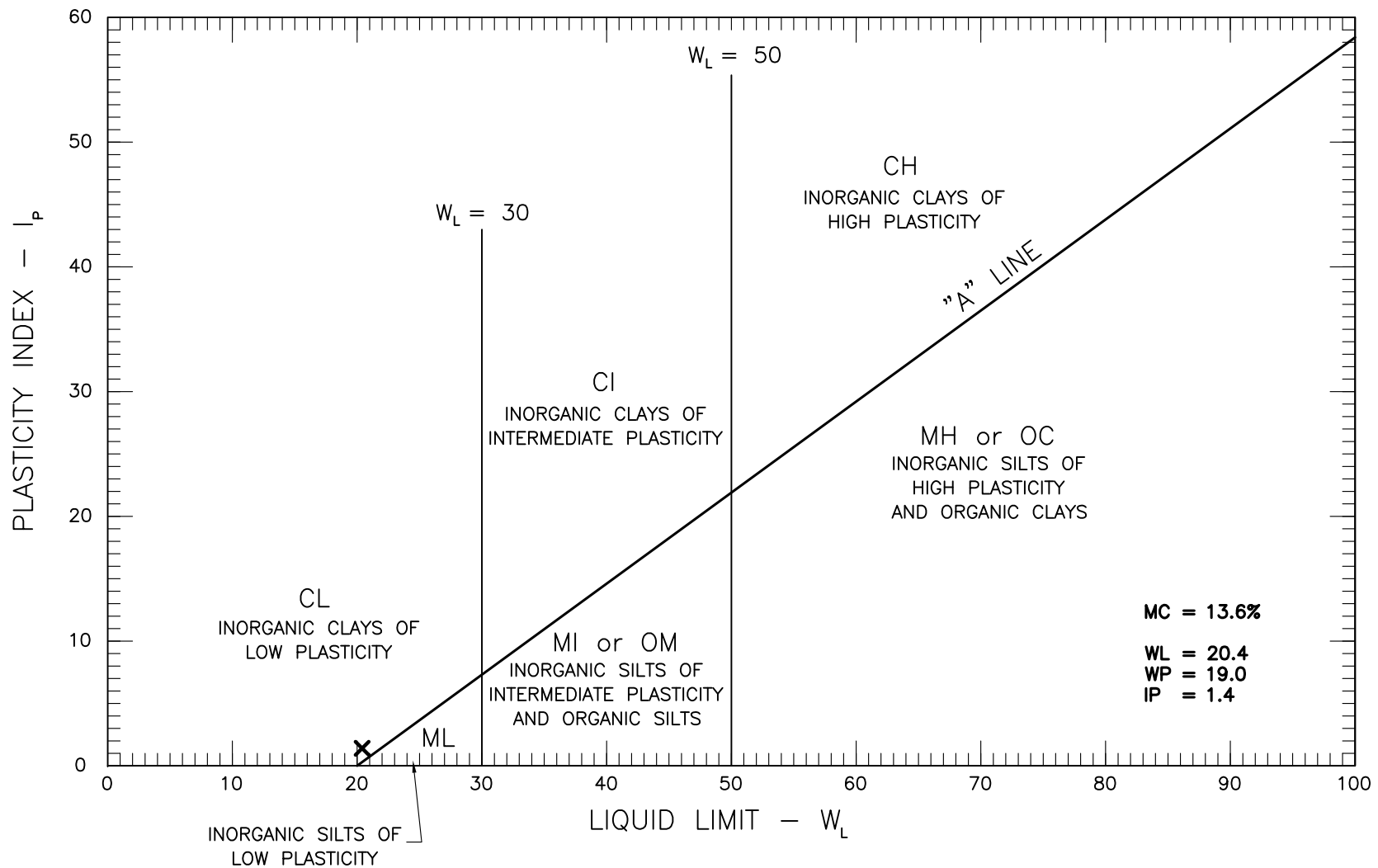
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PROJECT NO.
 K-3300

PLATE NO.
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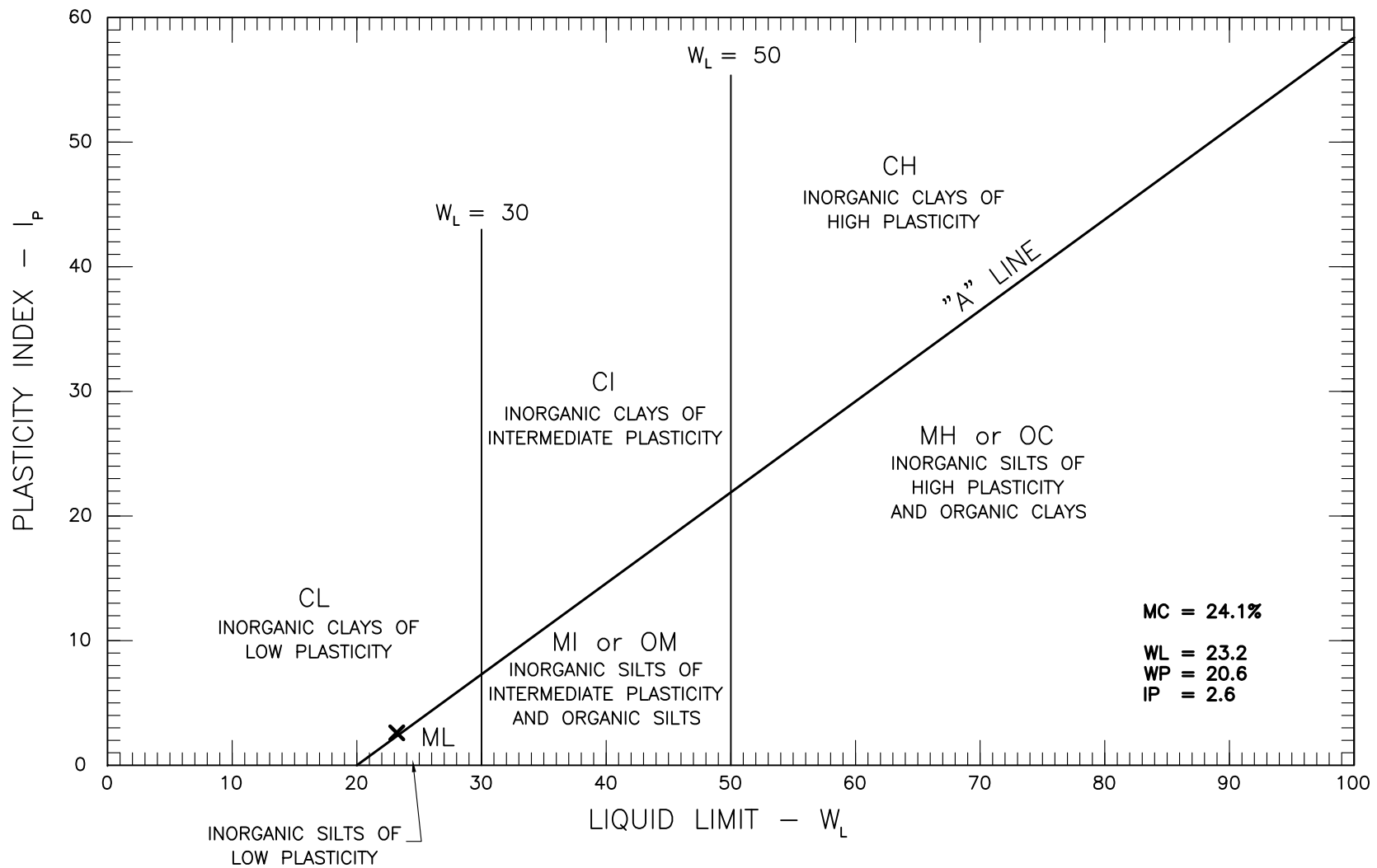
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PROJECT NO.
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PLATE NO.
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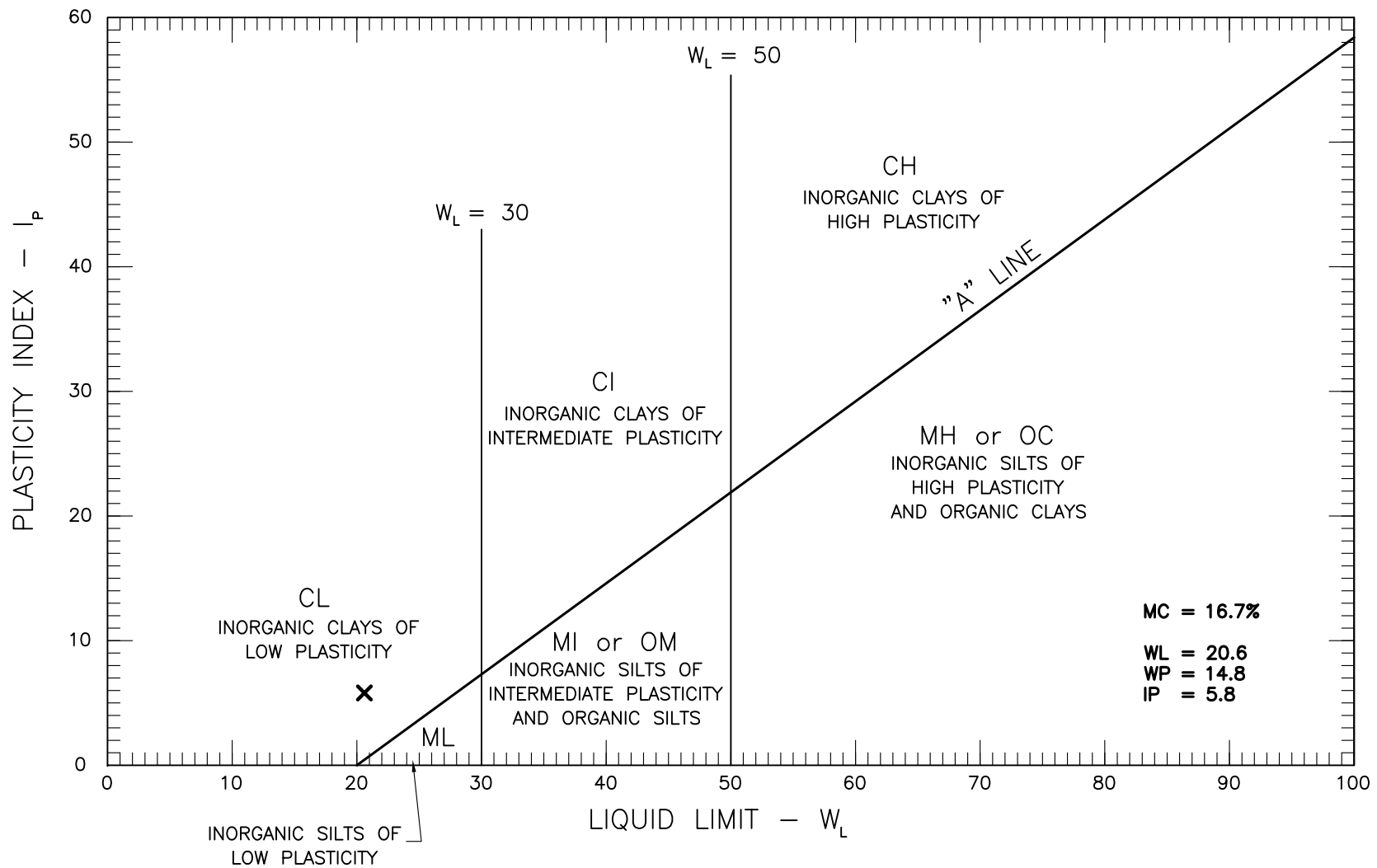
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PLATE NO.
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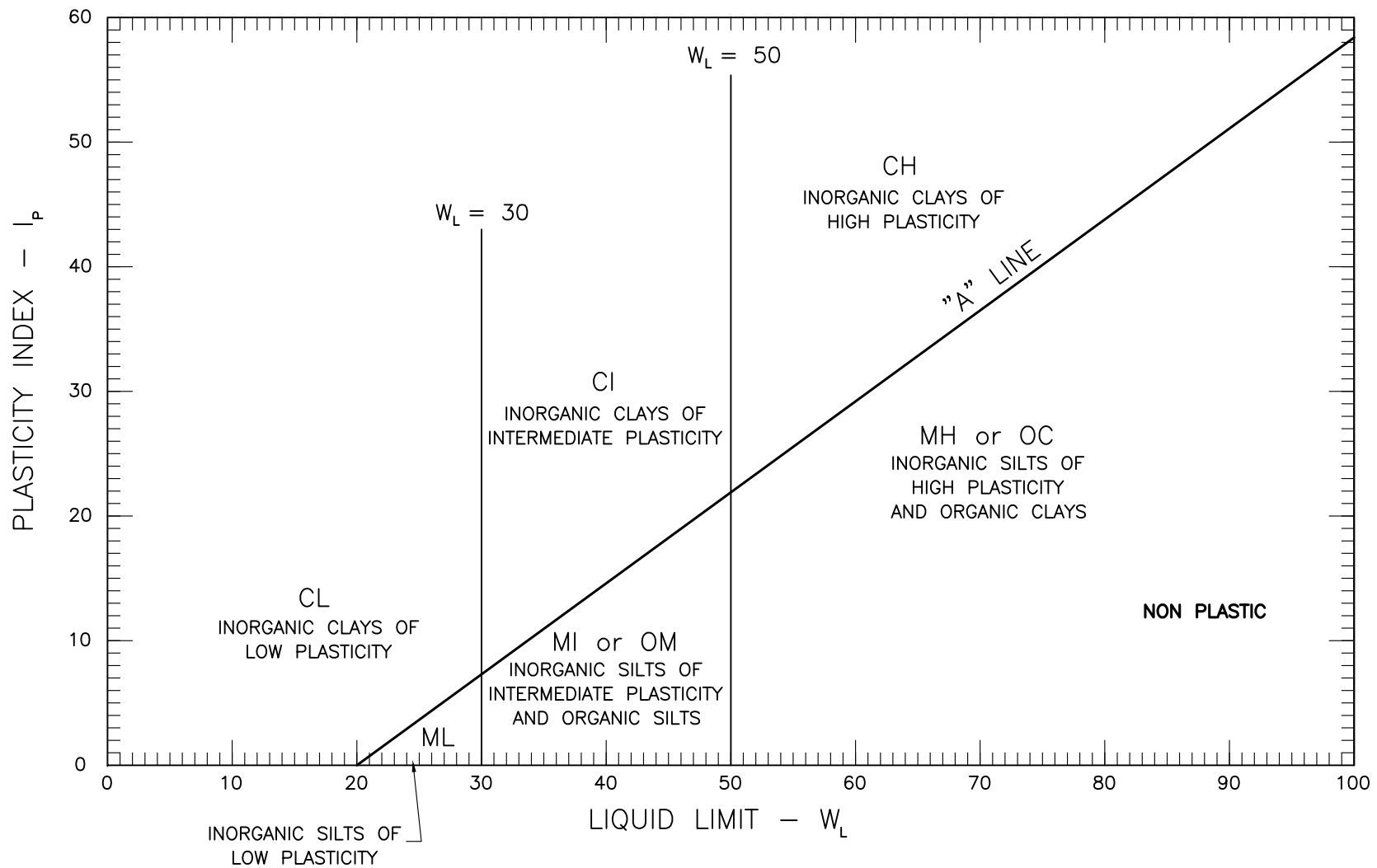
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PLATE NO.
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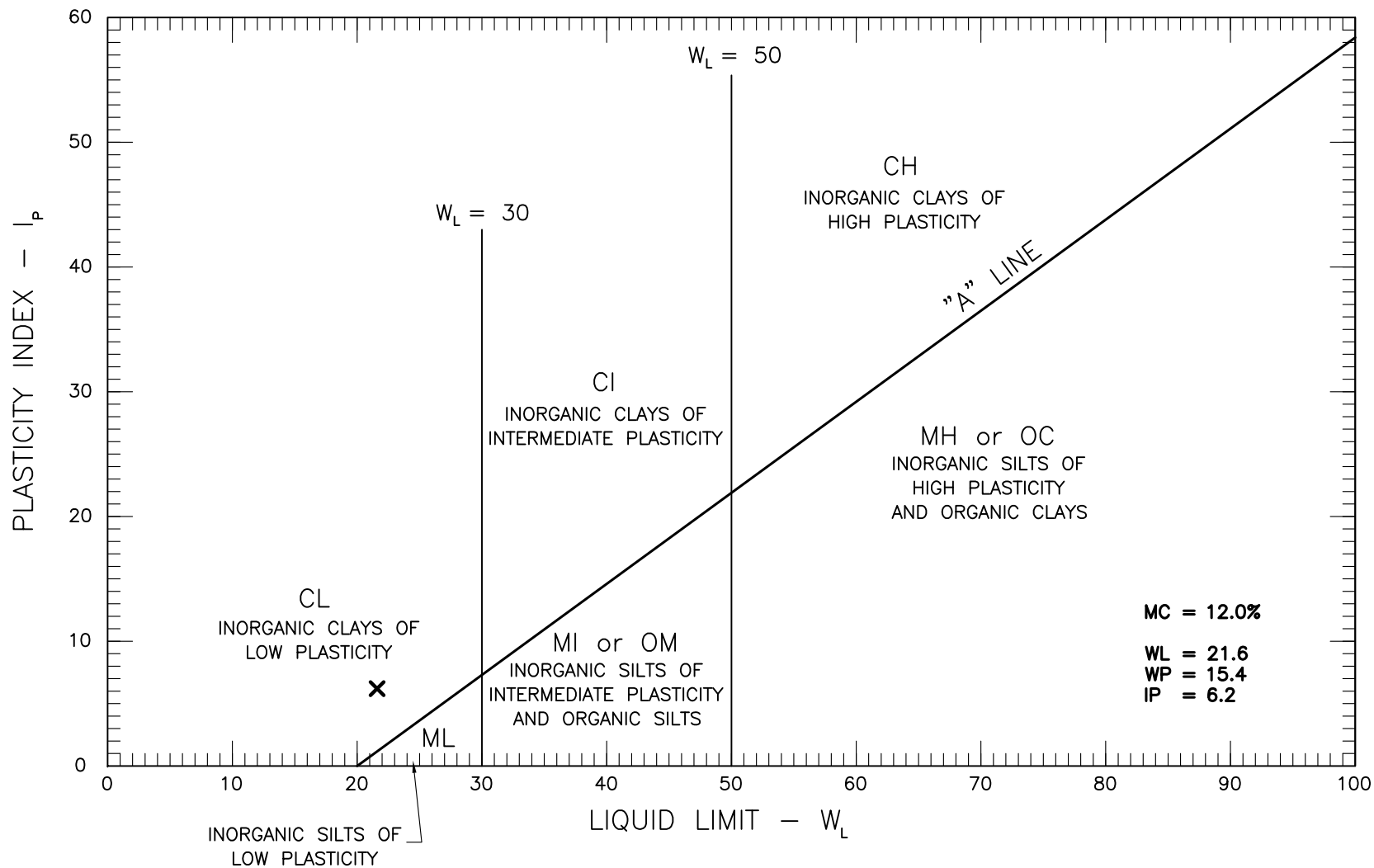
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PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH44-SPT2



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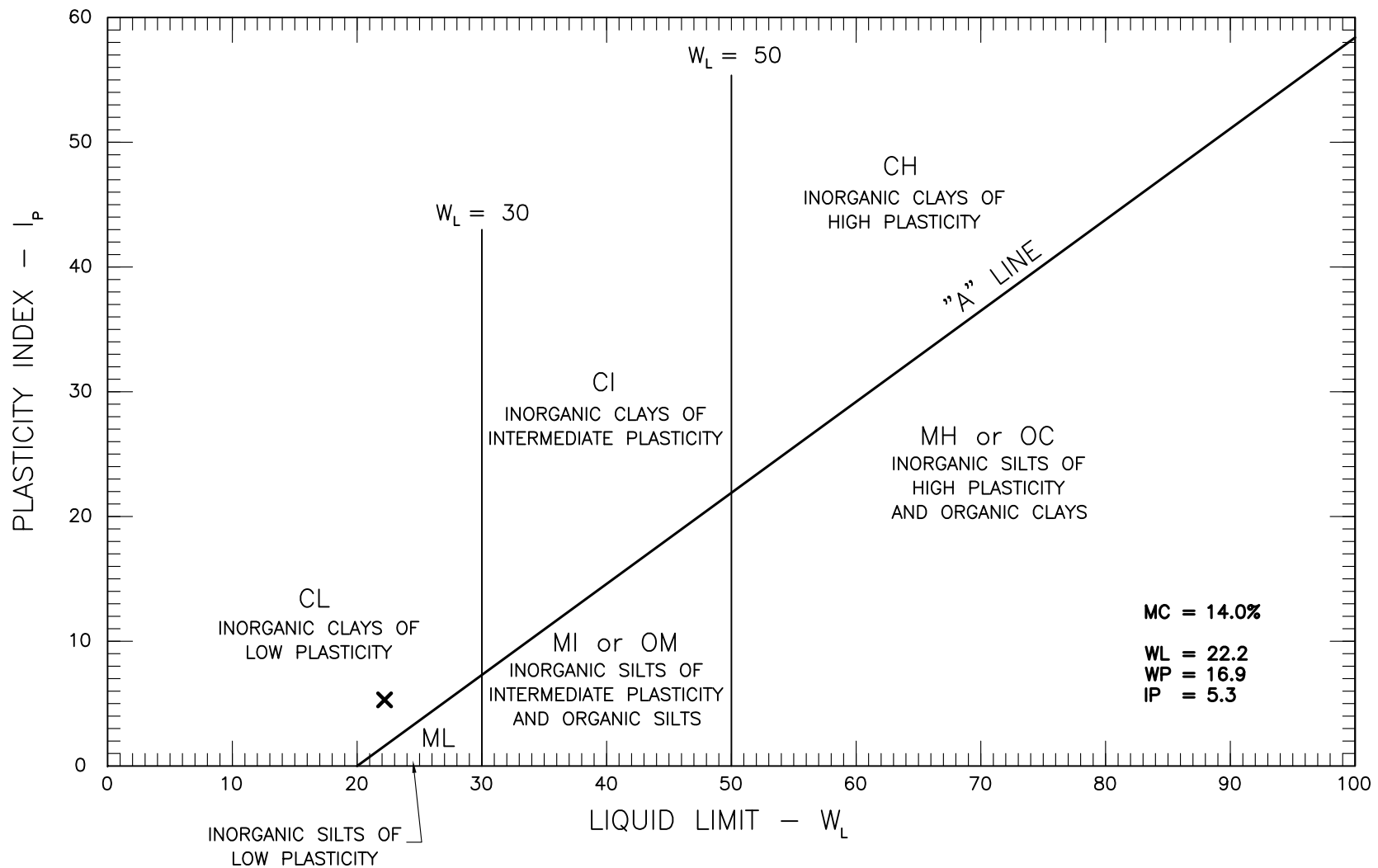
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PLATE NO.
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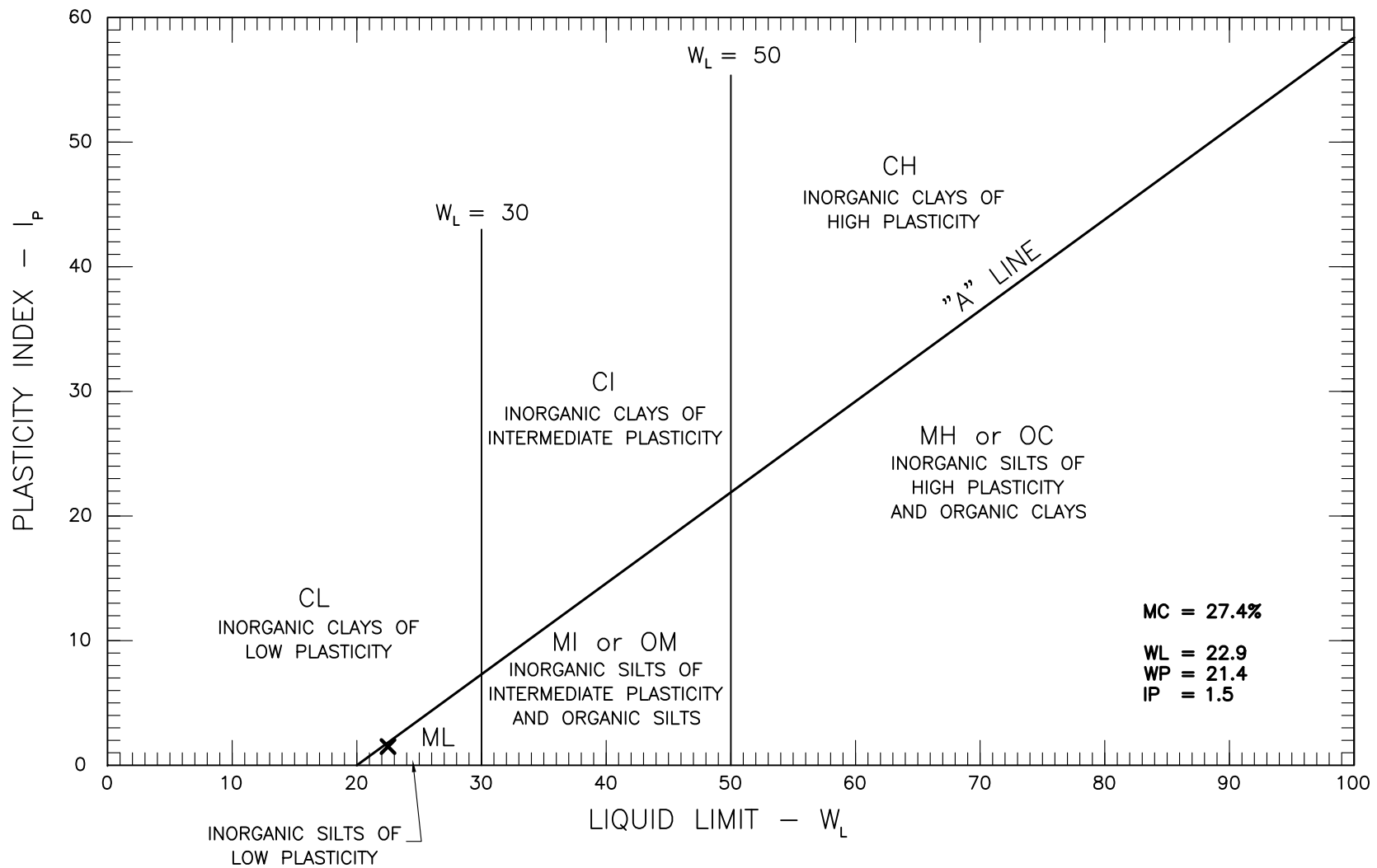
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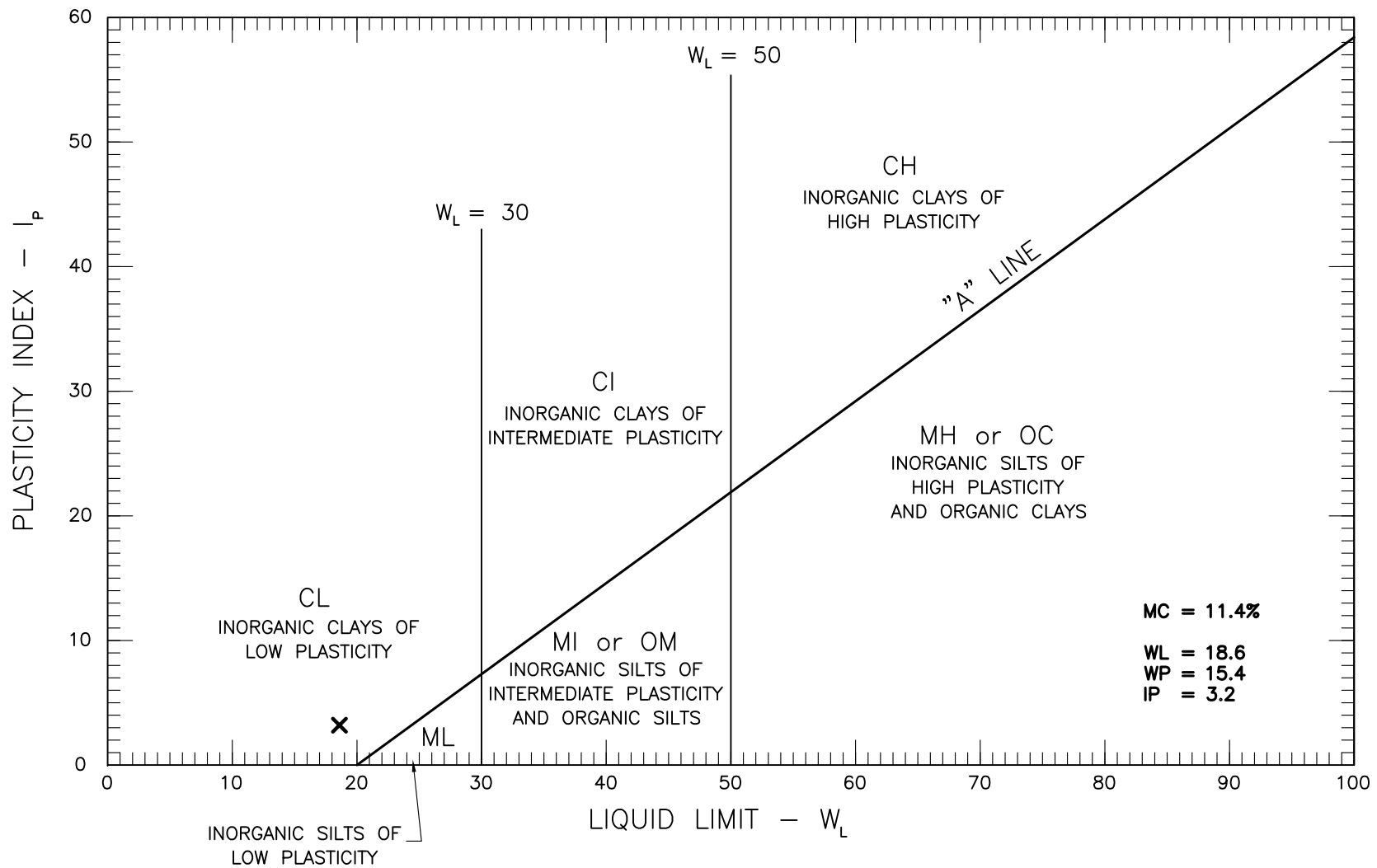
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PROJECT NO.
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PLATE NO.
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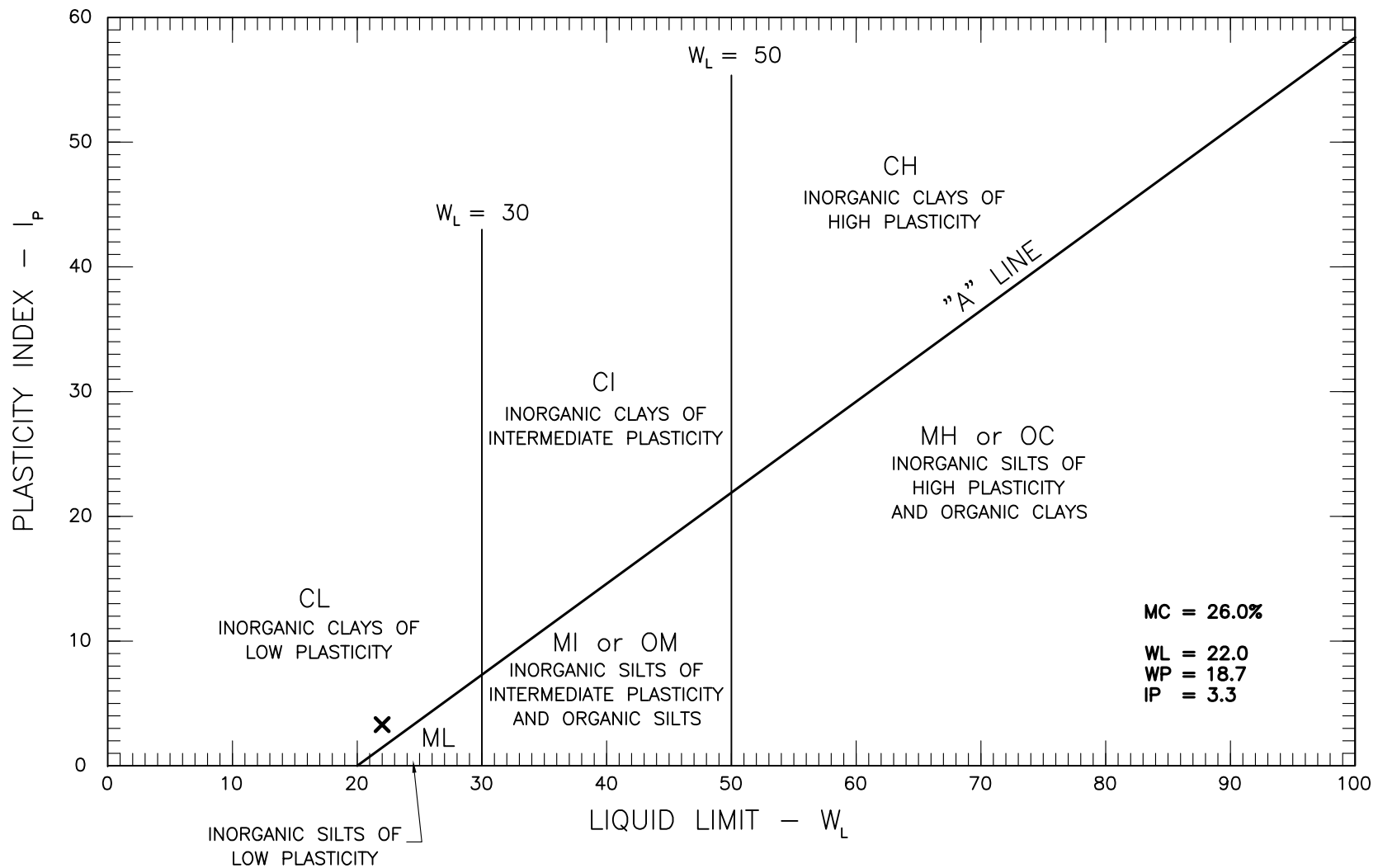
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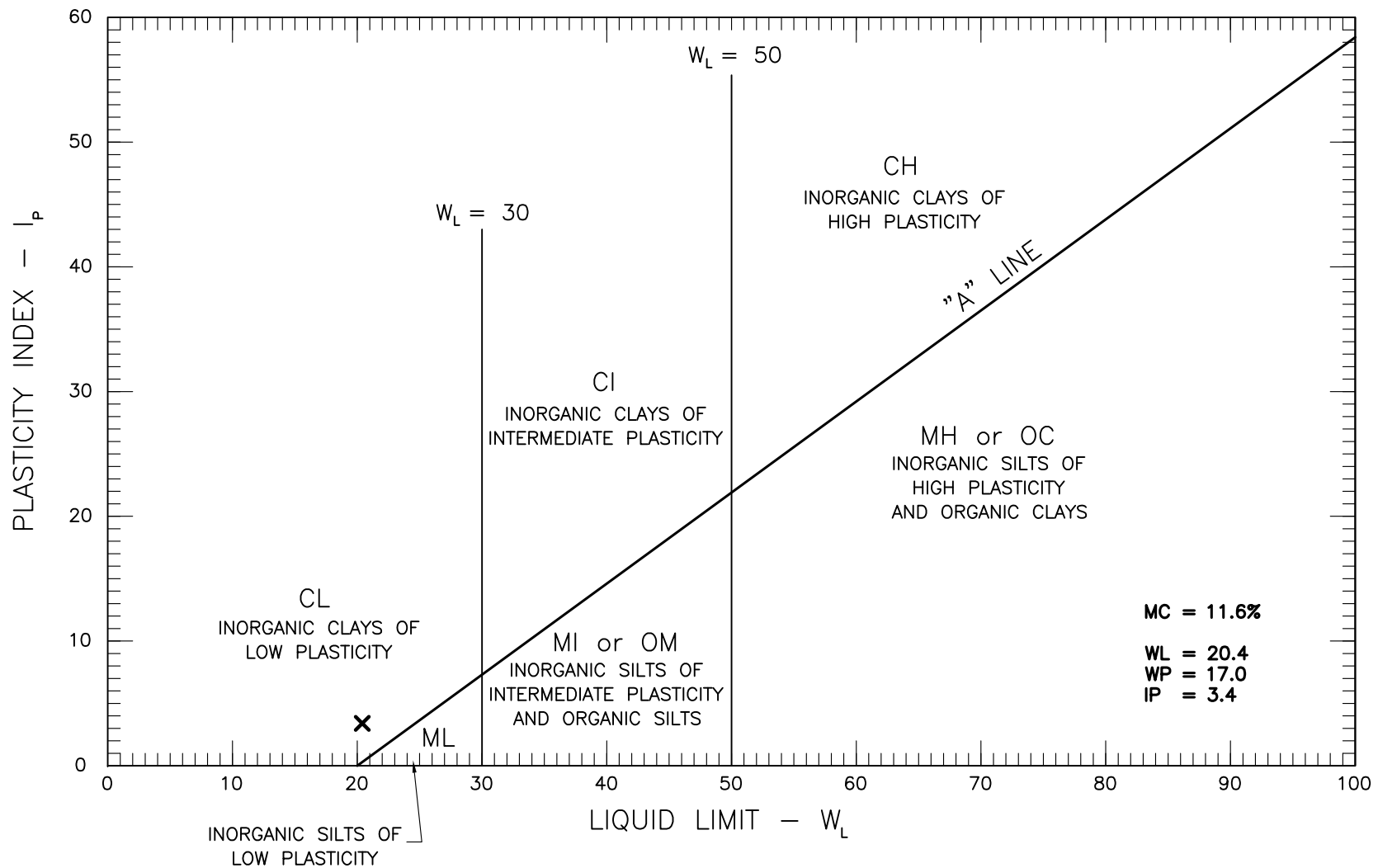
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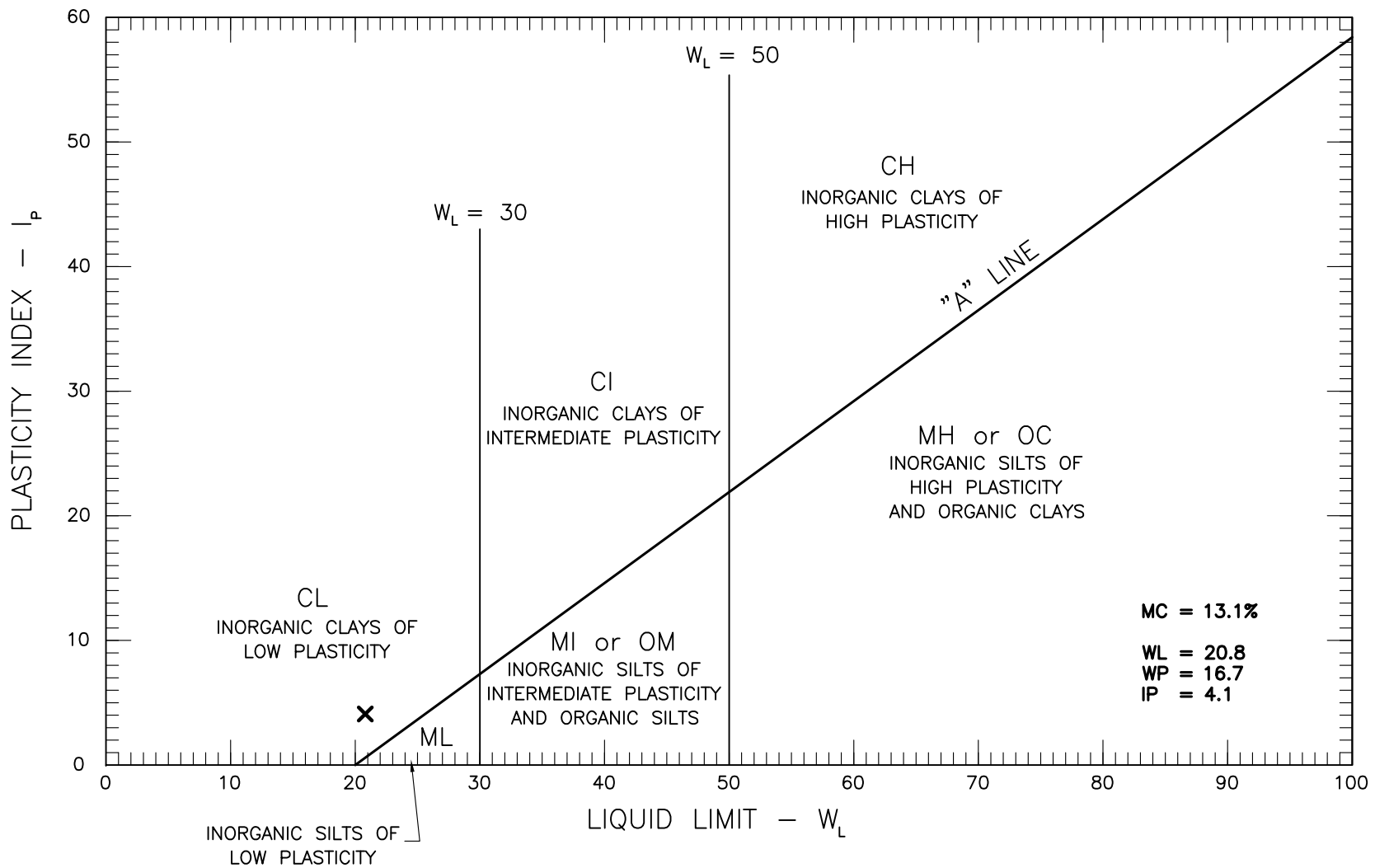
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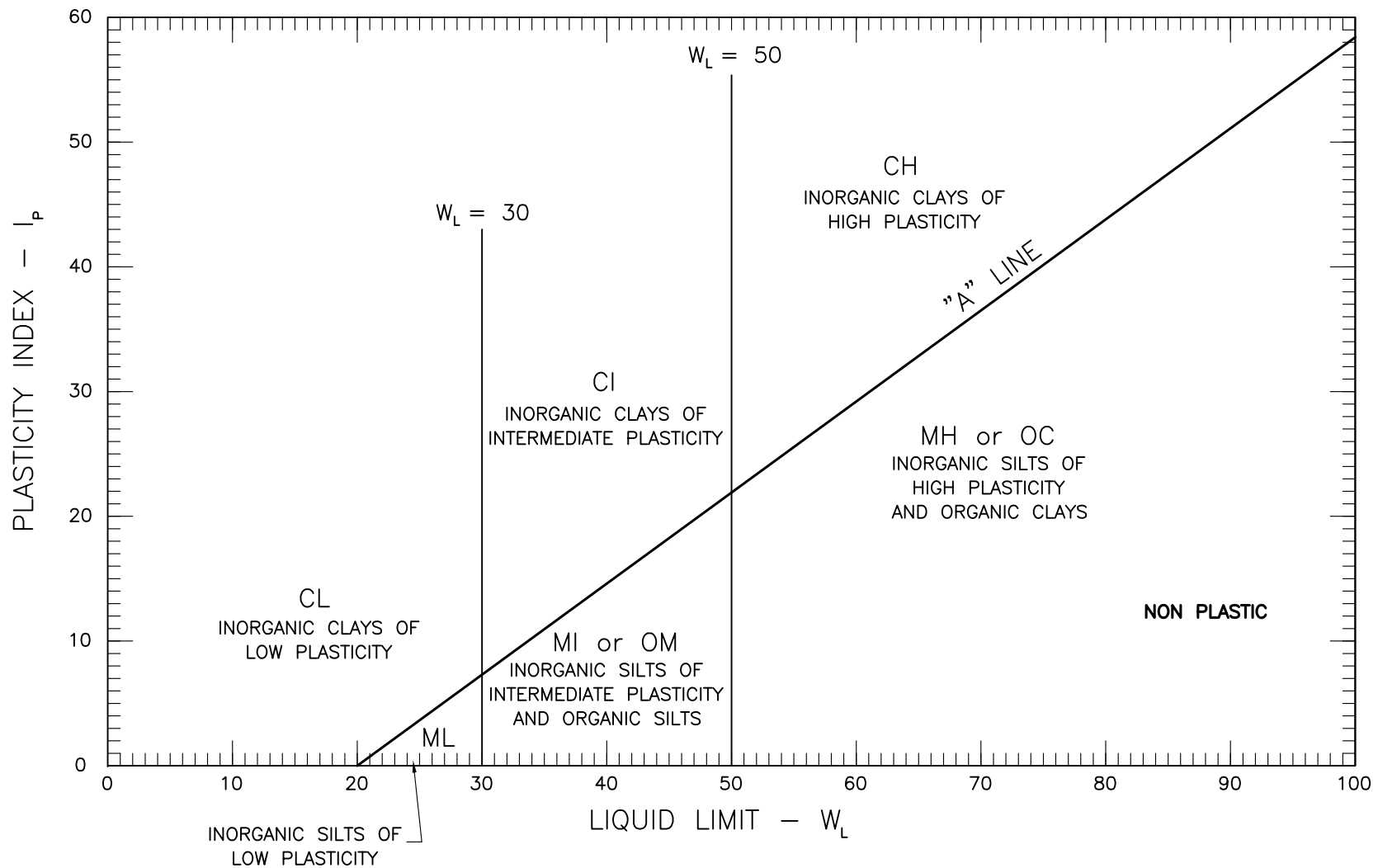
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PROJECT NO.
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PLATE NO.
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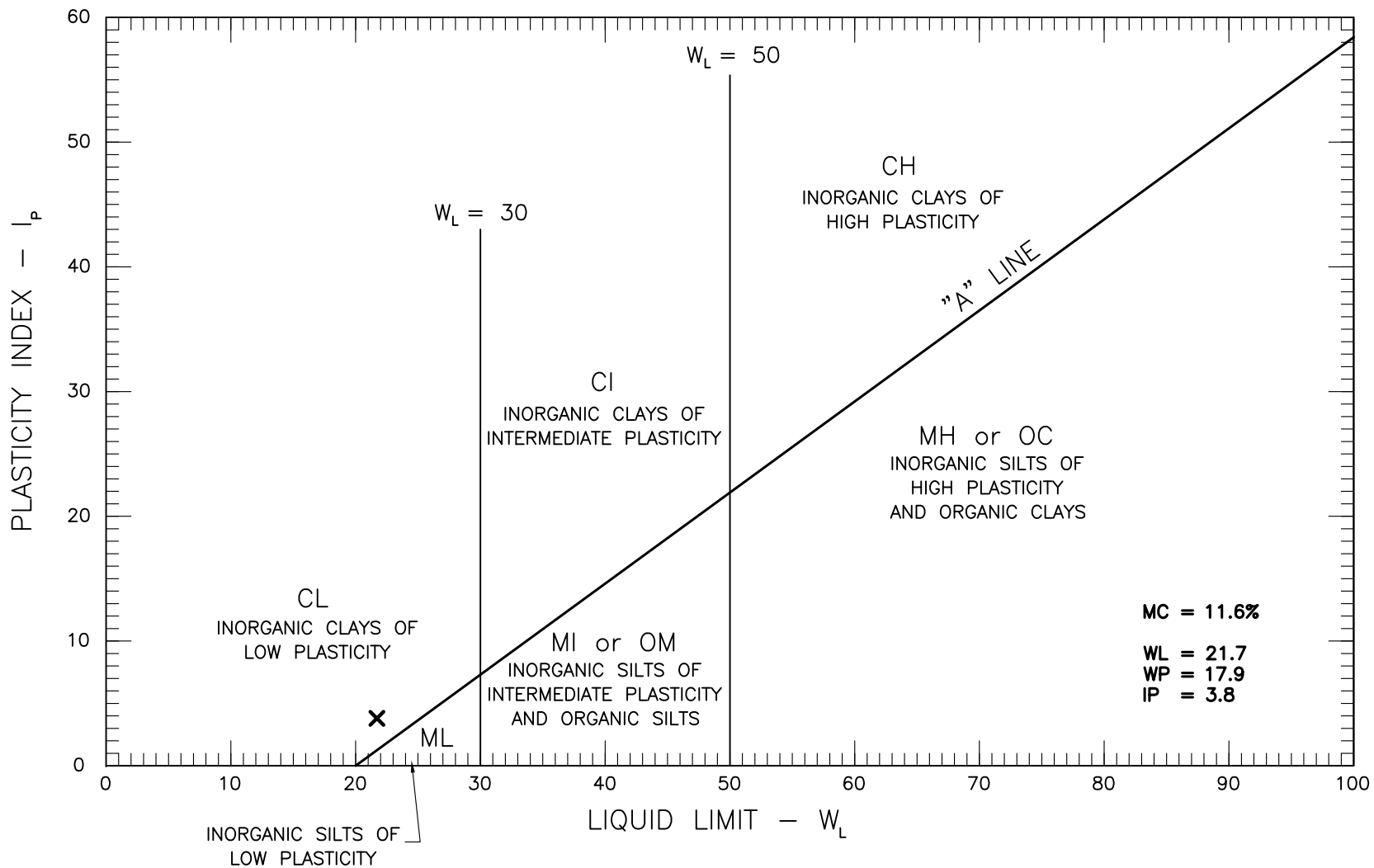
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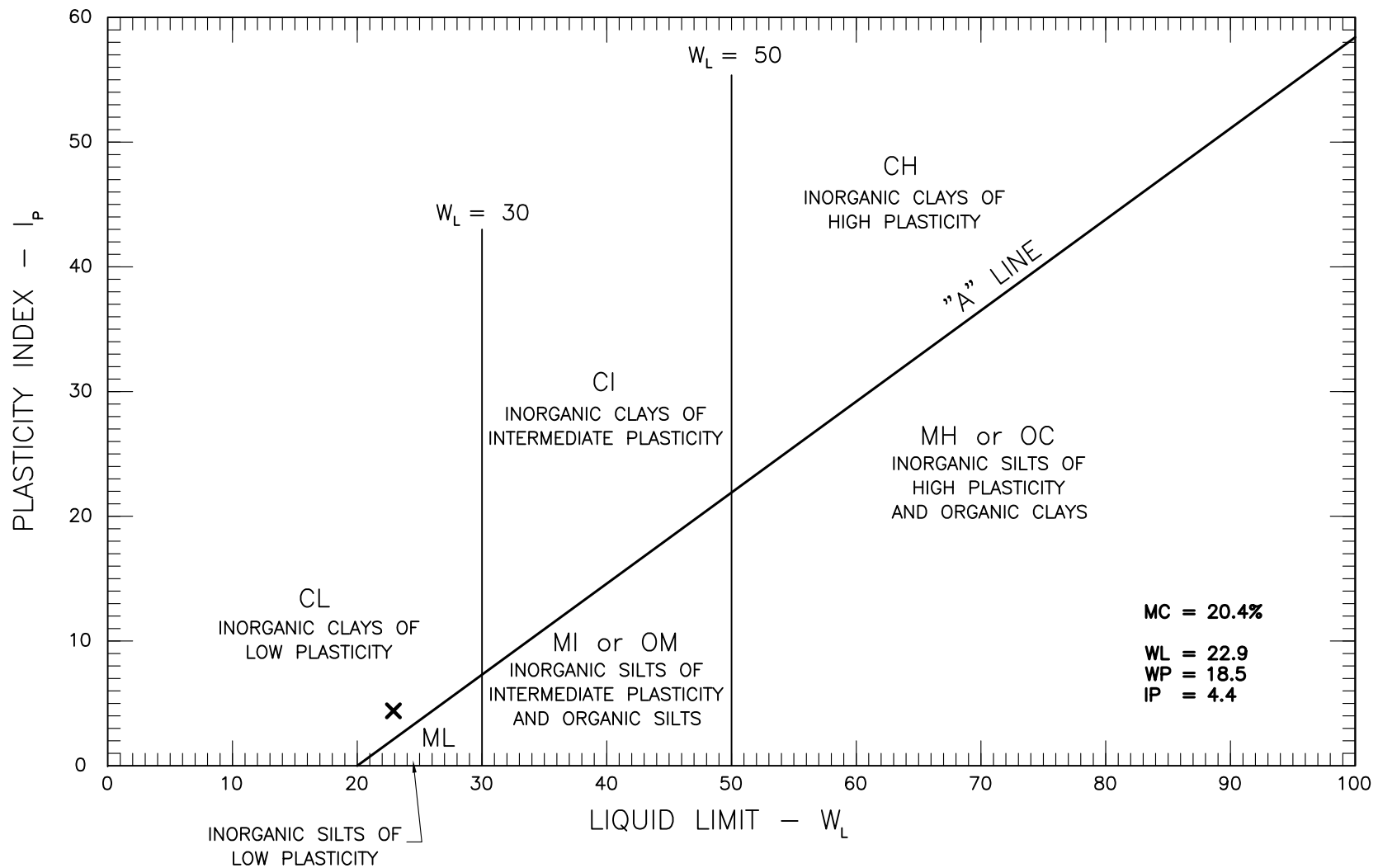
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PROJECT NO.
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PLATE NO.
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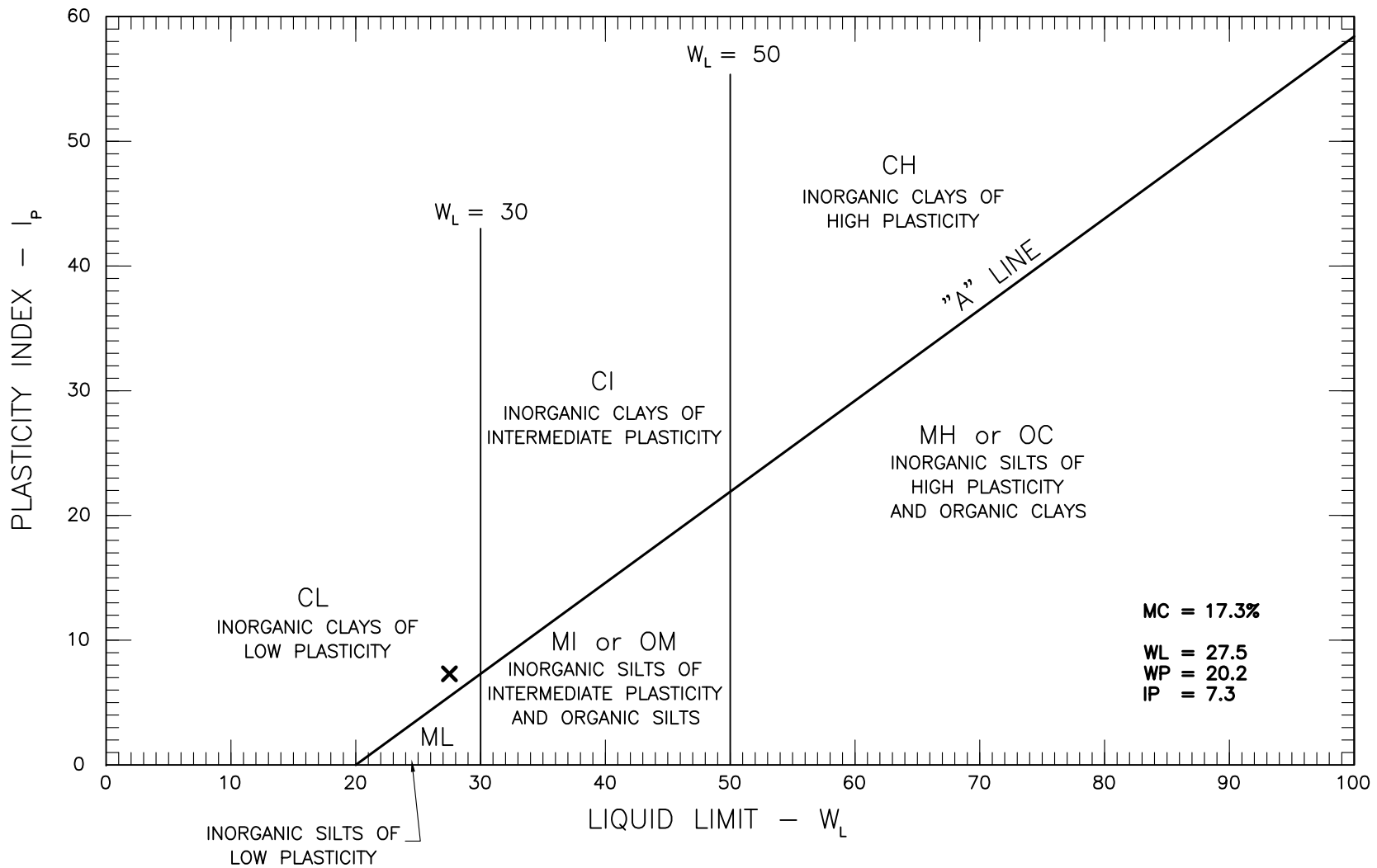
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ATTERBERG LIMITS OF BH-BGC11-49-SPT2

PROJECT NO.
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PLATE NO.
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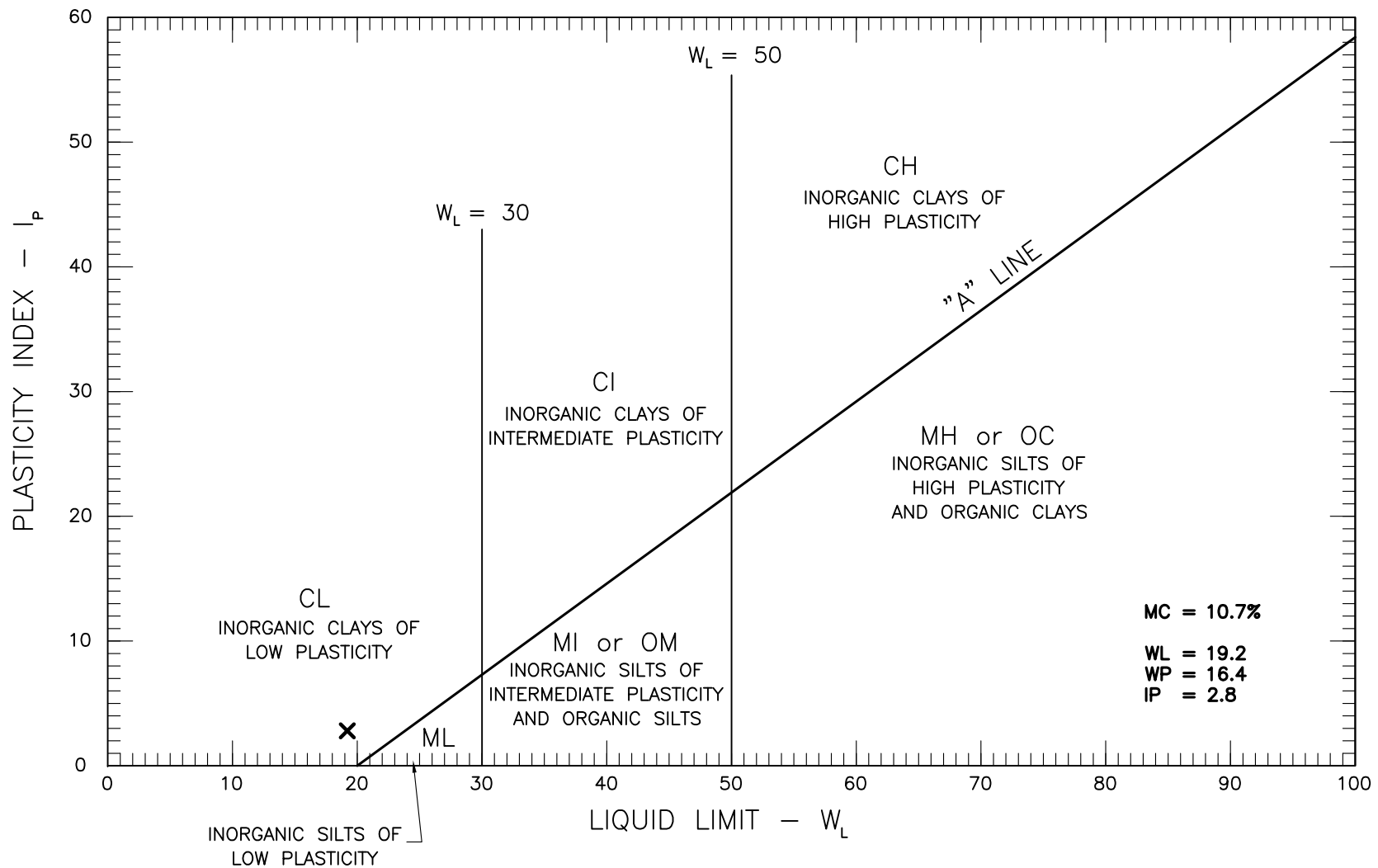
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 ATTERBERG LIMITS OF BH-BGC11-51-G6

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH51-G6



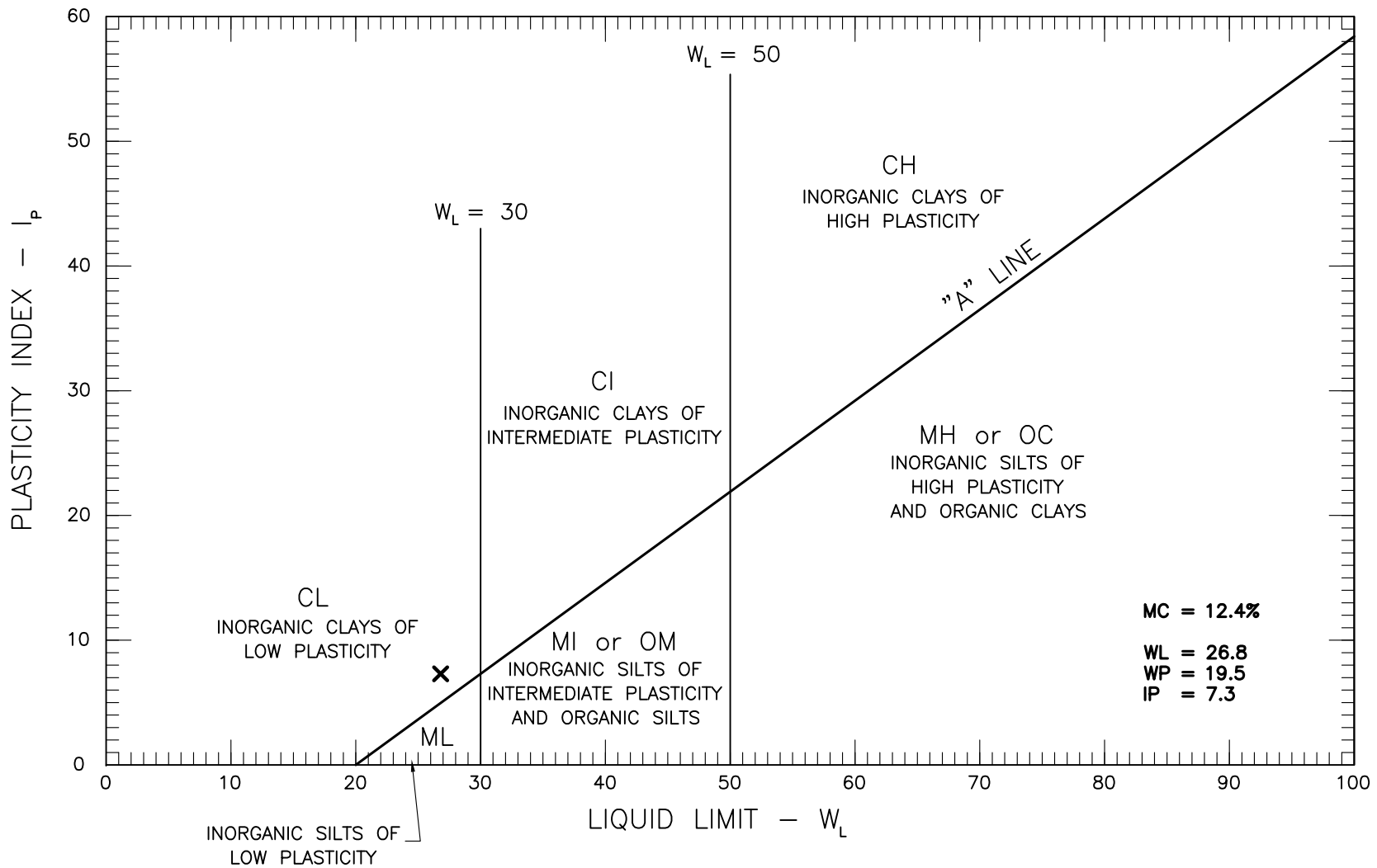
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PLATE NO.
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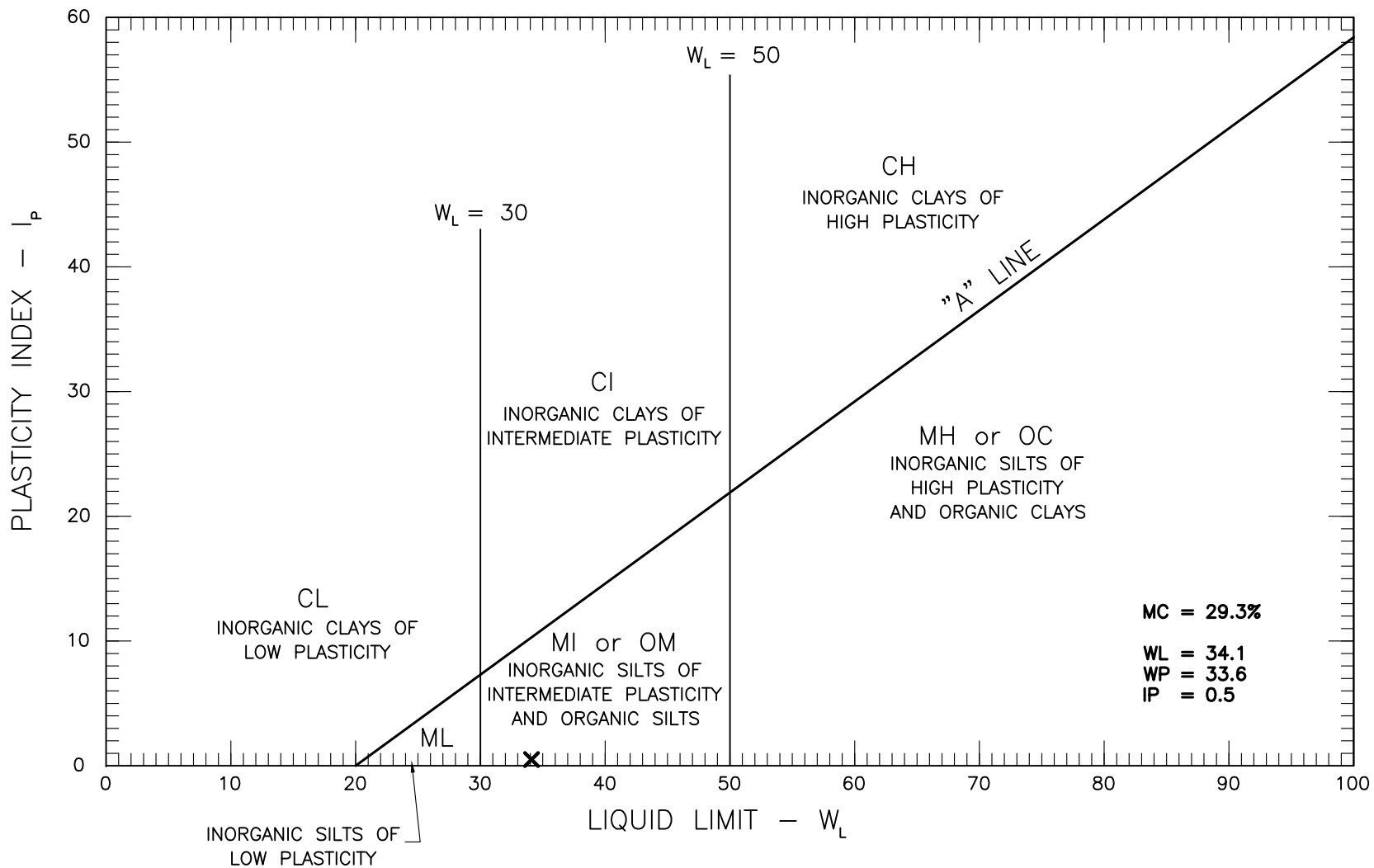


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 ATTERBERG LIMITS OF BH-BGC11-51-SPT1

PROJECT NO.
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 PLATE NO.
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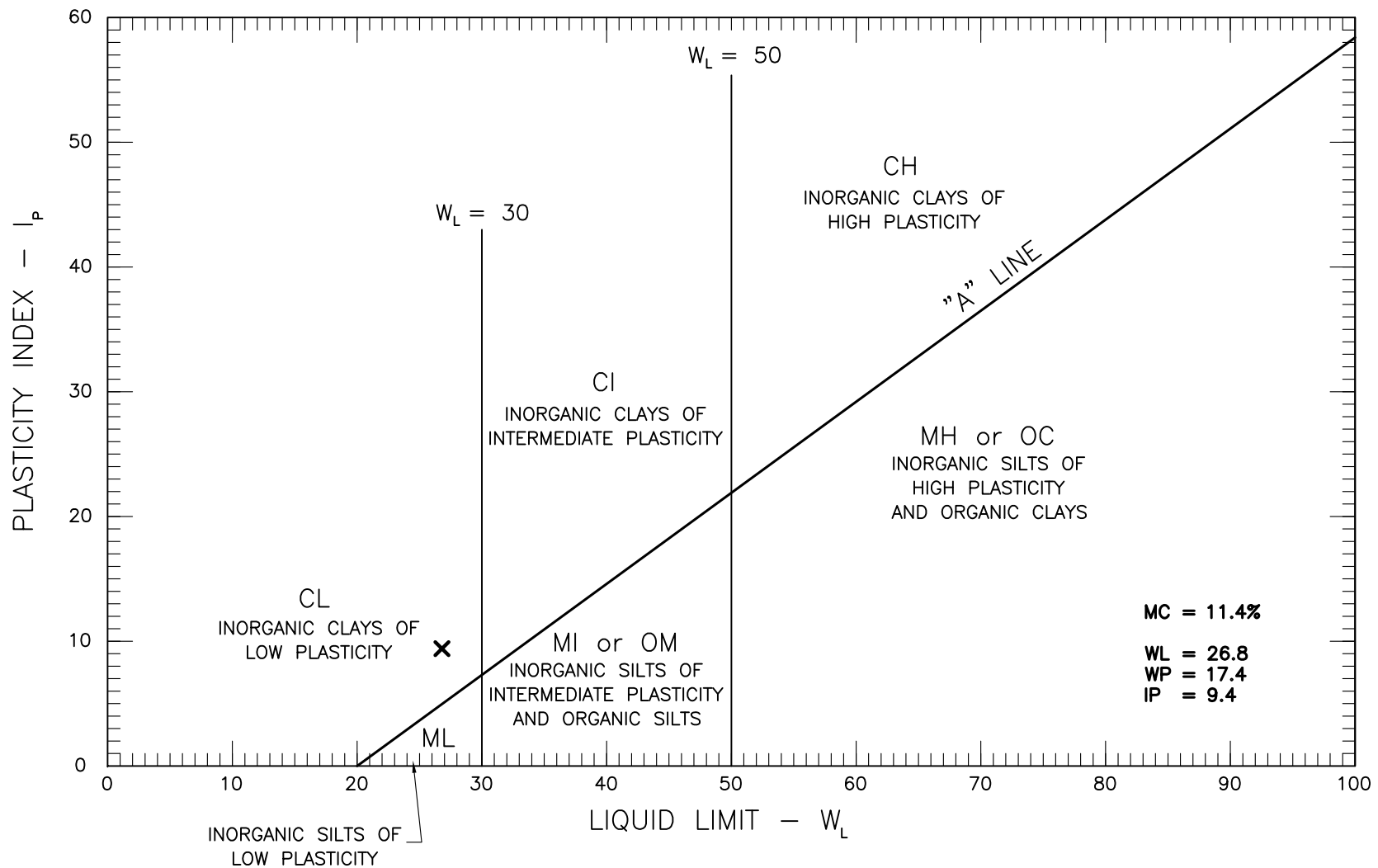
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 ATTERBERG LIMITS OF BH-BGC11-51-SPT3

PROJECT NO.
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PLATE NO.
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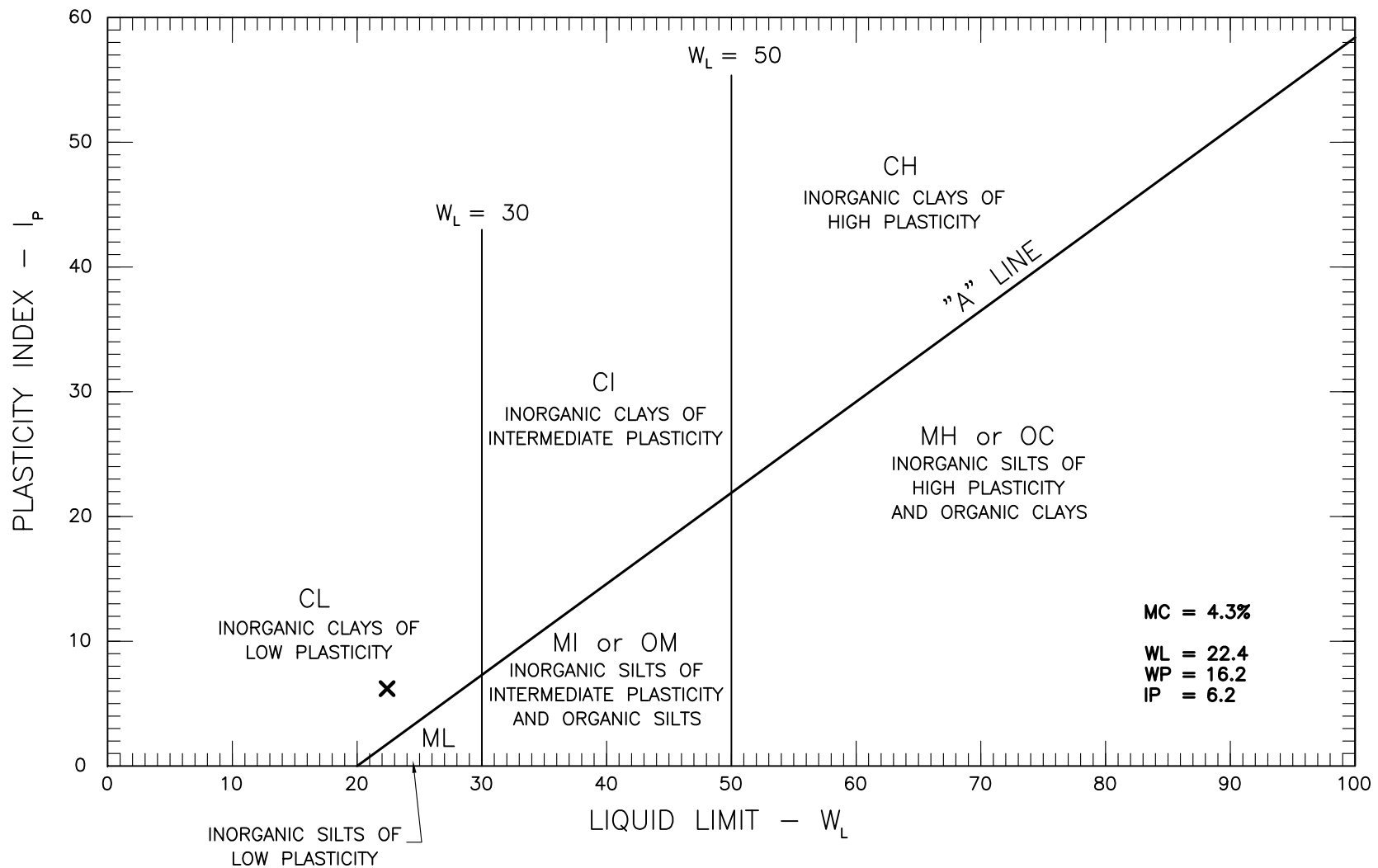
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PROJECT NO.
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PLATE NO.
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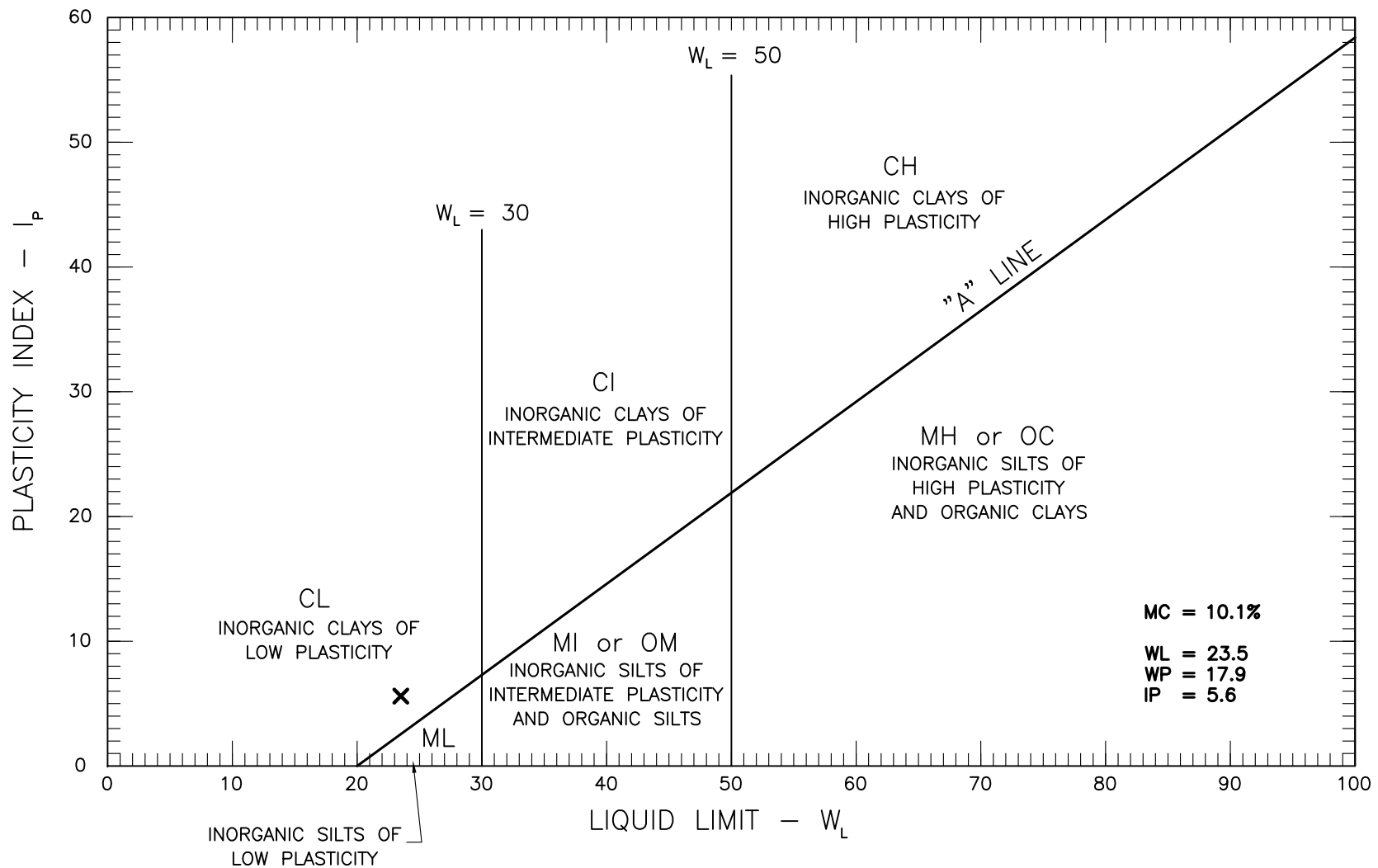
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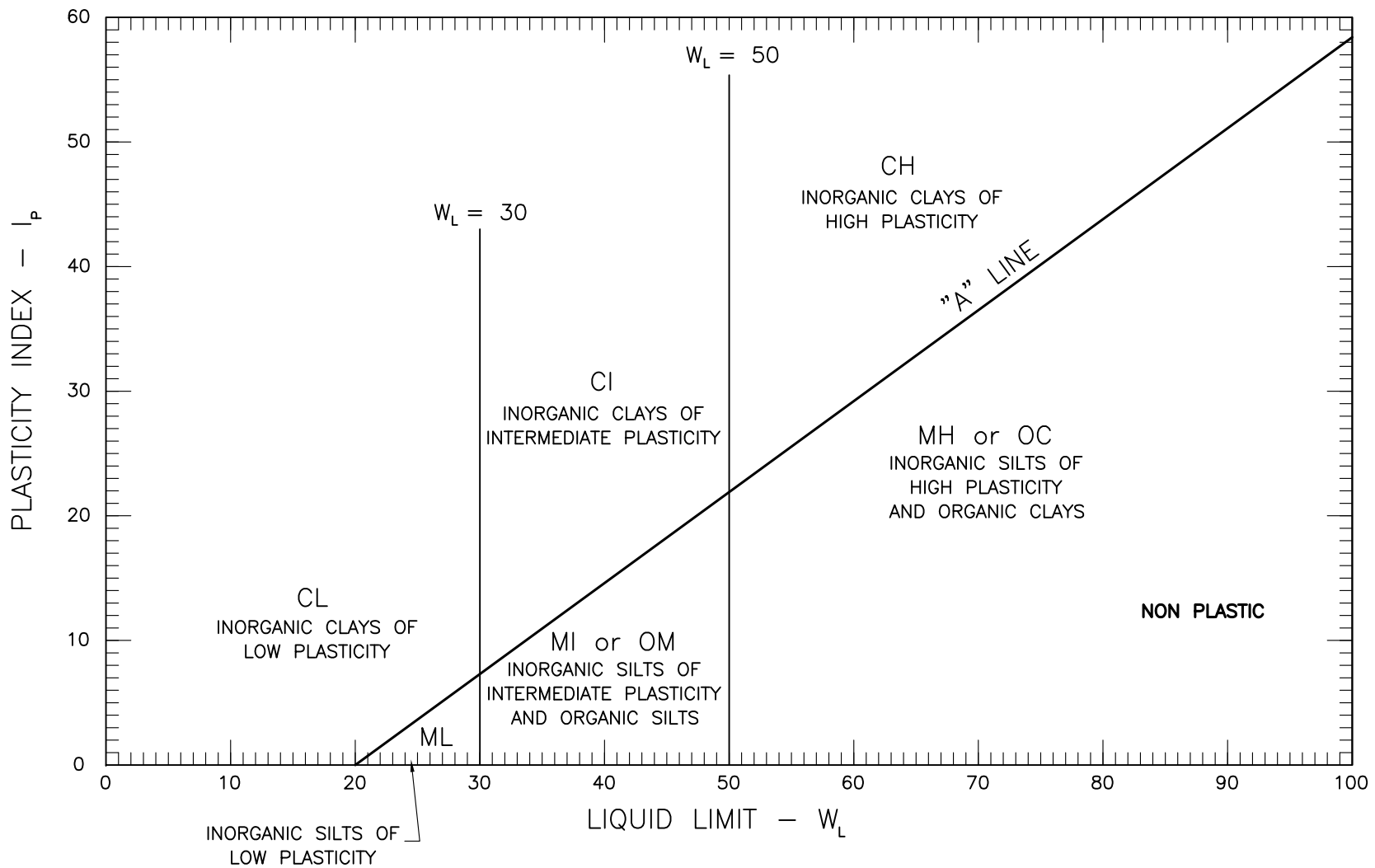
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ATTERBERG LIMITS OF BH-BGC11-53-SPT2

PROJECT NO.
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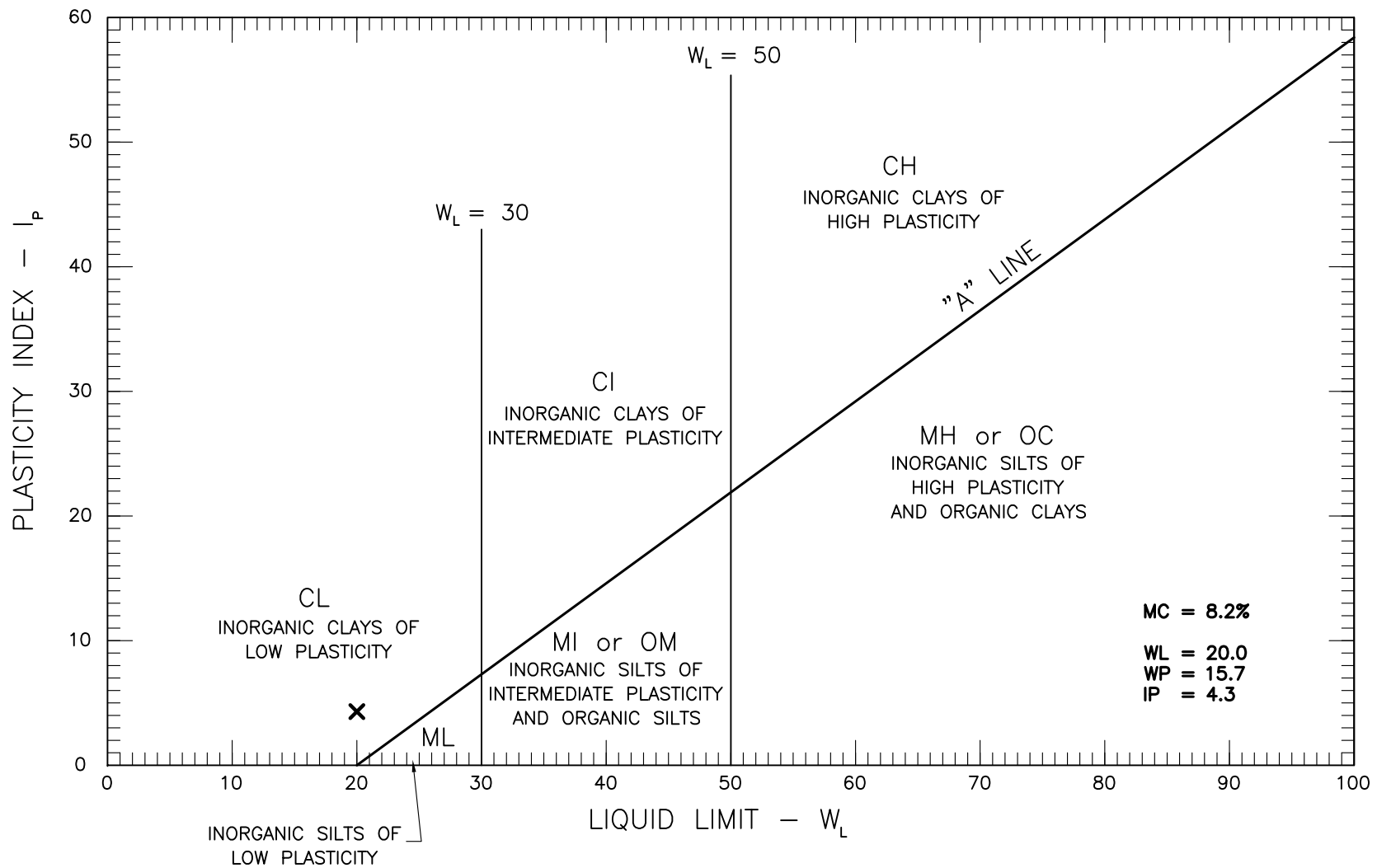


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PROJECT NO.
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 PLATE NO.
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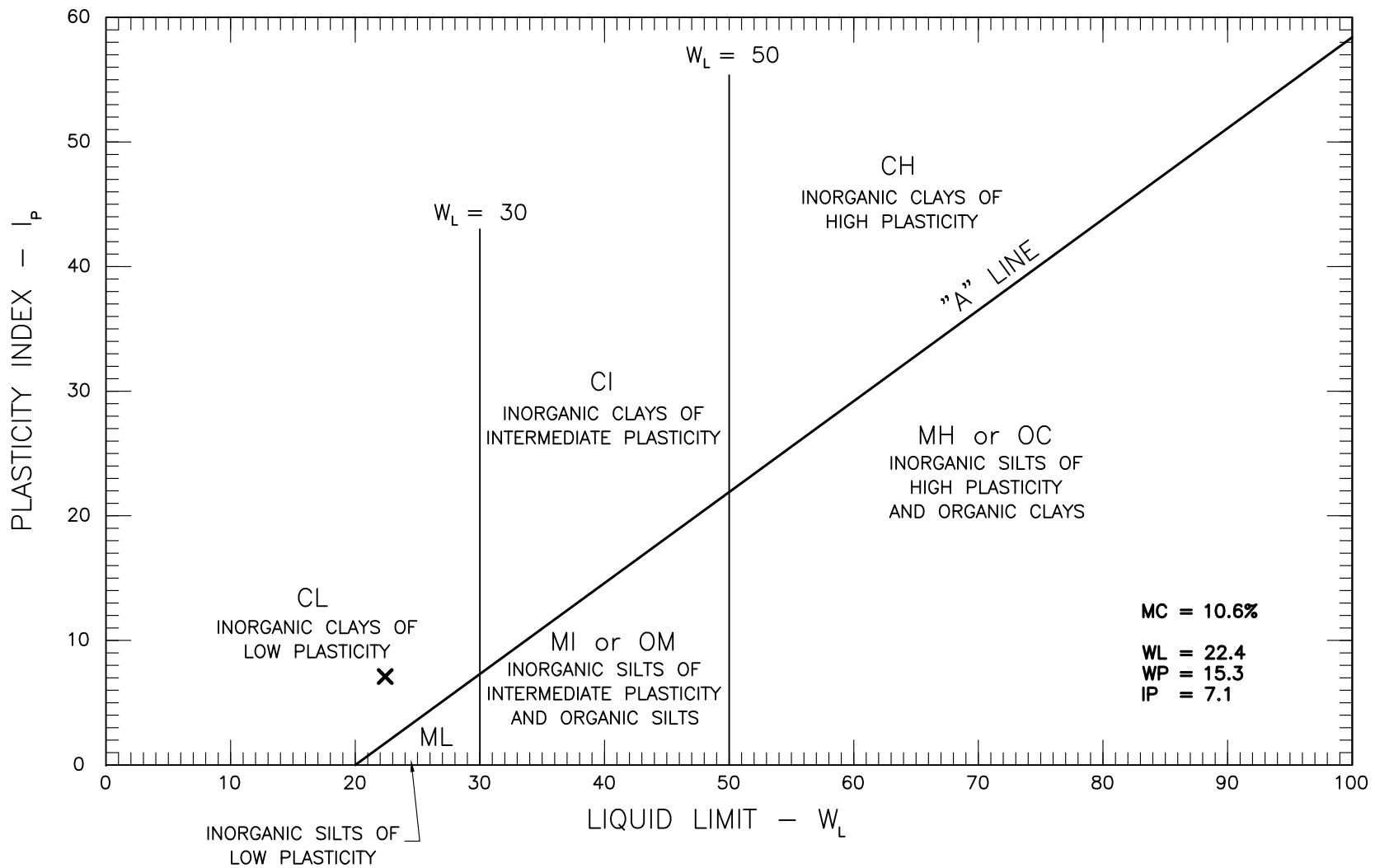
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-54-G1

PROJECT NO.
 K-3300

PLATE NO.
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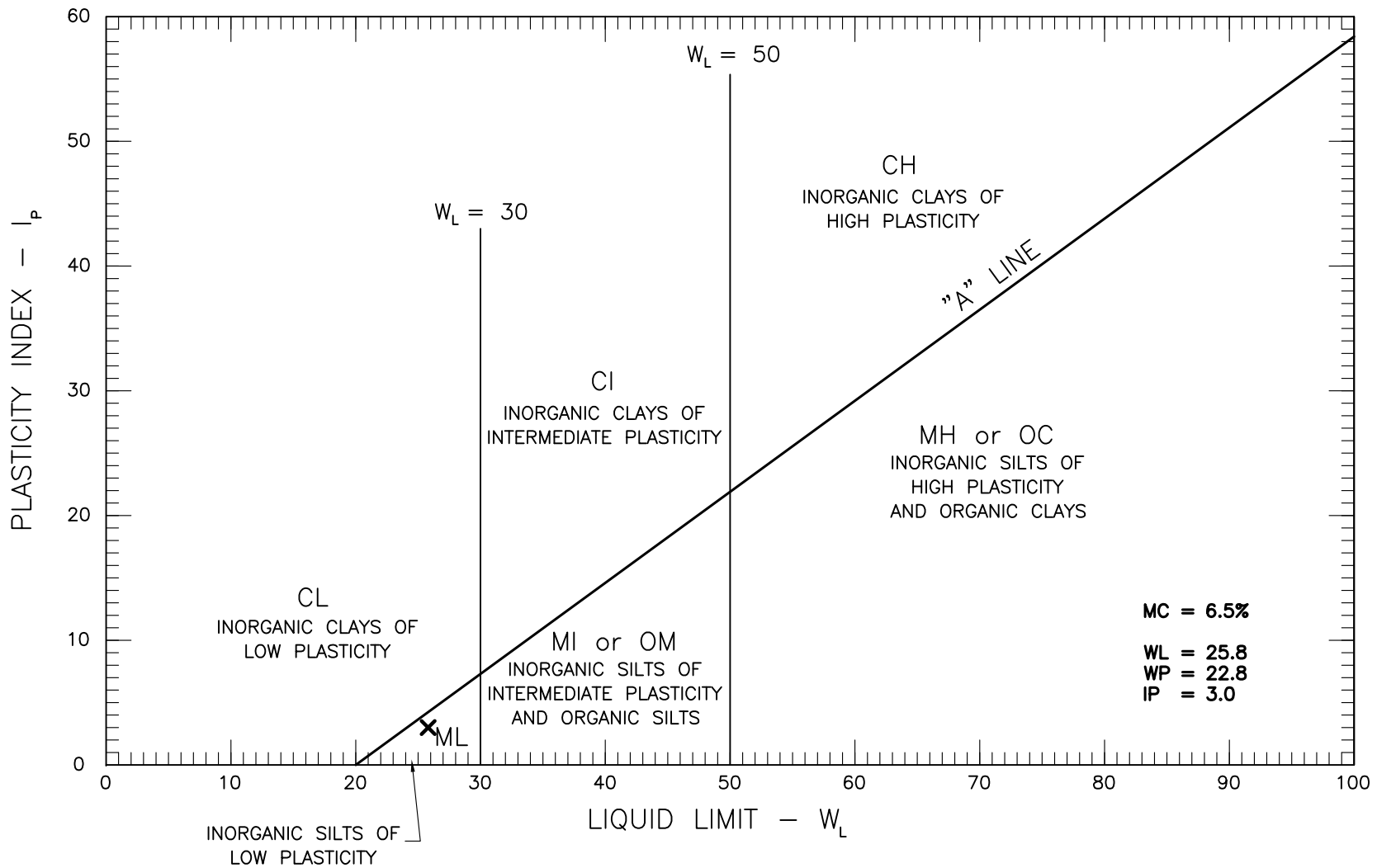
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 ATTERBERG LIMITS OF BH-BGC11-55-G4

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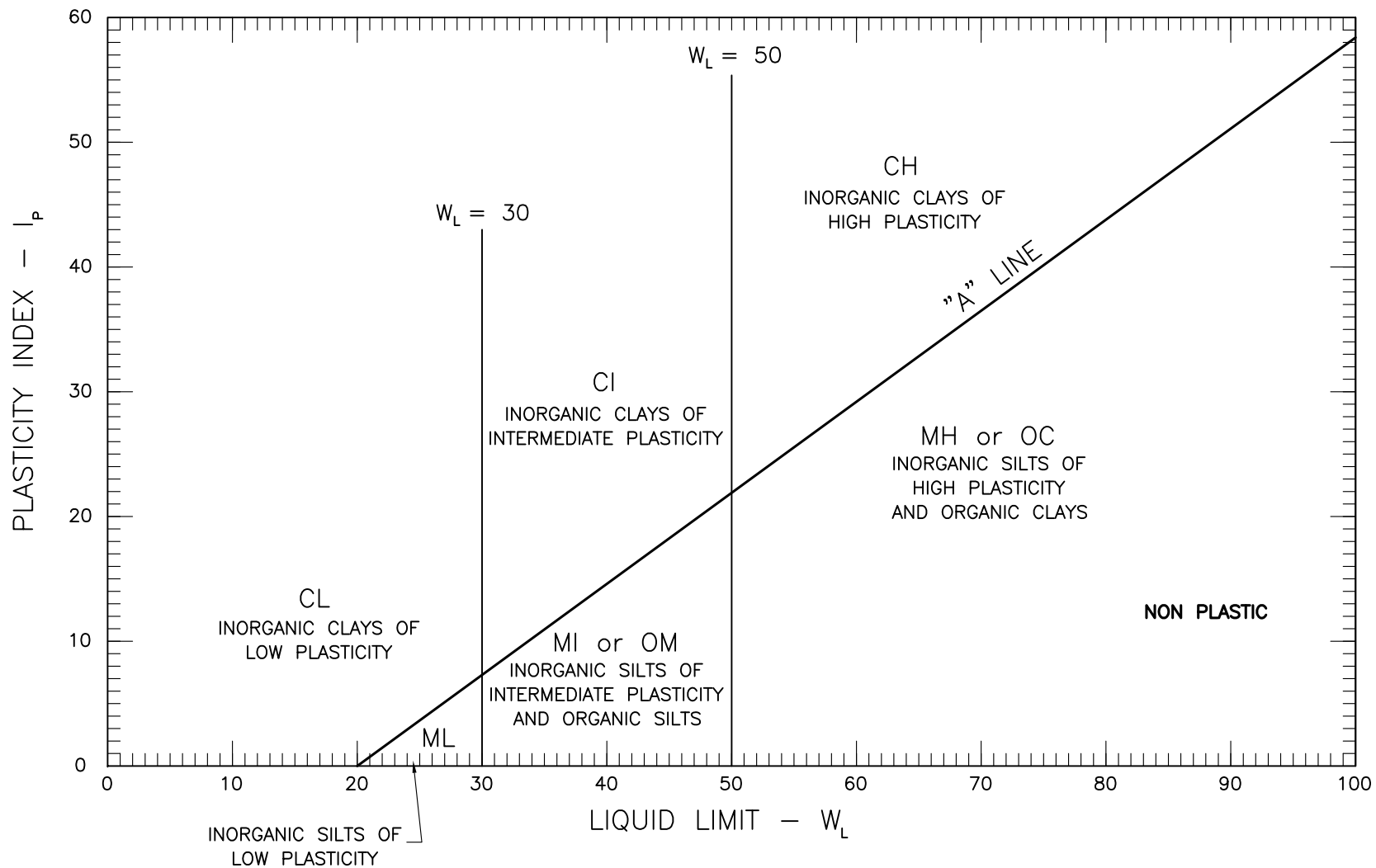


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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-55-SPT6

PROJECT NO.
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 PLATE NO.
 3300-AL-BH55-SPT6



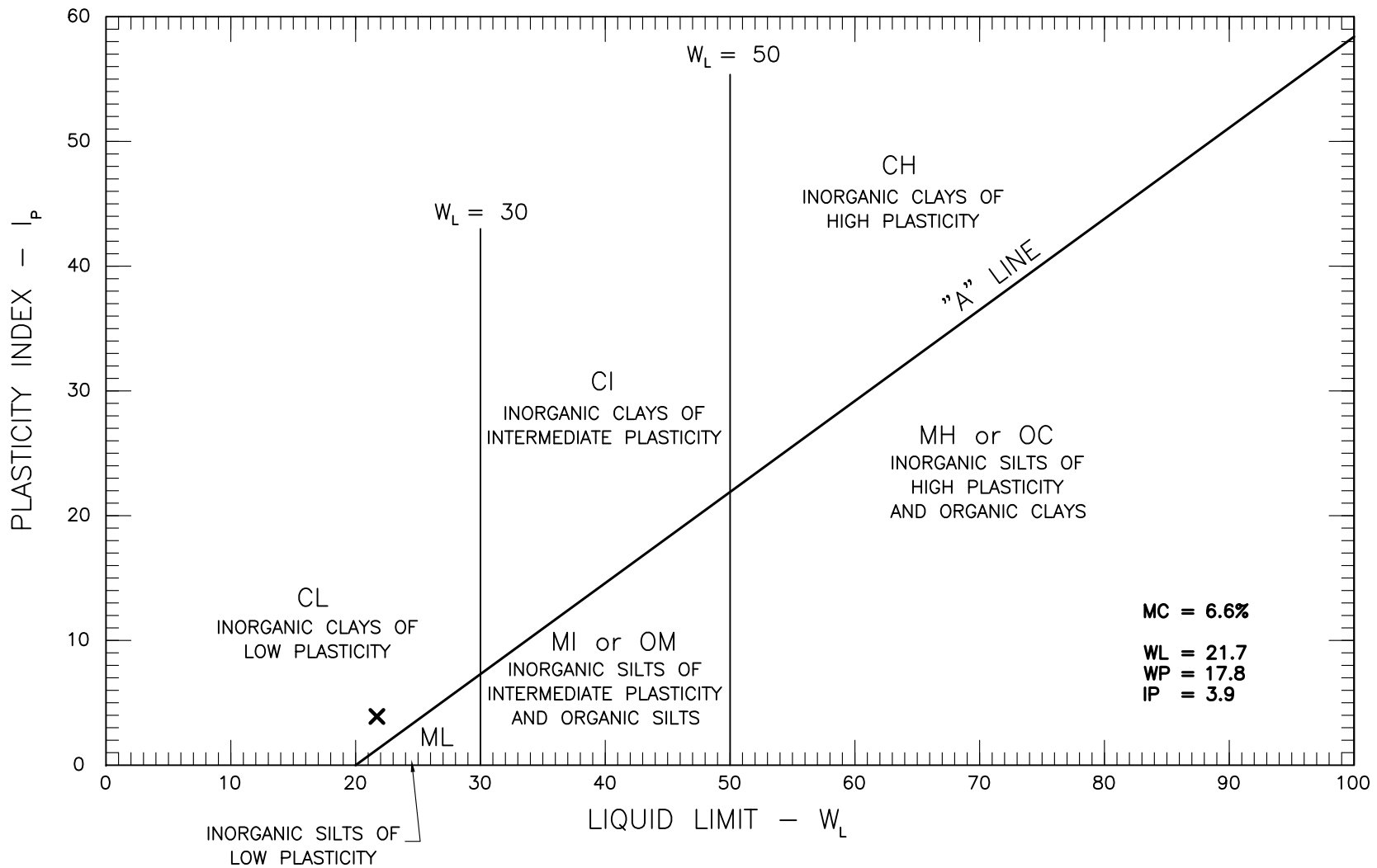
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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-56-SPT3

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH56-SPT3



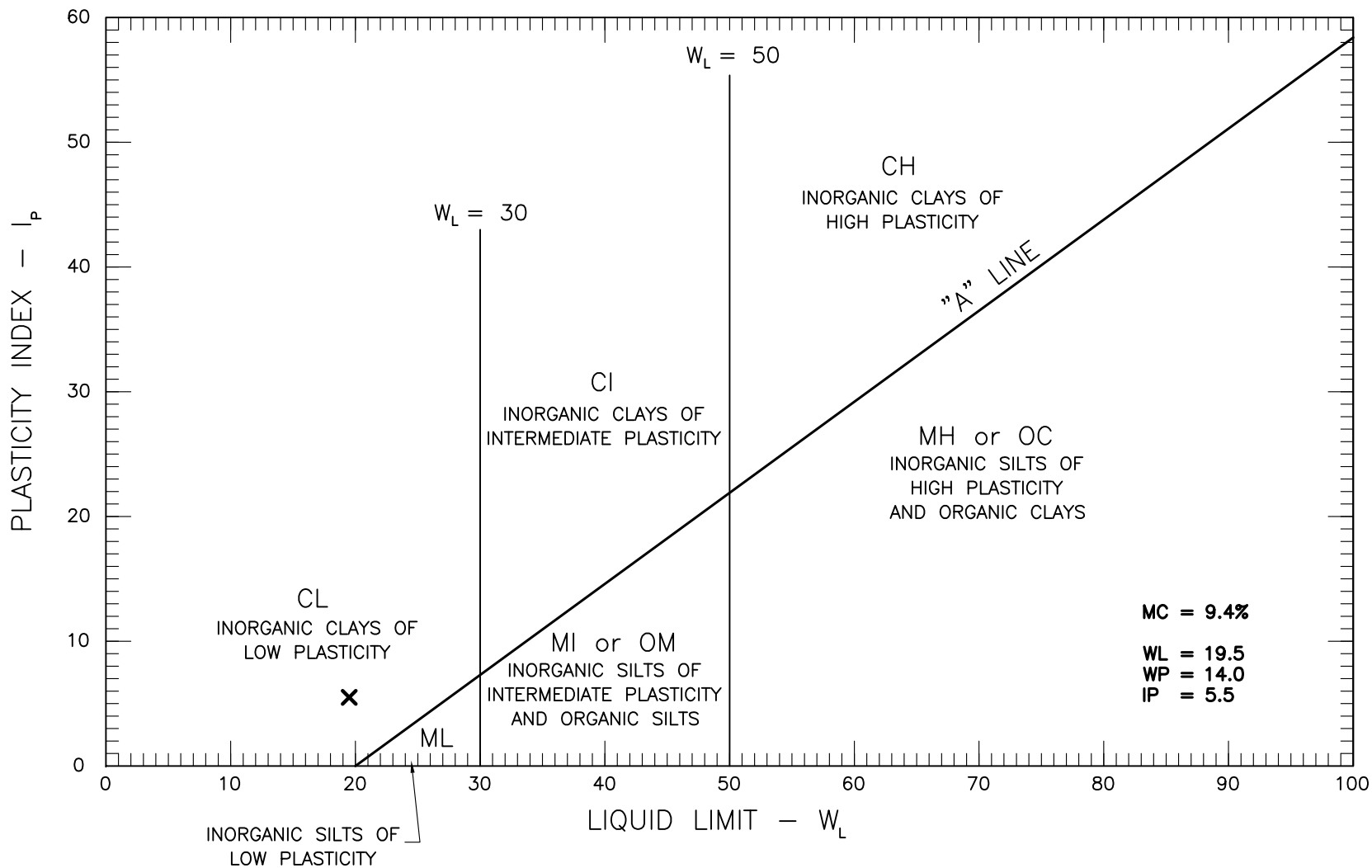
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PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH56-SPT4



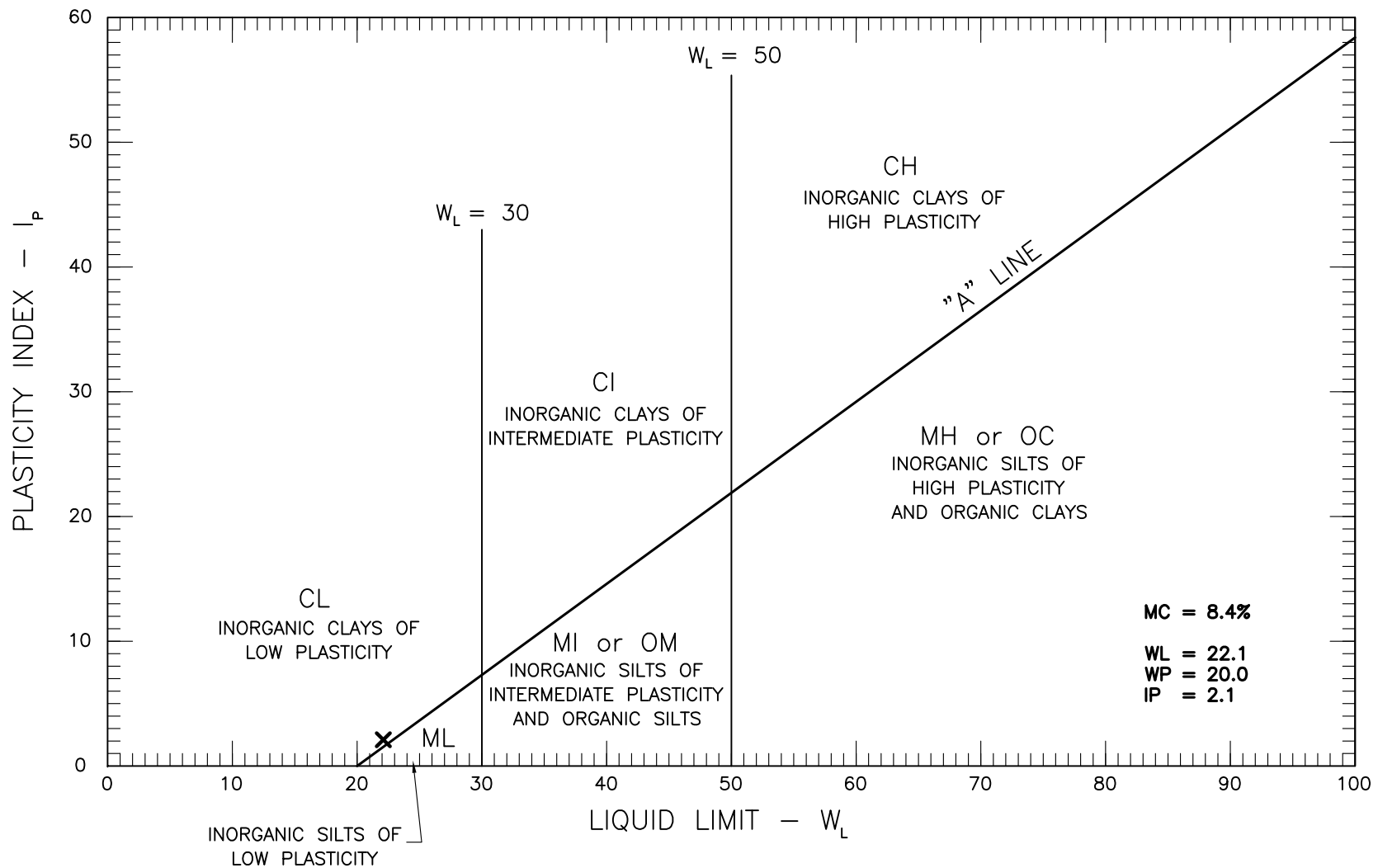
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PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH57-SPT2



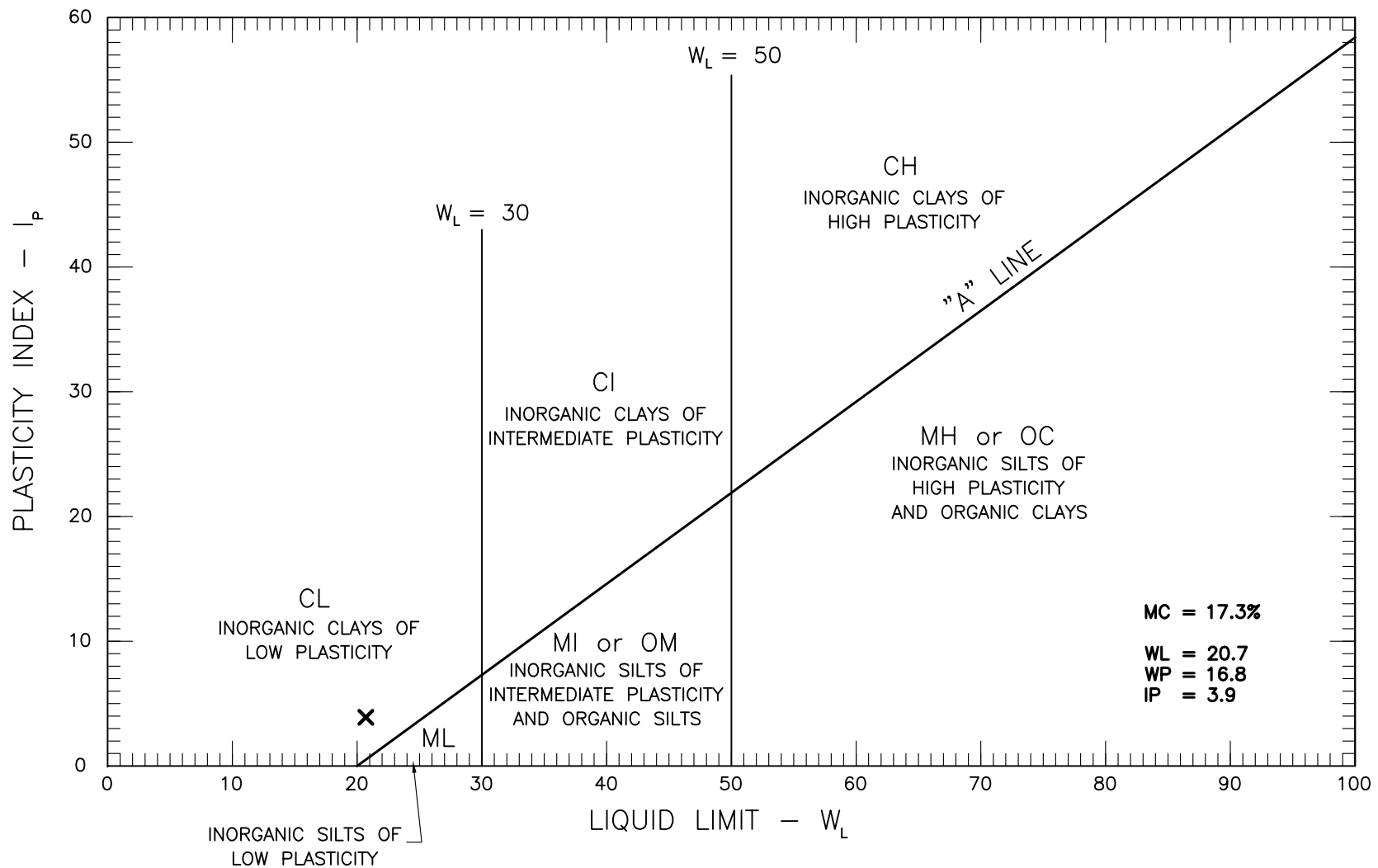
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 ATTERBERG LIMITS OF BH-BGC11-57-SPT4

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH57-SPT4



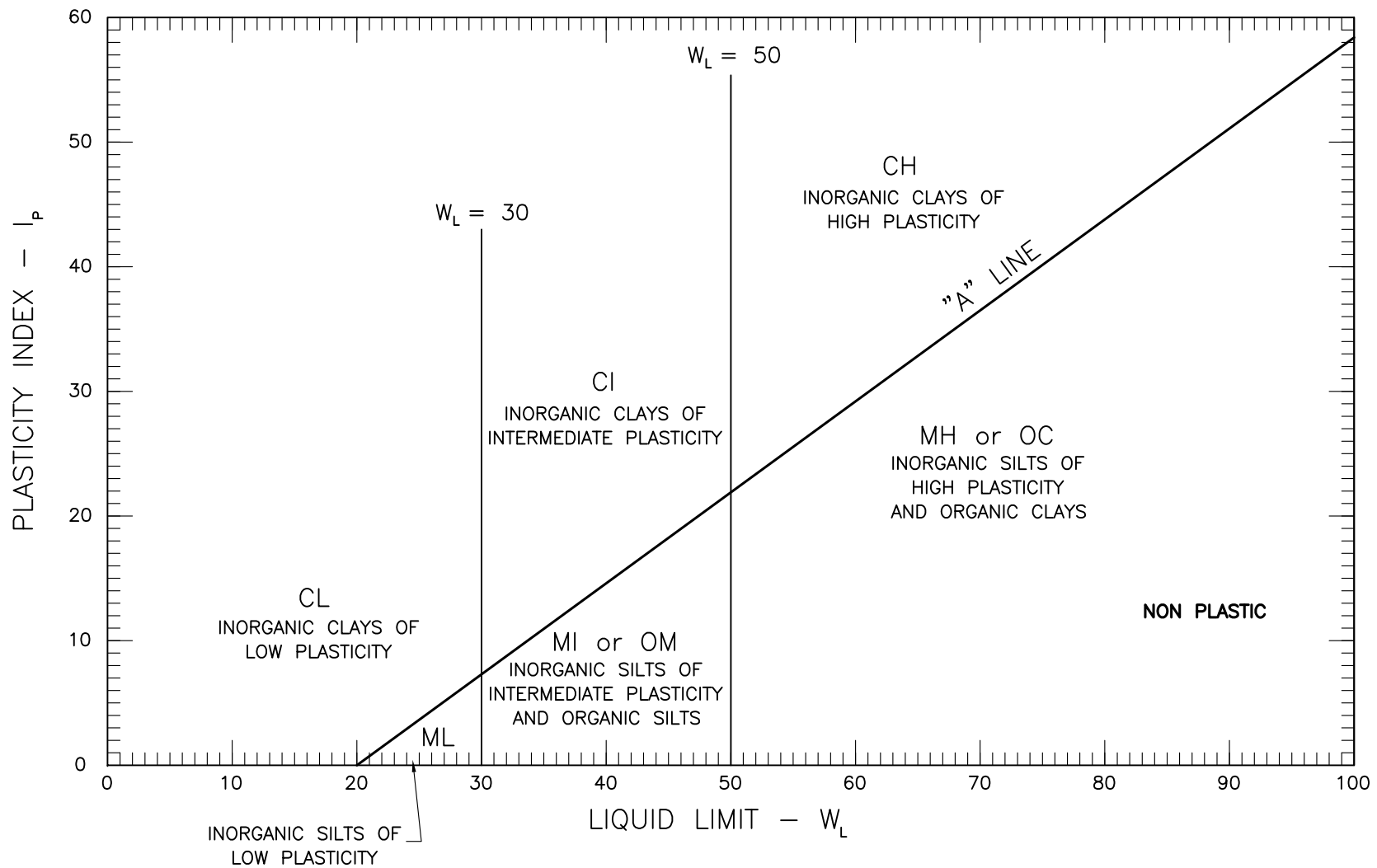
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 ATTERBERG LIMITS OF BH-BGC11-58-G2

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH58-G2



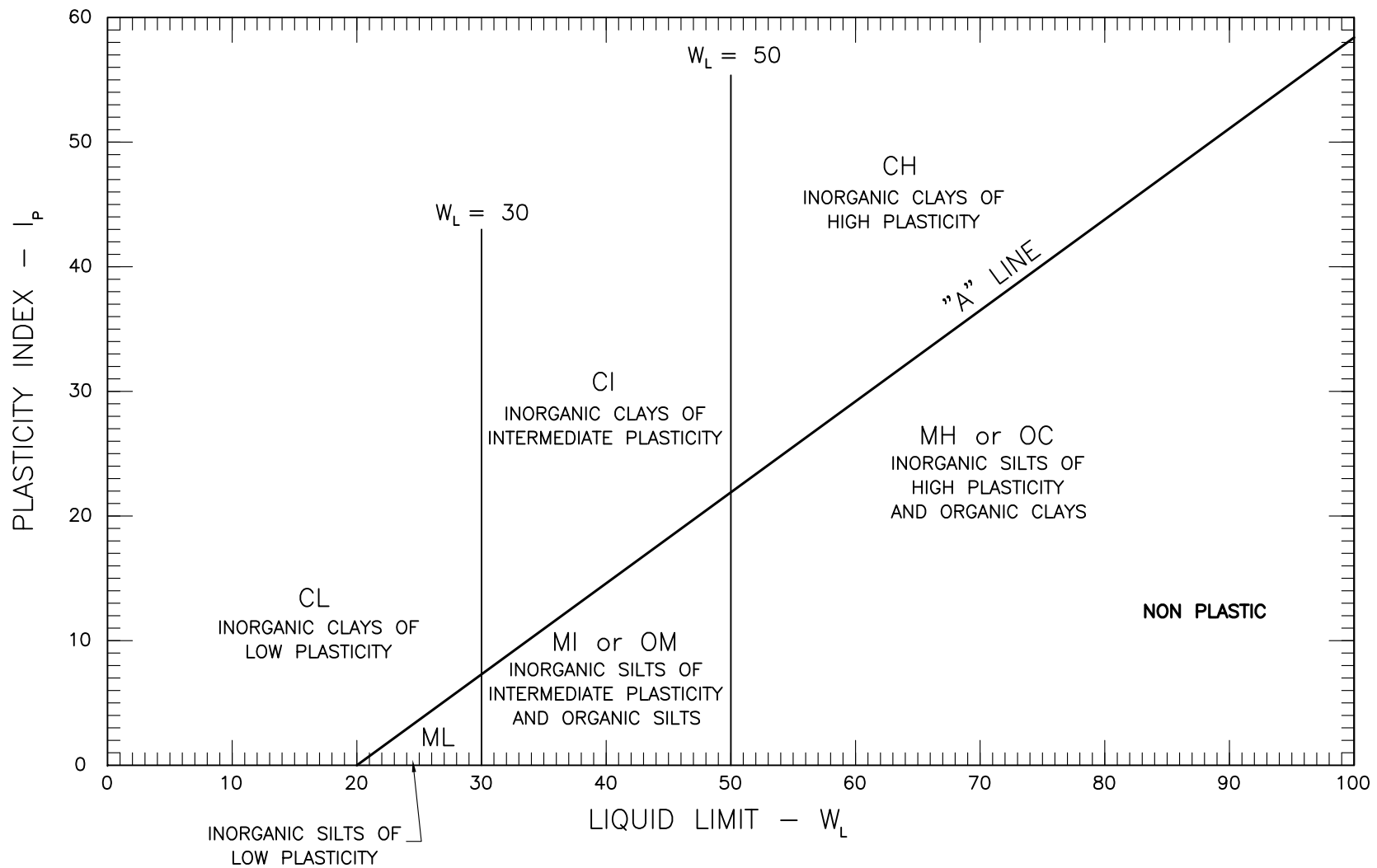
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 ATTERBERG LIMITS OF BH-BGC11-58-G4

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH58-G4



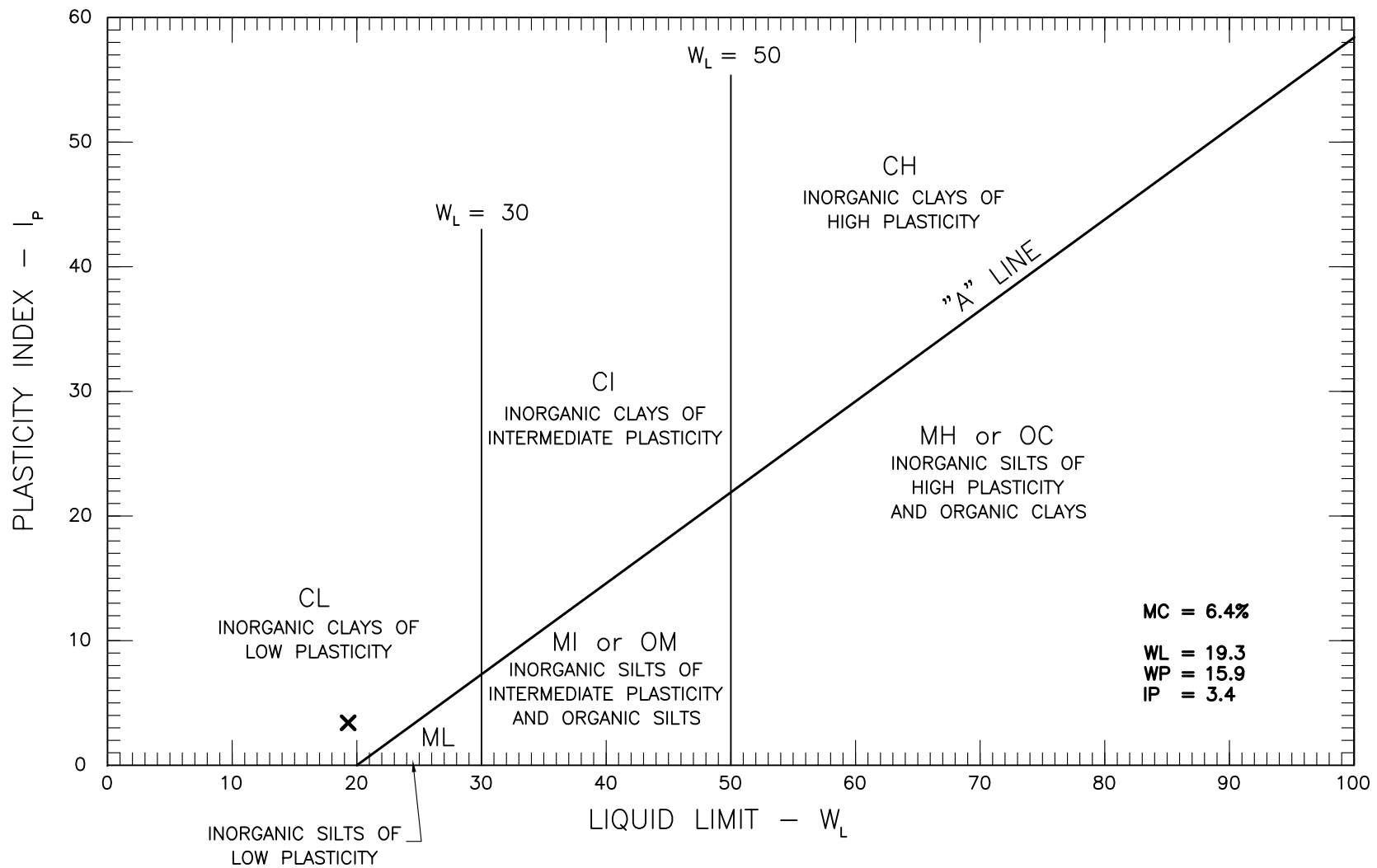
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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-58-G6

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH58-G6



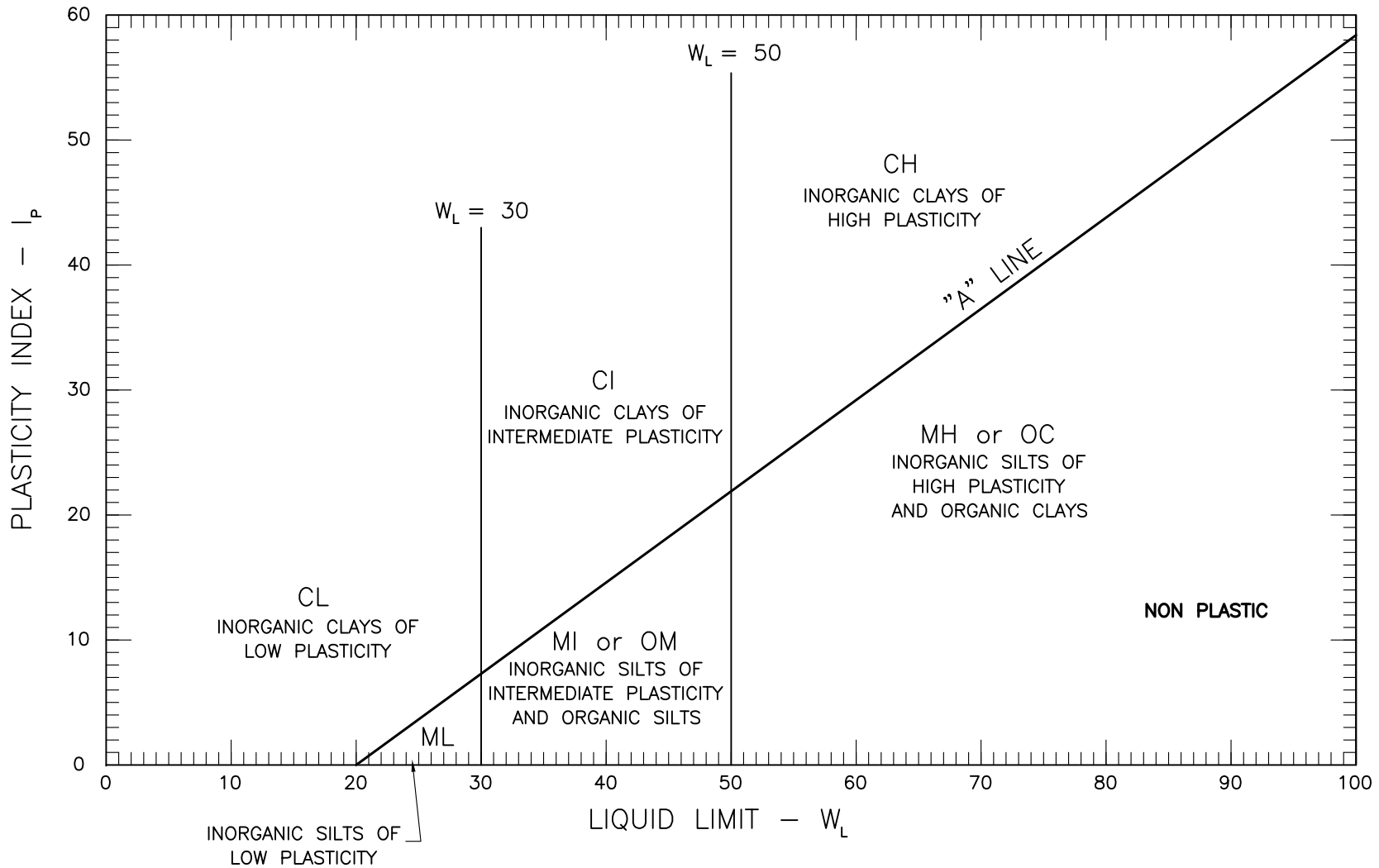
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DUBLIN GULCH, YUKON
ATTERBERG LIMITS OF BH-BGC11-58-G8

PROJECT NO.
K-3300

PLATE NO.
3300-AL-BH58-G8

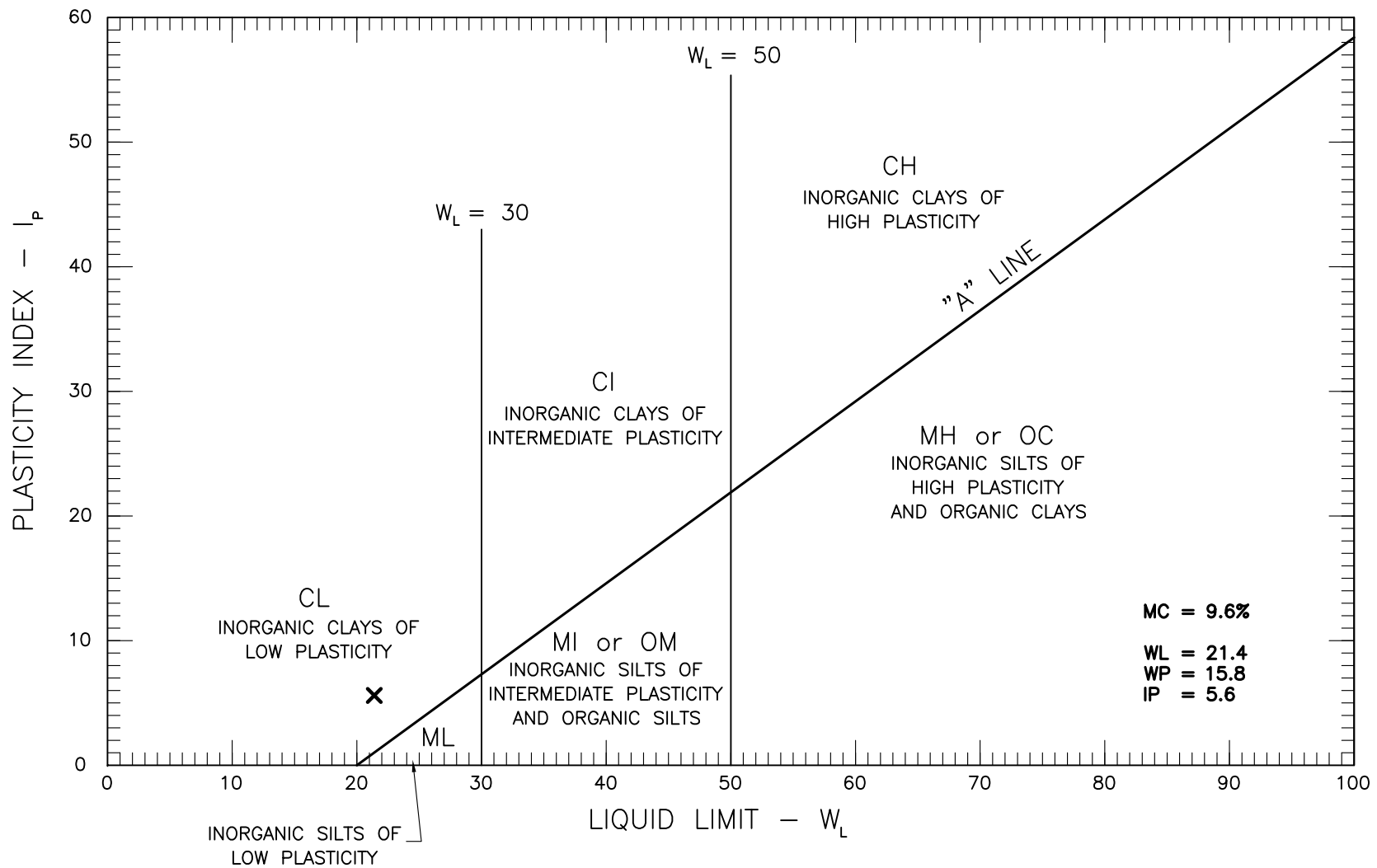


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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-58-G10

PROJECT NO.
 K-3300
 PLATE NO.
 3300-AL-BH58-G10



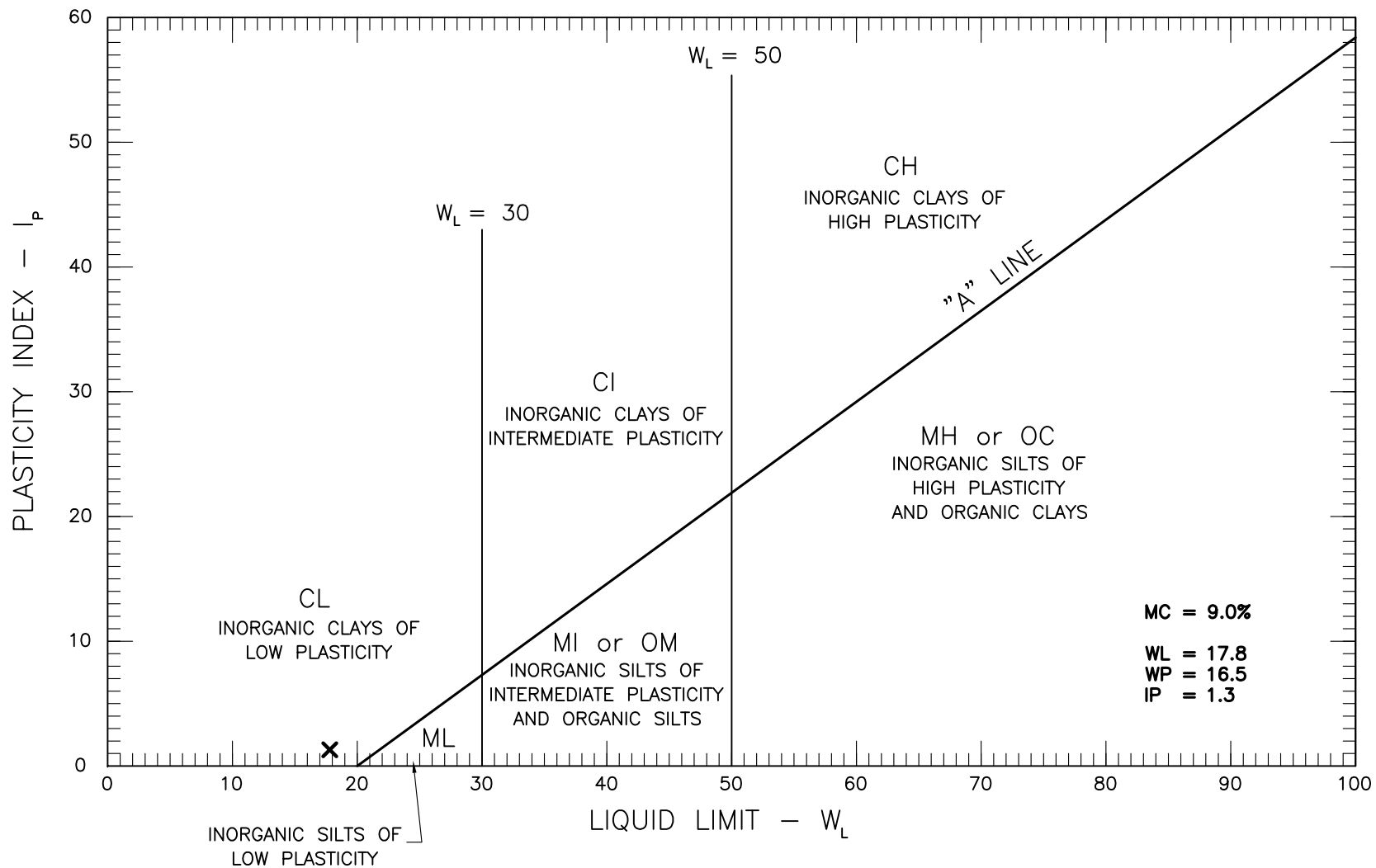
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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-60-G3

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH60-G3



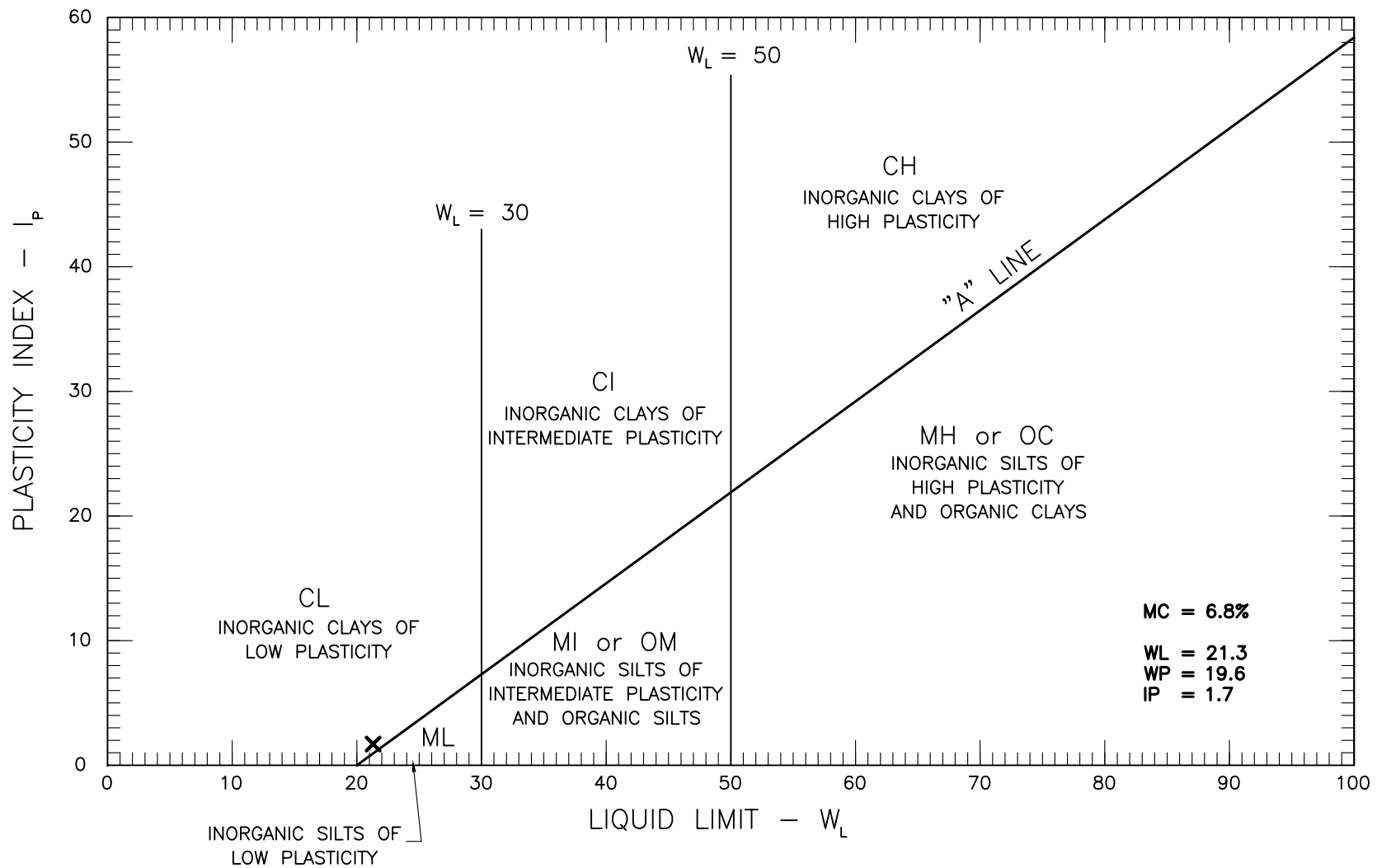
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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-60-G5

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH60-G5



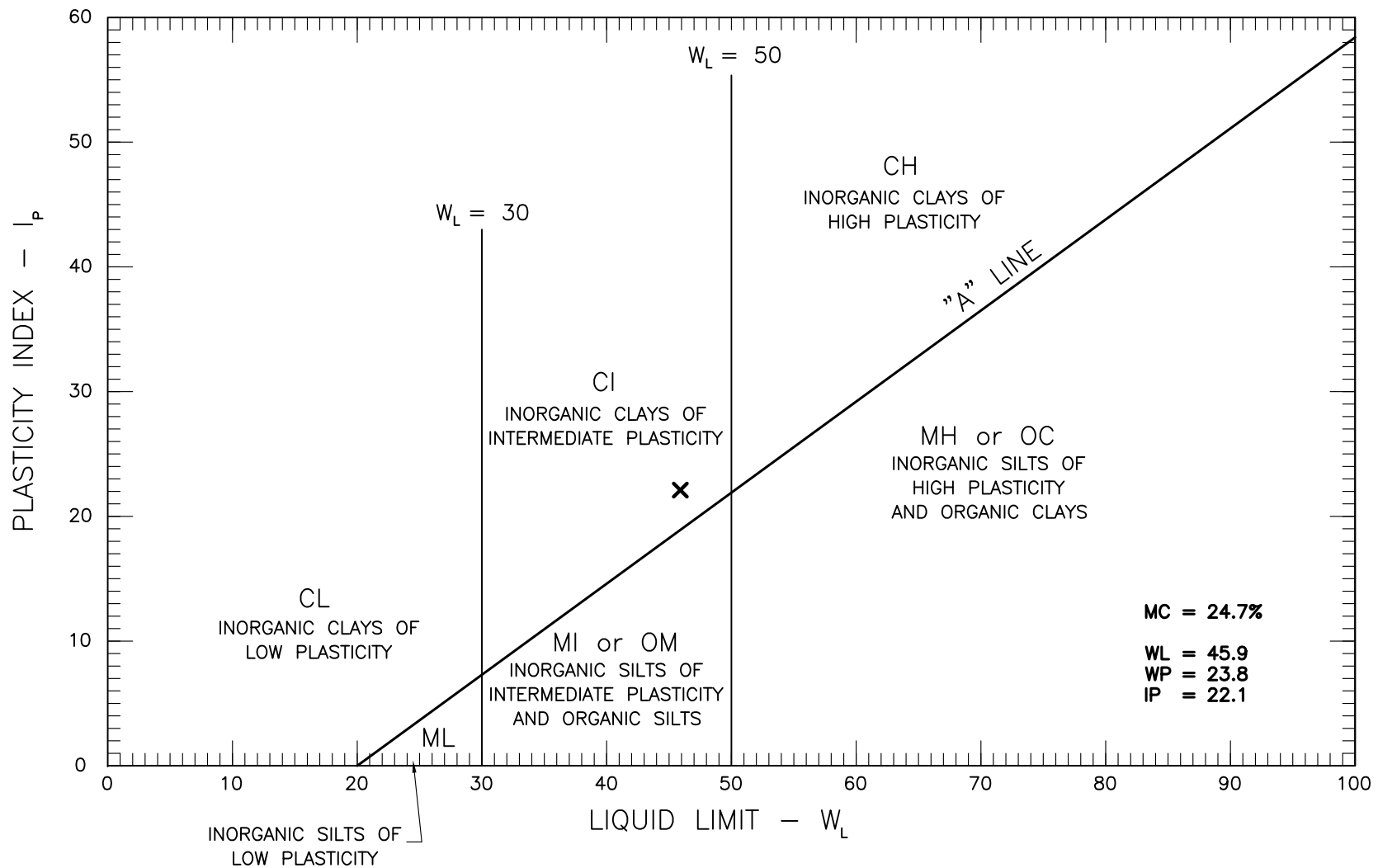
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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-60-G7

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH60-G7



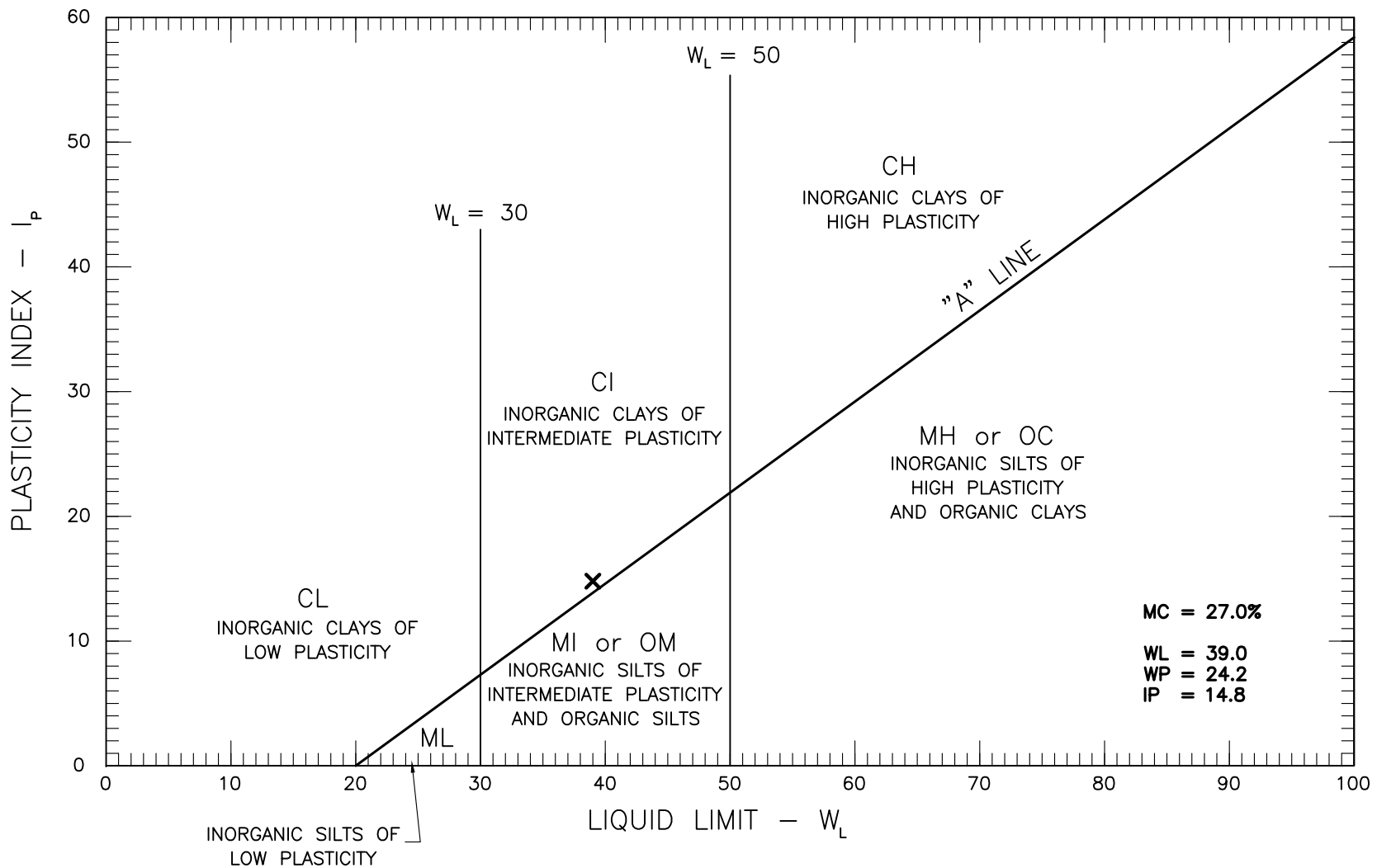
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BGC ENGINEERING INC.
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-63-G2

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH63-G2



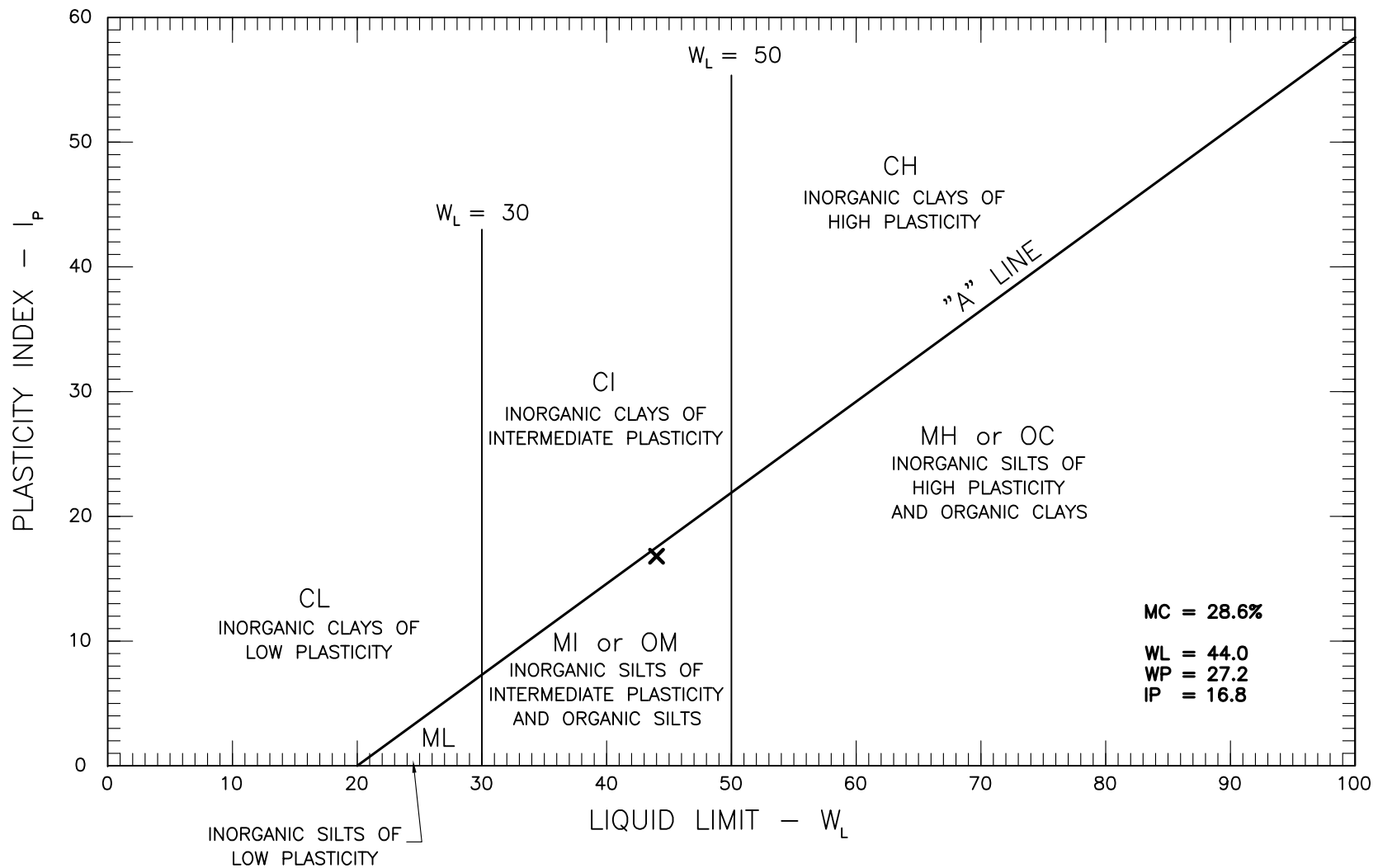
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-63-G5

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH63-G5



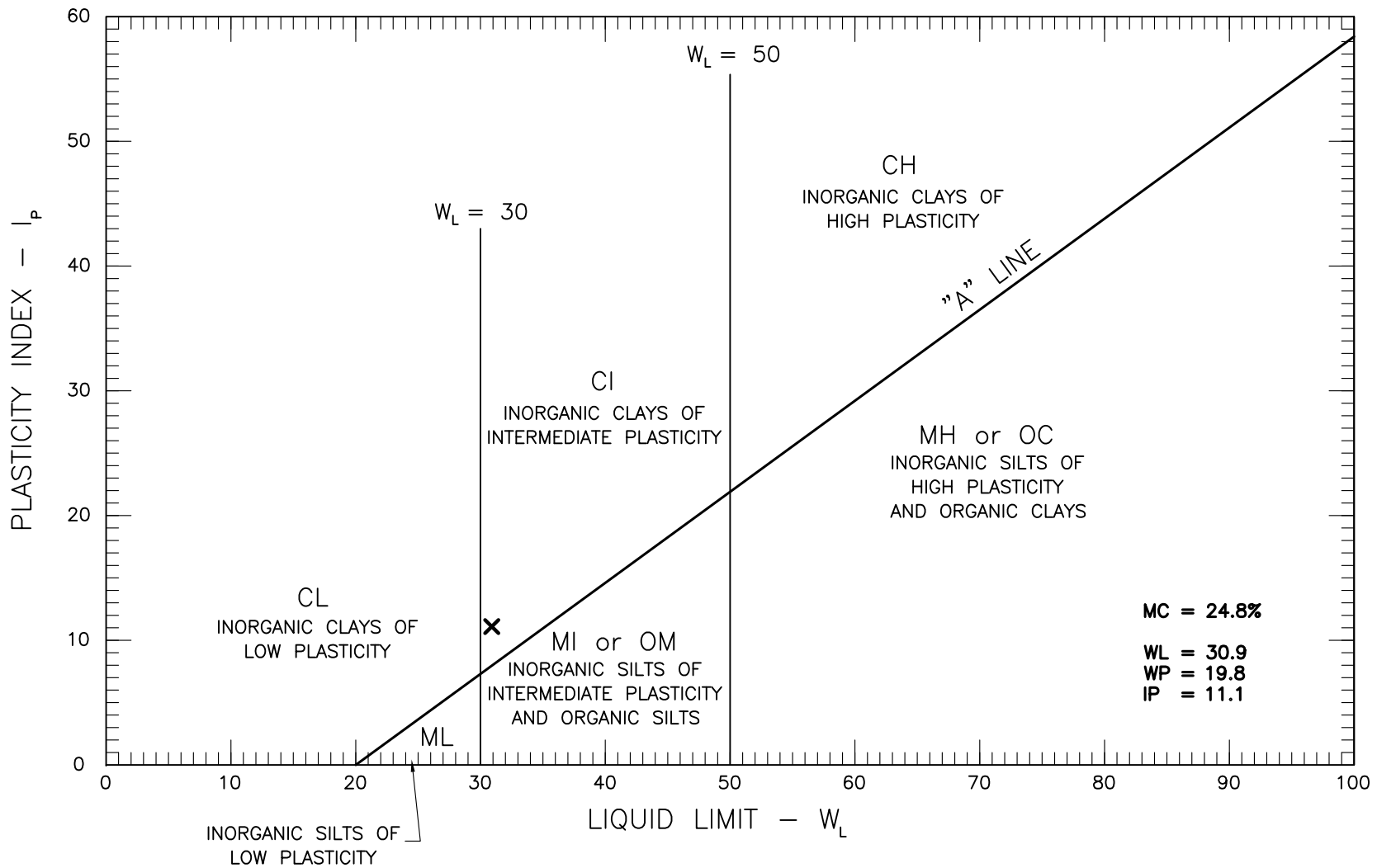
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BGC ENGINEERING INC.
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-63-G9

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH63-G9



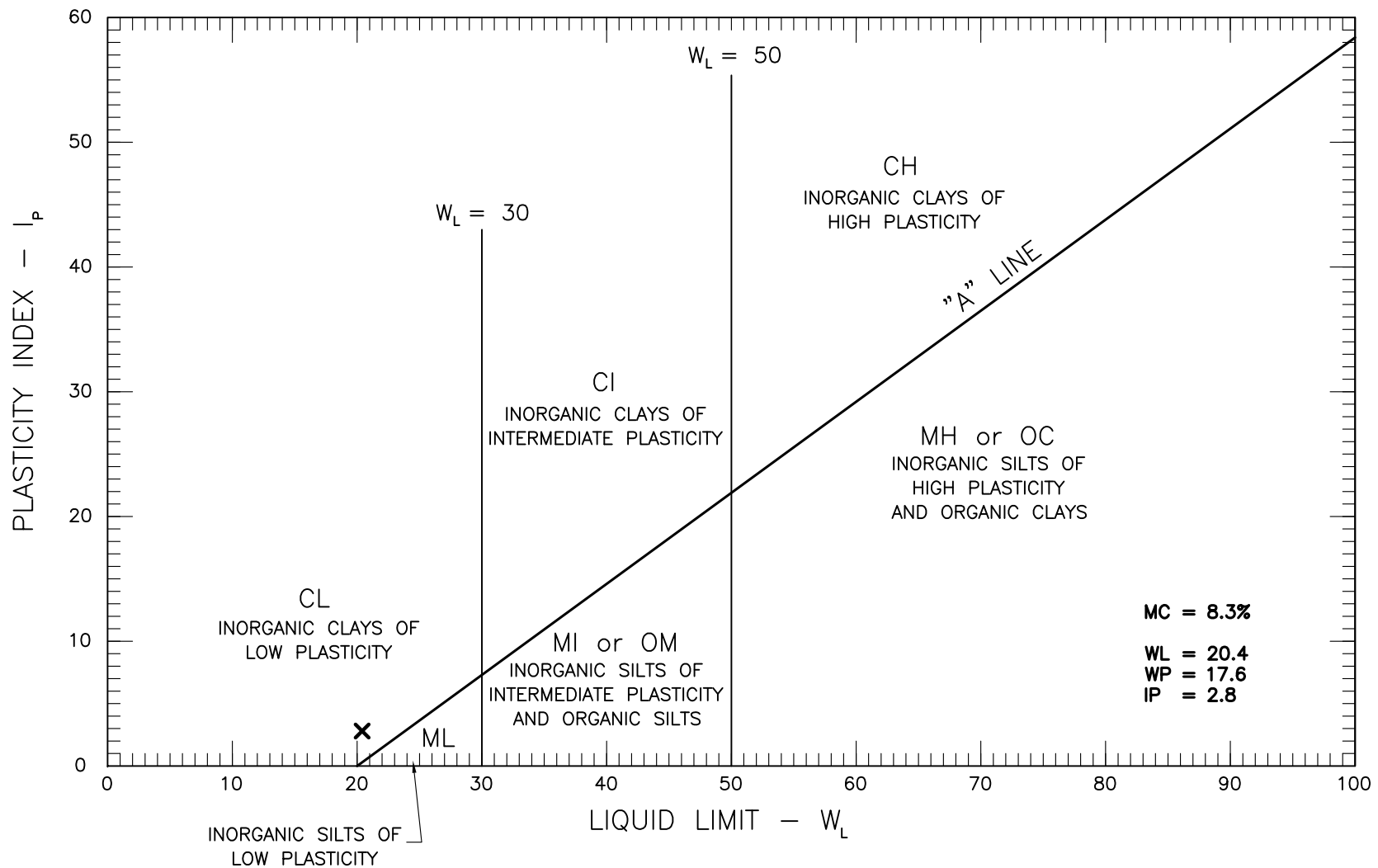
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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-63-G12

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH63-G12



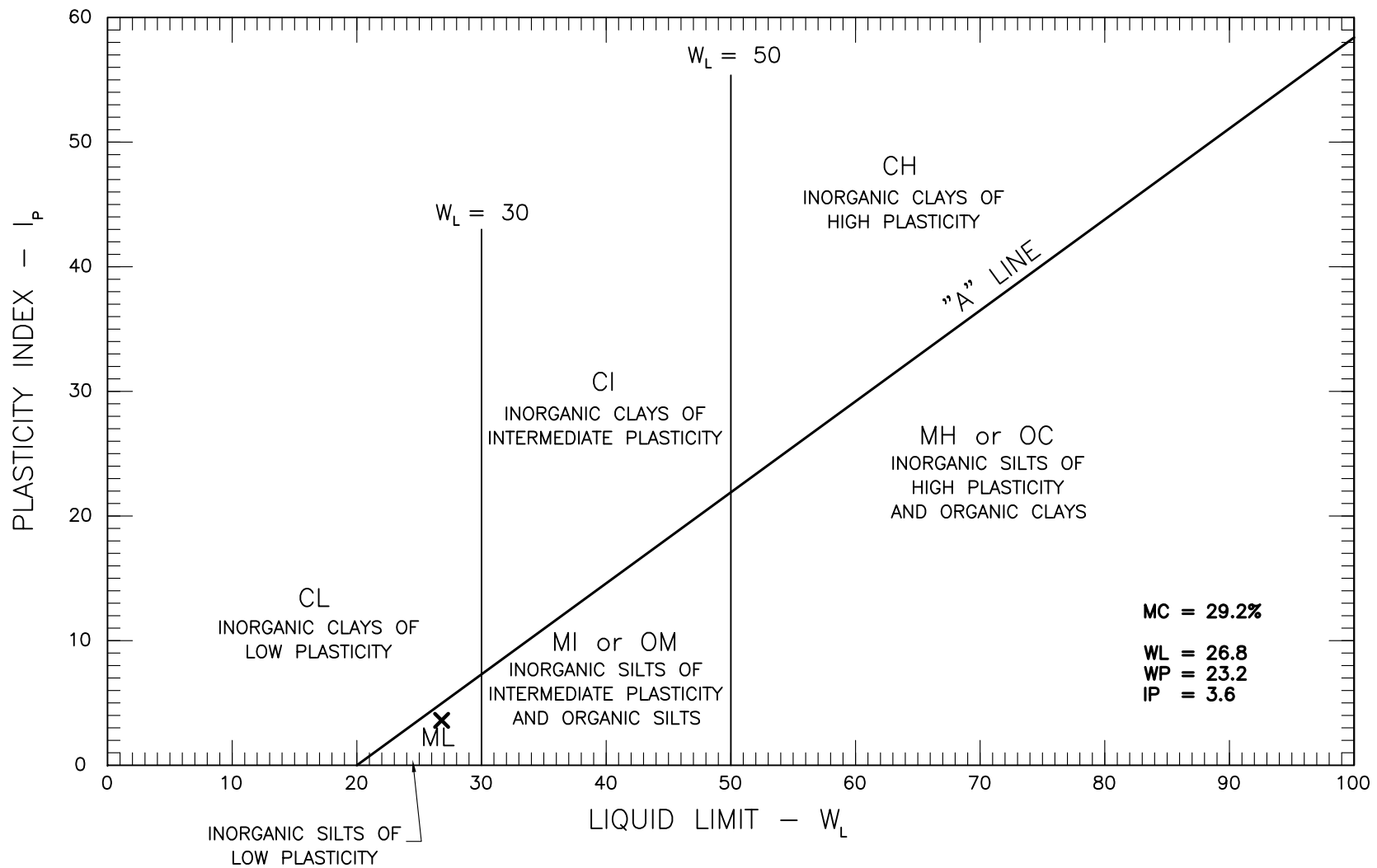
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BGC ENGINEERING INC.
 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF BH-BGC11-116-G2

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-BH116-G2



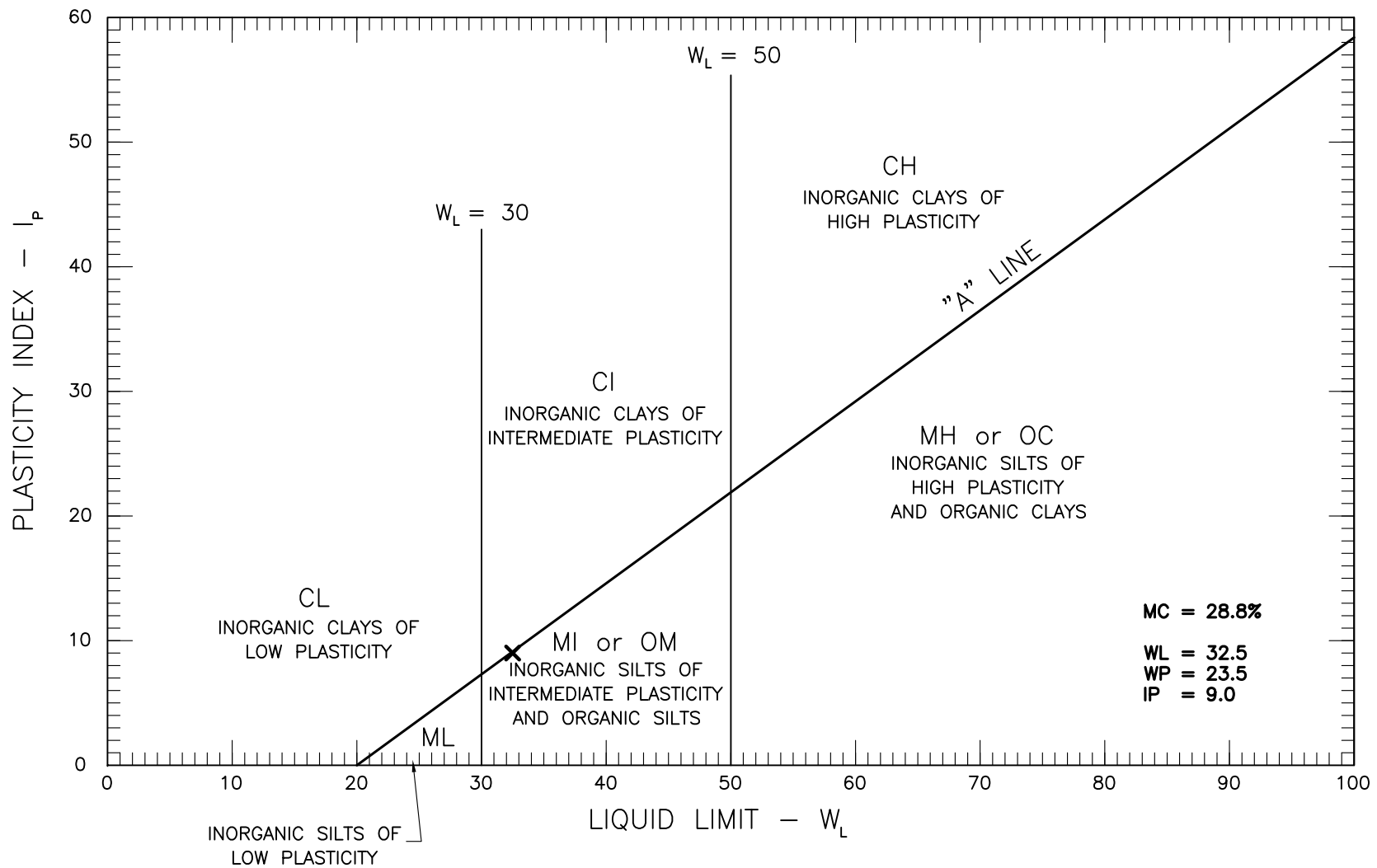
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ATTERBERG LIMITS OF TP-BGC11-119-G1

PROJECT NO.
K-3300

PLATE NO.
3300-AL-TP119-G1



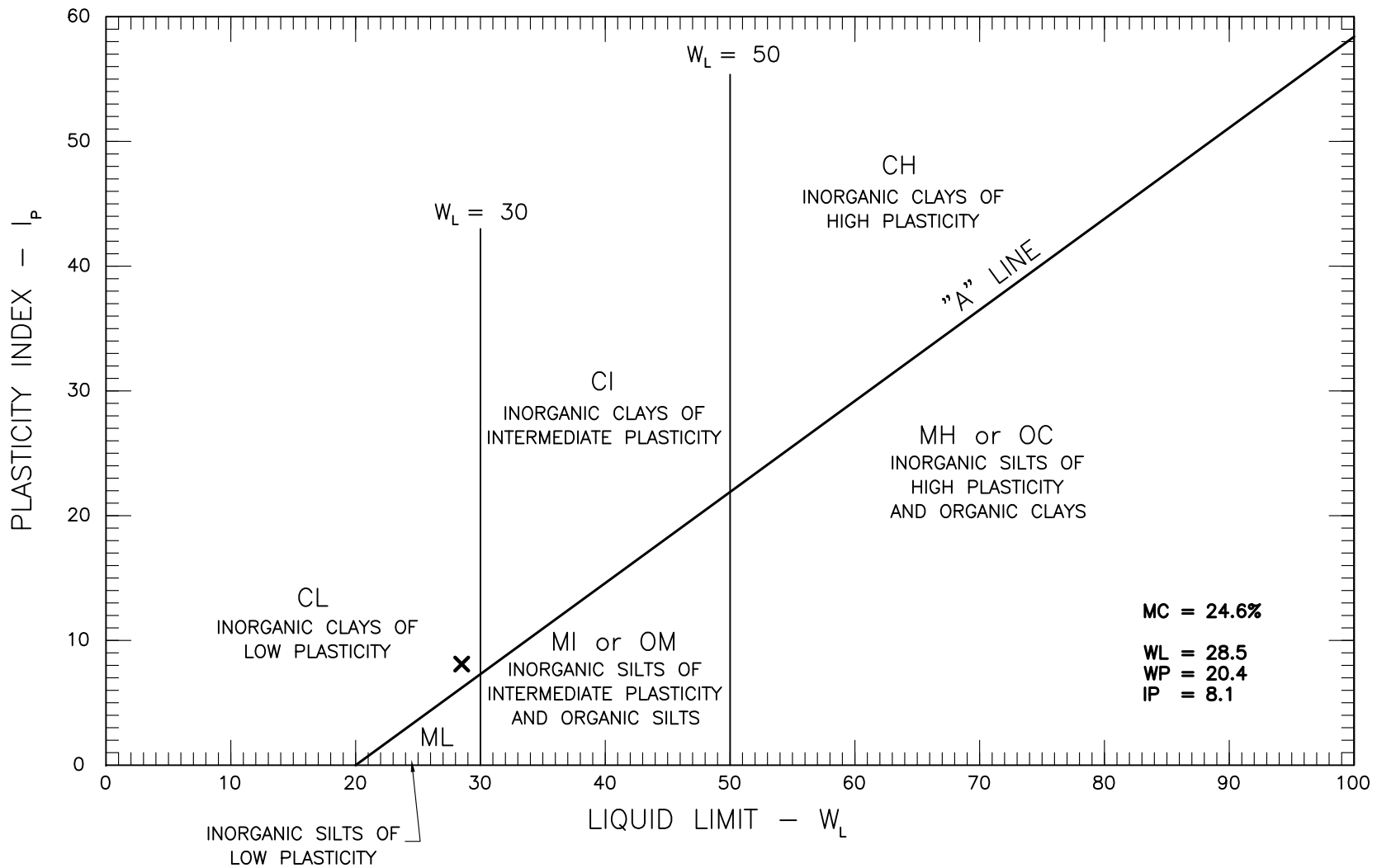
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-120-G1

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-TP120-G1



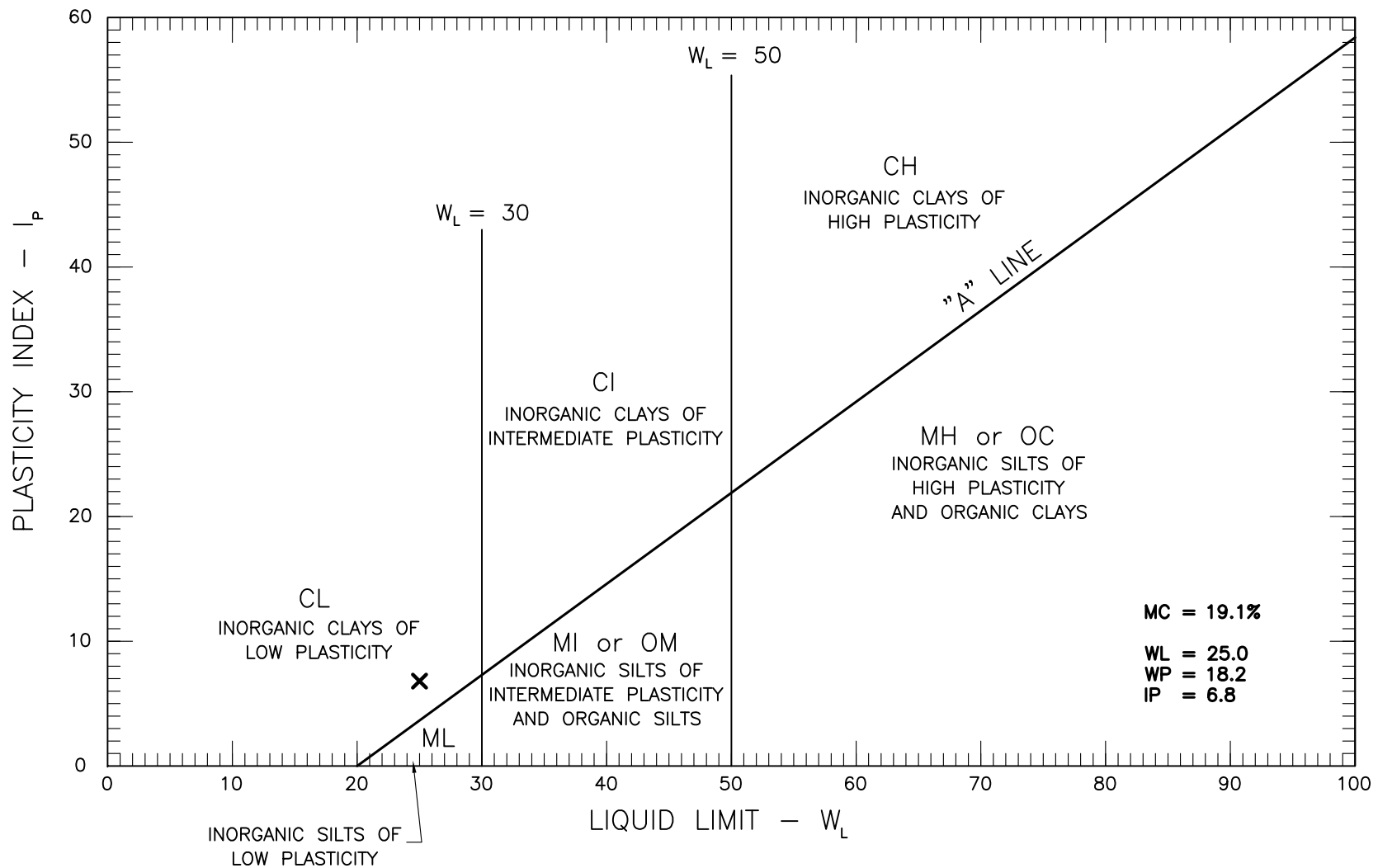
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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-121, 2.9-3.1 m DEPTH

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-TP121



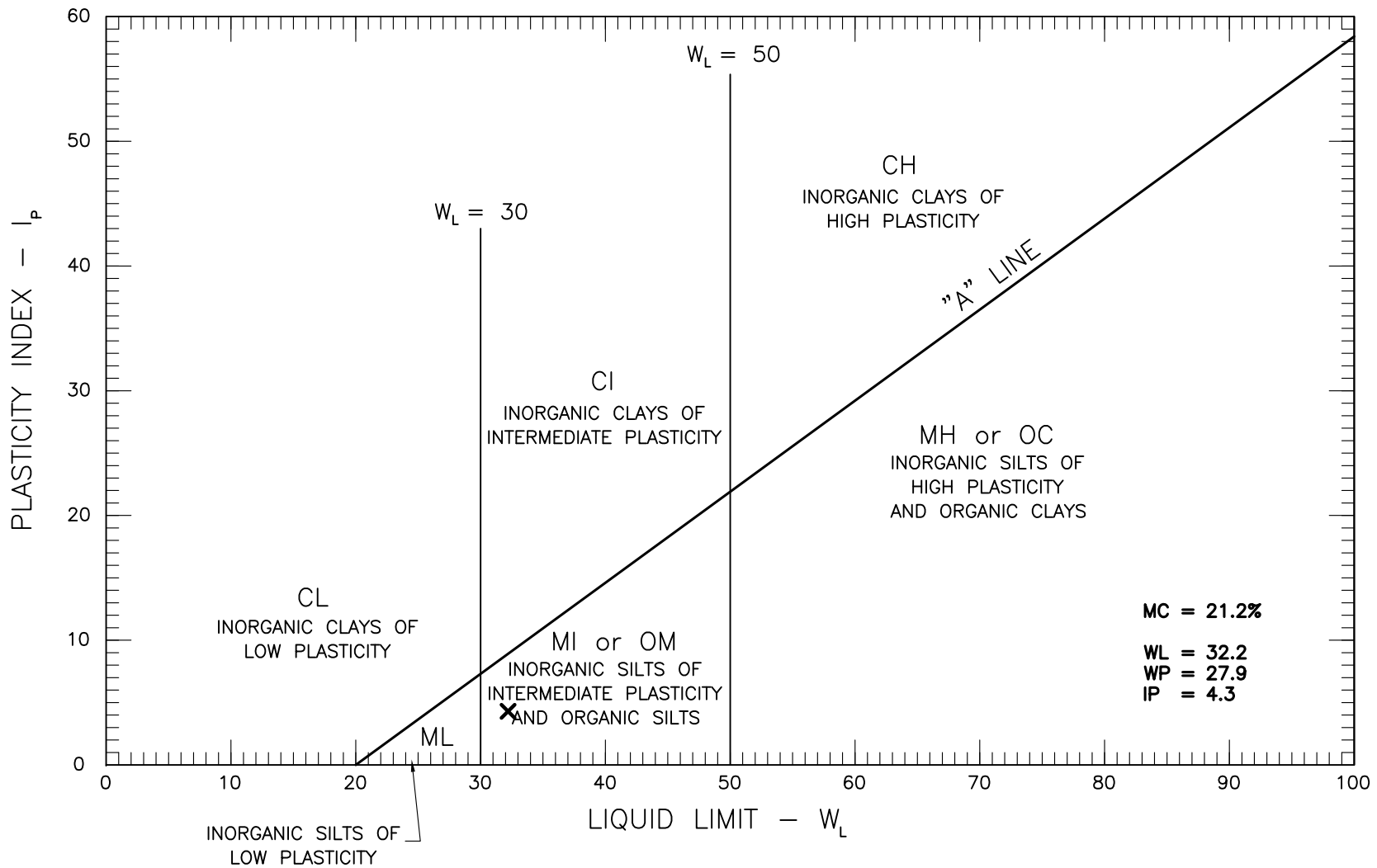
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-122, 5.4-5.8 m DEPTH

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-TP122



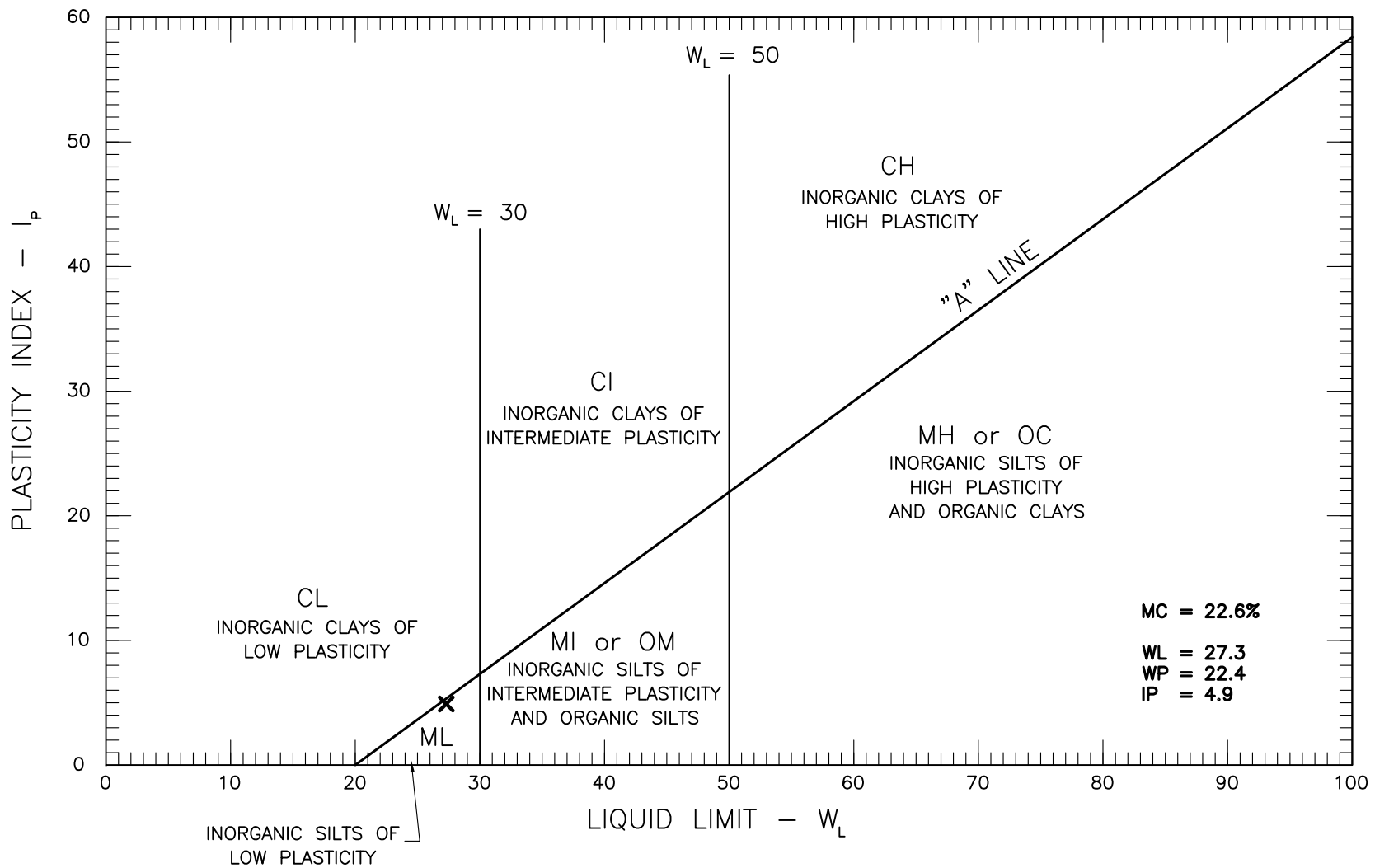
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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-124-G1

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-TP124-G1



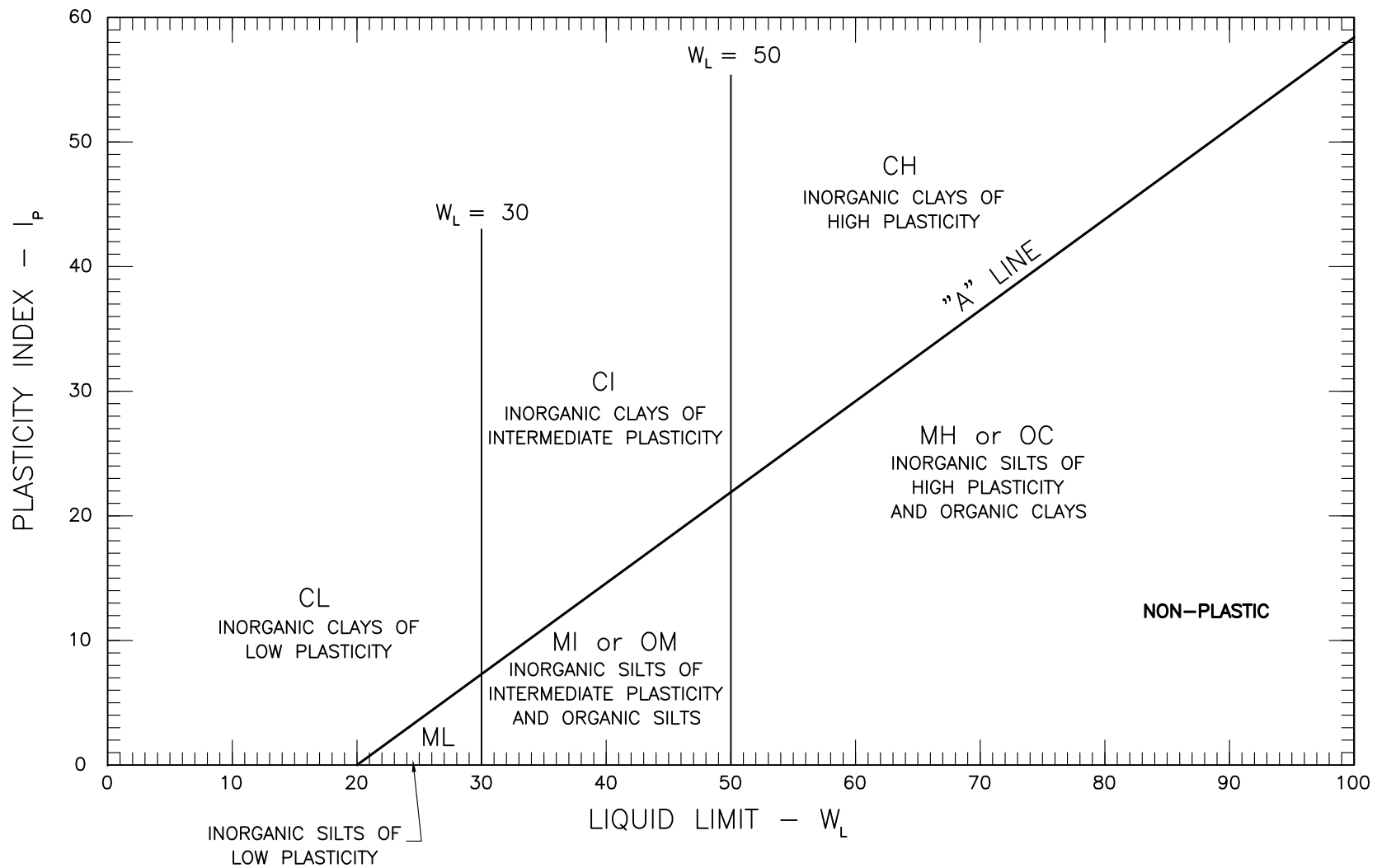
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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-129-G1

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-TP129-G1



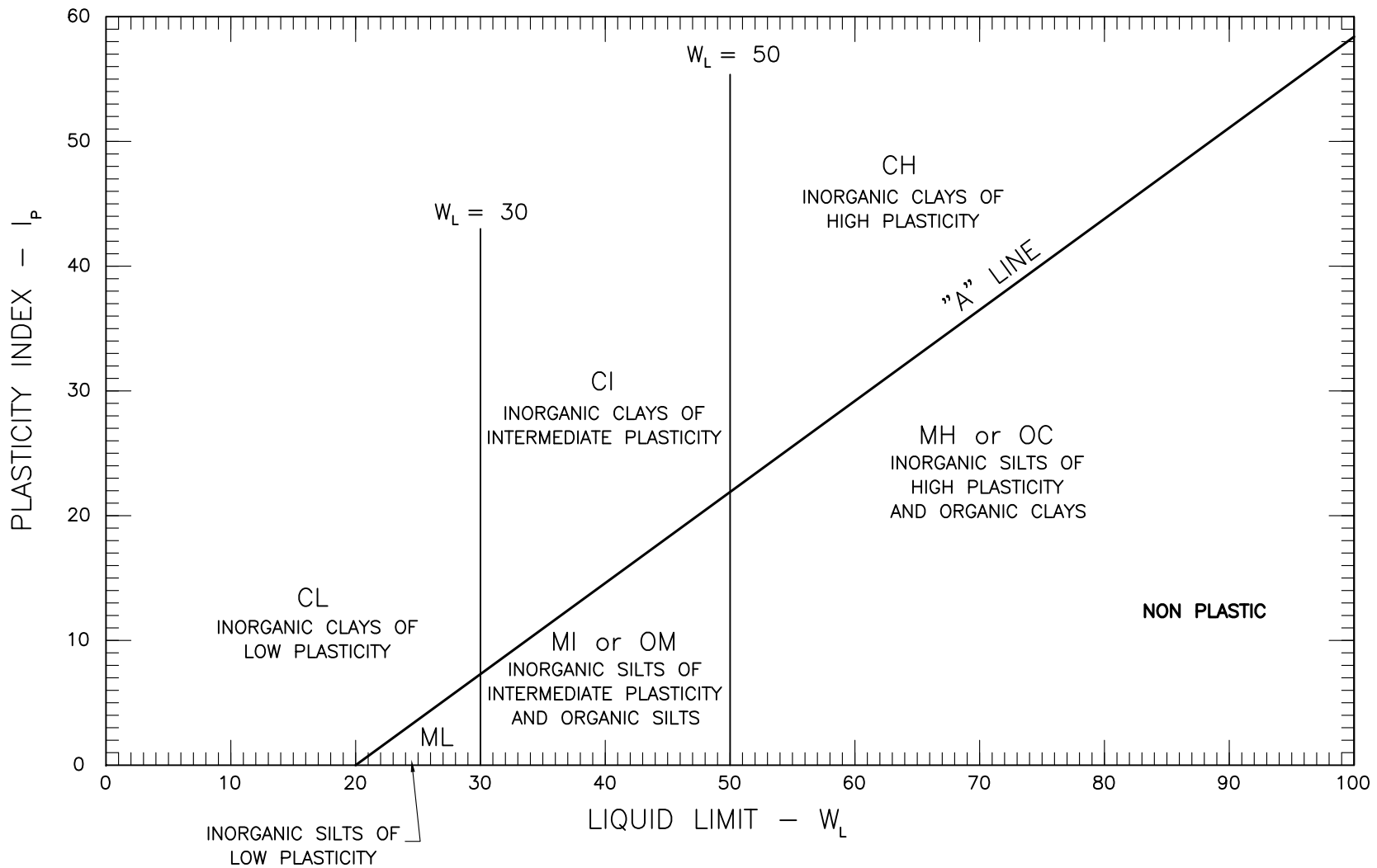
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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-130, 5.8-6.0 m DEPTH

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-TP130

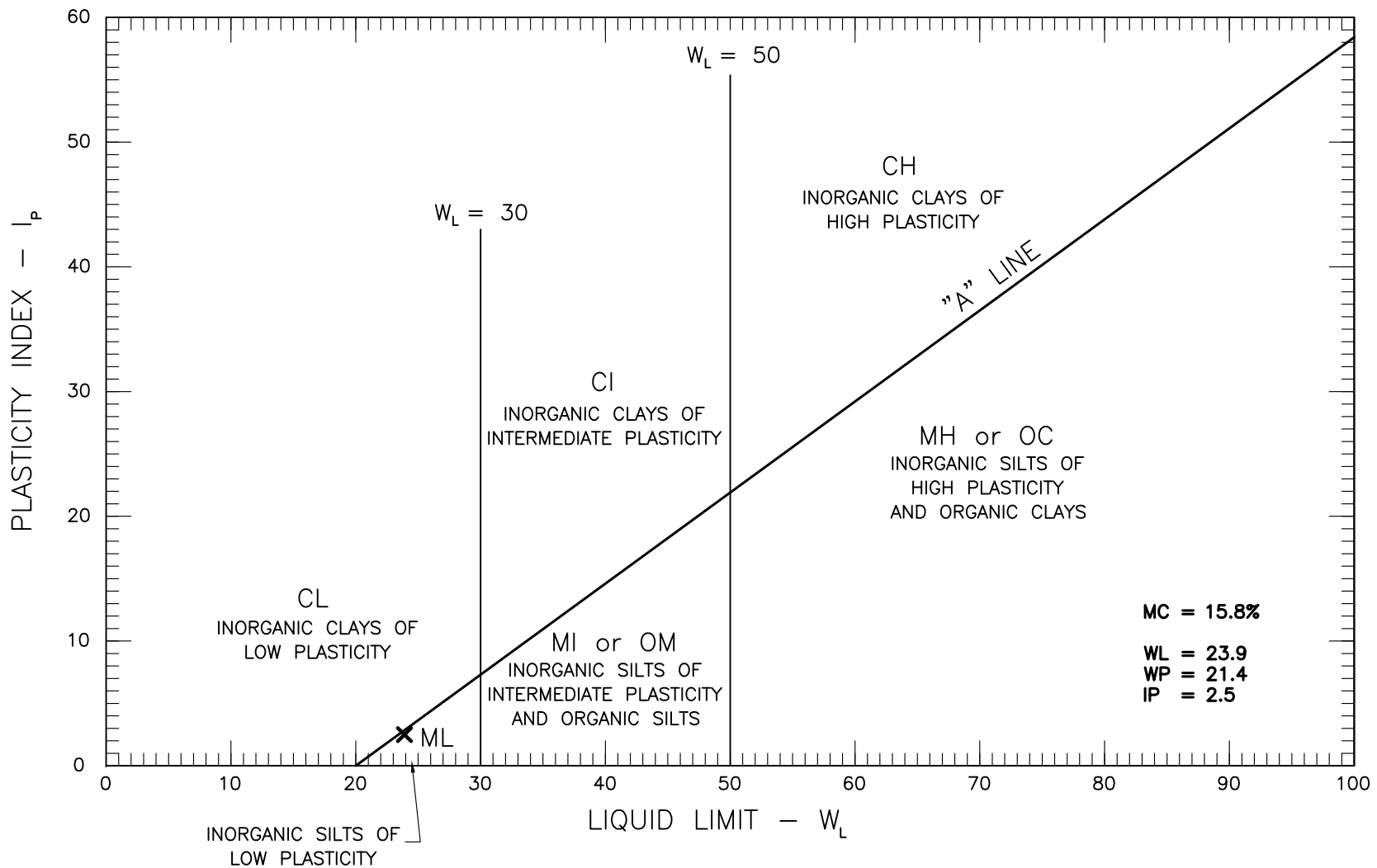


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 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-131-G1

PROJECT NO.
 K-3300
 PLATE NO.
 3300-AL-TP131-G1



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 EAGLE GOLD SOILS AND AGGREGATE TESTING
 DUBLIN GULCH, YUKON
 ATTERBERG LIMITS OF TP-BGC11-140-G1

PROJECT NO.
 K-3300

PLATE NO.
 3300-AL-TP140-G1

Project #: 11-1415-0029/7000
 Short Title: Eagle Gold Mine Site Infra FS SI
 Client: BGC (Proj 0792-006-04)
 Location: Dublin Gulch, Yukon
 Lab ID: 141

Borehole	BH-BGC-29	BH-BGC-34				
Sample Number	1	1				
Depth (m)	27.46-27.77	26.60-36.96				
Bulk G _s (Oven Dry)	2.71	2.56				
Bulk G _s (SSD)	2.73	2.64				
Apparent G _s	2.77	2.78				
% Absorption	0.79	3.14				

Borehole						
Sample Number						
Depth (m)						
Bulk G _s (Oven Dry)						
Bulk G _s (SSD)						
Apparent G _s						
% Absorption						

Borehole						
Sample Number						
Depth (m)						
Bulk G _s (Oven Dry)						
Bulk G _s (SSD)						
Apparent G _s						
% Absorption						

Borehole						
Sample Number						
Depth (m)						
Bulk G _s (Oven Dry)						
Bulk G _s (SSD)						
Apparent G _s						
% Absorption						

G. Patton

September 19, 2011

LPerrey

October 27, 2011

TESTED BY

DATE TESTED

CHECKED BY

DATE CHECKED



Project #: 11-1415-0029/7000
 Short Title: Eagle Gold
 Client: BGC Engineering Inc.
 Location: Yukon
 Lab ID: 141

Borehole	BH-BGC11-40A	BH-BGC11-40B	BH-BGC11-43	BH-BGC11-45	BH-BGC11-46	BH-BGC11-62
Sample Number	2	UCS 1	1	UCS	UCS	UCS
Depth (m)	30.98-31.30	21.81-22.06	4.24-4.48	17.80-18.04	13.11-13.65	28.59-28.84
Bulk G _s (Oven Dry)	2.67	2.74	2.71	2.76	2.69	2.59
Bulk G _s (SSD)	2.68	2.75	2.71	2.78	2.70	2.63
Apparent G _s	2.70	2.78	2.73	2.81	2.72	2.69
% Absorption	0.36	0.54	0.30	0.61	0.48	1.33

Borehole	BH-BGC11-69					
Sample Number	UCS 1					
Depth (m)	12.26-12.54					
Bulk G _s (Oven Dry)	2.52					
Bulk G _s (SSD)	2.59					
Apparent G _s	2.70					
% Absorption	2.64					

Borehole						
Sample Number						
Depth (m)						
Bulk G _s (Oven Dry)						
Bulk G _s (SSD)						
Apparent G _s						
% Absorption						

Borehole						
Sample Number						
Depth (m)						
Bulk G _s (Oven Dry)						
Bulk G _s (SSD)						
Apparent G _s						
% Absorption						

G. Patton
TESTED BY

September 19, 2011
DATE TESTED

LP
CHECKED BY

October 7, 2011
DATE CHECKED



SPECIFIC GRAVITY OF SOIL SOLIDS

Reference
 ASTM C 127-07
 ASTM D 854-06 Method B

Project No.:	11-1415-0029/7000	Borehole	TP-BGC11-50
Client:	BGC (Proj 0792-006-04)	Sample No.:	SA1
Project:	Eagle Gold Mine Site Infra FS SI	Depth (m):	N/A
Location:	Dublin Gulch, Yukon	Lab Sch No:	141

Specific Gravity of Fine Fraction (ASTM D 854-06)

Percentage Passing #4 sieve		15.54	
Test Number		1	2
Flask Number		6	7
Air Removal Method		Vacuum	Vacuum
Mass of Flask (g)		173.40	171.33
Mass of Flask + Dry Soil (g)	M_P	271.20	262.18
Mass of Dry Soil (g)		97.64	90.59
Mass of Flask + Soil + Water (g)	$M_{pws,t}$	734.09	727.40
Test Temperature (g)	T_t	20.50	20.50
Mass of Flask + Water (g)	$M_{pw,t}$	671.34	669.18
Mass of Dish + Dry Soil (g)		467.92	443.84
Mass of Dish (g)		370.28	353.25
Mass of Oven Dry Soil (g)	M_S	97.64	90.59
Temperature Coefficient	K	1.00	1.00
Density of Solids (g/cm ³)	ρ_s	2.80	2.80
Specific Gravity at Test Temperature	G_t	2.80	2.80
Specific Gravity at 20°C	$G_{20^\circ C}$	2.80	2.80
AVERAGE SPECIFIC GRAVITY		2.80	

Specific Gravity of Coarse Fraction (ASTM C 127-07)

Percentage Retained on #4 sieve		84.46
Mass of Sample in Water (g)	A	457.51
Mass of Sample @ SSD (g)	B	718.88
Mass of Oven Dried Sample (g)	C	682.7
Bulk G (Oven Dry)	C/(B-A)	2.61
Bulk G (SSD)	B/(B-A)	2.75
Apparent	C/(C-A)	3.03
Absorbtion (%)	(B-C)/C	5.30

Combined Specific Gravity

COMBINED SPECIFIC GRAVITY	$G_{avg @ 20^\circ C}$	2.99
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** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.*

SK	September 28, 2011	LP	October 21, 2011
TESTED BY	DATE	CHECKED BY	DATE

SPECIFIC GRAVITY OF SOIL SOLIDS

Reference
 ASTM C 127-07
 ASTM D 854-06 Method B

Project No.:	11-1415-0029/7000	Borehole	TP-BGC11-58
Client:	BGC (Proj 0792-006-04)	Sample No.:	SA2
Project:	Eagle Gold	Depth (m):	1.9-2.0
Location:	Dublin Gulch, Yukon	Lab Sch No:	141

Specific Gravity of Fine Fraction (ASTM D 854-06)

Percentage Passing #4 sieve		100	
Test Number		1	2
Flask Number		6	7
Air Removal Method		Vacuum	Vacuum
Mass of Flask (g)		173.41	171.35
Mass of Flask + Dry Soil (g)	M_P	277.90	273.49
Mass of Dry Soil (g)		104.10	101.83
Mass of Flask + Soil + Water (g)	$M_{pws,t}$	738.77	734.87
Test Temperature (g)	T_t	21.70	21.50
Mass of Flask + Water (g)	$M_{pw,t}$	671.22	669.08
Mass of Dish + Dry Soil (g)		466.51	472.53
Mass of Dish (g)		362.41	370.70
Mass of Oven Dry Soil (g)	M_S	104.10	101.83
Temperature Coefficient	K	1.00	1.00
Density of Solids (g/cm ³)	ρ_s	2.85	2.83
Specific Gravity at Test Temperature	G_t	2.85	2.83
Specific Gravity at 20°C	$G_{20^\circ C}$	2.85	2.83
AVERAGE SPECIFIC GRAVITY		2.84	

Specific Gravity of Coarse Fraction (ASTM C 127-07)

Percentage Retained on #4 sieve		
Mass of Sample in Water (g)	A	0
Mass of Sample @ SSD (g)	B	0
Mass of Oven Dried Sample (g)	C	0
Bulk G (Oven Dry)	C/(B-A)	
Bulk G (SSD)	B/(B-A)	
Apparent	C/(C-A)	
Absorbion (%)	(B-C)/C	

Combined Specific Gravity

COMBINED SPECIFIC GRAVITY	$G_{avg @ 20^\circ C}$
----------------------------------	------------------------

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.*

SK	November 1, 2002	LP	NOVEMBER 3, 2011
TESTED BY	DATE	CHECKED BY	DATE

SPECIFIC GRAVITY OF SOIL SOLIDS

Reference
 ASTM C 127-07
 ASTM D 854-06 Method B

Project No.:	11-1415-0029/7000	Borehole	TP-BGC11-67
Client:	BGC (Proj 0792-006-04)	Sample No.:	SA1
Project:	Eagle Gold	Depth (m):	0.7-0.9
Location:	Dublin Gulch, Yukon	Lab Sch No:	141

Specific Gravity of Fine Fraction (ASTM D 854-06)

Percentage Passing #4 sieve		100	
Test Number		1	2
Flask Number		4	5
Air Removal Method		Vacuum	Vacuum
Mass of Flask (g)		172.75	174.83
Mass of Flask + Dry Soil (g)	M_P	272.17	278.92
Mass of Dry Soil (g)		99.45	104.02
Mass of Flask + Soil + Water (g)	$M_{pws,t}$	733.82	738.64
Test Temperature (g)	T_t	21.60	21.80
Mass of Flask + Water (g)	$M_{pw,t}$	671.06	672.89
Mass of Dish + Dry Soil (g)		452.55	461.38
Mass of Dish (g)		353.10	357.36
Mass of Oven Dry Soil (g)	M_S	99.45	104.02
Temperature Coefficient	K	1.00	1.00
Density of Solids (g/cm3)	ρ_s	2.71	2.72
Specific Gravity at Test Temperature	G_t	2.72	2.72
Specific Gravity at 20°C	$G_{20^\circ C}$	2.72	2.72
AVERAGE SPECIFIC GRAVITY		2.72	

Specific Gravity of Coarse Fraction (ASTM C 127-07)

Percentage Retained on #4 sieve		
Mass of Sample in Water (g)	A	0
Mass of Sample @ SSD (g)	B	0
Mass of Oven Dried Sample (g)	C	0
Bulk G (Oven Dry)	C/(B-A)	
Bulk G (SSD)	B/(B-A)	
Apparent	C/(C-A)	
Absorbion (%)	(B-C)/C	

Combined Specific Gravity

COMBINED SPECIFIC GRAVITY	$G_{avg @ 20^\circ C}$	
----------------------------------	------------------------	--

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.*

SK	November 1, 2002	LP	NOVEMBER 3, 2011
TESTED BY	DATE	CHECKED BY	DATE

GEO NORTH ENGINEERING LTD.

3975 18th Avenue
Prince George, B.C., V2N 1B2
Phone 250-564-4304
Fax 250-564-9323
E-mail mail@geonorth.ca

December 7, 2011

K-3300

BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC V6Z 2A9

Attention: Pete Quinn

Dear Sirs:

Re: Eagle Gold BH-BGC11-42 Sample 5 Specific Gravity Test Results

Listed below are results for the specific gravity test that you requested for the above mentioned sample.

Bulk Specific Gravity:	2.533
Bulk Specific Gravity Saturated Surface Dry	2.577
Apparent Specific Gravity	2.649
Absorption	1.718%

If you require any further information, please do not hesitate to call me at our office at (250) 564-4304.

Yours truly,

GeoNorth Engineering Ltd.

Per: Hans Jorgensen
Supervisor, Technical Services



DETERMINATION OF TOTAL OR WATER-SOLUBLE SULPHATE ION CONTENT OF SOIL CSA A23.2-3B

October 27, 2011
Project Number: 11-1415-0029-7000

BGC Engineering Inc.
500 - 1045 Howe Street
Vancouver, BC
V6Z 2A9

ATTENTION: Mr. Peter Quinn, Ph.D, P.Eng.

PROJECT: Eagle Gold Mine Site Infra FS S1, Dublin Gulch, Yukon

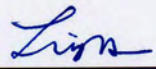
Date Tested: October 25, 2011

Sample ID	Date Sampled	Total Sulphate Ion Content %	Water-Soluble Sulphate Ion Content %
TP-BGC11-51 SA2 2.4-2.8 m	29 June 2011	0.13	Not Applicable *
TP-BGC11-60 SA1 0.7-0.9 m	7 July 2011	0.07	Not Applicable *
TP-BGC11-84 SA1 0.4-0.6 m	13 July 2011	0.07	Not Applicable *

Note:

- * Per Clause 5.1.4, the water-soluble sulphate ion content need not be tested when the total sulphate ion content is less than 0.20%
- Detection limit for the test is 0.005%

Tested by: S. John, AScT

Reviewed by: 
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the samples provided. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

GOLDER ASSOCIATES LTD., #245 - 12388 - 88th Avenue, Surrey, B.C. Canada V3W 7R7 Tel: 604-591-6616 Fax: 604-591-6608

GeoNorth Engineering Ltd.

3975 18th Avenue Prince George, BC V2N 1B2

Phone (250)564-4304; Fax (250)564-9323

MOISTURE - DENSITY RELATIONSHIP REPORT

PROJECT NO. K-3300

CLIENT BGC Engineering Inc.

C.C.

TO
BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC
V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

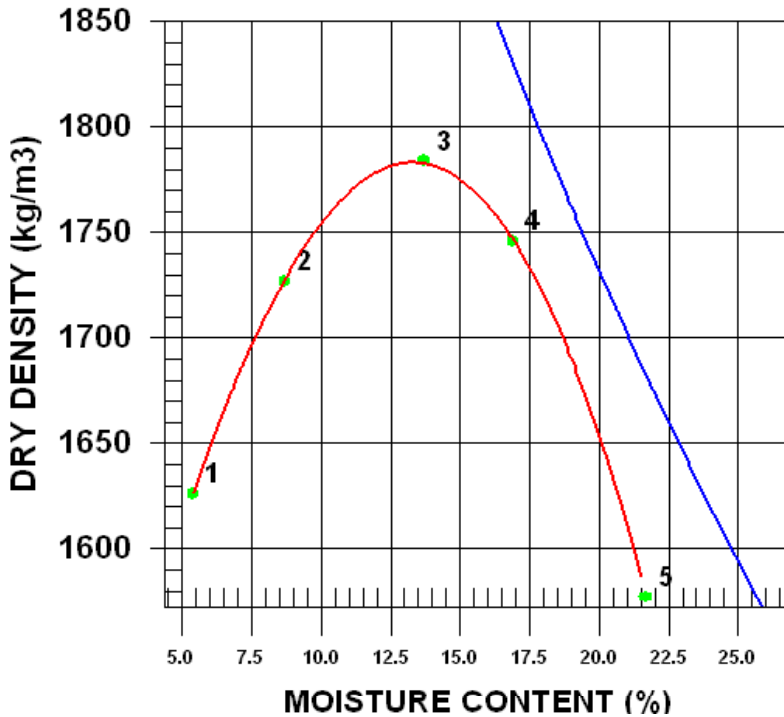
PROCTOR NO. 3

DATE TESTED 2011.Nov.10

DATE RECEIVED 2011.Sep.13

DATE SAMPLED 2011.Aug.18

INSITU MOISTURE	N/A %	COMPACTION STANDARD	Modified Proctor,
SAMPLED BY	Client		ASTM D1557
TESTED BY	MM	COMPACTION PROCEDURE	A: 101.6mm Mold,
SUPPLIER	TP-BGC11-119		Passing 4.75mm
SOURCE	G1 @ 0.9 - 1.1m	RAMMER TYPE	Automatic
MATERIAL IDENTIFICATION		PREPARATION	Moist
MAJOR COMPONENT	Silt	OVERSIZE CORRECTION METHOD	None
SIZE	4.75mm	RETAINED 4.75mm SCREEN	%
DESCRIPTION		OVERSIZE SPECIFIC GRAVITY	
ROCK TYPE		TOTAL NUMBER OF TRIALS	5



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1714	1626	5.4
2	1877	1727	8.7
3	2028	1784	13.7
4	2041	1746	16.9
5	1919	1577	21.7

ZERO AIR VOIDS CURVE FOR ESTIMATED SPECIFIC GRAVITY OF 2.65	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED OVERSIZE CORRECTED	1780	13.5

COMMENTS

Permeability at 95% MPD (Over Optimum Moisture Conditions): 4.2×10^{-8} cm/s

PROJECT NO. K-3300

CLIENT BGC Engineering Inc.

C.C.

TO
BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC
V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

PROCTOR NO. 4

DATE TESTED 2011.Nov.10

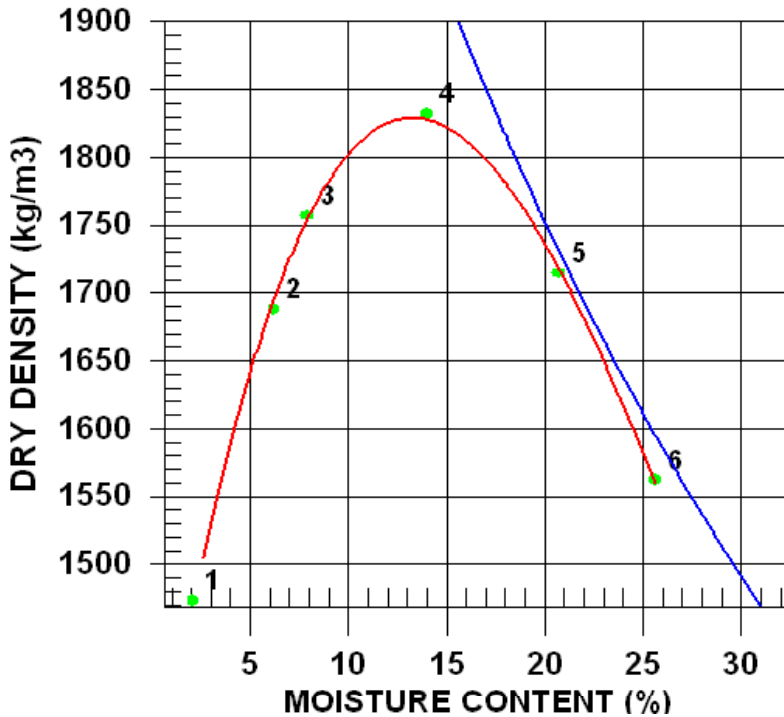
DATE RECEIVED 2011.Sep.13

DATE SAMPLED 2011.Aug.18

INSITU MOISTURE N/A %
SAMPLED BY Client
TESTED BY MM
SUPPLIER TP-BGC11-120
SOURCE G1 @ 3.8 - 4.0m

COMPACTION STANDARD Modified Proctor,
ASTM D1557
COMPACTION PROCEDURE A: 101.6mm Mold,
Passing 4.75mm
RAMMER TYPE Automatic
PREPARATION Moist
OVERSIZE CORRECTION METHOD ASTM 4718
RETAINED 4.75mm SCREEN 2.4 %
OVERSIZE SPECIFIC GRAVITY 2.65
TOTAL NUMBER OF TRIALS 6

MATERIAL IDENTIFICATION
MAJOR COMPONENT Silt
SIZE 25 mm
DESCRIPTION
ROCK TYPE



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1504	1473	2.1
2	1793	1688	6.2
3	1896	1757	7.9
4	2089	1832	14.0
5	2070	1715	20.7
6	1962	1562	25.6

ZERO AIR VOIDS CURVE FOR ESTIMATED SPECIFIC GRAVITY OF 2.70	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1830	13.5
OVERSIZE CORRECTED	1840	13.0

COMMENTS

Permeability at 95% MPD (Over Optimum Moisture Conditions): 6.5×10^{-8} cm/s

GeoNorth Engineering Ltd.

3975 18th Avenue Prince George, BC V2N 1B2

Phone (250)564-4304; Fax (250)564-9323

MOISTURE - DENSITY RELATIONSHIP REPORT

PROJECT NO. K-3300

CLIENT BGC Engineering Inc.

C.C.

TO
BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC
V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

PROCTOR NO. 1

DATE TESTED 2011.Nov.09

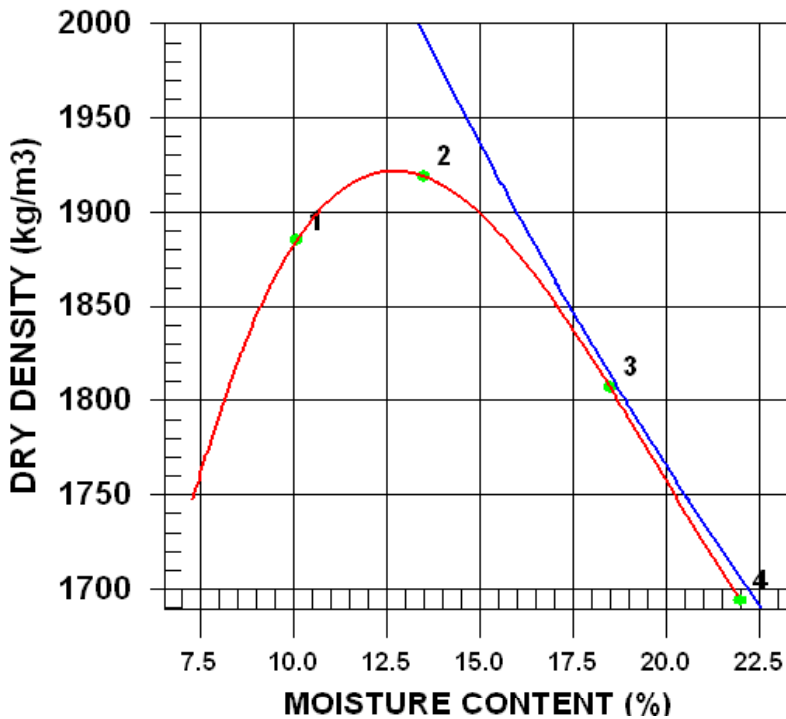
DATE RECEIVED 2011.Sep.13

DATE SAMPLED 2011.Aug.18

INSITU MOISTURE N/A %
SAMPLED BY Client
TESTED BY RO
SUPPLIER TP-BGC11-121
SOURCE G1 @ 2.9-3.1m

COMPACTION STANDARD Modified Proctor,
ASTM D1557
COMPACTION PROCEDURE A: 101.6mm Mold,
Passing 4.75mm
RAMMER TYPE Automatic
PREPARATION Moist
OVERSIZE CORRECTION METHOD ASTM 4718
RETAINED 4.75mm SCREEN 4.3 %
OVERSIZE SPECIFIC GRAVITY 2.65
TOTAL NUMBER OF TRIALS 4

MATERIAL IDENTIFICATION
MAJOR COMPONENT Silt, sandy
SIZE 37.5mm
DESCRIPTION
ROCK TYPE



TRIAL NUMBER	WET DENSITY (kg/m ³)	DRY DENSITY (kg/m ³)	MOISTURE CONTENT (%)
1	2075	1885	10.1
2	2178	1919	13.5
3	2141	1807	18.5
4	2067	1694	22.0

ZERO AIR VOIDS CURVE FOR ESTIMATED SPECIFIC GRAVITY OF 2.73	MAXIMUM DRY DENSITY (kg/m ³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1920	12.5
OVERSIZE CORRECTED	1940	12.0

COMMENTS

Permeability at 95% MPD (Over Optimum Moisture Conditions): 4.0×10^{-8} cm/s

GeoNorth Engineering Ltd.

3975 18th Avenue Prince George, BC V2N 1B2

Phone (250)564-4304; Fax (250)564-9323

MOISTURE - DENSITY RELATIONSHIP REPORT

PROJECT NO. K-3300

CLIENT BGC Engineering Inc.

C.C.

TO
BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC
V6Z 2A9

ATTN: Peter Quinn

PROJECT Eagle Gold Soils and Aggregate Testing

Dublin Gulch, Yukon

CONTRACTOR

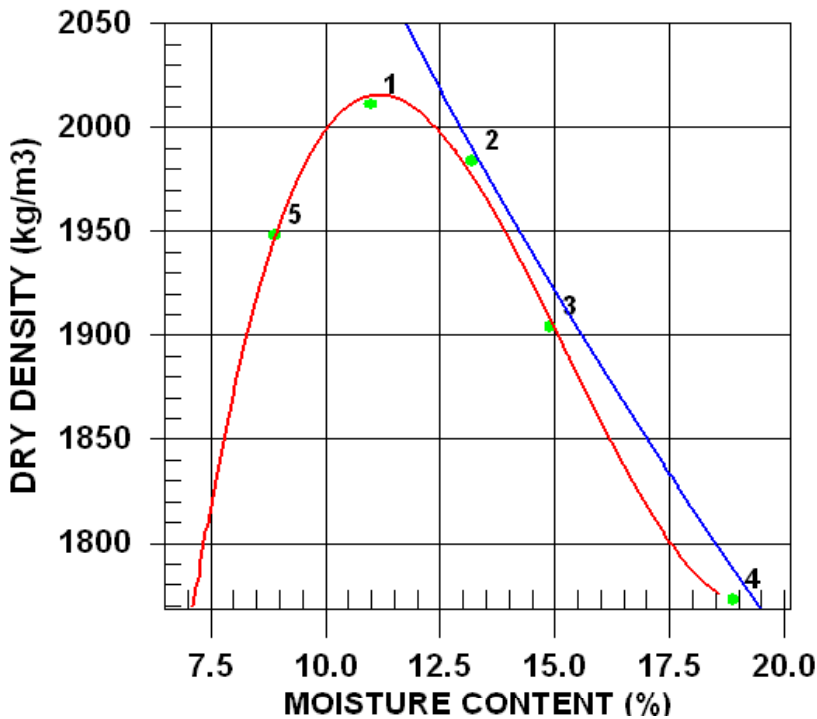
PROCTOR NO. 2

DATE TESTED 2011.Nov.09

DATE RECEIVED 2011.Sep.13

DATE SAMPLED 2011.Aug.18

INSITU MOISTURE	N/A %	COMPACTION STANDARD	Modified Proctor,
SAMPLED BY	Client		ASTM D1557
TESTED BY	RO	COMPACTION PROCEDURE	A: 101.6mm Mold,
SUPPLIER	TP-BGC11-122		Passing 4.75mm
SOURCE	G1 @ 5.4-5.8m	RAMMER TYPE	Automatic
MATERIAL IDENTIFICATION		PREPARATION	Moist
MAJOR COMPONENT	Silt, sandy	OVERSIZE CORRECTION METHOD	ASTM 4718
SIZE	75 mm	RETAINED 4.75mm SCREEN	17.8 %
DESCRIPTION		OVERSIZE SPECIFIC GRAVITY	2.65
ROCK TYPE		TOTAL NUMBER OF TRIALS	5

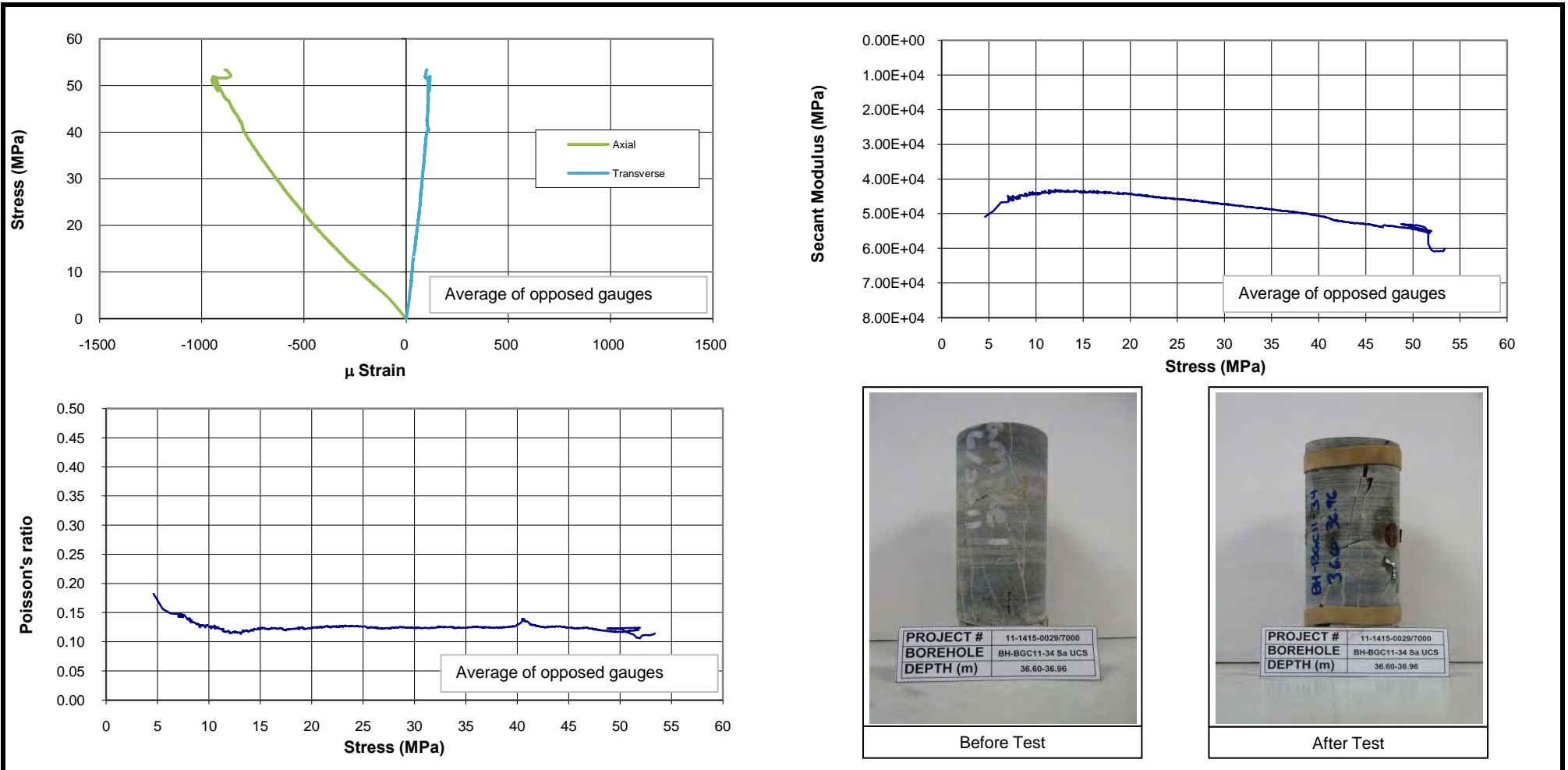


TRIAL NUMBER	WET DENSITY (kg/m ³)	DRY DENSITY (kg/m ³)	MOISTURE CONTENT (%)
1	2232	2011	11.0
2	2246	1984	13.2
3	2188	1904	14.9
4	2108	1773	18.9
5	2121	1948	8.9

ZERO AIR VOIDS CURVE FOR ESTIMATED SPECIFIC GRAVITY OF 2.70	MAXIMUM DRY DENSITY (kg/m ³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	2020	11.0
OVERSIZE CORRECTED	2110	9.0

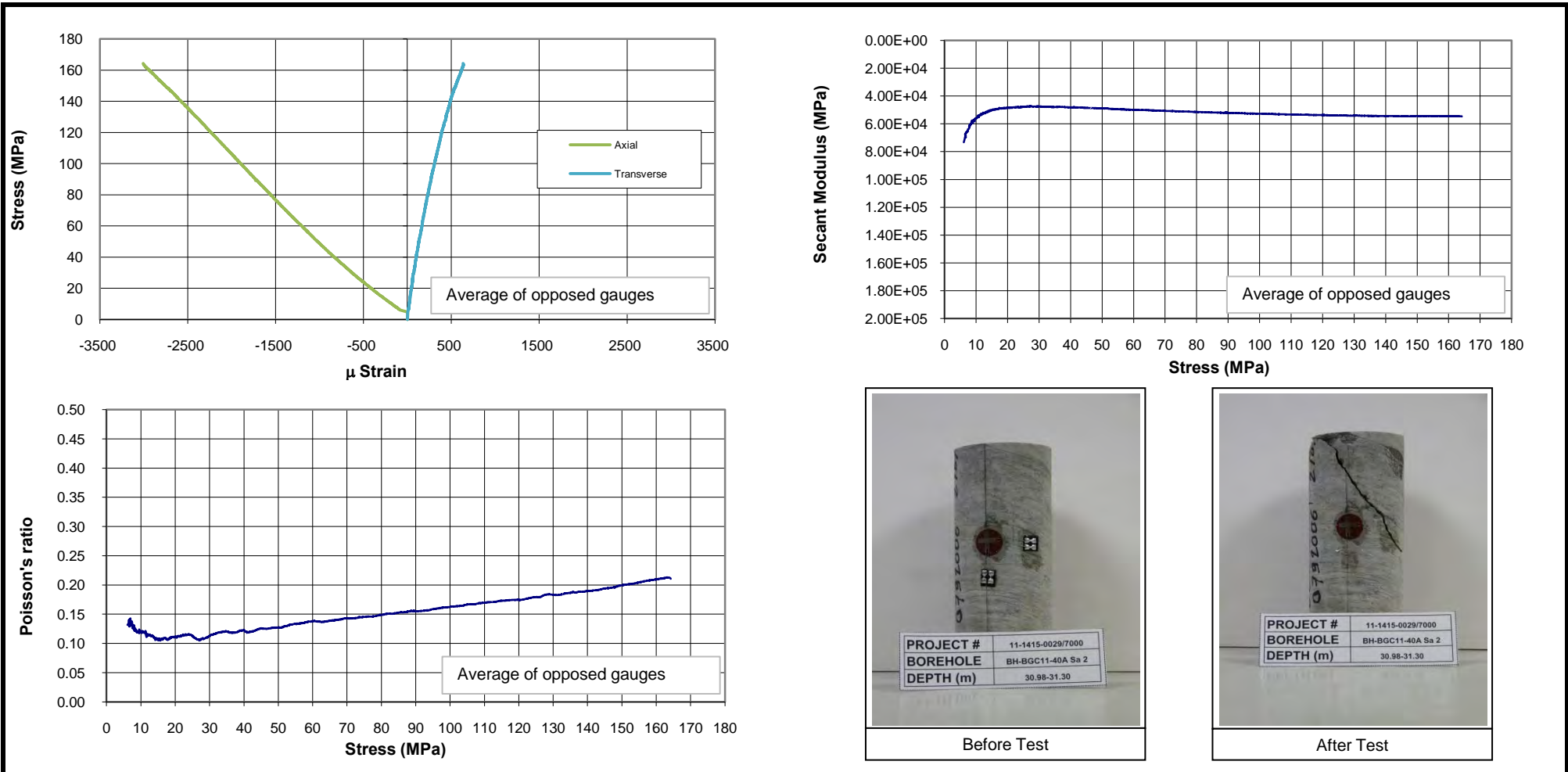
COMMENTS

Permeability at 95% MPD (Over Optimum Moisture Conditions): 3.15×10^{-8} cm/s



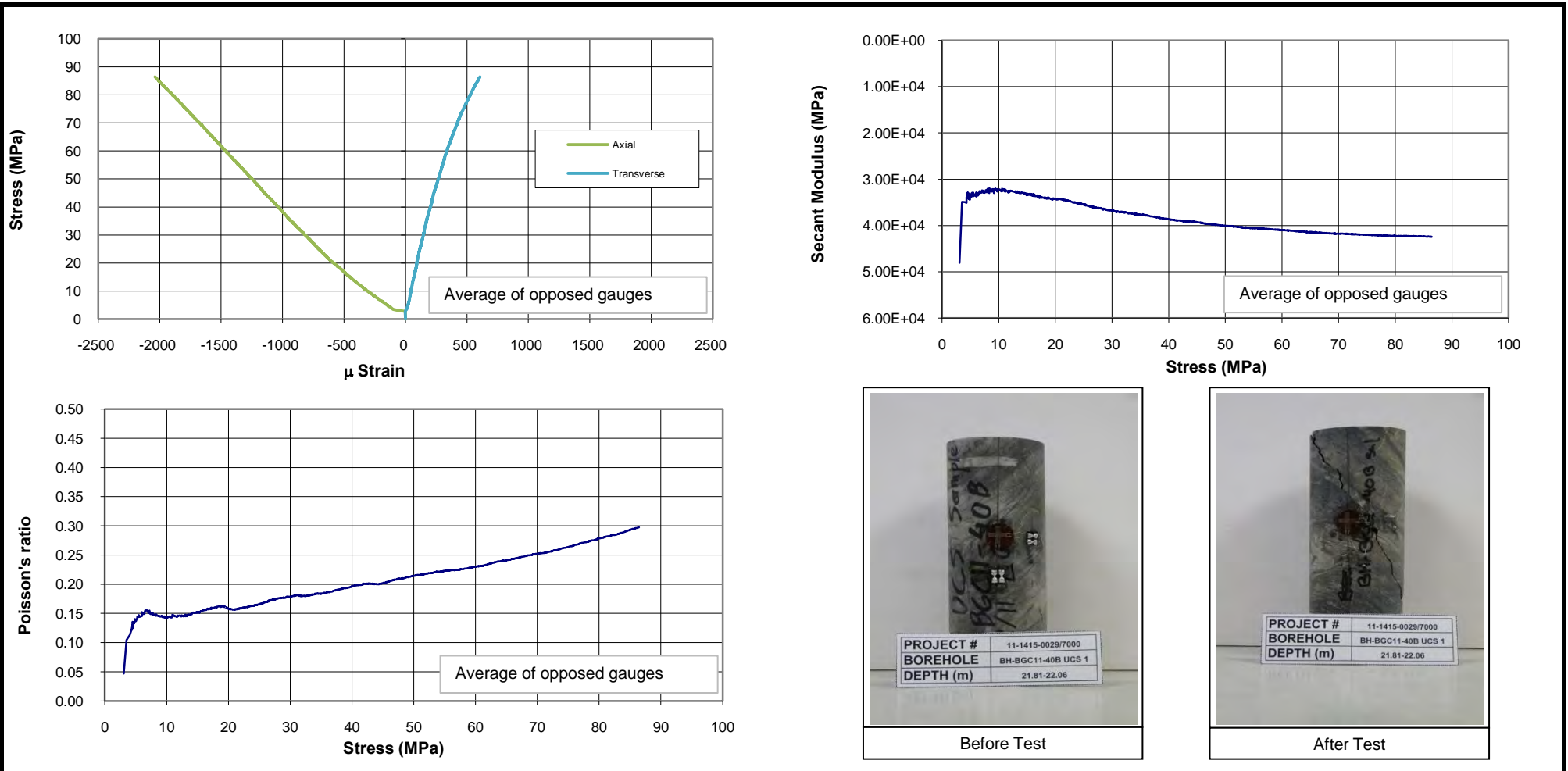
Test Summary	ASTM D7012-07 Method D Modulus in Uniaxial Compression	Project Details
Peak Stress σ_{peak} 53.4 MPa	Borehole: BH-BGC11-34	Project No.: 11-1415-0029/7000
Secant Modulus, ϵ_{50} 46.3 GPa	Sample: UCS	Project: Eagle Gold Mine Site Infra FS SI
Poisson's Ratio, ν_{50} 0.12	Depth (m): 36.60-36.96	Location Dublin Gulch, Yukon
Height: 126.80 mm	Tested By: G. Patton	Client BGC (Proj 0792-006-04)
Diameter: 60.83 mm	Reviewed By LPerrey	
Failure Mode: Along Discontinuities		

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.



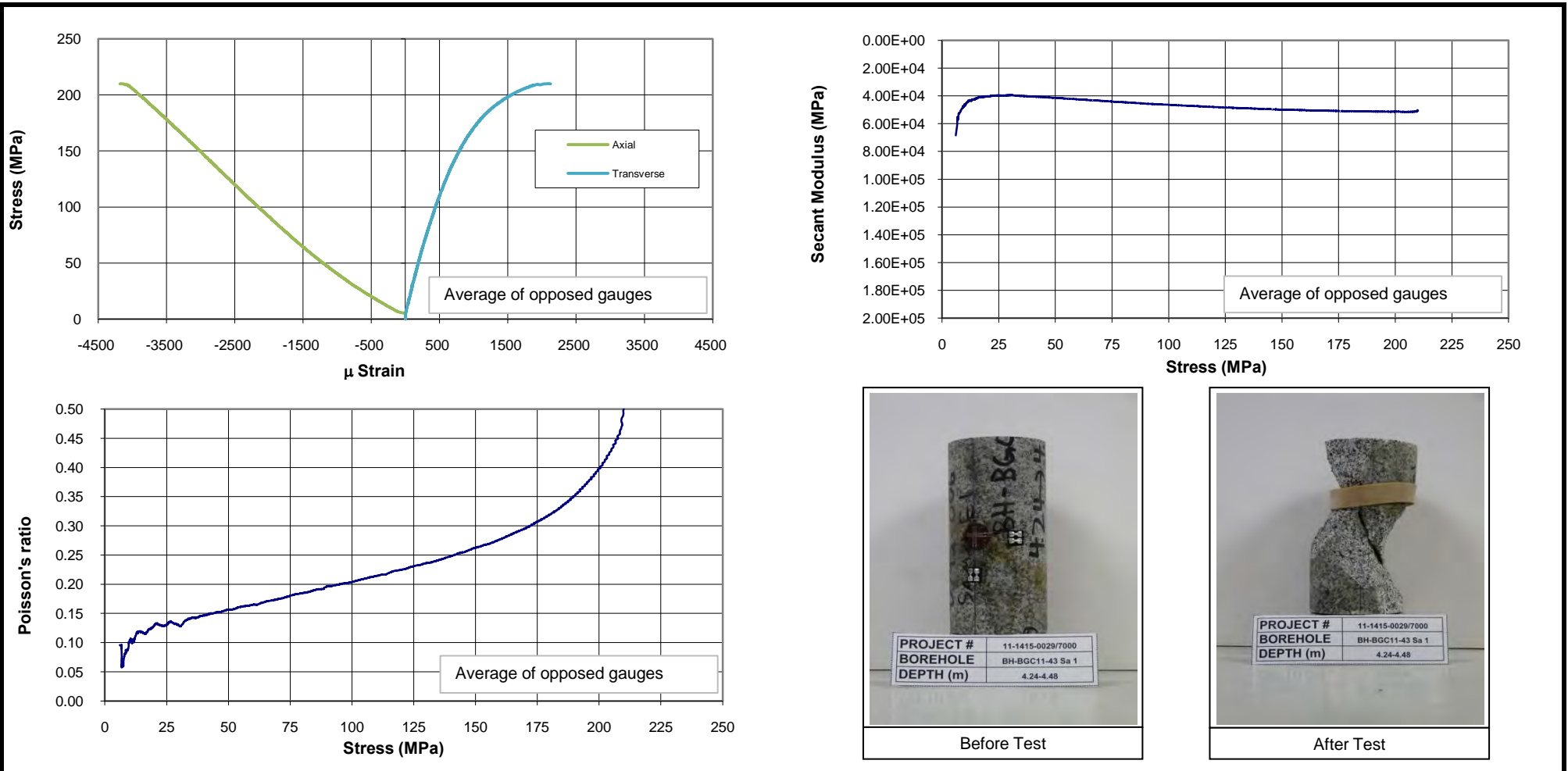
Test Summary		ASTM D7012-07 Method D Modulus in Uniaxial Compression		Project Details	
Peak Stress σ_{peak}	164.2 MPa	Borehole:	BH-BGC11-40A	Project No.:	11-1415-0029/7000
Secant Modulus, ϵ_{50}	51.6 GPa	Sample:	2	Project:	Eagle Gold
Poisson's Ratio, ν_{50}	0.15	Depth (m):	30.98-31.30	Location:	Yukon
Height:	122.85 mm	Tested By:	G. Patton	Client:	BGC Engineering Inc.
Diameter:	60.62 mm	Reviewed By:	EK		
Failure Mode:	Shear 33°				

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.



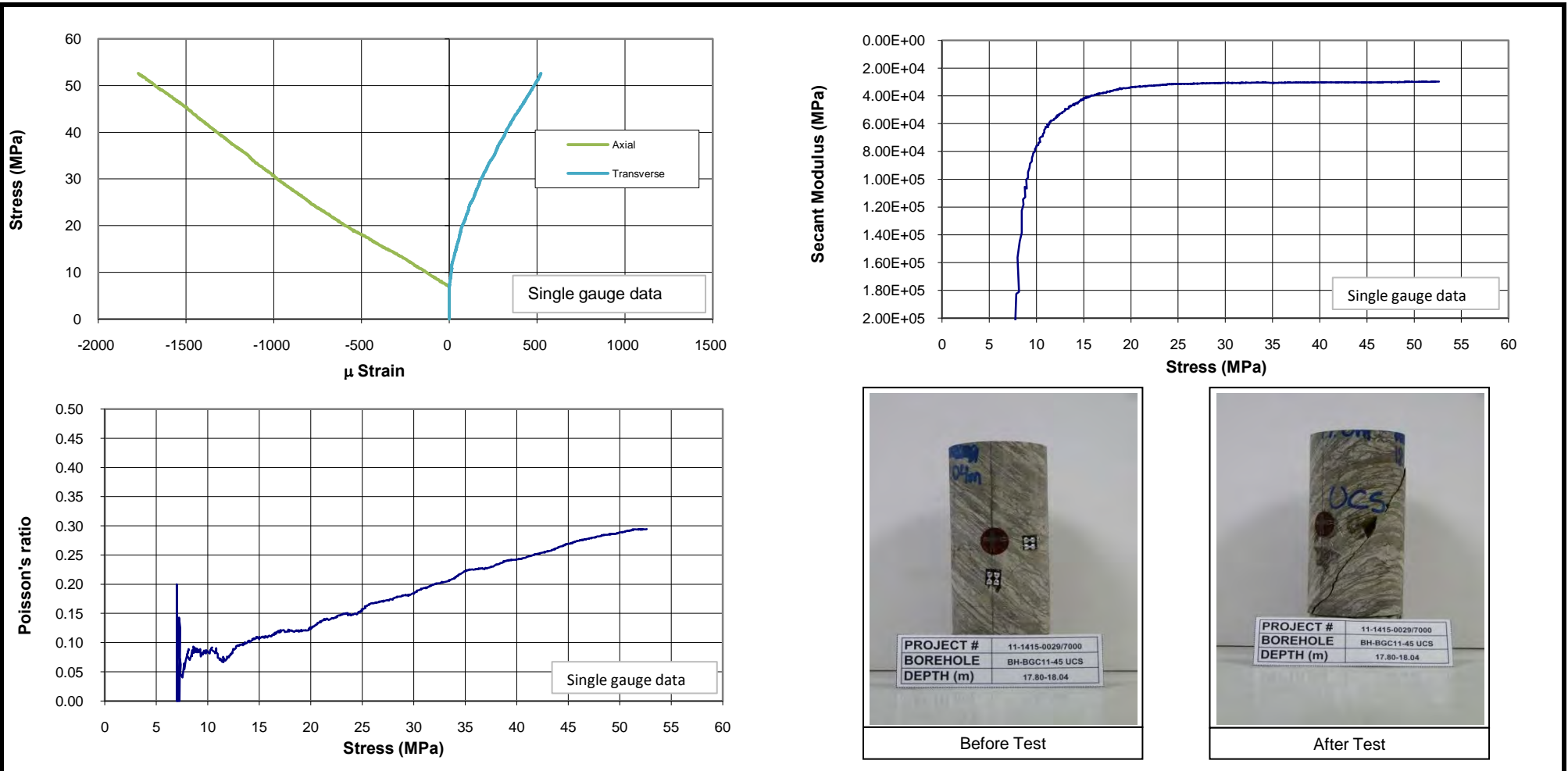
Test Summary		ASTM D7012-07 Method D Modulus in Uniaxial Compression		Project Details	
Peak Stress σ_{peak}	86.5 MPa	Borehole:	BH-BGC11-40B	Project No.:	11-1415-0029/7000
Secant Modulus, ϵ_{50}	3.9 GPa	Sample:	UCS 1	Project:	Eagle Gold
Poisson's Ratio, ν_{50}	0.20	Depth (m):	21.81-22.06	Location:	Yukon
Height:	125.77 mm	Tested By:	G. Patton	Client:	BGC Engineering Inc.
Diameter:	61.05 mm	Reviewed By:	EK		
Failure Mode:	Shear along foliation 30°				

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.



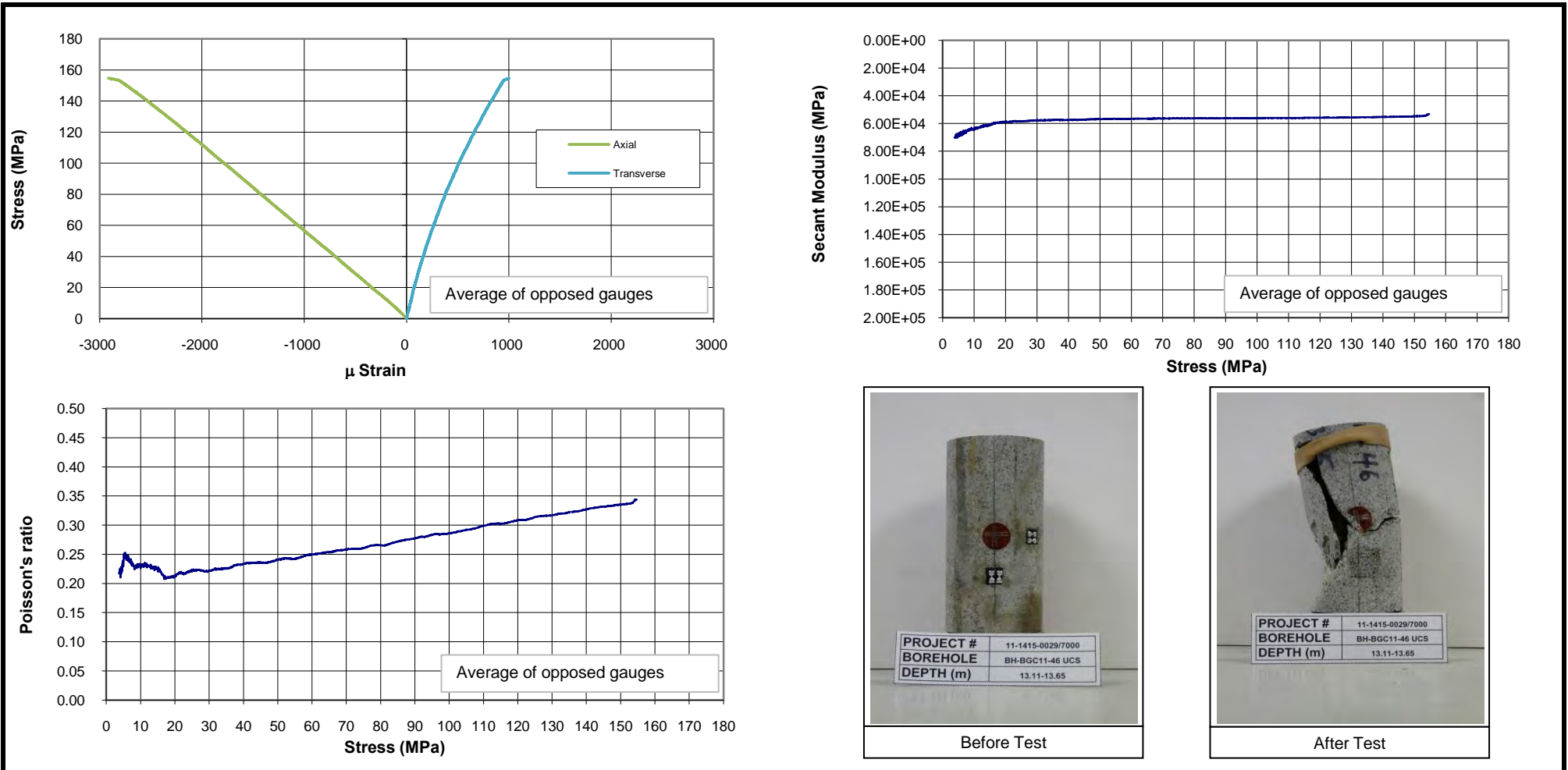
Test Summary		ASTM D7012-07 Method D Modulus in Uniaxial Compression		Project Details	
Peak Stress σ_{peak}	210.1 MPa	Borehole:	BH-BGC11-43	Project No.:	11-1415-0029/7000
Secant Modulus, ϵ_{50}	46.8 GPa	Sample:	1	Project:	Eagle Gold
Poisson's Ratio, ν_{50}	0.21	Depth (m):	4.24-4.48	Location:	Yukon
Height:	126.25 mm	Tested By:	G. Patton	Client:	BGC Engineering Inc.
Diameter:	60.95 mm	Reviewed By:	EK		
Failure Mode:	Shear 27°				

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.



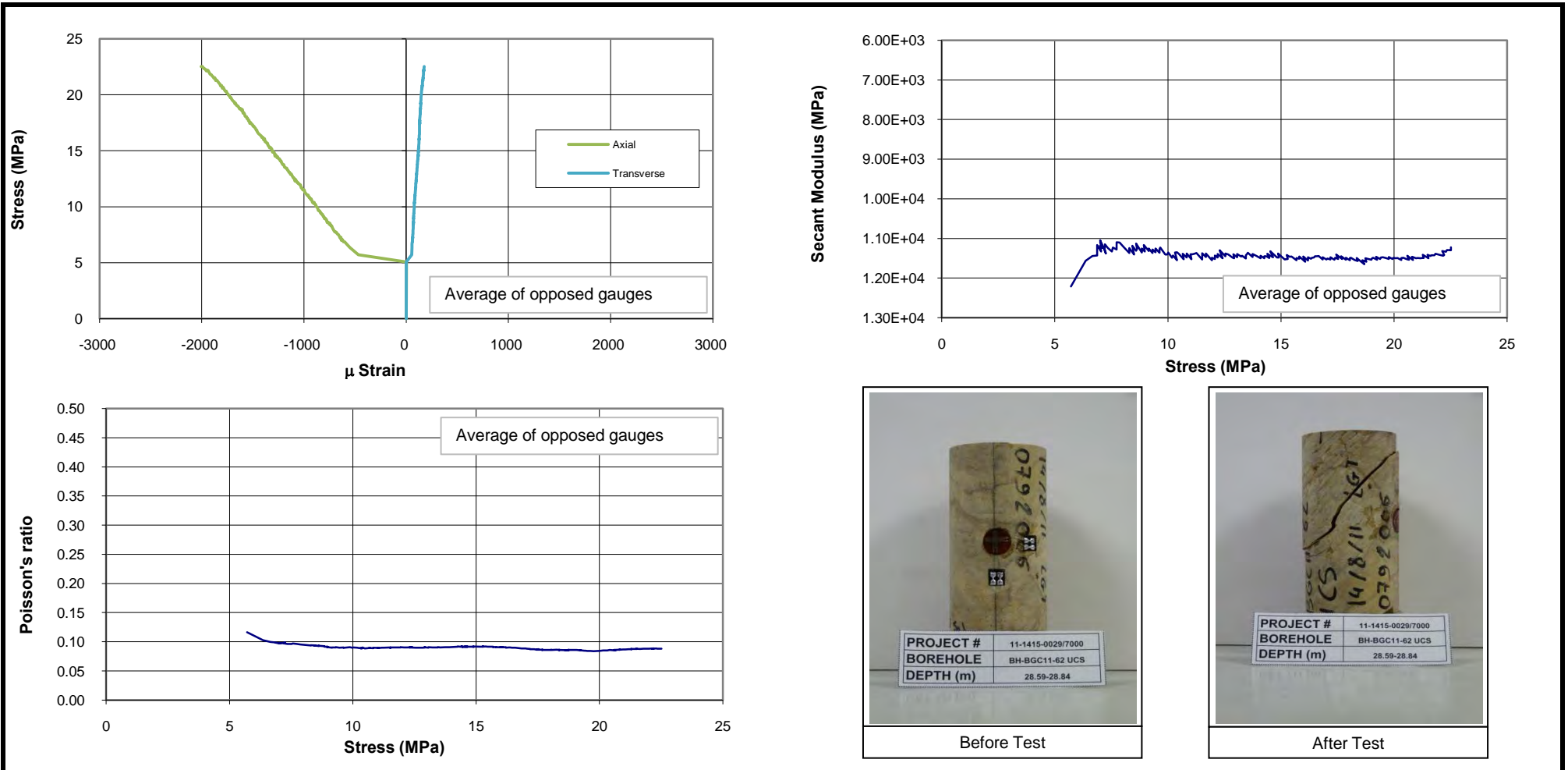
Test Summary	ASTM D7012-07 Method D Modulus in Uniaxial Compression	Project Details
Peak Stress σ_{peak} 52.6 MPa	Borehole: BH-BGC11-45	Project No.: 11-1415-0029/7000
Secant Modulus, ϵ_{50} 31.1 GPa	Sample: UCS	Project: Eagle Gold
Poisson's Ratio, ν_{50} 0.17	Depth (m): 17.80-18.04	Location Yukon
Height: 123.14 mm	Tested By: G. Patton	Client BGC Engineering Inc.
Diameter: 60.77 mm	Reviewed By EK	
Failure Mode: Shear along foliation 33°		

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.



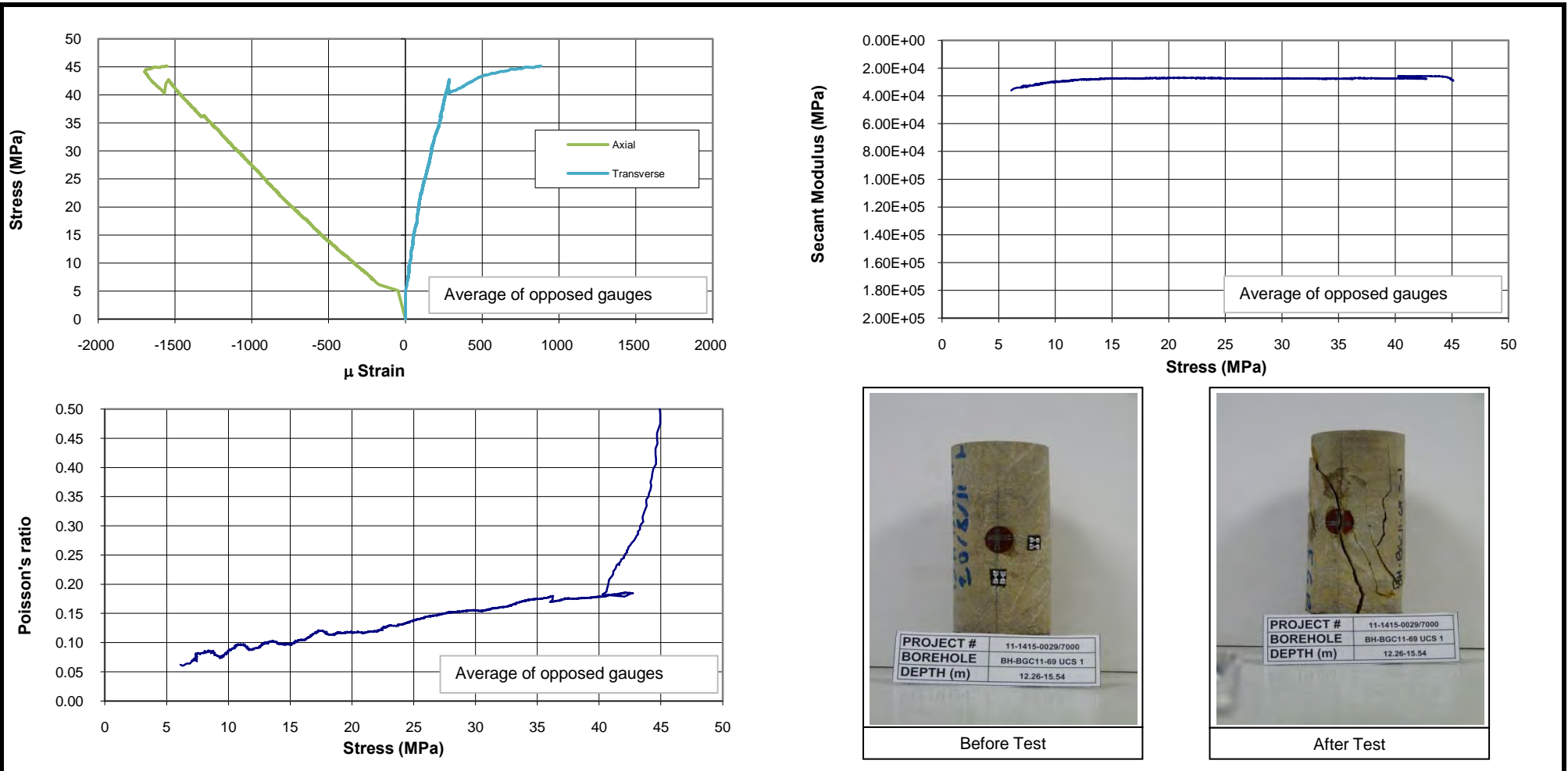
Test Summary		ASTM D7012-07 Method D Modulus in Uniaxial Compression		Project Details	
Peak Stress σ_{peak}	154.7 MPa	Borehole:	BH-BGC11-46	Project No.:	11-1415-0029/7000
Secant Modulus, ϵ_{50}	56.3 GPa	Sample:	UCS	Project:	Eagle Gold
Poisson's Ratio, ν_{50}	0.27	Depth (m):	13.11-13.65	Location:	Yukon
Height:	126.37 mm	Tested By:	G. Patton	Client:	BGC Engineering Inc.
Diameter:	60.80 mm	Reviewed By:	EK		
Failure Mode:	Multi-Vertical				

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.



Test Summary		ASTM D7012-07 Method D Modulus in Uniaxial Compression		Project Details	
Peak Stress σ_{peak}	22.5 MPa	Borehole:	BH-BGC11-62	Project No.:	11-1415-0029/7000
Secant Modulus, ϵ_{50}	11.4 GPa	Sample:	UCS	Project:	Eagle Gold
Poisson's Ratio, ν_{50}	0.09	Depth (m):	28.59-28.84	Location:	Yukon
Height:	122.75 mm	Tested By:	G. Patton	Client:	BGC Engineering Inc.
Diameter:	60.89 mm	Reviewed By:	EK		
Failure Mode:	Shear along foliation 38°				

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.



Test Summary		ASTM D7012-07 Method D Modulus in Uniaxial Compression		Project Details	
Peak Stress σ_{peak}	45.1 MPa	Borehole:	BH-BGC11-69	Project No.:	11-1415-0029/7000
Secant Modulus, ϵ_{50}	27.1 GPa	Sample:	UCS 1	Project:	Eagle Gold
Poisson's Ratio, ν_{50}	0.13	Depth (m):	12.26-12.54	Location:	Yukon
Height:	123.03 mm	Tested By:	G. Patton	Client:	BGC Engineering Inc.
Diameter:	60.83 mm	Reviewed By:	EK		
Failure Mode:	Shear 14°				

The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.

Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-29
Project: Eagle Gold Mine Site Infra FS SI	Sample Number:	Sa 1 BTS 1
Location: Dublin Gulch, Yukon	Depth (m):	27.46-27.77
Client: BGC (Proj 0792-006-04)	Lab ID No:	141

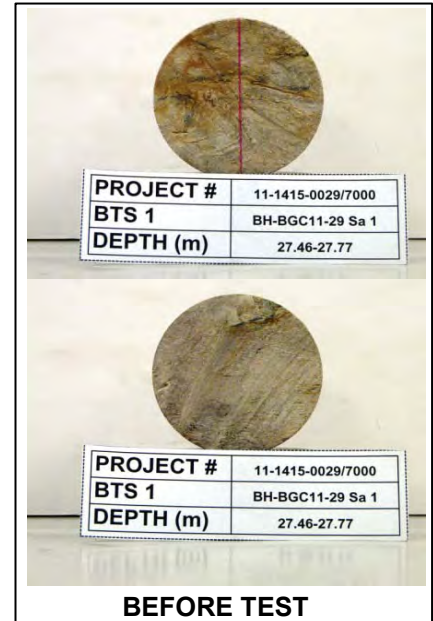
Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0163</u>	Diameter (mm)	<u>60.93</u>
Tensile Stress (MPa)	<u>5.6</u>	Thickness (mm)	<u>30.64</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.16</u>
		Volume (cm ³)	<u>89.34</u>
		Mass (g)	<u>232.70</u>
		Moisture Content (%)	<u>1.05</u>
		Wet Density (Kg/m ³)	<u>2604.69</u>
		Dry Density (Kg/m ³)	<u>2577.62</u>

Failure Mode		Calibration	
Type:	<u>Along Discontinuities</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.*



G. Patton	October 24, 2011	LPerrey	October 27, 2011
TESTED BY	DATE	CHECKED BY	DATE

Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-29
Project: Eagle Gold Mine Site Infra FS SI	Sample Number:	Sa 1 BTS 2
Location: Dublin Gulch, Yukon	Depth (m):	27.46-27.77
Client: BGC (Proj 0792-006-04)	Lab ID No:	141

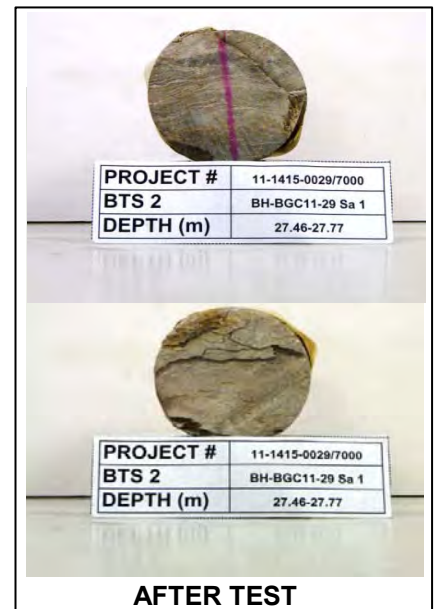
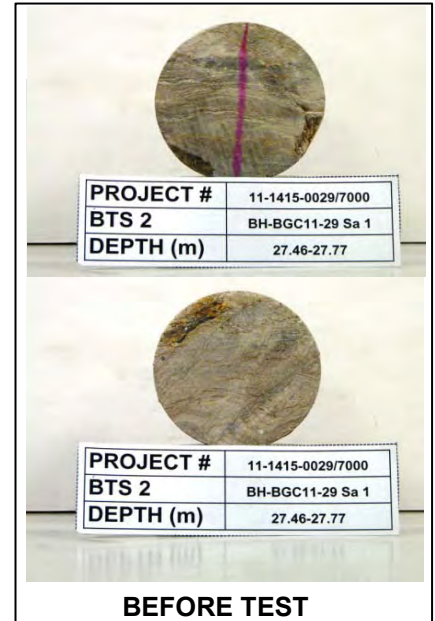
Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0087</u>	Diameter (mm)	<u>60.87</u>
Tensile Stress (MPa)	<u>3.2</u>	Thickness (mm)	<u>28.35</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.10</u>
		Volume (cm ³)	<u>82.50</u>
		Mass (g)	<u>215.90</u>
		Moisture Content (%)	<u>1.05</u>
		Wet Density (Kg/m ³)	<u>2617.00</u>
		Dry Density (Kg/m ³)	<u>2589.80</u>

Failure Mode		Calibration	
Type:	<u>Along Discontinuities</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.*



G. Patton	October 24, 2011	LPerrey	October 27, 2011
TESTED BY	DATE	CHECKED BY	DATE

Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-34
Project: Eagle Gold Mine Site Infra FS SI	Sample Number:	UCS BTS 1
Location: Dublin Gulch, Yukon	Depth (m):	36.60-36.96
Client: BGC (Proj 0792-006-04)	Lab ID No:	141

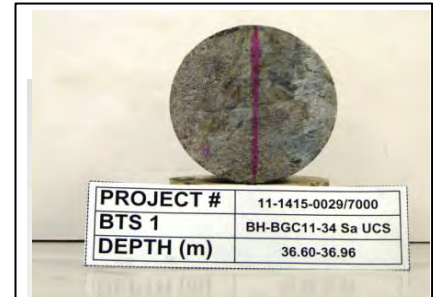
Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0246</u>	Diameter (mm)	<u>60.81</u>
Tensile Stress (MPa)	<u>9.2</u>	Thickness (mm)	<u>27.92</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.04</u>
		Volume (cm ³)	<u>81.09</u>
		Mass (g)	<u>212.20</u>
		Moisture Content (%)	<u>0.32</u>
		Wet Density (Kg/m ³)	<u>2616.92</u>
		Dry Density (Kg/m ³)	<u>2608.57</u>

Failure Mode		Calibration	
Type:	<u>Shear</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided

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Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-34
Project: Eagle Gold Mine Site Infra FS SI	Sample Number:	UCS BTS 2
Location: Dublin Gulch, Yukon	Depth (m):	36.60-36.96
Client: BGC (Proj 0792-006-04)	Lab ID No:	141

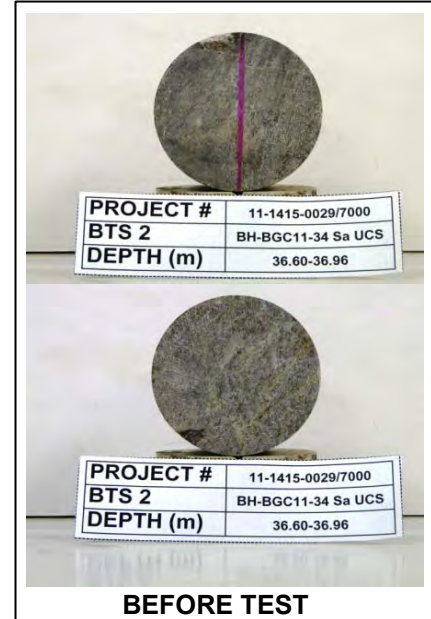
Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0379</u>	Diameter (mm)	<u>60.76</u>
Tensile Stress (MPa)	<u>11.8</u>	Thickness (mm)	<u>33.54</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.00</u>
		Volume (cm ³)	<u>97.25</u>
		Mass (g)	<u>261.30</u>
		Moisture Content (%)	<u>0.32</u>
		Wet Density (Kg/m ³)	<u>2686.90</u>
		Dry Density (Kg/m ³)	<u>2678.33</u>

Failure Mode		Calibration	
Type:	<u>Shear</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided

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Tensile Strength By The Brazil Test
Reference
 ISRM 1981 (p120-121)

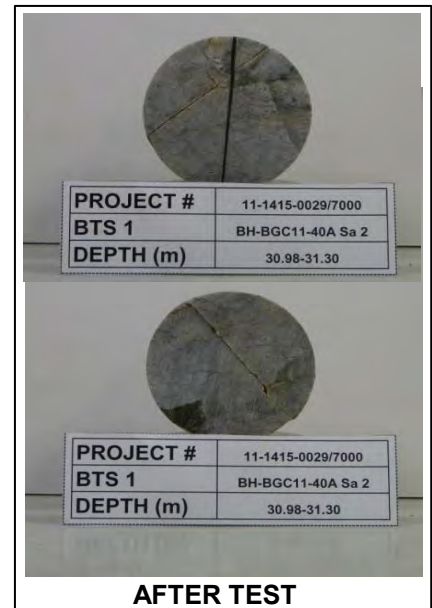
Project No.:	11-1415-0029/7000	Borehole:	BH-BGC11-40A
Project:	Eagle Gold	Sample Number:	Sa 1 BTS 1
Location:	Yukon	Depth (m):	30.98-31.30
Client:	BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0389</u>	Diameter (mm)	<u>60.66</u>
Tensile Stress (MPa)	<u>13.0</u>	Thickness (mm)	<u>31.32</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>28.90</u>
		Volume (cm ³)	<u>90.51</u>
		Mass (g)	<u>239.70</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2648.20</u>
		Dry Density (Kg/m ³)	<u>2645.56</u>

Failure Mode		Calibration	
Type:	<u>Along Discontinuity</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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Tensile Strength By The Brazil Test
Reference
 ISRM 1981 (p120-121)

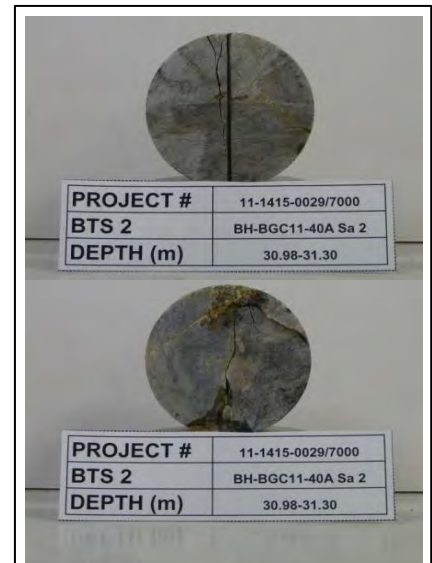
Project No.:	11-1415-0029/7000	Borehole:	BH-BGC11-40A
Project:	Eagle Gold	Sample Number:	Sa 1 BTS 2
Location:	Yukon	Depth (m):	30.98-31.30
Client:	BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0333</u>	Diameter (mm)	<u>60.64</u>
Tensile Stress (MPa)	<u>12.2</u>	Thickness (mm)	<u>28.58</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>28.88</u>
		Volume (cm ³)	<u>82.54</u>
		Mass (g)	<u>219.90</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2664.13</u>
		Dry Density (Kg/m ³)	<u>2661.46</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-40B
Project: Eagle Gold	Sample Number:	UCS 1 BTS 1
Location: Yukon	Depth (m):	21.81-22.06
Client: BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0334</u>	Diameter (mm)	<u>61.02</u>
Tensile Stress (MPa)	<u>11.3</u>	Thickness (mm)	<u>30.92</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.24</u>
		Volume (cm ³)	<u>90.42</u>
		Mass (g)	<u>253.10</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2799.10</u>
		Dry Density (Kg/m ³)	<u>2796.30</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

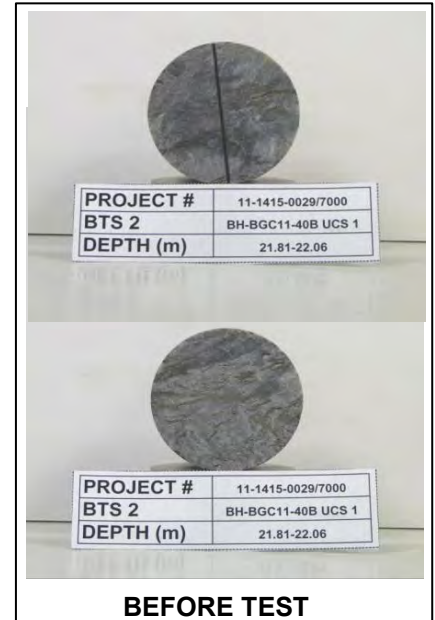
Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-40B
Project: Eagle Gold	Sample Number:	UCS 1 BTS 2
Location: Yukon	Depth (m):	21.81-22.06
Client: BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0374</u>	Diameter (mm)	<u>61.05</u>
Tensile Stress (MPa)	<u>13.0</u>	Thickness (mm)	<u>29.88</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.27</u>
		Volume (cm ³)	<u>87.47</u>
		Mass (g)	<u>237.50</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2715.32</u>
		Dry Density (Kg/m ³)	<u>2712.61</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided



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Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-43
Project: Eagle Gold	Sample Number:	Sa 1 BTS 1
Location: Yukon	Depth (m):	4.24-4.48
Client: BGC Engineering Inc.	Lab ID No:	141

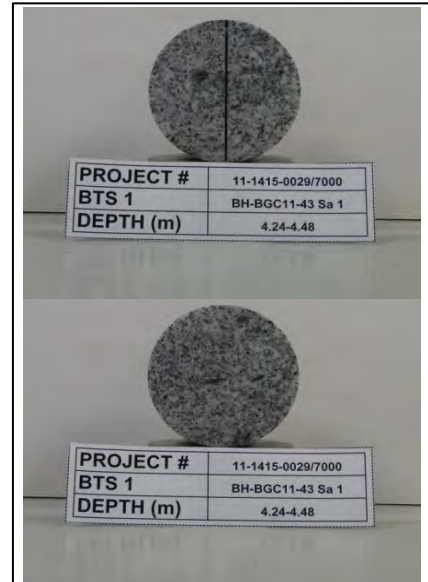
Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0444</u>	Diameter (mm)	<u>60.91</u>
Tensile Stress (MPa)	<u>15.6</u>	Thickness (mm)	<u>29.66</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.14</u>
		Volume (cm ³)	<u>86.42</u>
		Mass (g)	<u>233.50</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2701.77</u>
		Dry Density (Kg/m ³)	<u>2699.07</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided

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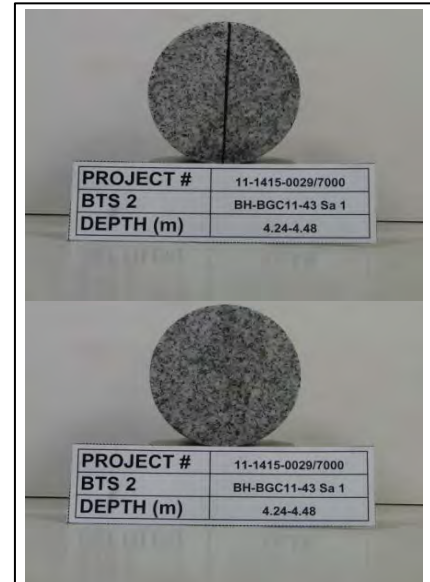
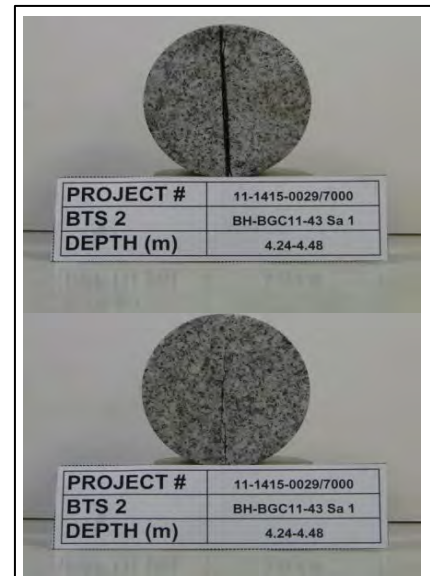
Project No.:	11-1415-0029/7000	Borehole:	BH-BGC11-43
Project:	Eagle Gold	Sample Number:	Sa 1 BTS 2
Location:	Yukon	Depth (m):	4.24-4.48
Client:	BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0421</u>	Diameter (mm)	<u>60.95</u>
Tensile Stress (MPa)	<u>14.0</u>	Thickness (mm)	<u>31.28</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.18</u>
		Volume (cm ³)	<u>91.26</u>
		Mass (g)	<u>244.80</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2682.30</u>
		Dry Density (Kg/m ³)	<u>2679.62</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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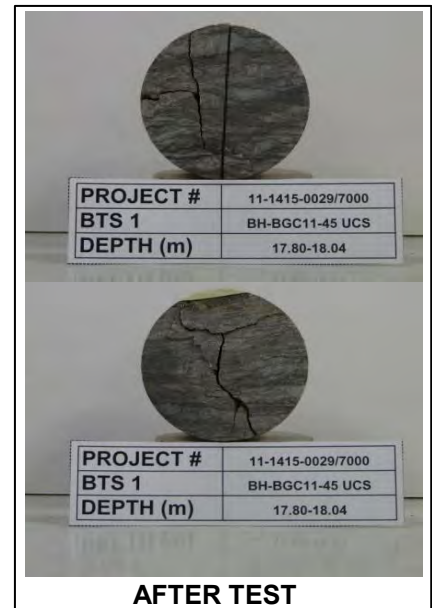
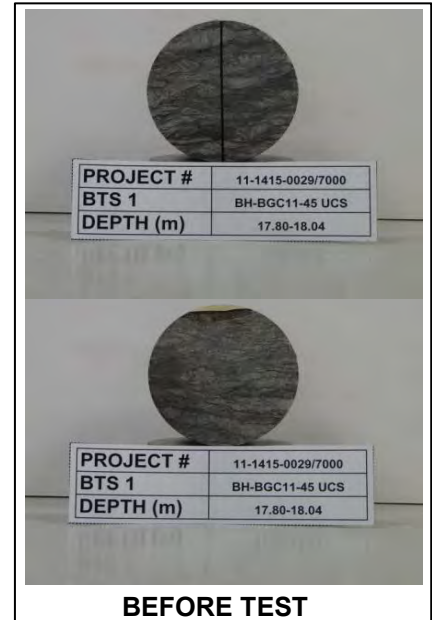
Project No.:	11-1415-0029/7000	Borehole:	BH-BGC11-45
Project:	Eagle Gold	Sample Number:	UCS BTS 1
Location:	Yukon	Depth (m):	17.80-18.04
Client:	BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0381</u>	Diameter (mm)	<u>60.77</u>
Tensile Stress (MPa)	<u>13.6</u>	Thickness (mm)	<u>29.24</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.00</u>
		Volume (cm ³)	<u>84.81</u>
		Mass (g)	<u>232.80</u>
		Moisture Content (%)	<u>0.20</u>
		Wet Density (Kg/m ³)	<u>2744.97</u>
		Dry Density (Kg/m ³)	<u>2739.49</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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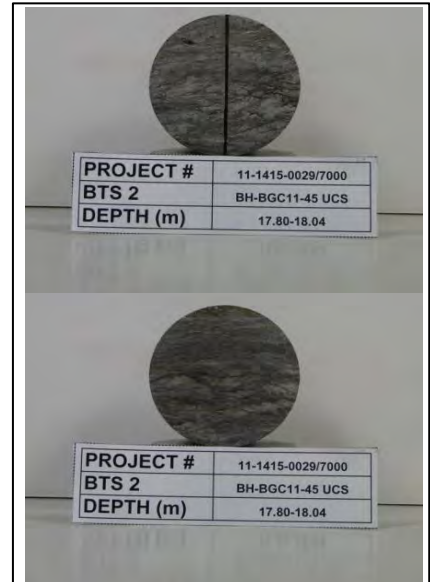
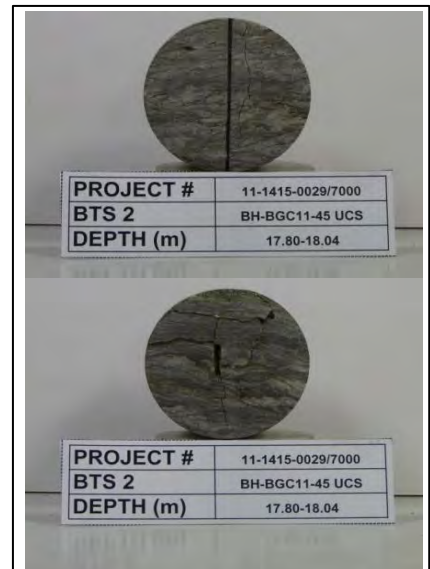
Project No.:	11-1415-0029/7000	Borehole:	BH-BGC11-45
Project:	Eagle Gold	Sample Number:	UCS BTS 2
Location:	Yukon	Depth (m):	17.80-18.04
Client:	BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0375</u>	Diameter (mm)	<u>60.79</u>
Tensile Stress (MPa)	<u>11.6</u>	Thickness (mm)	<u>33.81</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.02</u>
		Volume (cm ³)	<u>98.13</u>
		Mass (g)	<u>271.20</u>
		Moisture Content (%)	<u>0.20</u>
		Wet Density (Kg/m ³)	<u>2763.70</u>
		Dry Density (Kg/m ³)	<u>2758.18</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-46
Project: Eagle Gold	Sample Number:	UCS BTS 1
Location: Yukon	Depth (m):	13.11-13.65
Client: BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0402</u>	Diameter (mm)	<u>60.81</u>
Tensile Stress (MPa)	<u>15.0</u>	Thickness (mm)	<u>28.04</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.04</u>
		Volume (cm ³)	<u>81.44</u>
		Mass (g)	<u>217.60</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2672.03</u>
		Dry Density (Kg/m ³)	<u>2669.36</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided

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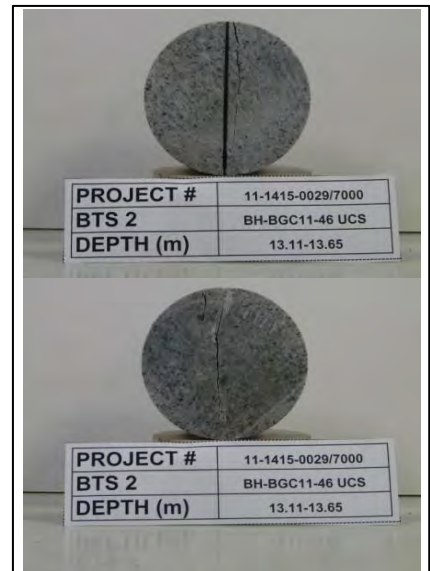
Project No.:	11-1415-0029/7000	Borehole:	BH-BGC11-46
Project:	Eagle Gold	Sample Number:	UCS BTS 2
Location:	Yukon	Depth (m):	13.11-13.65
Client:	BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0431</u>	Diameter (mm)	<u>60.80</u>
Tensile Stress (MPa)	<u>15.3</u>	Thickness (mm)	<u>29.42</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.03</u>
		Volume (cm ³)	<u>85.42</u>
		Mass (g)	<u>229.30</u>
		Moisture Content (%)	<u>0.10</u>
		Wet Density (Kg/m ³)	<u>2684.51</u>
		Dry Density (Kg/m ³)	<u>2681.82</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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Tensile Strength By The Brazil Test
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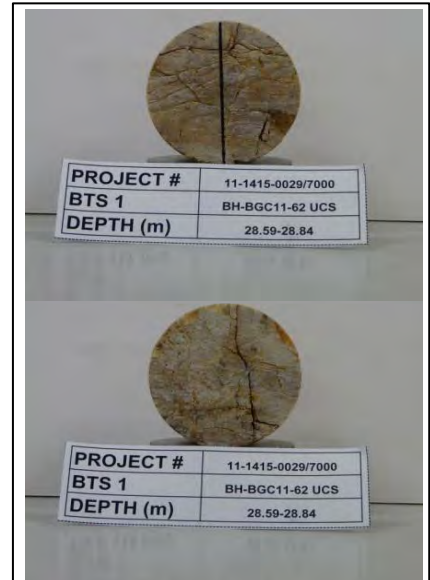
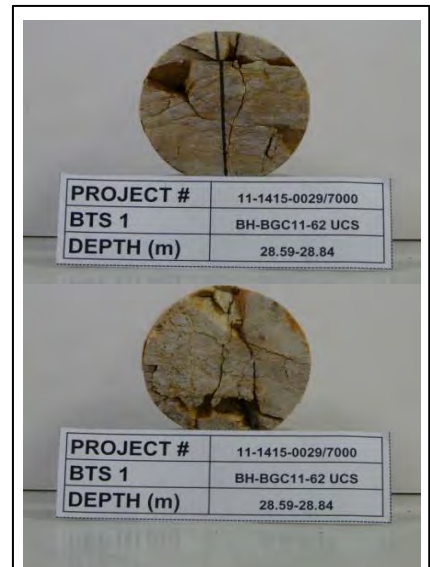
Project No.:	11-1415-0029/7000	Borehole:	BH-BGC11-62
Project:	Eagle Gold	Sample Number:	UCS BTS 1
Location:	Yukon	Depth (m):	28.59-28.84
Client:	BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0167</u>	Diameter (mm)	<u>60.97</u>
Tensile Stress (MPa)	<u>5.4</u>	Thickness (mm)	<u>32.31</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.20</u>
		Volume (cm ³)	<u>94.33</u>
		Mass (g)	<u>242.30</u>
		Moisture Content (%)	<u>0.50</u>
		Wet Density (Kg/m ³)	<u>2568.59</u>
		Dry Density (Kg/m ³)	<u>2555.81</u>

Failure Mode		Calibration	
Type:	<u>Along Discontinuities</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments	
Lithology:	Not Provided

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BEFORE TEST

AFTER TEST

G. Patton	September 20, 2011	EK	OCTOBER 7, 2011
TESTED BY	DATE	CHECKED BY	DATE

Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-69
Project: Eagle Gold	Sample Number:	UCS 1 BTS 1
Location: Yukon	Depth (m):	12.26-12.54
Client: BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0082</u>	Diameter (mm)	<u>60.77</u>
Tensile Stress (MPa)	<u>2.8</u>	Thickness (mm)	<u>31.03</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.00</u>
		Volume (cm ³)	<u>90.00</u>
		Mass (g)	<u>229.50</u>
		Moisture Content (%)	<u>0.40</u>
		Wet Density (Kg/m ³)	<u>2549.96</u>
		Dry Density (Kg/m ³)	<u>2539.80</u>

Failure Mode		Calibration	
Type:	<u>Along Discontinuities</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

Comments

Lithology: Not Provided

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.*



BEFORE TEST



AFTER TEST

G. Patton	September 20, 2011	EK	OCTOBER 7, 2011
TESTED BY	DATE	CHECKED BY	DATE

Tensile Strength By The Brazil Test

Reference
ISRM 1981 (p120-121)

Project No.: 11-1415-0029/7000	Borehole:	BH-BGC11-69
Project: Eagle Gold	Sample Number:	UCS 1 BTS 2
Location: Yukon	Depth (m):	12.26-12.54
Client: BGC Engineering Inc.	Lab ID No:	141

Testing Results		Sample Measurements	
Max Load (MN)	<u>0.0180</u>	Diameter (mm)	<u>60.87</u>
Tensile Stress (MPa)	<u>6.3</u>	Thickness (mm)	<u>29.92</u>
Rate of Loading (kN/s)	<u>0.5</u>	Area (cm ²)	<u>29.10</u>
		Volume (cm ³)	<u>87.07</u>
		Mass (g)	<u>220.90</u>
		Moisture Content (%)	<u>0.40</u>
		Wet Density (Kg/m ³)	<u>2537.10</u>
		Dry Density (Kg/m ³)	<u>2526.99</u>

Failure Mode		Calibration	
Type:	<u>Vertical Splitting</u>	Machine ID	<u>ELE</u>
Load Orientation		Transducer ID	<u>N/A</u>
Direction of loading axis with respect to bedding or foliation plane in degrees	<u>90</u>	Ram Area cm ²	<u>N/A</u>
		Data Logger	<u>N/A</u>

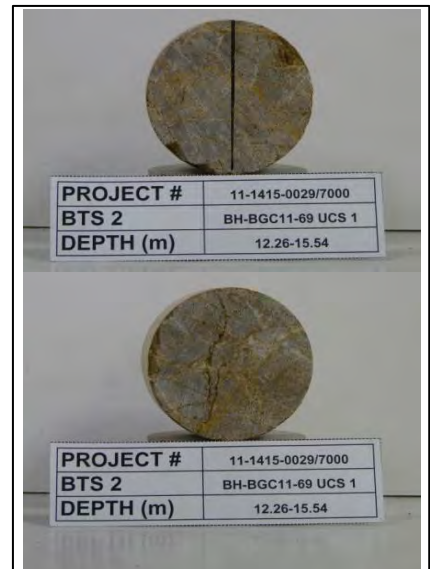
Comments

Lithology: Not Provided

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.*



BEFORE TEST



AFTER TEST

G. Patton	September 20, 2011	EK	OCTOBER 7, 2011
TESTED BY	DATE	CHECKED BY	DATE



METRO TESTING GROUP
 Materials Testing & Engineering Services
 CCIL & CSA Certified Company
 Innovative Quality Solutions

METROTESTING LABORATORIES LTD.
 6991 Curragh Avenue, Burnaby, B.C. V5J 4V6
 Tel.: (604) 436-9111 Fax.: (604) 436-9050

TO: GEONORTH ENGINEERING LTD.
 3975 18TH AVENUE
 PRINCE GEORGE, BC
 V2N 1B2

ATTN: Mr. Barry Tessmer

**TEST METHOD FOR THE RESISTANCE OF UNCONFINED COARSE
 AGGREGATE TO FREEZING AND THAWING**

(CSA A23.2-24A)

Project No:	15585
Project:	Aggregate Suitability Test for Concrete
Scope:	Aggregate Testing
Source:	Placer Trailings
Type of sample:	40mm Crushed Stone
Sample by:	Client
Date Sampled:	n/a
Date Received:	9-Nov-2011
Test No:	1
Tested by:	Mimi
Date Tested:	November 15 - 23, 2011
Report Date	24-Nov-2011

COARSE AGGREGATE

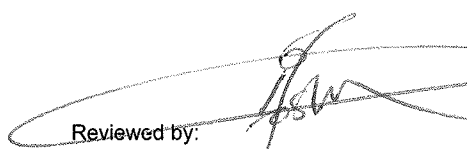
SIEVE SIZES	SUGGESTED WEIGHTS OF TEST SAMPLES	MASS OF TEST FRACTIONS BEFORE TEST	MASS OF TEST FRACTIONS AFTER TEST	FREEZE THAW LOSS	PERCENT LOSS	GRADING OF COARSE PORTION	D LOSS	WEIGHTED LOSS OF CONTROL AGGREGATE
	(grams)	(grams)	(grams)	(grams)	(%)	(%)	(%)	(%)
80mm to 28mm	4500.0	4750.5	4730.3	20.2	0.4	13.8	0.06	0.3
28mm to 20mm	2500.0	2509.5	2496.3	13.2	0.5	4.3	0.02	0.8
20mm to 14mm	1250.0	1263.5	1250.3	13.2	1.0	3.5	0.04	4.2
14mm to 10mm	1000.0	1055.0	1034.8	20.2	1.9	10	0.19	3.7
10mm to 5mm	500.0	567.4	550.9	16.5	2.9	30.8	0.90	5.5
(-) 5mm						37.6		
						100	1.2%	14.5

Comment:

Weighted sample loss of 1.2% meets the CSA requirement of 6% max. for concrete exposed to freezing and thawing.
 Weighted loss of control aggregates is 14.5% which is within the specified limits of 8.5% - 15.3% (Drain Quarry)

Per:

Mimi Baraquio, ASCT
 Laboratory Technologist

Reviewed by: 
 Prakash Joshi, ASCT, Eng.L.
 Senior Materials Engineering Technologist

Association of Professional Engineers and Geoscientists of the Province of British Columbia
 P. V. JOSHI
 ENGINEERING LICENSEE
 Limited Licence 31356

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 Engineering interpretation or evaluation of test results is provided only on written request.



METRO TESTING LABORATORIES LTD.

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**SIEVE ANALYSIS REPORT
 SI Standard SERIES**

PROJECT NO. 15585
 CLIENT GEONORTH ENGINEERING LTD.
 C.C.

TO
 GEONORTH ENGINEERING LTD.
 3975 18TH AVENUE
 PRINCE GEORGE, BRITISH COLUMBI
 V2N 1B2

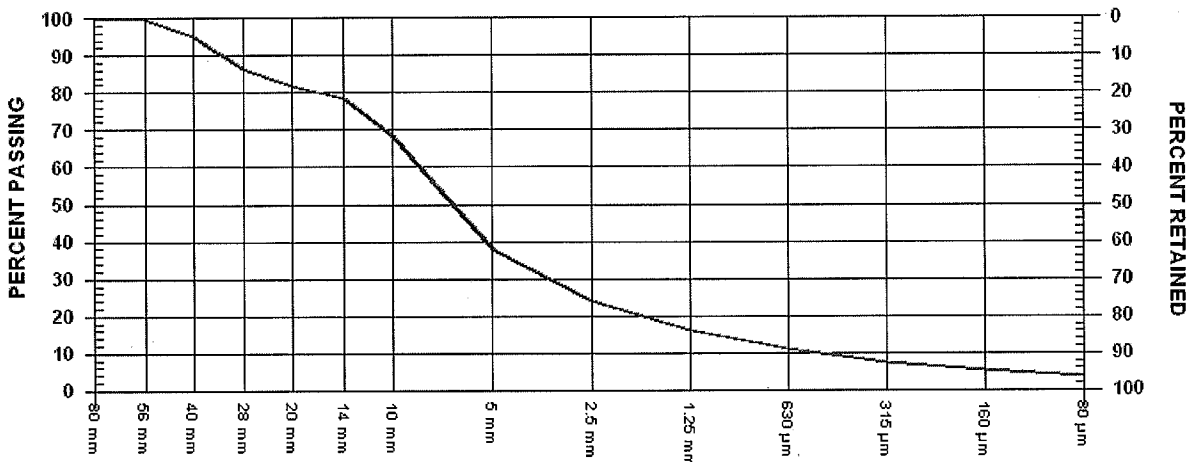
ATTN: BARRY TESSMER

PROJECT SUITABILITY OF AGGREGATE FOR CONCRTE
 CONCRETE
 CONTRACTOR GEONORTH ENGINEERING LTD.

SIEVE TEST NO. 1 DATE RECEIVED 2011.Nov.09 DATE TESTED 2011.Nov.09 DATE SAMPLED 2011.Nov.08

SUPPLIER
 SOURCE PLACER TAILING
 SPECIFICATION
 MATERIAL TYPE CRUSHED STONE

SAMPLED BY CLIENT
 TESTED BY AP
 TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
80 mm	100.0	
56 mm	99.5	
40 mm	95.0	
28 mm	86.2	
20 mm	81.9	
14 mm	78.4	
10 mm	68.4	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
5 mm	37.6	
2.5 mm	24.1	
1.25 mm	16.1	
630 µm	11.1	
315 µm	7.6	
160 µm	5.3	
80 µm	3.6	

COMMENTS

[Signature]
 Association of Professional
 Engineers and Geoscientists
 of the Province of
 British Columbia
 P. V. JOSHI
 ENGINEERING
 LICENSEE



METRO TESTING GROUP
 Materials Testing & Engineering Services
 CCIL & CSA Certified Company
Innovative Quality Solutions

TO: GEONORTH ENGINEERING LTD.
 3975 18TH AVENUE
 PRINCE GEORGE, BC
 V2N 1B2

LOG No.: 112
PROJECT No: 15585
TESTED BY: Andrey

ATTN: Mr. Barry Tessmer
PROJECT: Suitability of Aggregate for Concrete
SCOPE: Aggregate Testing

**Resistance to Degradation of
 Small Size Coarse Aggregate by
 Abrasion & Impact in the Los
 Angeles Machine
 CSA A23.2-16A**

SAMPLE DATA		TEST DATA	
Sample I.D.	N/A	Number of Revolutions:	500
Sample Source:	Placer Tailing	Number of Spheres:	8
Sample Type:	40mm Crushed Stone	Mass of Spheres:	3334.0 grams
Date Sampled:	N/A	Mass of Sample Tested:	5001.8 grams
Date Received:	09-Nov-11	Grading Category:	C
Date Tested:	14-Nov-11	Date Reported	14-Nov-11

TESTED SAMPLE DATA				CSA A23.2-16A, SPECIFIED TEST REQUIREMENTS					
				CSA A23.2-16A, Table 1: Numbers of Sphere and Mass					
Sieve Sizes		Tested Sample	After 500 Revolutions		A	B	C	D	E
Passing	Retained	(Grams)	+1.8mm (Grams)	-1.8mm (Grams)	12 Balls 5000+/- 25 g	11 Balls 4584+/- 25 g	8 Balls 3330+/- 20 g	6 Balls 2500+/- 15 g	
				CSA A23.2-16A, Table 2: Suggested Gradings of Test Samples					
		(Grams)	(Grams)	(Grams)	(Grams)	(Grams)	(Grams)	(Grams)	(Grams)
40.0 mm	- 28.0 mm				1250+/-25				
28.0 mm	- 20.0 mm				1250+/-25				
20.0 mm	- 14.0 mm				1250+/-10	2500+/-10			
14.0 mm	- 10.0 mm	2501.8			1250+/-10	2500+/-10	2500+/-10		
10.0 mm	- 5.0 mm	2500.0					2500+/-10	5000+/-10	
5.0 mm	- 2.5 mm								5000+/-10
Totals		5001.8	3506.8	1495.0	5000+/-10	5000+/-10	5000+/-10	5000+/-10	5000+/-10

Loss at 500 Revolutions	29.9%	APPLICABLE REQUIREMENTS			
		Standard	Section	Reference	Max Limit
Comments:		CSA A23.1		Table 12	50
* For concrete exposed to freezing and thawing or for other exposure conditions.					

Per:

Tatyana Tsvetkova, ASCT
 Laboratory Supervisor

Reviewed by:

Cengiz Guldemet, P.Eng
 Materials Engineer

Reporting of these test results constitutes a testing service only.
 Engineering interpretation or evaluation of test results is provided only on written request





METRO TESTING GROUP
 Materials Testing & Engineering Services
 CCIL & CSA Certified Company
Innovative Quality Solutions

METROTESTING LABORATORIES LTD.
 6991 Curragh Avenue, Burnaby, B.C. V5J 4V6
 Tel.: (604) 436-9111 Fax.: (604) 436-9050

TO: GEONORTH ENGINEERING LTD.
 3975 18TH AVENUE
 PRINCE GEORGE, BC
 V2N 1B2

ATTN: Mr. Barry Tessmer

**TEST METHOD FOR THE RESISTANCE OF UNCONFINED COARSE
 AGGREGATE TO FREEZING AND THAWING**

(CSA A23.2-24A)

Project No:	15585
Project:	Aggregate Suitability Test for Concrete
Scope:	Aggregate Testing
Source:	Pit Outcrop
Type of sample:	40mm Crushed Stone
Sample by:	Client
Date Sampled:	n/a
Date Received:	9-Nov-2011
Test No:	1
Tested by:	Mimi
Date Tested:	November 15 - 23, 2011
Report Date	24-Nov-2011

COARSE AGGREGATE

SIEVE SIZES	SUGGESTED WEIGHTS OF TEST SAMPLES	MASS OF TEST FRACTIONS BEFORE TEST	MASS OF TEST FRACTIONS AFTER TEST	FREEZE THAW LOSS	PERCENT LOSS	GRADING OF COARSE PORTION	D LOSS	WEIGHTED LOSS OF CONTROL AGGREGATE
	(grams)	(grams)	(grams)	(grams)	(%)	(%)	(%)	(%)
80mm to 28mm	4500.0	4690.3	4670.3	20.0	0.4	16.5	0.07	0.3
28mm to 20mm	2500.0	2520.5	2510.3	10.2	0.4	5.1	0.02	0.8
20mm to 14mm	1250.0	1283.6	1260.4	23.2	1.8	4.7	0.08	4.2
14mm to 10mm	1000.0	1067.0	1047.5	19.5	1.8	14.2	0.26	3.7
10mm to 5mm	500.0	579.9	570.9	9.0	1.6	28.5	0.44	5.5
(-) 5mm						31		
						100	0.9%	14.5

Comment:

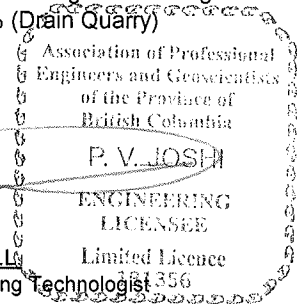
Weighted sample loss of 0.9% meets the CSA requirement of 6% max. for concrete exposed to freezing and thawing.
 Weighted loss of control aggregates is 14.5% which is within the specified limits of 8.5% - 15.3% (Drain Quarry)

Per:

Mimi Baraquio, ASCT
 Laboratory Technologist

Reviewed by:

Prakash Joshi, ASCT, Eng. Ld
 Senior Materials Engineering Technologist



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METRO TESTING LABORATORIES

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Tel: (604) 436-9111 Fax: (604) 436-9050

SIEVE ANALYSIS REPORT CSA SIEVES SERIES

PROJECT NO. 15585

CLIENT GEONORTH ENGINEERING LTD.
C.C.

TO
GEONORTH ENGINEERING LTD.
3975 18TH AVENUE
PRINCE GEORGE, BRITISH COLUMBI
V2N 1B2

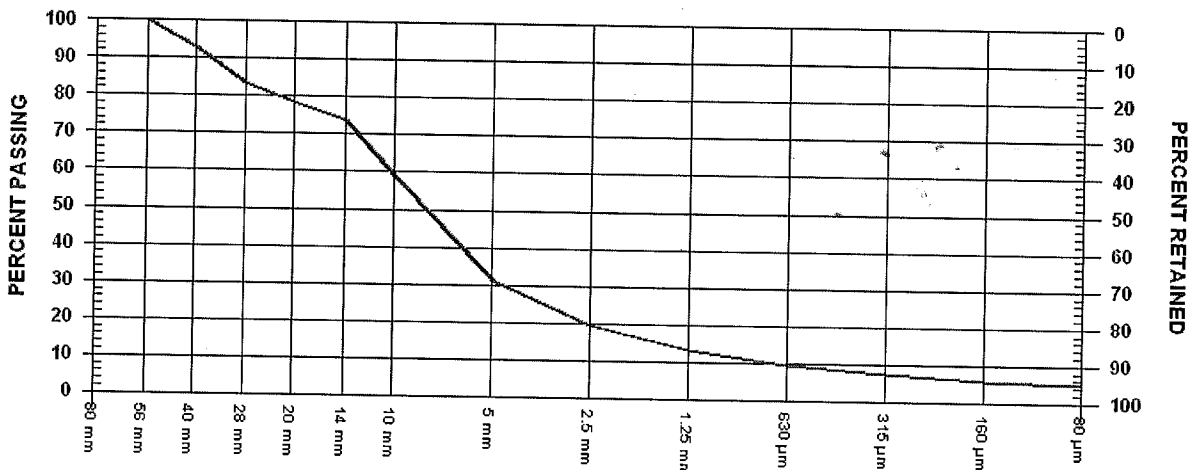
ATTN: BARRY TESSMER

PROJECT SUITABILITY OF AGGREGATE FOR CONCRTE
CONCRETE
CONTRACTOR GEONORTH ENGINEERING LTD.

SIEVE TEST NO. 2 DATE RECEIVED 2011.Nov.09 DATE TESTED 2011.Nov.14 DATE SAMPLED 2011.Nov.08

SUPPLIER
SOURCE PIT OUTCROP
SPECIFICATION
MATERIAL TYPE CRUSHED STONE

SAMPLED BY CLIENT
TESTED BY AP
TEST METHOD WASHED



GRAVEL SIZES	PERCENT PASSING	GRADATION LIMITS
80 mm		
56 mm	100.0	
40 mm	93.0	
28 mm	83.5	
20 mm	78.4	
14 mm	73.7	
10 mm	59.5	

SAND SIZES AND FINES	PERCENT PASSING	GRADATION LIMITS
5 mm	31.0	
2.5 mm	19.7	
1.25 mm	13.3	
630 µm	9.6	
315 µm	7.3	
160 µm	5.9	
80 µm	4.8	

COMMENTS

[Signature]
 Association of Professional
 Engineers and Geoscientists
 of the Province of
 British Columbia
 P. V. JOSHI
 ENGINEERING
 Limited Licence
 131856

**METRO TESTING GROUP**

Materials Testing & Engineering Services
 CCIL & CSA Certified Company
Innovative Quality Solutions

TO: GEONORTH ENGINEERING LTD.
 3975 18TH AVENUE
 PRINCE GEORGE, BC
 V2N 1B2

LOG No.: 112
 PROJECT No: 15585
 TESTED BY: Andrey

ATTN: Mr. Barry Tessmer

**Resistance to Degradation of
 Small Size Coarse Aggregate by
 Abrasion & Impact in the Los
 Angeles Machine**

PROJECT: Suitability of Aggregate for Concrete
 SCOPE: Aggregate Testing

CSA A23.2-16A

SAMPLE DATA		TEST DATA	
Sample I.D.	N/A	Number of Revolutions:	500
Sample Source:	Pit Outcrop	Number of Spheres:	8
Sample Type:	40mm Crushed Stone	Mass of Spheres:	3333.6 grams
Date Sampled:	N/A	Mass of Sample Tested:	5001.0 grams
Date Received:	09-Nov-11	Grading Category:	C
Date Tested:	14-Nov-11	Date Reported	14-Nov-11

TESTED SAMPLE DATA					CSA A23.2-16A, SPECIFIED TEST REQUIREMENTS				
					CSA A23.2-16A, Table 1: Numbers of Sphere and Mass				
Sieve Sizes		Tested Sample	After 500 Revolutions		A	B	C	D	E
Passing	Retained	(Grams)	+1.8mm (Grams)	-1.8mm (Grams)	12 Balls 5000+/- 25 g	11 Balls 4584+/- 25 g	8 Balls 3330+/- 20 g	6 Balls 2500+/- 15 g	
					CSA A23.2-16A, Table 2: Suggested Gradings of Test Samples				
		(Grams)	(Grams)	(Grams)	(Grams)	(Grams)	(Grams)	(Grams)	(Grams)
40.0 mm	- 28.0 mm				1250+/-25				
28.0 mm	- 20.0 mm				1250+/-25				
20.0 mm	- 14.0 mm				1250+/-10	2500+/-10			
14.0 mm	- 10.0 mm	2500.0			1250+/-10	2500+/-10	2500+/-10		
10.0 mm	- 5.0 mm	2501.0					2500+/-10	5000+/-10	
5.0 mm	- 2.5 mm								5000+/-10
Totals		5001.0	4316.4	684.6	5000+/-10	5000+/-10	5000+/-10	5000+/-10	5000+/-10

Loss at 500 Revolutions	13.7%	APPLICABLE REQUIREMENTS			
		Standard	Section	Reference	Max Limit
Comments:		CSA A23.1		Table 12	50
* For concrete exposed to freezing and thawing or for other exposure conditions.					

Per:

Tatyana Tsvetkova, AScT
 Laboratory Supervisor

Reviewed by:

Cengiz Guldemet, P.Eng
 Materials Engineer

Reporting of these test results constitutes a testing service only.
 Engineering interpretation or evaluation of test results is provided only on written request





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**#18 – 3275 McCallum Rd,
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(250) 860-9955
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(604) 740-0920
- Squamish
1-604-898-1420
- Salmon Arm
(250) 803 0248
- Williams Lake
(250) 296-4118
- Fort St. John
(250) 261-6615
- Dawson Creek
(250) 261-6615



INNOVATION QUALITY SOLUTIONS

GeoNorth Engineering Ltd. Preliminary Source Acceptance Testing

Project: Aggregate Quality Assessment
Examination of Coarse Aggregate
50mm to 9.5mm Fraction
Samples From “Placer Tailings” and “Pit Outcrop”

Petrographic Examination in accordance with:
CSA A23.2-04 15A Petrographic Examination Of Aggregates

Submitted To:

Metro Testing Laboratories Ltd.
Burnaby, BC

Submitted By:

Valley Testing Services Ltd.
Abbotsford, BC

December 2, 2011

MTL 15585

December 2, 2011

Metro Testing Laboratories Ltd.
6992 Curragh Avenue
Burnaby, British Columbia

Attention: Mr. Prakash Joshi

**RE: Petrographic Examination Of Coarse Aggregate
Examination of Coarse Aggregate
Samples From “Placer Tailings” and “Pit Outcrop”**

Dear Sir:

As requested Valley Testing Services Ltd. (Valley Testing) has examined two samples of material as supplied by GeoNorth Engineering Ltd from the above-mentioned locations. The material supplied is summarized in the table below. The purpose of the examination is to determine the suitability of the material for concrete applications.

Table 1.

Sample ID	Material Type	Source	Petrographic Grade	Intended Use
Placer Tailings	crushed rock	tailings pile	117	concrete
Pit outcrop	crushed rock	outcrop	121	concrete

The petrographic examination carried out is based on CSA A23.2-04 15A *Petrographic Examination of Aggregates*, and the MOT SS Section 202, Appendix 2 test method. The worksheet “MTO LS609” in the appendix A is derived from the Ontario test method LS609 for coarse aggregates. The examination assigns a “Petrographic Number” (PN) to the material, which correlates to classifications such as Good, Fair, Poor or Deleterious.

Summary

Placer Tailings: The examination was performed on a screened crushed rock aggregate. The material has been assigned a grade of “Good” with a PN of 117. Medium-hard to hard granodiorite and siliceous sheared rocks dominated the sample with minor amounts of medium hard mafic phyllite, and quartz vein. Particles ranged in size from 55mm to dust; estimated fracture count was 100%. Particles were angular and generally cubic. Slight to moderate weathering was observed, with some intense weathering noted in some sheared rock particles. The porosity of the sample is considered low to medium.

This coarse aggregate material is deemed physically and mechanically suitable for use in ready mix concrete and asphalt.

Pit Outcrop: The examination was performed on a screened crushed rock aggregate. The material has been assigned a grade of “Good” with a PN of 121. Medium-hard to hard sheared rocks dominated the sample with a moderate amount of medium hard diorite. Particles ranged in size from 50mm to dust; estimated fracture count was 100%. Particles were angular and generally cubic. Slight to moderate weathering was observed (mostly surface weathering), with some intense weathering noted in some sheared rock particles. The porosity of the sample is considered low to medium.

This coarse aggregate material is deemed physically and mechanically suitable for use in some construction materials including concrete and asphalt.

Method

As stated previously the basis for the examination was the CSA A23.2-04 15A test method and the MOT test method described in Appendix 2 of Section 202 of their Standard Specifications. The aggregate samples were prepared according to the procedures as outlined in the MOT test method¹. The resulting coarse fraction material, comprised of aggregate between the 19mm and 9.5mm screens, was washed and analysed. Due to the mass of this fraction the larger particles were re-introduced to produce a large enough sample. Fracture count was estimated, and then the material was examined for its physical and mechanical properties.

Using a hand lens (16X) and microscope, as well as a magnet, steel nail and knife, and dilute HCl, the material was classified by rock type and physical condition. Classifications for the aggregate was noted on the analysis sheets in Appendix A.

Discussion of Coarse Aggregates

Placer Tailings: The material used for this examination was a 55mm-9.5mm coarse fraction of a crushed rock aggregate from the “Placer Tailings” source. The coarse fraction material used was comprised of 19mm to 9.5mm material as per the MOT test method as outlined in Appendix 2 of Section 202 (with larger material re-introduced as mentioned above).

The sample material was generally found to be medium hard to hard, moderately tough with minor to moderate surface weathering. The porosity of the rock types were generally low to moderate, with minor amounts of highly porous sheared material. The majority of the sample was composed of granodiorite and sheared rocks with minor amounts of mafic phyllite, and quartz vein. Much of the sheared rock has rusty limonite surfaces does not appear to have excessive deleterious effects on the aggregate. No significant amounts of flat or elongated particles were observed.

The rock types identified were (in order of magnitude):

Granodiorite: no organics or coatings were noted, shape was angular and generally cubic, medium grain size, crystalline grain arrangement, low porosity, mineralogy dominated by

¹ MOT SS Section 202 Appendix 2, page 202 (11 of 12)

feldspar, quartz and biotite, structure of the particles was massive with uneven fracturing, particles were fresh with some surface weathered particles, Mohs hardness ranged from 5.0 - 5.5, color was specked black & white or gray overall.

Sheared Rock: no organics or coatings were noted, shape was very angular and generally cubic (minor flat particles), fine to medium grain size, cryptocrystalline grain arrangement, low to moderate porosity, mineralogy dominated by feldspar, quartz and muscovite, particle structure showed moderate to strong foliation, minor jointing with uneven fracturing, mainly surface weathered with minor deeply weathered particles, Mohs hardness ranged from 5.0 - >5.5, color was pale gray to yellowish gray with brown rusty limonite surfaces.

Mafic Phyllite: no organics or coatings were noted, shape was angular and generally cubic (minor elongated particles), fine to coarse grain size, crystalline and cryptocrystalline grain arrangement, low to moderate porosity, mineralogy dominated by hornblende, calcite, feldspar, and biotite, particle structure showed weak to moderate foliation with uneven fracturing, mainly fresh and surface weathered particles, Mohs hardness ranged from 5.0 - 5.5, color was dark gray to black.

Quartz Vein: no organics or coatings were noted, shape was angular and generally cubic (minor flat particles), fine grain size, cryptocrystalline grain arrangement, low porosity, mineralogy dominated by quartz with some limonite, particle structure massive with conchoidal fracturing, mainly fresh particles, Mohs hardness was >5.5, color was white with minor rusty limonite.

Pit Outcrop: The material used for this examination was a 50mm-9.5mm coarse fraction of a crushed rock aggregate from the “Pit Outcrop” source. The coarse fraction material used was comprised of 19mm to 9.5mm material as per the MOT test method as outlined in Appendix 2 of Section 202 (with larger material re-introduced as mentioned above).

The sample material was generally found to be medium hard to hard, moderately tough with minor to moderate weathering. The porosity of the rock types were generally low to moderate, with minor amounts of highly porous sheared material. The majority of the sample was composed of sheared rock with moderate amounts of diorite. Much of the sheared rock has rusty limonite surfaces does not appear to have excessive deleterious effects on the aggregate. No significant amounts of flat or elongated particles were observed.

The rock types identified were (in order of magnitude):

Sheared Rock: no organics or coatings were noted, shape was very angular and generally cubic (minor flat particles), fine to medium grain size, cryptocrystalline grain arrangement, low to moderate porosity, mineralogy dominated by feldspar, hornblende, biotite (minor quartz), particle structure showed moderate to strong foliation, minor jointing with uneven fracturing, mainly surface weathered particles, Mohs hardness ranged from 5.0 - >5.5, color was rusty brown to dark gray.

Diorite: no organics or coatings were noted, shape was very angular to angular and generally cubic (minor flat and elongated particles), fine to medium grain size, crystalline grain

arrangement, low porosity, mineralogy dominated by feldspar, quartz and biotite, structure of the particles was massive with uneven fracturing, particles were fresh with some surface weathered particles, Mohs hardness ranged from 5.0 - 5.5, color was specked black & white or gray overall.

Alkali Aggregate Reactivity

Alkali Silica Reactivity

The petrographic examination revealed that the aggregate material contained rock types with mineralogy that may have a potential for alkali silica reactivity (ASR). These include particular types of quartz mineralogy found in the sheared rocks. Historical usage of similar material as well as ASR evaluations of similar material should be taken into account when assessing the potential for expansive reactivity in Portland cement.

This is a reaction between the hydroxide ions associated with the dissolved salts of sodium and potassium and the silica molecules of certain imperfectly crystallized siliceous rocks and minerals, such as opal, chert, and cristobalite, or highly siliceous volcanic glasses or highly strained or granulated siliceous rocks, such as the metaquartzites, other stressed silicates, and exceptionally fine-grained siliceous rocks (such as siltstone and argillite). The reaction produces a silica gel that will expand in the presence of moisture. The expanding gel causes cracks in the aggregate and paste. The cracks allow more moisture to enter and expansive gel to fill the cracks and cause more expansion.

The aggregate material, once processed for use, should be evaluated for ASR in accordance with CSA A23.2-27A prior to being used in ready mix concrete. If the aggregate will be used as concrete aggregate before any ASR testing can be done (e.g. CSA A23.2-25A) then the ready mix supplier should reference A23.2-27A, Tables 1 to 6 for “Measures to Avoid Deleterious Expansion in Concrete” using supplementary cementing materials such as fly ash and silica fume.

Conclusions

Placer Tailings: PN= 117 *Pit Outcrop:* PN= 121

This aggregate material is physically and mechanically suitable for asphalt and concrete applications. In particular, aggregates with petrographic numbers at or below 125 are considered suitable for all classes of ready mix concrete. Aggregates with petrographic numbers below 140 are suitable for most classes of concrete with the exception of Class C1, C2, and F1 (these are concrete mixes that have entrained air and may be exposed to freeze/thaw) (CSA A23.2 – 15A).

As noted earlier in this report, the potential exists for alkali aggregate reactivity. If the aggregate material has not been evaluated for AAR, this is a recommended action.

As many of the aggregate particles have an oxidized rusty surface, there is the chance that this oxidation could find its way to the surface and cause rusty streaks to form on the surface of the concrete.

I trust this report meets your needs. Please do not hesitate to call if you require any further services or information.

Yours truly,

John Burton, P.Ge
Senior Geoscientist



References

CSA A23.2-04 15A Petrographic Examination Of Aggregates
MTO LS 609 Test Method
Best, Igneous and Metamorphic Petrology, 1982
MOT Standard Specifications for Highway Construction 2009

Appendix A
Aggregate Worksheets

Coarse Aggregate Petrographic Analysis- MTO LS609

Client Name: GeoNorth Engineering			
Source: Placer Tailings		Project Name: Aggregate Qualification	
Date: 02-Dec-11	Fraction: 30-9.5mm	Sample ID crush rock	Analyst: J. Burton
Type	Type No.	Mass (g)	%
Carbonate [hard; silty, hard]	01		
Carbonate [surface weathered; silty, surface weathered; medium hard: silty medium hard]	20		
Carbonate [sandy, hard or medium hard]	02		
Carbonate [slightly cherty; <5% chert]	21		
Chert [hard or medium hard]	23		
Conglomerate-Sandstone-Arkose [hard]	03		
Conglomerate-Sandstone-Arkose [medium hard]	22		
Greywacke-Argillite [hard or medium hard]	06		
Gneiss-Amphibolite-Schist [hard]	04	694.9	46.4
Quartzite [hard]	05		
Granite-Diorite-Gabbro [hard]	08	639.6	42.7
Volcanic [hard or medium hard]	07		
Minerals [hard/medium]	09		
Quartz vein	10	36.6	2.4
Total Good Aggregate		1371.1	91.5
Carbonate [soft; silty soft; slightly shaley]	35		
Carbonate [soft, pitted]	41		
Carbonate [deeply weathered; silty, deeply weathered]	42		
Carbonate [sandy, soft]	40		
Marble [brittle]	24		
Chert-Cherty Carbonate [<20% leached chert]	26		
Conglomerate-Sandstone-Arkose [brittle]	30		
Greywacke [brittle]	29		
Quartzite [slightly weathered]	52		
Gneiss-Amphibolite-Schist [brittle]	25	100.5	6.7
Minerals [soft]	34		
Granite-Diorite-Gabbro [brittle]	27	26.7	1.8
Volcanic [soft]	28		
Total Fair Aggregate		127.2	8.5
Carbonate [shaley; clayey; silty, clayey]	43		
Carbonate [ochreous: sandy ochreous]	44		
Marble [friable]	49		
Chert-Cherty Carbonate [>20% leached chert]	45		
Conglomerate-Sandstone-Arkose [friable]	46		
Siltstone	56		
Cementation [partial]	53		
Cementation [total]	54		
Gneiss-Amphibolite [friable]	50		
Schist [soft]	55		
Granite-Diorite-Gabbro [friable]	51		
Volcanic [very soft, porous]	48		
Total Poor Aggregate		0	0.0
Ochre	60		
Carbonate/Shale	61		
Clay	62		
Volcanic-Gneiss Schist [decomposed]	63		
Total Deleterious Aggregate		0	0.0
% GOOD	91.5	x 1 =	91.5
% FAIR	8.5	x 3 =	25.5
% POOR	0.0	x 6 =	0.0
% DELETERIOUS	0.0	x 10 =	0.0
		Estimated % Crushed:	100%
		Estimated % Flat and Elongated:	<3%
Asphalt and Ready-Mix Concrete PN:		117	

Coarse Aggregate Petrographic Analysis- MTO LS609

Client Name: GeoNorth Engineering			
Source: Pit Outcrop	Project Name: Aggregate Qualification		
Date: 02-Dec-11	Fraction: 30-9.5mm		
Sample ID: crush rock	Analyst: J. Burton		
Type			
	Type No.	Mass (g)	%
Carbonate [hard; silty, hard]	01		
Carbonate [surface weathered; silty, surface weathered; medium hard: silty medium hard]	20		
Carbonate [sandy, hard or medium hard]	02		
Carbonate [slightly cherty; <5% chert]	21		
Chert [hard or medium hard]	23		
Conglomerate-Sandstone-Arkose [hard]	03		
Conglomerate-Sandstone-Arkose [medium hard]	22		
Greywacke-Argillite [hard or medium hard]	06		
Gneiss-Amphibolite-Schist [hard]	04	974.2	64.5
Quartzite [hard]	05		
Granite-Diorite-Gabbro [hard]	08	397.3	26.3
Volcanic [hard or medium hard]	07		
Minerals [hard/medium]	09		
Quartz vein	10		
Total Good Aggregate		1371.5	90.8
Carbonate [soft; silty soft; slightly shaley]	35		
Carbonate [soft, pitted]	41		
Carbonate [deeply weathered; silty, deeply weathered]	42		
Carbonate [sandy, soft]	40		
Marble [brittle]	24		
Chert-Cherty Carbonate [<20% leached chert]	26		
Conglomerate-Sandstone-Arkose [brittle]	30		
Greywacke [brittle]	29		
Quartzite [slightly weathered]	52		
Gneiss-Amphibolite-Schist [brittle]	25	96.3	6.4
Minerals [soft]	34		
Granite-Diorite-Gabbro [brittle]	27	29.9	2.0
Volcanic [soft]	28		
Total Fair Aggregate		126.2	8.4
Carbonate [shaley; clayey; silty, clayey]	43		
Carbonate [ochreous: sandy ochreous]	44		
Marble [friable]	49		
Chert-Cherty Carbonate [>20% leached chert]	45		
Conglomerate-Sandstone-Arkose [friable]	46		
Siltstone	56		
Cementation [partial]	53		
Cementation [total]	54		
Gneiss-Amphibolite [friable]	50		
Schist [soft]	55	12.3	0.8
Granite-Diorite-Gabbro [friable]	51		
Volcanic [very soft, porous]	48		
Total Poor Aggregate		12.3	0.8
Ochre	60		
Carbonate/Shale	61		
Clay	62		
Volcanic-Gneiss Schist [decomposed]	63		
Total Deleterious Aggregate		0	0.0
% GOOD	90.8	x 1 = 90.8	Estimated % Crushed: 100%
% FAIR	8.4	x 3 = 25.1	Estimated % Flat and Elongated: <5%
% POOR	0.8	x 6 = 4.9	
% DELETERIOUS	0.0	x 10 = 0.0	
Asphalt and Ready-Mix Concrete PN:		121	



Analytical Report

Bill To: GeoNorth Engineering Inc.
Report To: GeoNorth Engineering Inc.
3975 18 Avenue
Prince George, BC, Canada
V2N 1B2
Attn: Hans Jorgensen
Sampled By:
Company:

Project:
ID: K-3300
Name:
Location:
LSD:
P.O.:
Acct code:

Lot ID: **837489**
Control Number:
Date Received: Nov 9, 2011
Date Reported: Nov 17, 2011
Report Number: 1528815

	Reference Number	837489-1	837489-2		
	Sample Date				
	Sample Time				
	Sample Location				
	Sample Description	Placer Tailings	Pit Outcrop		
	Matrix	Soil	Soil		
Analyte	Units	Results	Results	Results	Nominal Detection Limit
Available Nutrients					
Sulfate-S	Available	mg/kg	4	8	1

Approved by: 
Darlene Lintott, MSc
Consulting Scientist

Methodology and Notes

Bill To: GeoNorth Engineering Inc.	Project:	Lot ID: 837489
Report To: GeoNorth Engineering Inc.	ID: K-3300	Control Number:
3975 18 Avenue	Name:	Date Received: Nov 9, 2011
Prince George, BC, Canada	Location:	Date Reported: Nov 17, 2011
V2N 1B2	LSD:	Report Number: 1528815
Attn: Hans Jorgensen	P.O.:	
Sampled By:	Acct code:	
Company:		

Method of Analysis

Method Name	Reference	Method	Date Analysis Started	Location
Sulfate in General Soil	McKeague	* Sulfate Extractable by 0.1M CaCl ₂ , 4.47 <i>* Reference Method Modified</i>	10-Nov-11	Exova Edmonton

References

McKeague Manual on Soil Sampling and Methods of Analysis

Comments:

Please direct any inquiries regarding this report to our Client Services group.

Results relate only to samples as submitted.

The test report shall not be reproduced except in full, without the written approval of the laboratory.

GEO NORTH ENGINEERING LTD.

3975 18th Avenue
Prince George, B.C., V2N 1B2
Phone 250-564-4304
Fax 250-564-9323
E-mail mail@geonorth.ca

November 15, 2011

K-3300

BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC V6Z 2A9

Attention: Pete Quinn

Dear Sirs:

Re: Eagle Gold Concrete Aggregate Test Results, Pit Outcrop

Listed below are the tests that you requested on your proposed concrete aggregate to determine their conformation to CSA A23.2 specification:

		Results
CSA A23.2-3A	Clay lumps in Aggregate (5mm-10mm)	0.0%
CSA A23.2-4A	Low Density Granular Materials	0.0%
CSA A23.2-12A	Relative Density SSD (Fine Agg)	2.657
	Absorption	0.999
CSA A23.2-13A	Flat & Elongated 4:1 Ratio	35.0%
CSA A23.2-23A	Micro-Deval (Fine Agg)	13.3% Loss
CSA A23.2-29A	Micro-Deval (Coarse Agg)	12.9% Loss

If you require any further information, please do not hesitate to call me at our office at (250) 564-4304.

Yours truly,

GeoNorth Engineering Ltd.

Per: Hans Jorgensen
Manager, Technical Services

GEO NORTH ENGINEERING LTD.

3975 18th Avenue
Prince George, B.C., V2N 1B2
Phone 250-564-4304
Fax 250-564-9323
E-mail mail@geonorth.ca

November 15, 2011

K-3300

BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC V6Z 2A9

Attention: Pete Quinn

Dear Sirs:

Re: Eagle Gold Concrete Aggregate Test Results, Placer Tailings

Listed below are the tests that you requested on your proposed concrete aggregate to determine their conformation to CSA A23.2 specification:

		Results
CSA A23.2-3A	Clay lumps in Aggregate (5mm-10mm)	0.0%
CSA A23.2-4A	Low Density Granular Materials	0.0%
CSA A23.2-12A	Relative Density SSD (Fine Agg)	2.704
	Absorption	1.399%
CSA A23.2-13A	Flat & Elongated 4:1 Ratio	19.0%
CSA A23.2-23A	Micro-Deval (Fine Agg)	11.6% Loss
CSA A23.2-29A	Micro-Deval (Coarse Agg)	26.9% Loss

If you require any further information, please do not hesitate to call me at our office at (250) 564-4304.

Yours truly,

GeoNorth Engineering Ltd.

Per: Hans Jorgensen
Manager, Technical Services

GEO NORTH ENGINEERING LTD.

3975 18 th Avenue
Prince George, B.C., V2N 1B2

SOUNDNESS OF AGGREGATE SULPHATE TEST REPORT

PROJECT No.:	K-3300
DATE:	11/15/2011
P.O.:	
CC:	

PROJECT: EAGLE GOLD AGGREGATE TESTING		
SOURCE: PIT OUTCROP	TYPE OF SAMPLE: CRUSHED ROCK	SAMPLED BY: CLIENT
DATE SAMPLED: 8/23/2011	DATE RECEIVED: 9/13/2011	DATE TESTED: 11/6/2011

SOLUTION: MgSO ₄				NUMBER OF CYCLES: 5			
COARSE AGGREGATE				FINE AGGREGATE			
SIEVE SIZE		ORIGINAL GRADING PERCENT	WEIGHED AVERAGE PERCENT LOSS	SIEVE SIZE		ORIGINAL GRADING PERCENT	WEIGHED AVERAGE PERCENT LOSS
PASSING	RETAINED			PASSING	RETAINED		
80 mm	56 mm						
56 mm	40 mm	12.43	0.00	5 mm	2.5 mm	37.3	0.22
40 mm	28 mm	9.22	0.00	2.5 mm	1.25 mm	19.7	0.63
28 mm	20 mm	7.54	0.01	1.25 mm	630 µm	11.3	0.46
20 mm	14 mm	6.15	0.54	630 µm	315 µm	7.4	0.18
14 mm	10 mm	22.35	0.28	315 µm	160 µm	4.6	1.66
10 mm	5 mm	42.32	0.13	160 µm		19.7	
TOTALS		100.0	0.97	TOTALS		100.0	3.16

SIZE FRACTION	NO. PARTICLES	QUALITATIVE EXAMINATION OF PLUS 20 MM MATERIAL.
80 mm - 56 mm	ORIGINAL	
	FINAL	
56 mm - 40 mm	ORIGINAL	
	FINAL	
40 mm - 28 mm	ORIGINAL	
	FINAL	
28 mm - 20 mm	ORIGINAL	
	FINAL	

TESTED IN ACCORDANCE WITH CSA A23.2 - 9A

NOELCO

COMMENTS:

CERTIFIED:

REPORTING OF TEST RESULTS CONSTITUTES A TESTING SERVICE ONLY. ENGINEERING EVALUATION OF THE TEST RESULTS IS PROVIDED ONLY ON REQUEST.

GEO NORTH ENGINEERING LTD.

3975 18 th Avenue
Prince George, B.C., V2N 1B2

SOUNDNESS OF AGGREGATE SULPHATE TEST REPORT

PROJECT No.:	K-3300
DATE:	11/15/2011
P.O.:	
CC:	

PROJECT: EAGLE GOLD AGGREGATE TESTING		
SOURCE: PLACER TAILINGS	TYPE OF SAMPLE: CRUSHED ROCK	SAMPLED BY: CLIENT
DATE SAMPLED: 8/23/2011	DATE RECEIVED: 9/13/2011	DATE TESTED: 11/6/2011

SOLUTION: MgSO ₄				NUMBER OF CYCLES: 5			
COARSE AGGREGATE				FINE AGGREGATE			
SIEVE SIZE		ORIGINAL GRADING PERCENT	WEIGHED AVERAGE PERCENT LOSS	SIEVE SIZE		ORIGINAL GRADING PERCENT	WEIGHED AVERAGE PERCENT LOSS
PASSING	RETAINED			PASSING	RETAINED		
80 mm	56 mm						
56 mm	40 mm	9.53	0.00	5 mm	2.5 mm	34.2	0.50
40 mm	28 mm	10.28	0.00	2.5 mm	1.25 mm	22.4	0.76
28 mm	20 mm	5.23	0.03	1.25 mm	630 µm	14.6	1.03
20 mm	14 mm	5.42	0.02	630 µm	315 µm	9.2	0.94
14 mm	10 mm	15.14	0.10	315 µm	160 µm	6.0	0.64
10 mm	5 mm	54.4	0.12	160 µm		13.5	
TOTALS		100.0	0.27	TOTALS		100.0	3.86

SIZE FRACTION	NO. PARTICLES	QUALITATIVE EXAMINATION OF PLUS 20 MM MATERIAL.
80 mm - 56 mm	ORIGINAL	
	FINAL	
56 mm - 40 mm	ORIGINAL	
	FINAL	
40 mm - 28 mm	ORIGINAL	
	FINAL	
28 mm - 20 mm	ORIGINAL	
	FINAL	

TESTED IN ACCORDANCE WITH CSA A23.2 - 9A

NOELCO

COMMENTS:

CERTIFIED:

REPORTING OF TEST RESULTS CONSTITUTES A TESTING SERVICE ONLY. ENGINEERING EVALUATION OF THE TEST RESULTS IS PROVIDED ONLY ON REQUEST.



METRO TESTING LABORATORIES LTD.

6991 Curragh Avenue, Burnaby, BC V5J 4V6
Tel: (604) 436-9111 Fax: (604) 436-9050

Detection of Alkali-silica Reactive Aggregate by Accelerated Expansion of Mortar Bars (CSA A23.2 - 25A)

GeoNorth Engineering Ltd.
3975 18th Avenue
Prince George, BC V2N 1B2

Date: 16 December 2011

Our File: 15585 Set 2

Attn: Hans Jorgensen

PROJECT: Suitability of Aggregate for Concrete

Sample	Crushed Rock (Source: Placer Tailings)
---------------	---

Date sample received: 9 Nov. 2011

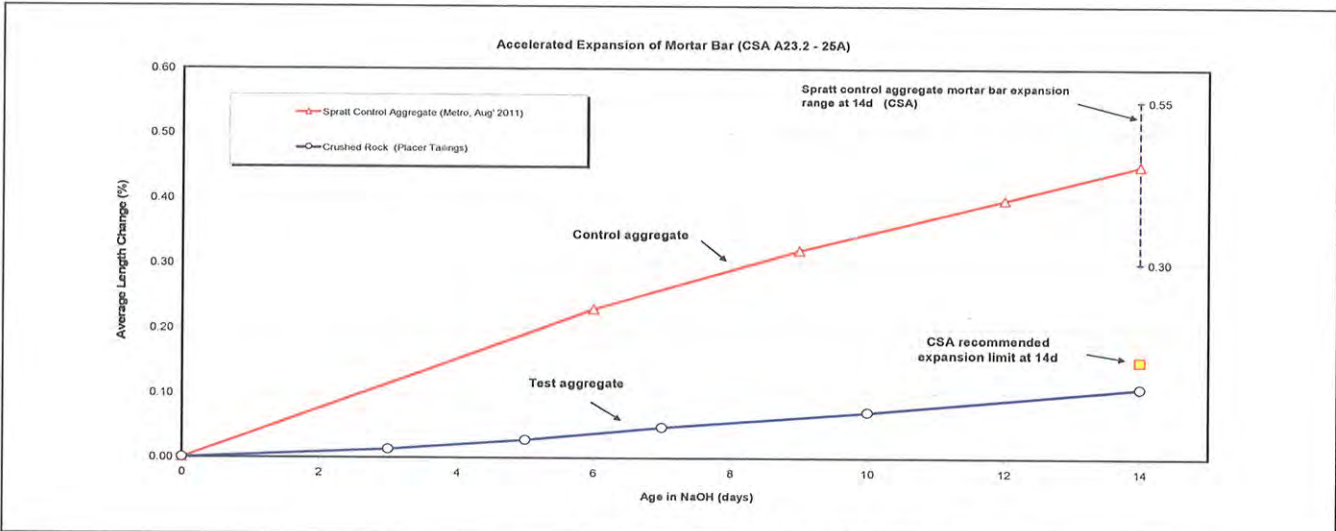
Sampled by: Client

Date specimens cast: 30 Nov. 2011

Water/Cement Ratio: 0.50

Portland Cement: Essroc type GU, alkali content of 0.94% Na₂O_{eq}.

Test method: CSA A23.2-25A, Test Method for Detection of Alkali-silica Reactive Aggregate by Accelerated Expansion of Mortar Bars



Mortar bar specimens	Expansion, % (age in NaOH solution, days)				
	3	5	7	10	14
1	0.014	0.029	0.050	0.073	0.111
2	0.015	0.029	0.050	0.074	0.112
3	0.011	0.028	0.047	0.072	0.109
4	0.012	0.025	0.043	0.066	0.102
Average	0.013	0.028	0.047	0.071	0.108
CSA recommended limit for deleterious expansion					0.15
Spratt control aggregate test results (Metro, Aug' 11)					0.45
Observation: White spots on mortar bars were observed at 7 days test age. Hairline cracks were observed at 14 days test age.					

Comments: Sample meets CSA maximum limit of 0.15% expansion at 14 days. According to CSA A23.2 - 27A, Table 2, the degree of alkali-silica reactivity of the aggregate may be classified as "Non-reactive".

Metro Testing Laboratories Ltd.

Des Guo, M.Eng., P.Eng.
Materials Engineer

Reviewed by:

Prakash V. Joshi, ASCE, Eng. Tech.
Senior Materials Engineering Technologist





METRO TESTING LABORATORIES LTD.

6991 Curragh Avenue, Burnaby, BC V5J 4V6
Tel: (604) 436-9111 Fax: (604) 436-9050

Detection of Alkali-silica Reactive Aggregate by Accelerated Expansion of Mortar Bars (CSA A23.2 - 25A)

GeoNorth Engineering Ltd.
3975 18th Avenue
Prince George, BC V2N 1B2

Date: 16 December 2011

Our File: 15585 Set 1

Attn: Hans Jorgensen

PROJECT: Suitability of Aggregate for Concrete

Sample	Crushed Rock (Source: Pit Outcrop)
---------------	---

Date sample received: 9 Nov. 2011

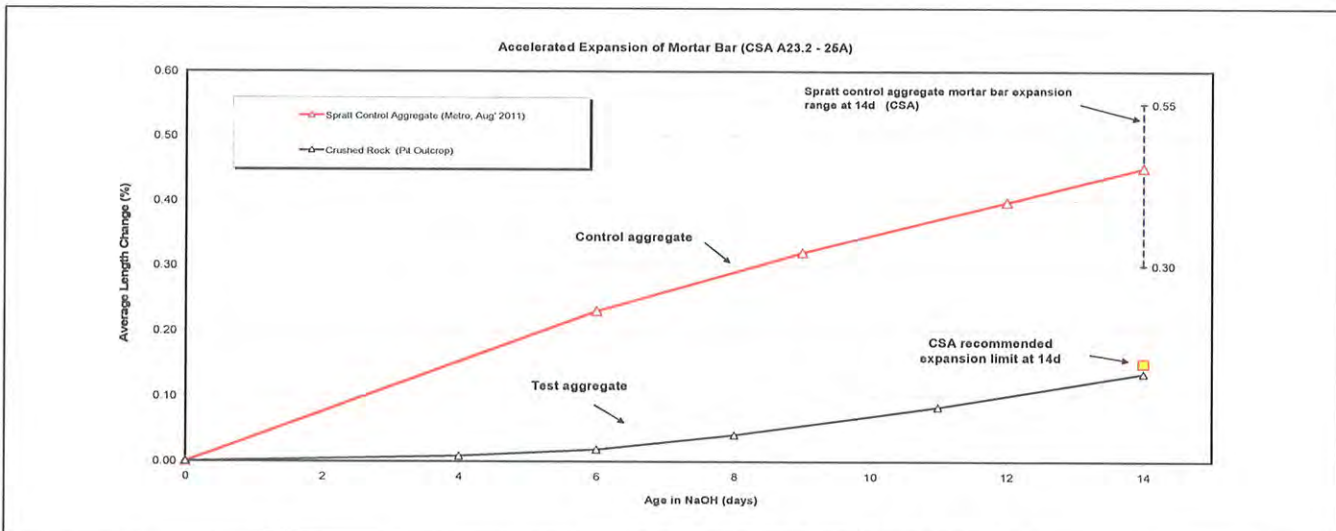
Sampled by: Client

Date specimens cast: 29 Nov. 2011

Water/Cement Ratio: 0.50

Portland cement: Essroc type GU, alkali content of 0.94% Na₂O_{eq.}

Test method: CSA A23.2-25A, Test Method for Detection of Alkali-silica Reactive Aggregate by Accelerated Expansion of Mortar Bars



Mortar bar specimens	Expansion, % (age in NaOH solution, days)				
	4	6	8	11	14
1	0.008	0.017	0.040	0.081	0.136
2	0.008	0.016	0.040	0.080	0.139
3	0.009	0.017	0.039	0.083	0.131
4	0.008	0.022	0.042	0.088	0.133
Average	0.009	0.018	0.040	0.083	0.135
CSA recommended limit for deleterious expansion					0.15
Spratt control aggregate test results (Metro, Aug' 11)					0.45
Observation: White spots on mortar bars were observed at 8 days test age.					

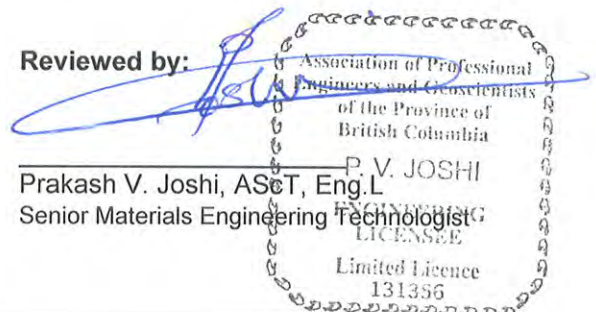
Comments: Sample meets CSA maximum limit of 0.15% expansion at 14 days. According to CSA A23.2 – 27A, Table 2, the degree of alkali-silica reactivity of the aggregate may be classified as **“Non-reactive”**.

Metro Testing Laboratories Ltd.

Des Guo, M.Eng., P.Eng.
Materials Engineer

Reviewed by:

Prakash V. Joshi, ASCT, Eng.L
Senior Materials Engineering Technologist



GEO NORTH ENGINEERING LTD.

3975 18th Avenue
Prince George, B.C., V2N 1B2
Phone 250-564-4304
Fax 250-564-9323
E-mail mail@geonorth.ca

January, 10 2012

K-3300

BGC Engineering Inc.
Suite 500 - 1045 Howe Street
Vancouver, BC V6Z 2A9

Attention: Pete Quinn

Dear Sirs:

Re: Eagle Gold Aggregate Test Results, Placer Steiner Outcrop

Listed below are the tests that you requested on your proposed concrete aggregate to determine their conformation to CSA A23.2 specification:

		Results
CSA A23.2-6A	Relative Density SSD (Fine Agg)	2.718
	Absorption	1.122%
CSA A23.2-29A	Micro-Deval (Coarse Agg)	11.0% Loss

If you require any further information, please do not hesitate to call me at our office at (250) 564-4304.

Yours truly,

GeoNorth Engineering Ltd.

Per: Hans Jorgensen
Supervisor, Technical Services

LABORATORY TESTS COMMISSIONED BY TETRA TECH

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

November 4, 2011
Project number: 11-1415-0013/1000

ATTENTION: Mr. Troy Meyer, P.E.

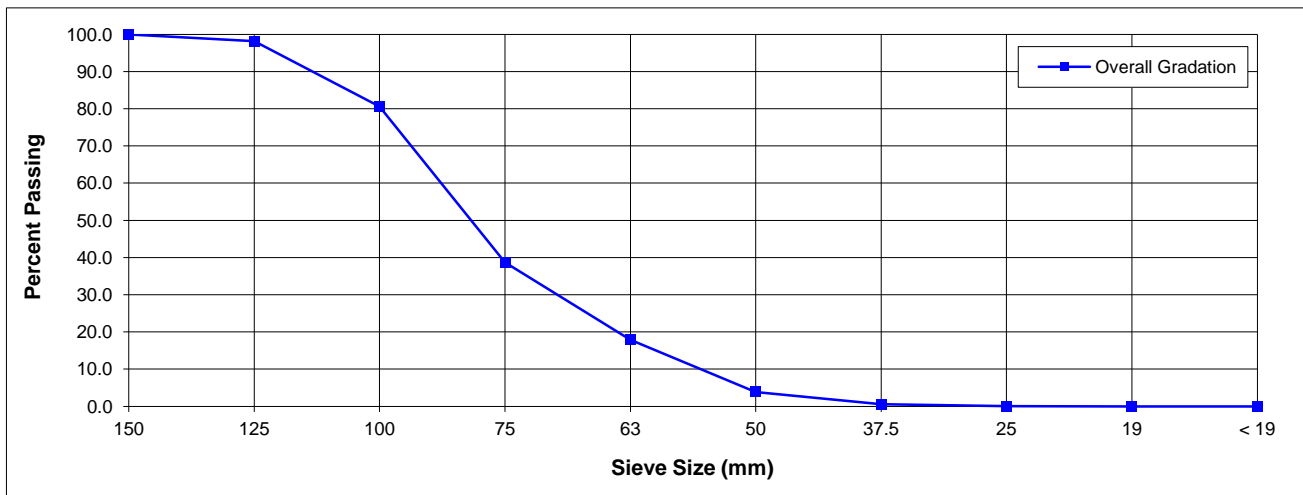
PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF , As Received
Source	Eagle Gold, Yukon

DATE RECEIVED: November 3, 2011
DATE TESTED: November 4, 2011

SAMPLED BY: BGC
TESTED BY: IC

SIEVE ANALYSIS					MATERIAL SPECIFICATION :
Sieve Size (mm)	% Retained	% Passing	Individual % Retained (Split values)		
			+ < 19	- < 19	
150	0.0	100.0	0.0		
125	1.8	98.2	1.8		
100	17.6	80.6	17.6		
75	41.9	38.7	41.9		
63	20.8	17.9	20.8		
50	14.0	3.9	14.0		
37.5	3.3	0.6	3.3		
25	0.5	0.0	0.5		
19	0.0	0.0	0.0		
< 19	0.0	0.0	0.0		
Total	100.0		100.0		



Remarks: Entire sample received sieved; total mass: 174928 g

Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other zones/depths. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

November 4, 2011
Project number: 11-1415-0013/1000

ATTENTION: Mr. Troy Meyer, P.E.

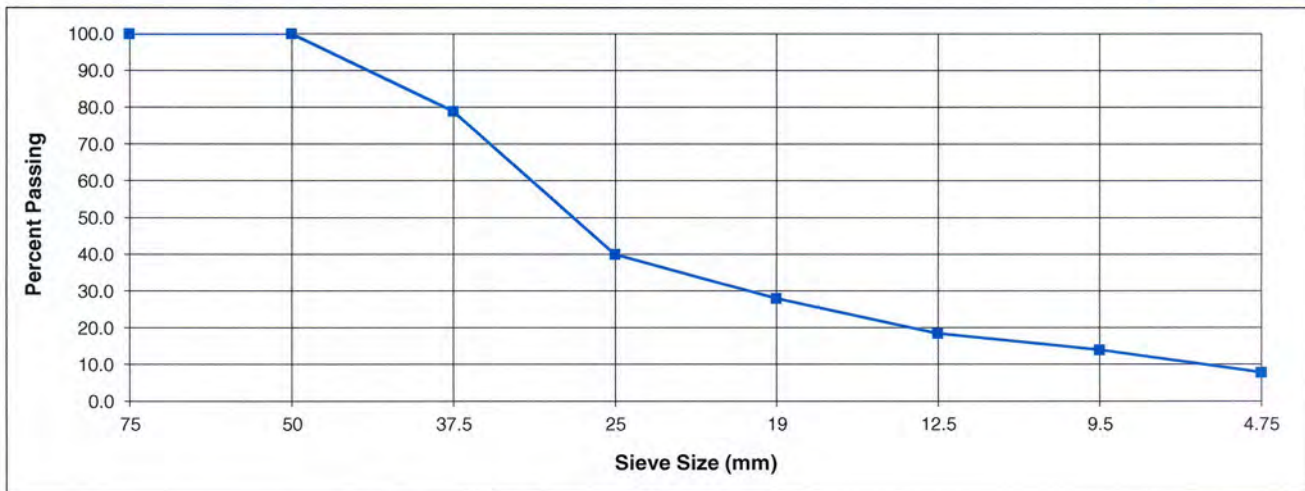
PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF, 50mm minus lab crushed
Source	Eagle Gold, Yukon

DATE RECEIVED: November 3, 2011
DATE TESTED: November 7, 2011

SAMPLED BY: BGC
TESTED BY: IC

SIEVE ANALYSIS					MATERIAL SPECIFICATION :
Sieve Size (mm)	% Retained	% Passing	Individual % Retained (Split values)		
			+ 4.75	- 4.75	
75	0.0	100.0	0.0		
50	0.0	100.0	0.0		
37.5	21.1	78.9	22.9		
25	38.9	40.0	42.2		
19	12.0	28.0	13.0		
12.5	9.6	18.4	10.4		
9.5	4.5	14.0	4.8		
4.75	6.1	7.9	6.7		
PAN	7.9			100.0	
Total	100.0		100.0	100.0	



Remarks: Combined Sample(BGC-GD-1, 2,3,4,5 and BGC-MS-1, 2,3,4,5)
Sample crushed to 50mm minus by lab jaw crusher.

Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other zones/depths. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



**MAXIMUM AND MINIMUM INDEX DENSITY
USING A VIBRATORY TABLE**
ASTM D 4253 & D 4254

January 12, 2012
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 50 mm minus lab crushed
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: December 21, 2011

Date received: November 3, 2011
Tested by: DC

Trial	Maximum Index Density (Dry method 2A) kg/m ³	Maximum Index Density (Wet method 2B) kg/m ³	Minimum Index Density (Method A) kg/m ³
1	1755	1764	1400
2	--	--	--
AVERAGE	1755	1764	1400

Note: Tests were carried out with the large mould.

Reported by: I. Chung

Reviewed by: _____
L. Hu, MSc.E



Notice: The test data given herein pertain to the sample provided. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



RELATIVE DENSITY (SPECIFIC GRAVITY) AND ABSORPTION OF COARSE AGGREGATE

ASTM C 127

December 12, 2011
 Project number: 11-1415-0013/1000

Tetra Tech
 Ste 204, 120 West Park Dr.
 Grand Junction, CO 81505
 United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 50mm minus crushed, Coarse Portions
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: November 30, 2011

Date received: November 3, 2011
Tested by: DC

Size Fractions	Relative Density (oven dry basis) g/cm ³	Relative Density (SSD basis) g/cm ³	Relative Density (Apparent) g/cm ³	Absorption (%)
50 x 37.5mm	2.661	2.683	2.722	0.84
37.5 x 25.0mm	2.643	2.665	2.702	0.82
25.0 x 12.5mm	2.634	2.659	2.701	0.94
12.5 x 4.75mm	2.603	2.638	2.697	1.33

Reported by: I. Chung

Reviewed by: _____
 L. Hu, MSc.E



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other production zones/periods. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

GOLDER ASSOCIATES LTD., 245. 12388 88th Avenue, Surrey, B.C. Canada V3W 7R7 Tel: 604-591-6616 Fax: 604-591-6608



RELATIVE DENSITY (SPECIFIC GRAVITY) AND ABSORPTION OF FINE AGGREGATE ASTM C 128

December 12, 2011
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 50mm minus crushed, Passing 4.75mm Portion
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: November 30, 2011

Date received: November 3, 2011
Tested by: DC

Trial No.	Relative Density (dry basis)	Relative Density (SSD basis)	Apparent Relative Density	Absorption (%)
1	2.452	2.545	2.704	3.79
2	2.458	2.550	2.707	3.73
AVERAGE	2.455	2.548	2.705	3.76

Reported by: I. Chung

Reviewed by: _____
L. Hu, MSc.E



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other production zones/periods. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

GOLDER ASSOCIATES LTD., 245. 12388 88th Avenue, Surrey, B.C. Canada V3W 7R7 Tel: 604-591-6616 Fax: 604-591-6608

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

January 12, 2012
Project number: 11-1415-0013/1000

ATTENTION: Mr. Troy Meyer, P.E.

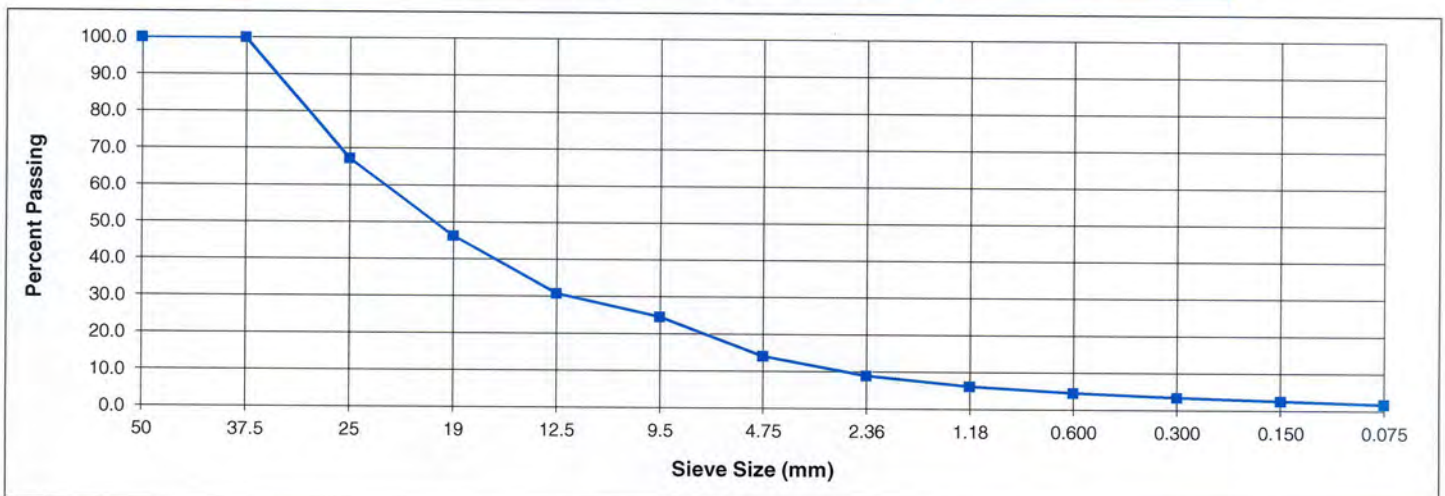
PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF, 37.5mm minus lab crushed
Source	Eagle Gold, Yukon

DATE RECEIVED: November 3, 2011
DATE TESTED: December 20, 2011

SAMPLED BY: BGC
TESTED BY: IC

SIEVE ANALYSIS					MATERIAL SPECIFICATION :
Sieve Size (mm)	% Retained	% Passing	Individual % Retained (Split values)		
			+ 4.75	- 4.75	
50	0.0	100.0	0.0		
37.5	0.0	100.0	0.0		
25	32.7	67.3	38.1		
19	21.0	46.3	24.4		
12.5	15.5	30.8	18.1		
9.5	6.2	24.6	7.2		
4.75	10.4	14.2	12.2		
2.36	5.2	9.0		36.5	
1.18	2.7	6.3		19.3	
0.600	1.6	4.6		11.4	
0.300	1.2	3.5		8.2	
0.150	0.8	2.7		5.8	
0.075	0.8	1.8		5.9	
PAN	1.8	0		12.9	
Total	100.0		100.0	100.0	



Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other zones/depths. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



**MAXIMUM AND MINIMUM INDEX DENSITY
USING A VIBRATORY TABLE**
ASTM D 4253 & D 4254

January 12, 2012
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 37.5 mm minus lab crushed
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: January 5, 2012

Date received: November 3, 2011
Tested by: DC

Trial	Maximum Index Density (Dry method 2A) kg/m ³	Maximum Index Density (Wet method 2B) kg/m ³	Minimum Index Density (Method A) kg/m ³
1	1854	1908	1553
2	1891	--	1532
AVERAGE	1872	1908	1543

Note: Tests were carried out with the large mould.

Reported by: I. Chung

Reviewed by: _____
L. Hu, MSc.E



Notice: The test data given herein pertain to the sample provided. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



RELATIVE DENSITY (SPECIFIC GRAVITY) AND ABSORPTION OF FINE AGGREGATE
ASTM C 128

January 12, 2012
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 37.5mm minus lab crushed, passing 4.75mm portion
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: December 21, 2011

Date received: November 3, 2011
Tested by: DC

Trial No.	Relative Density (dry basis)	Relative Density (SSD basis)	Apparent Relative Density	Absorption (%)
1	2.609	2.637	2.683	1.05
2	2.610	2.635	2.678	0.97
AVERAGE	2.610	2.636	2.681	1.01

Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other production zones/periods. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



**RESISTANCE TO DEGRADATION OF
SMALL-SIZE COARSE AGGREGATE
BY ABRASION & IMPACT IN THE
LOS ANGELES MACHINE
ASTM C 131**

January 12, 2012
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 37.5mm minus lab crushed
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: January 9, 2012

Date received: November 3, 2011
Tested by: DC

Grading	A
Number of revolutions	500
Loss after 500 revolutions (%)	28.4

Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other production zones/periods. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

GOLDER ASSOCIATES LTD., 245, 12388 88th Avenue, Surrey, B.C. Canada V3W 7R7 Tel: 604-591-6616 Fax: 604-591-6608

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

January 12, 2012
Project number: 11-1415-0013/1000

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF, 25mm minus lab crushed
Source	Eagle Gold, Yukon

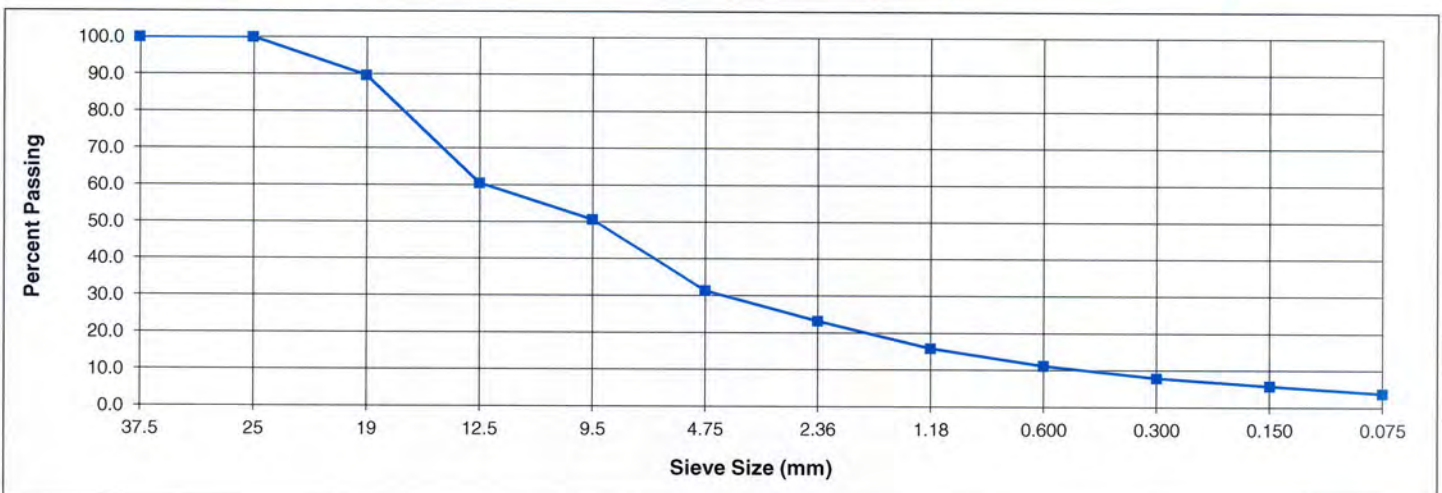
DATE RECEIVED: November 3, 2011

SAMPLED BY: BGC

DATE TESTED: January 9, 2012

TESTED BY: DC

SIEVE ANALYSIS					MATERIAL SPECIFICATION :
Sieve Size (mm)	% Retained	% Passing	Individual % Retained (Split values)		
			+ 4.75	- 4.75	
37.5	0.0	100.0	0.0		
25	0.0	100.0	0.0		
19	10.3	89.7	15.1		
12.5	29.2	60.5	42.5		
9.5	9.8	50.7	14.3		
4.75	19.3	31.4	28.1		
2.36	8.2	23.2		26.0	
1.18	7.3	16.0		23.1	
0.600	4.7	11.3		15.0	
0.300	3.3	8.0		10.5	
0.150	2.1	5.8		6.8	
0.075	2.0	3.8		6.4	
PAN	3.8	0		12.2	
Total	100.0		100.0	100.0	



Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



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**MAXIMUM AND MINIMUM INDEX DENSITY
USING A VIBRATORY TABLE
ASTM D 4253 & D 4254**

January 12, 2012
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF, 25 mm minus lab crushed
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: January 11, 2012

Date received: November 3, 2011
Tested by: DC

Trial	Maximum Index Density (Dry method 2A) kg/m ³	Maximum Index Density (Wet method 2B) kg/m ³	Minimum Index Density (Method A) kg/m ³
1	1991	2108	1620
2	1984	--	1634
AVERAGE	1988	2108	1627

Note: Tests were carried out with the large mould.

Reported by: I. Chung

Reviewed by: _____
L. Hu, MSc.E



Notice: The test data given herein pertain to the sample provided. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



RELATIVE DENSITY (SPECIFIC GRAVITY) AND ABSORPTION OF COARSE AGGREGATE
ASTM C 127

January 12, 2012
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 25mm minus crushed, retain on 4.75mm portions
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: January 10, 2012

Date received: November 3, 2011
Tested by: DC

Trial No.	Relative Density (dry basis)	Relative Density (SSD basis)	Apparent Relative Density	Absorption (%)
1	2.638	2.661	2.699	0.87
2	2.634	2.658	2.700	0.93
AVERAGE	2.636	2.660	2.700	0.90

Reported by: I. Chung

Reviewed by: _____
L. Hu, MSc.E



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GOLDER ASSOCIATES LTD., 245, 12388 88th Avenue, Surrey, B.C. Canada V3W7R7 Tel: 604-591-6616 Fax: 604-591-6608



RELATIVE DENSITY (SPECIFIC GRAVITY) AND ABSORPTION OF FINE AGGREGATE
ASTM C 128

January 12, 2012
Project number: 11-1415-0013/1000

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	ODF 25mm minus lab crushed, passing 4.75mm portion
Source:	Eagle Gold, Yukon

Date sampled: Not provided
Date tested: January 10, 2012

Date received: November 3, 2011
Tested by: IC

Trial No.	Relative Density (dry basis)	Relative Density (SSD basis)	Apparent Relative Density	Absorption (%)
1	2.601	2.639	2.704	1.48
2	2.610	2.646	2.708	1.39
AVERAGE	2.605	2.643	2.706	1.43

Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other production zones/periods. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

GOLDER ASSOCIATES LTD., 245, 12388 88th Avenue, Surrey, B.C. Canada V3W7R7 Tel: 604-591-6616 Fax: 604-591-6608



Compaction of Soil Using Standard Effort				Reference ASTM D 698-07	
Client:	Tetra Tech	Sample No.:	Section 2 Silt		
Project:	Aggregate Testing	Sample Location:			
Location:	Eagle Gold, Yukon	Depth (m):	N/A		
Project No.:	11-1415-0013/1000	Lab ID No:	211		
Effort Type:	Standard	Visual Description:	-		
Test Method:	A	Type of Rammer:	Mechanical		
Preparation Method:	Moist	Mould Volume (m³):	0.000945		
Remarks:					
Compaction Results					
Trial No.:	1	2	3	4	5
Unit Weight (KN/m³)	16.51	16.75	16.94	16.19	16.7
Water Content (%)	12.3	13.9	15.2	18.3	16.4
<p>The graph plots Unit Weight (KN/m³) on the y-axis (ranging from 15.00 to 18.00) against Water Content (%) on the x-axis (ranging from 10 to 30). It features three data series: Proctor Results (open circles), a 100% Saturation Curve (dashed line), and a Polynomial fit to the Proctor Results (solid line). The Proctor Results show a peak unit weight of approximately 16.94 KN/m³ at 15.2% water content. The 100% Saturation Curve is a straight line with a negative slope, starting at approximately 17.8 KN/m³ at 18% water content and ending at approximately 15.0 KN/m³ at 29% water content.</p>					
Uncorrected Optimum w (%)	15.2		Uncorrected Unit Weight (KN/m³)	16.93	
ASTM D 4718-87 (2007) Correction for oversize particles					
Max Percent Oversize (%)	25		Sieve Size (mm)	4.75	
Oversized Fraction			Test Fraction		
Percent Oversized Fraction (%)	0.0		Percent Test Fraction (%)	100.0	
G_{sc} (calculated)	2.74		G_{sf} (calculated)	2.74	
Water Content of Oversized (%)	16.3		Water Content of Test (%)	15.2	
Final Results					
Corrected Optimum w (%)	15.2		Corrected Unit Weight (KN/m³)	16.93	
			Corrected Dry Density (Kg/m³)	1726	
SK	December-09-11		LP	December 21,2011	
TESTED BY	DATE		CHECKED BY	DATE	

SPECIFIC GRAVITY OF SOIL SOLIDS

Reference
 ASTM C 127-07
 ASTM D 854-06 Method B

Project No.:	11-1415-0013/1000	Borehole	Section 2 Silt
Client:	Tetra Tech	Sample No.:	N/A
Project:	Aggregate Testing	Depth (m):	N/A
Location:	Eagle Gold, Yukon	Lab Sch No:	211

Specific Gravity of Fine Fraction (ASTM D 854-06)

Percentage Passing #4 sieve		100	
Test Number		1	2
Flask Number		5	6
Air Removal Method		Vacuum	Vacuum
Mass of Flask (g)		174.83	173.41
Mass of Flask + Dry Soil (g)	M_P	248.33	251.02
Mass of Dry Soil (g)		73.37	77.76
Mass of Flask + Soil + Water (g)	$M_{pws,t}$	719.75	720.71
Test Temperature (g)	T_t	18.50	18.40
Mass of Flask + Water (g)	$M_{pw,t}$	673.08	671.53
Mass of Dish + Dry Soil (g)		252.00	424.26
Mass of Dish (g)		178.63	346.50
Mass of Oven Dry Soil (g)	M_S	73.37	77.76
Temperature Coefficient	K	1.00	1.00
Density of Solids (g/cm ³)	ρ_s	2.75	2.72
Specific Gravity at Test Temperature	G_t	2.75	2.72
Specific Gravity at 20°C	$G_{20^\circ C}$	2.75	2.73
AVERAGE SPECIFIC GRAVITY		2.74	

Specific Gravity of Coarse Fraction (ASTM C 127-07)

Percentage Retained on #4 sieve		0
Mass of Sample in Water (g)	A	
Mass of Sample @ SSD (g)	B	
Mass of Oven Dried Sample (g)	C	
Bulk G (Oven Dry)	C/(B-A)	
Bulk G (SSD)	B/(B-A)	
Apparent	C/(C-A)	
Absorbion (%)	(B-C)/C	

Combined Specific Gravity

COMBINED SPECIFIC GRAVITY	$G_{avg @ 20^\circ C}$	
----------------------------------	------------------------	--

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.*

SK/RS	December 5, 2011	LP	December 21, 2011
TESTED BY	DATE	CHECKED BY	DATE



SIEVE ANALYSIS OF FINE AND COARSE AGGREGATE

ASTM C 136

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

December 16, 2011
Project number: 11-1415-0013/1000

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	Section 3 Colluvium
Source	Eagle Gold, Yukon

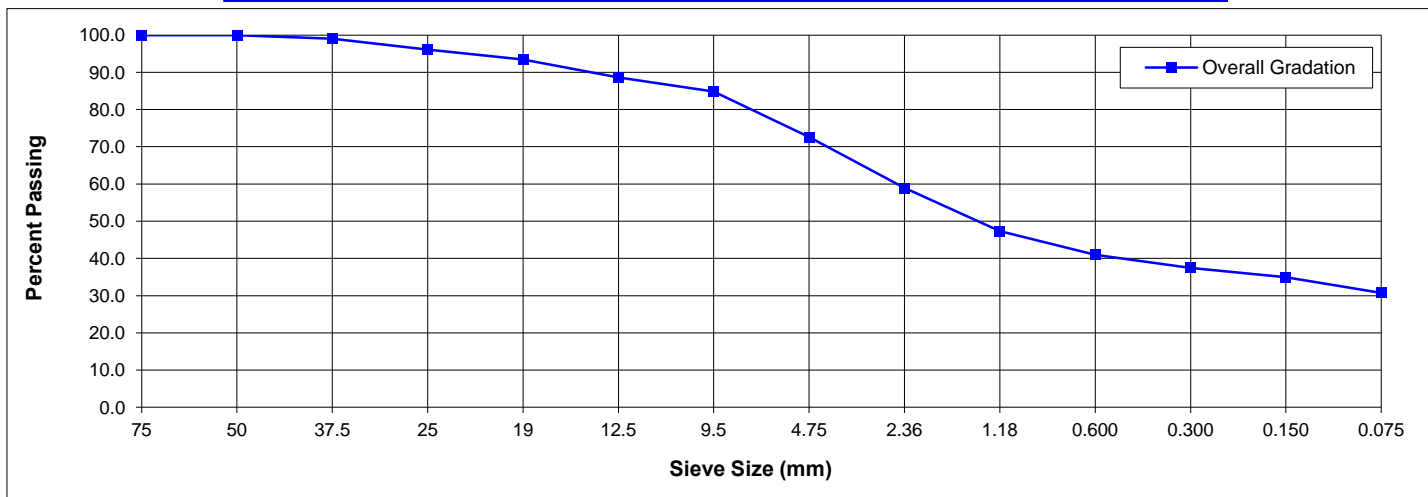
DATE RECEIVED: December 15, 2011

SAMPLED BY: BGC

DATE TESTED: December 16, 2012

TESTED BY: IC

SIEVE ANALYSIS					MATERIAL SPECIFICATION :	
Sieve Size (mm)	% Retained	% Passing	Individual % Retained (Split values)			
			+ 4.75	- 4.75		
75	0.0	100.0	0.0			
50	0.0	100.0	0.0			
37.5	0.9	99.1	3.5			
25	2.9	96.1	10.7			
19	2.7	93.4	9.8			
12.5	4.8	88.6	17.5			
9.5	3.8	84.8	13.9			
4.75	12.2	72.6	44.7			
2.36	13.7	58.9		18.8		
1.18	11.6	47.3		15.9		
0.600	6.3	41.0		8.7		
0.300	3.5	37.5		4.8		
0.150	2.5	34.9		3.5		
0.075	4.2	30.7		5.8		
PAN	30.7	0		42.4		
Total	100.0		100.0	100.0		



Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other zones/depths. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



**RESISTANCE OF COARSE AGGREGATE
TO DEGRADATION BY ABRASION IN
THE MICRO-DEVAL APPARATUS
ASTM D 6928**

January 18, 2012
Project number: 11-1415-0013/1000

TETRA TECH
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
U.S.A.

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	Section 3 Colluvium
Source:	Eagle Gold, Yukon

Date sampled: Not Given
Date tested: January 16, 2012

Sampled by: Client
Tested by: VN

Grading	Section 8.2 – 19x16, 16x12.5, 12.5x9.5mm
Loss at conclusion of test (%)	16.6

Validation Test Data: Control Aggregate (Drain Bros. stone)	
Test date	January 10, 2012
Percent loss	12.7%
Valid range	11.4 – 14.8%

Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other production zones/periods. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

GOLDER ASSOCIATES LTD., 245, 12388 88th Avenue, Surrey, B.C. Canada V3W 7R7 Tel: 604-591-6616 Fax: 604-591-6608



Compaction of Soil Using Standard Effort				Reference ASTM D 698-07	
Client:	Tetra Tech	Sample No.:	Section 3 Colluvium		
Project:	Aggregate Testing	Sample Location:			
Location:	Eagle Gold, Yukon	Depth (m):	N/A		
Project No.:	11-1415-0013/1000	Lab ID No:	211		
Effort Type:	Standard	Visual Description:	-		
Test Method:	C	Type of Rammer:	Mechanical		
Preparation Method:	Moist	Mould Volume (m³):	0.002122		
Remarks:					
Compaction Results					
Trial No.:	1	2	3	4	5
Unit Weight (KN/m³)	19.57	19.78	19.86	19.76	19.4
Water Content (%)	8.0	9.1	10.0	10.7	12.0
<p>The graph plots Unit Weight (KN/m³) on the y-axis (ranging from 19.00 to 21.00) against Water Content (%) on the x-axis (ranging from 0 to 20). It features three data series: Proctor Results (open circles), a 100% Saturation Curve (dashed line), and a polynomial fit to the Proctor Results (solid line). The Proctor Results show a peak unit weight of approximately 19.86 KN/m³ at 10.0% water content. The 100% Saturation Curve starts at approximately 19.86 KN/m³ at 10.0% water content and decreases to about 19.10 KN/m³ at 15.0% water content.</p>					
Uncorrected Optimum w (%)	10.0		Uncorrected Unit Weight (KN/m³)	19.86	
ASTM D 4718-87 (2007) Correction for oversize particles					
Max Percent Oversize (%)	25		Sieve Size (mm)	4.75	
Oversized Fraction			Test Fraction		
Percent Oversized Fraction (%)	17.0		Percent Test Fraction (%)	83.0	
Gsc (calculated)	2.75		Gsf (calculated)	2.75	
Water Content of Oversized (%)	16.3		Water Content of Test (%)	10.0	
Final Results					
Corrected Optimum w (%)	11.1		Corrected Unit Weight (KN/m³)	20.79	
			Corrected Dry Density (Kg/m³)	2119	
SK	December-13-11		LP	December 21,2011	
TESTED BY	DATE		CHECKED BY	DATE	

SPECIFIC GRAVITY OF SOIL SOLIDS

Reference
 ASTM C 127-07
 ASTM D 854-06 Method B

Project No.:	11-1415-0013/1000	Borehole	Section 3 Colluvium
Client:	Tetra Tech	Sample No.:	N/A
Project:	Aggregate Testing	Depth (m):	N/A
Location:	Eagle Gold, Yukon	Lab Sch No:	211

Specific Gravity of Fine Fraction (ASTM D 854-06)

Percentage Passing #4 sieve		65.82	
Test Number		1	2
Flask Number		5	6
Air Removal Method		Vacuum	Vacuum
Mass of Flask (g)		174.83	173.38
Mass of Flask + Dry Soil (g)	M_P	266.76	270.74
Mass of Dry Soil (g)		91.66	97.03
Mass of Flask + Soil + Water (g)	$M_{pws,t}$	731.54	733.22
Test Temperature (g)	T_t	19.40	19.40
Mass of Flask + Water (g)	$M_{pw,t}$	673.04	671.44
Mass of Dish + Dry Soil (g)		202.22	207.55
Mass of Dish (g)		110.56	110.52
Mass of Oven Dry Soil (g)	M_S	91.66	97.03
Temperature Coefficient	K	1.00	1.00
Density of Solids (g/cm ³)	ρ_s	2.76	2.75
Specific Gravity at Test Temperature	G_t	2.77	2.76
Specific Gravity at 20°C	$G_{20^\circ C}$	2.77	2.76
AVERAGE SPECIFIC GRAVITY		2.76	

Specific Gravity of Coarse Fraction (ASTM C 127-07)

Percentage Retained on #4 sieve		31.39
Mass of Sample in Water (g)	A	
Mass of Sample @ SSD (g)	B	
Mass of Oven Dried Sample (g)	C	
Bulk G (Oven Dry)	C/(B-A)	
Bulk G (SSD)	B/(B-A)	2.54
Apparent	C/(C-A)	2.71
Absorbtion (%)	(B-C)/C	4.09

Combined Specific Gravity

COMBINED SPECIFIC GRAVITY	$G_{avg @ 20^\circ C}$	2.75
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** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.*

SK/RS	November 15, 2011	LP	December 21, 2011
TESTED BY	DATE	CHECKED BY	DATE

Tetra Tech
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
United States

December 16, 2011
Project number: 11-1415-0013/1000

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	Section 4 Placer Tails
Source	Eagle Gold, Yukon

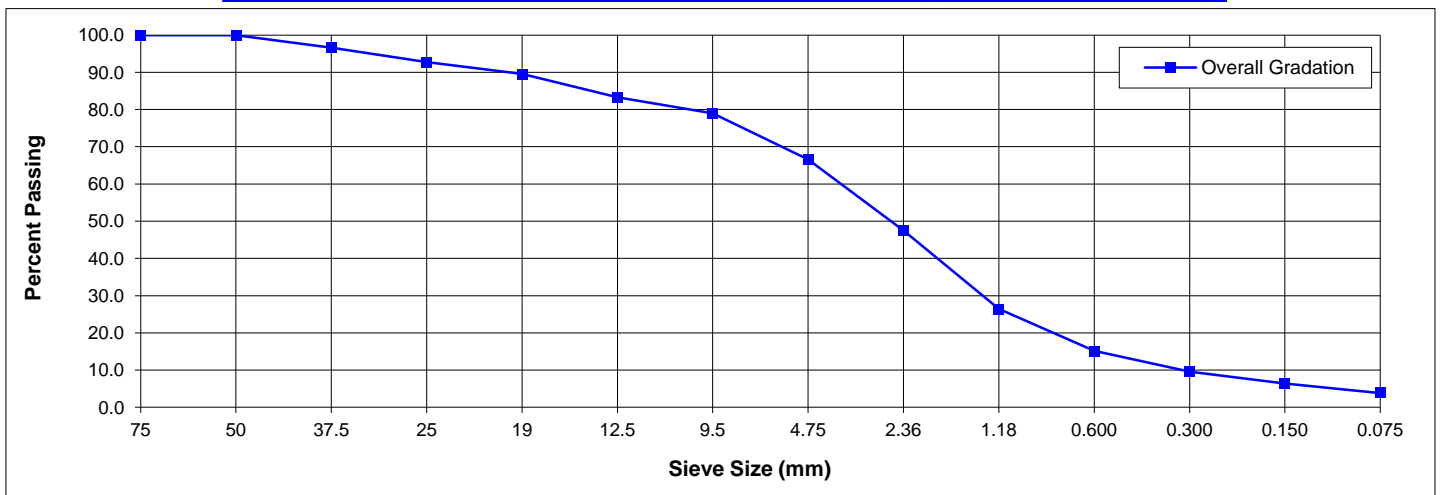
DATE RECEIVED: December 15, 2011

SAMPLED BY: BGC

DATE TESTED: December 16, 2012

TESTED BY: IC

SIEVE ANALYSIS					MATERIAL SPECIFICATION :	
Sieve Size (mm)	% Retained	% Passing	Individual % Retained (Split values)			
			+ 4.75	- 4.75		
75	0.0	100.0	0.0			
50	0.0	100.0	0.0			
37.5	3.4	96.6	10.1			
25	3.9	92.7	11.7			
19	3.2	89.5	9.6			
12.5	6.2	83.3	18.6			
9.5	4.3	79.0	12.9			
4.75	12.4	66.6	37.2			
2.36	19.1	47.5		28.6		
1.18	21.1	26.4		31.7		
0.600	11.3	15.1		16.9		
0.300	5.6	9.6		8.3		
0.150	3.2	6.4		4.8		
0.075	2.5	3.9		3.8		
PAN	3.9	0		5.8		
Total	100.0		100.0	100.0		



Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other zones/depths. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.



**RESISTANCE OF COARSE AGGREGATE
TO DEGRADATION BY ABRASION IN
THE MICRO-DEVAL APPARATUS
ASTM D 6928**

January 18, 2012
Project number: 11-1415-0013/1000

TETRA TECH
Ste 204, 120 West Park Dr.
Grand Junction, CO 81505
U.S.A.

ATTENTION: Mr. Troy Meyer, P.E.

PROJECT: TETRA TECH/ AGGREGATE/ YUKON

Sample:	Section 4 Placer Tails
Source:	Eagle Gold, Yukon

Date sampled: Not Given
Date tested: January 16, 2012

Sampled by: Client
Tested by: VN

Grading	Section 8.2
Loss at conclusion of test (%)	32.1

Validation Test Data: Control Aggregate (Drain Bros. stone)	
Test date	January 10, 2012
Percent loss	12.7%
Valid range	11.4 – 14.8%

Reported by: I. Chung

Reviewed by: _____
L. Hu, M. Sc. E.



Notice: The test data given herein pertain to the sample provided, and may not be applicable to material from other production zones/periods. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.

GOLDER ASSOCIATES LTD., 245, 12388 88th Avenue, Surrey, B.C. Canada V3W 7R7 Tel: 604-591-6616 Fax: 604-591-6608



Compaction of Soil Using Standard Effort				Reference ASTM D 698-07	
Client:	Tetra Tech	Sample No.:	Section 4 - Placer Tails		
Project:	Aggregate Testing	Sample Location:			
Location:	Eagle Gold, Yukon	Depth (m):			
Project No.:	11-1415-0013/1000	Lab ID No:	211		
Effort Type:	Standard	Visual Description:	-		
Test Method:	C	Type of Rammer:	Mechanical		
Preparation Method:	Moist	Mould Volume (m³):	0.002122		
Remarks:					
Compaction Results					
Trial No.:	1	2	3	4	5
Unit Weight (KN/m³)	19.63	19.68	19.76	19.60	19.3
Water Content (%)	5.4	7.1	8.5	9.8	10.4
Uncorrected Optimum w (%)	8.5		Uncorrected Unit Weight (KN/m³)	19.80	
ASTM D 4718-87 (2007) Correction for oversize particles					
Max Percent Oversize (%)	20		Sieve Size (mm)	20.00	
Oversized Fraction			Test Fraction		
Percent Oversized Fraction (%)	30.0		Percent Test Fraction (%)	70.0	
Gsc (calculated)	2.70		Gsf (calculated)	2.70	
Water Content of Oversized (%)	1.5		Water Content of Test (%)	8.5	
Final Results					
Corrected Optimum w (%)	6.4		Corrected Unit Weight (KN/m³)	21.42	
			Corrected Dry Density (Kg/m³)	2183	
SK, RS	December-15-11		LP	December 21, 2011	
TESTED BY	DATE		CHECKED BY	DATE	

SPECIFIC GRAVITY OF SOIL SOLIDS

Reference
 ASTM C 127-07
 ASTM D 854-06 Method B

Project No.:	11-1415-0013/1000	Borehole	Section 4 Placer Tails
Client:	Tetra Tech	Sample No.:	N/A
Project:	Aggregate Testing	Depth (m):	N/A
Location:	Eagle Gold, Yukon	Lab Sch No:	211

Specific Gravity of Fine Fraction (ASTM D 854-06)

Percentage Passing #4 sieve		62.99	
Test Number		1	2
Flask Number		4	3
Air Removal Method		Vacuum	Vacuum
Mass of Flask (g)		172.71	173.97
Mass of Flask + Dry Soil (g)	M_P	286.80	285.41
Mass of Dry Soil (g)		113.77	111.24
Mass of Flask + Soil + Water (g)	$M_{pws,t}$	742.87	742.45
Test Temperature (g)	T_t	19.00	18.90
Mass of Flask + Water (g)	$M_{pw,t}$	671.38	672.50
Mass of Dish + Dry Soil (g)		480.30	221.58
Mass of Dish (g)		366.53	110.34
Mass of Oven Dry Soil (g)	M_S	113.77	111.24
Temperature Coefficient	K	1.00	1.00
Density of Solids (g/cm ³)	ρ_s	2.69	2.69
Specific Gravity at Test Temperature	G_t	2.70	2.70
Specific Gravity at 20°C	$G_{20^\circ C}$	2.70	2.70
AVERAGE SPECIFIC GRAVITY		2.70	

Specific Gravity of Coarse Fraction (ASTM C 127-07)

Percentage Retained on #4 sieve		31.39
Mass of Sample in Water (g)	A	
Mass of Sample @ SSD (g)	B	
Mass of Oven Dried Sample (g)	C	
Bulk G (Oven Dry)	C/(B-A)	
Bulk G (SSD)	B/(B-A)	2.63
Apparent	C/(C-A)	2.70
Absorbtion (%)	(B-C)/C	1.65

Combined Specific Gravity

COMBINED SPECIFIC GRAVITY	$G_{avg @ 20^\circ C}$	2.70
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** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data can be provided upon request.*

SK/RS	November 15, 2011	LP	December 21, 2011
TESTED BY	DATE	CHECKED BY	DATE

Slake Durability of Shales and Similar Weak Rocks
Reference
 ASTM D4644-08

Project No.:	11-1415-0013/1000	Borehole:	Weathered Rock
Project:	Aggregate Testing	Sample Number:	N/A
Location:	Eagle Gold, Yukon	Depth (m):	N/A
Client:	Tetra Tech	Lab ID No:	211

Test Results		
I_{d1} = Slake Durability index after 1 st Cycle	97.6	%
I_{d2} = Slake Durability index after 2 nd Cycle	95.6	%
I_{d3} = Slake Durability index after 3 rd Cycle	94.1	%

Initial Sample Description	Final Description of Fragments in Drum
10 irregular lumps of weathered rock	I_{d1} Type II - Retained pieces consists of large and small pieces
	I_{d2} Type II - Retained pieces consists of large and small pieces
	I_{d3} Type II - Retained pieces consists of large and small pieces

Temperature ° C		Calibration	
Max	<u>22.2</u>	Machine ID	<u>BUR010002</u>
Min	<u>18.1</u>	Drum ID	<u>A</u>
Average	<u>20.1</u>	Thermometer ID	<u>FB61328</u>

Comments


BEFORE

AFTER

** The test data given herein pertain to the sample provided only. This report constitutes a testing service only. Interpretation of the data given here may be provided upon request.*

G. Patton	December 19, 2011	LP	December 23, 2011
TESTED BY	DATE	CHECKED BY	DATE

POINT LOAD STRENGTH INDEX TEST				Reference ASTM D5731 - 08	
Project No	11-1415-0013/1000			Sch. #	211
Client	Tetra Tech			Machine ID	BUR010024
Project	Aggregate Testing				
Location	Eagle Gold, Yukon				

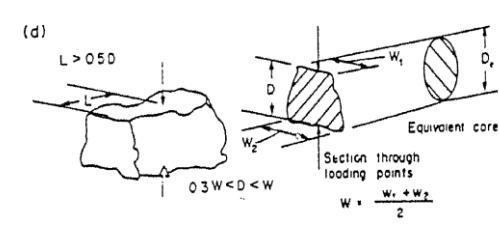
Lump Test														
Test #	Borehole	Sa#	Depth	D	W	Guage Reading	Load P	$D_e^2 = D^2$	$I_s = P/D_e^2$	$F = (D_e/50)^{0.45}$	$I_{s(50)}$	Type	Direction	Remarks
	No.		m	mm	mm	kN	MN	mm ²	MPa		MPa			
1	Section 5	1	Combined	30.00	52.40	2.053	0.00205	900.0	2.28	0.795	1.81	L	p	
2	Section 5	2	Combined	38.32	71.01	21.987	0.02199	1468.4	14.97	0.887	13.28	L	p	Sample was very strong - hard to cut into block shape
3	Section 5	3	Combined	31.19	51.98	4.253	0.00425	972.8	4.37	0.809	3.54	L	p	
4	Section 5	4	Combined	30.99	58.98	5.113	0.00511	960.4	5.32	0.806	4.29	L	p	
5	Section 5	5	Combined	39.69	59.61	5.130	0.00513	1575.3	3.26	0.901	2.94	L	p	
6	Section 5	6	Combined	32.81	48.90	2.289	0.00229	1076.5	2.13	0.827	1.76	L	p	
7	Section 5	7	Combined	32.62	69.34	1.428	0.00143	1064.1	1.34	0.825	1.11	L	p	
8	Section 5	8	Combined	48.39	61.74	1.48	0.00148	2341.4	0.63	0.985	0.62	L	∥	
9	Section 5	9	Combined	33.27	56.93	2.63	0.00263	1106.9	2.38	0.833	1.98	L	p	
10	Section 5	10	Combined	32.11	62.75	3.83	0.00383	1031.1	3.71	0.819	3.04	L	p	
11	Section 5	11	Combined	27.85	81.18	4.97	0.00497	775.6	6.41	0.768	4.92	L	∥	

$I_{s(50)}$ = Size Corrected Point Load Strength
 $I_{s(50)} = F I_s$
 I_s = Uncorrected Point Load Strength
 $I_s = P/D_e^2$

F = Size Correction Factor
 $F = (D_e/50)^{0.45}$
 D_e = equivalent core diameter (mm)
 $D_e^2 = D^2$ (mm²)

Lump Test

Direction
 ∥ = parallel
 p = perpendicular to planes of weakness



G. Patton	1/5/2012	LP	January 17, 2012
Tested By	Date	Checked By	Date

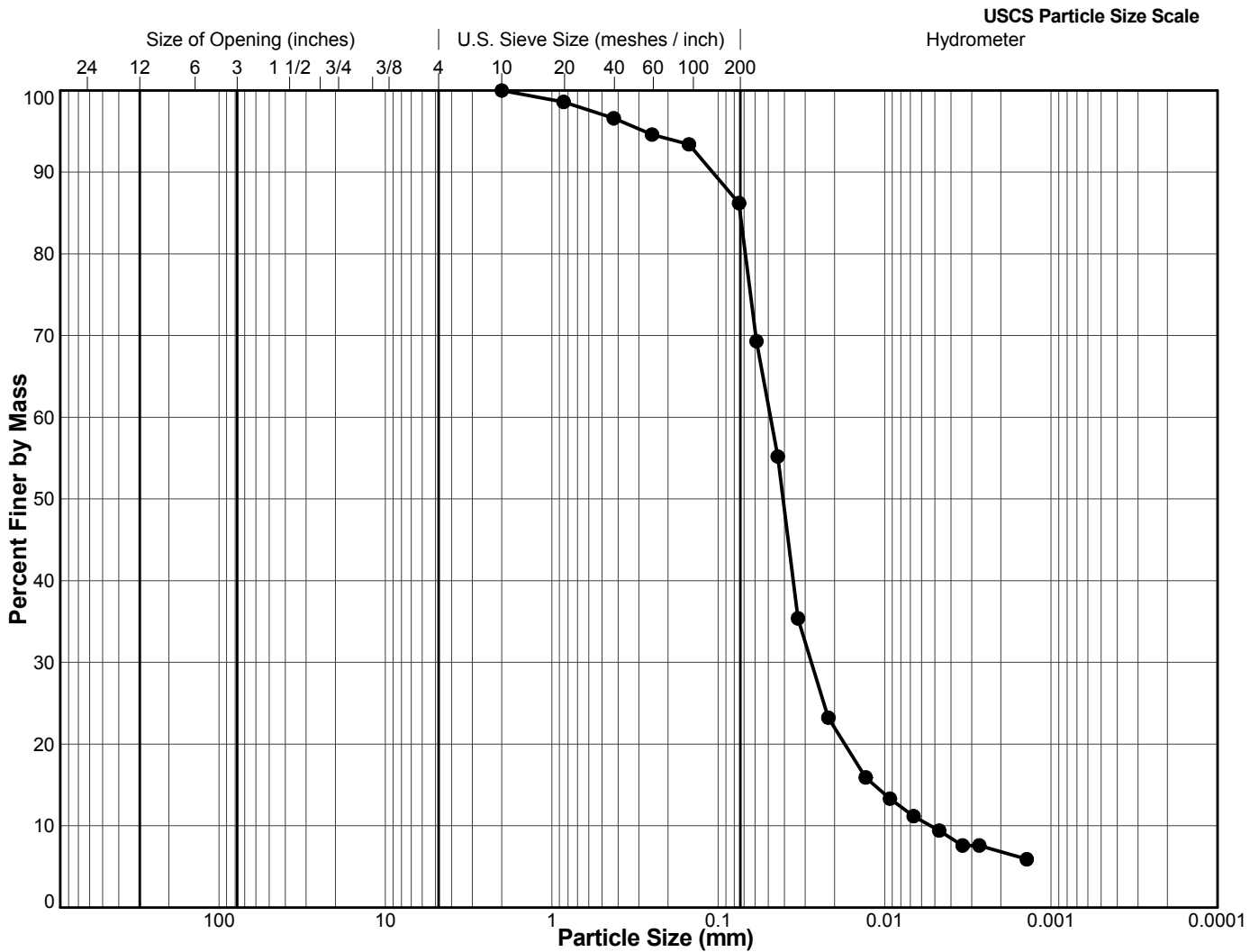
PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

Client: Tetra Tech	ID: Section 2 Silt
Project: Aggregate Testing	Sample No.: N/A
Location: Eagle Gold, Yukon	Depth Interval (m): N/A
Project No.: 11-1415-0013 Phase: 1000	Lab Schedule No.: 211

Other Remarks: N/A

Specific Gravity (assumed):	Shape:
Max. Particle Size Passing (mm): 2	Dispersion Method: Stirring
Method: Split, Washed	Dispersion Period (min): 1
Hydrometer ID: BURNABY - 87024	



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)				
		Coarse	Fine	Coarse	Medium	Fine					

SK	05/12/2011	LP	21/12/2011
Tech	Date	Checked	Date

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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

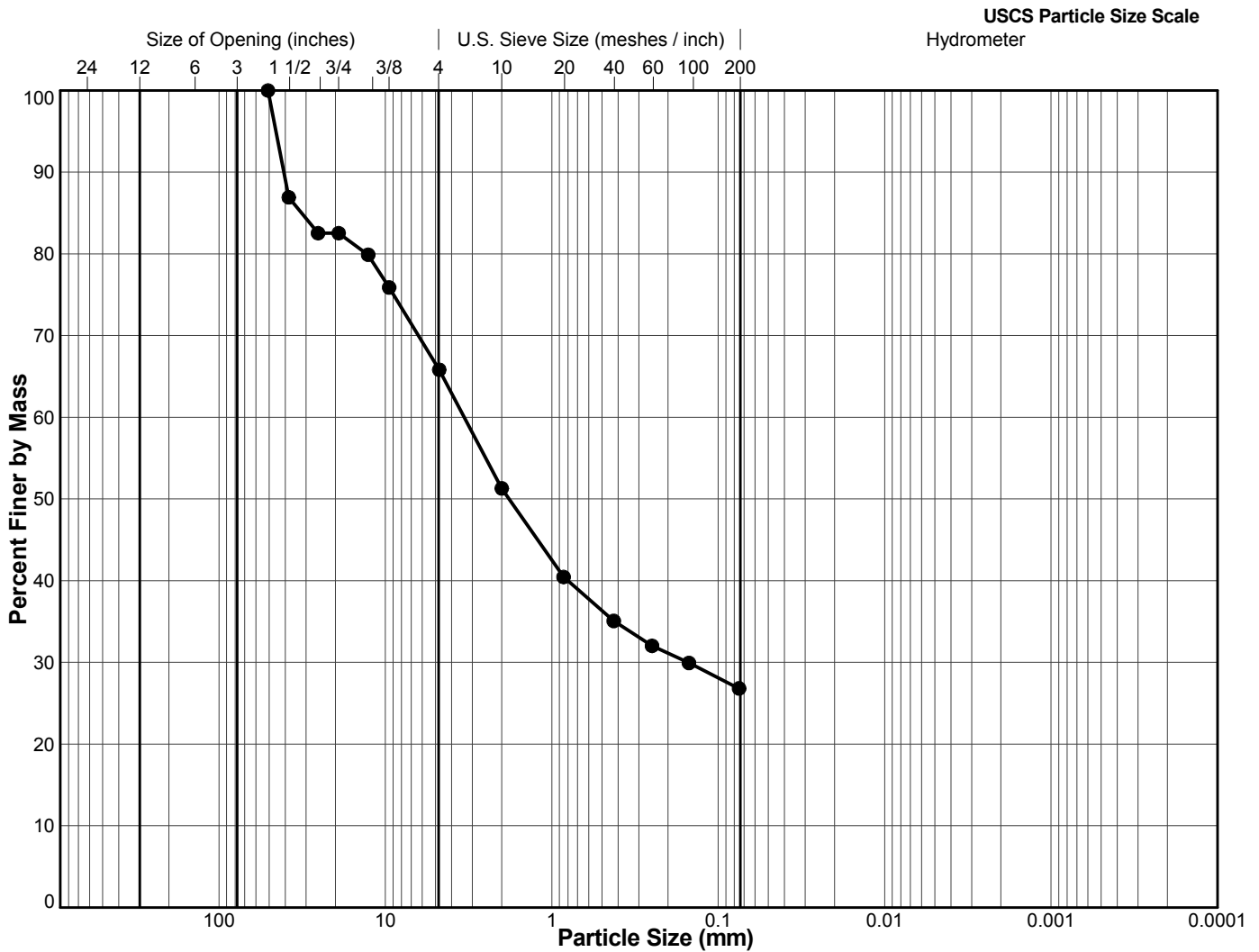
Client: Tetra Tech	ID: Section 3 Colluvium
Project: Aggregate Testing	Sample No.: N/A
Location: Eagle Gold, Yukon	Depth Interval (m): N/A
Project No.: 11-1415-0013 Phase: 1000	Lab Schedule No.: 211

Other Remarks: N/A

Specific Gravity (assumed): _____ **Shape:** _____

Max. Particle Size Passing (mm): 50.8

Method: Split, Washed



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

SK	08/12/2011	LP	21/12/2011
Tech	Date	Checked	Date

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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

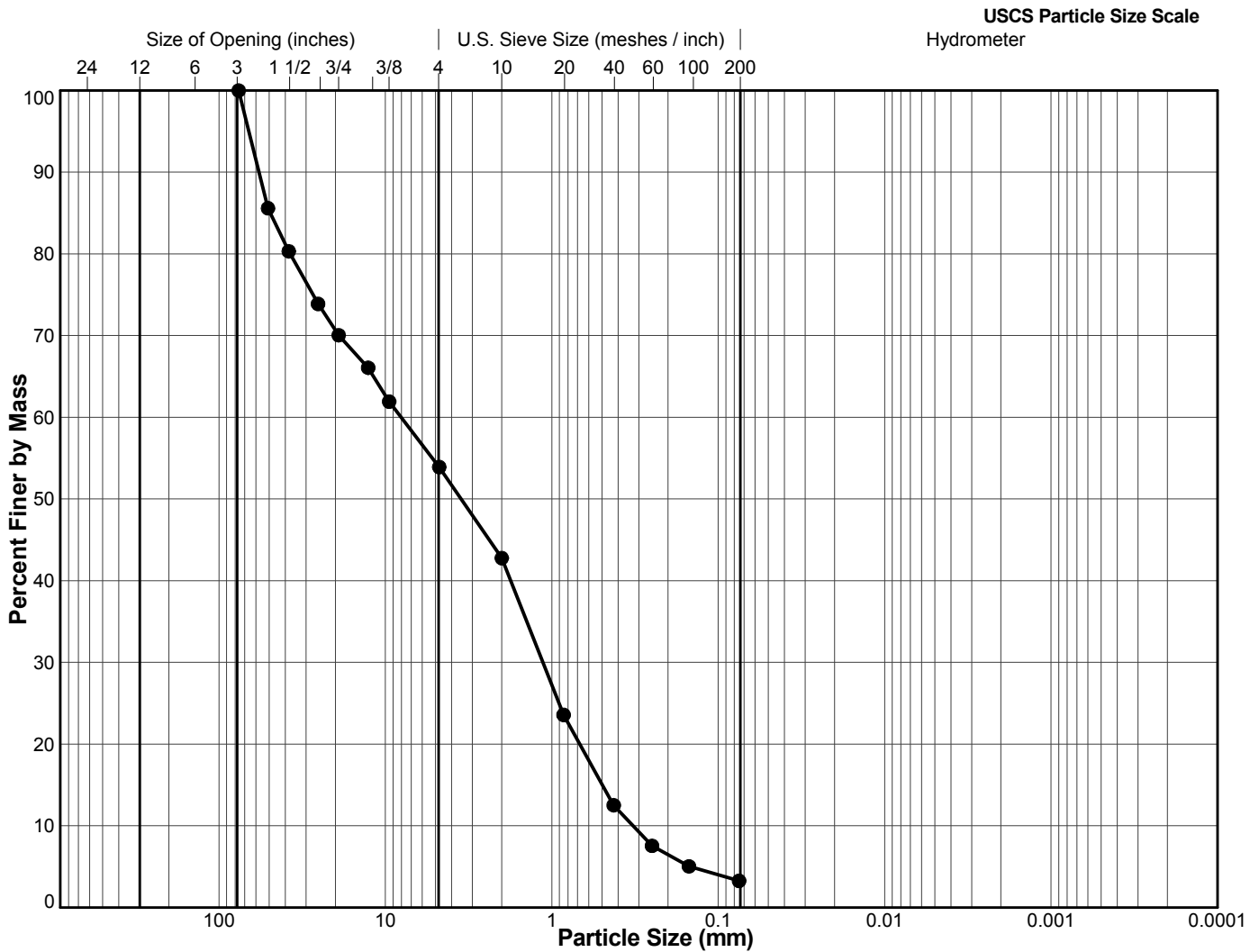
Client: Tetra Tech	ID: Section 4 Placer Tails
Project: Aggregate Testing	Sample No.: N/A
Location: Eagle Gold, Yukon	Depth Interval (m): N/A
Project No.: 11-1415-0013 Phase: 1000	Lab Schedule No.: 211

Other Remarks: N/A

Specific Gravity (assumed): _____ **Shape:** _____

Max. Particle Size Passing (mm): 76.2

Method: Split, Washed



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)
		Coarse	Fine	Coarse	Medium	Fine	

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Tech	Date	LP Checked	21/12/2011 Date
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PARTICLE SIZE ANALYSIS OF SOILS

Reference(s)
ASTM D 422-63 (2007)

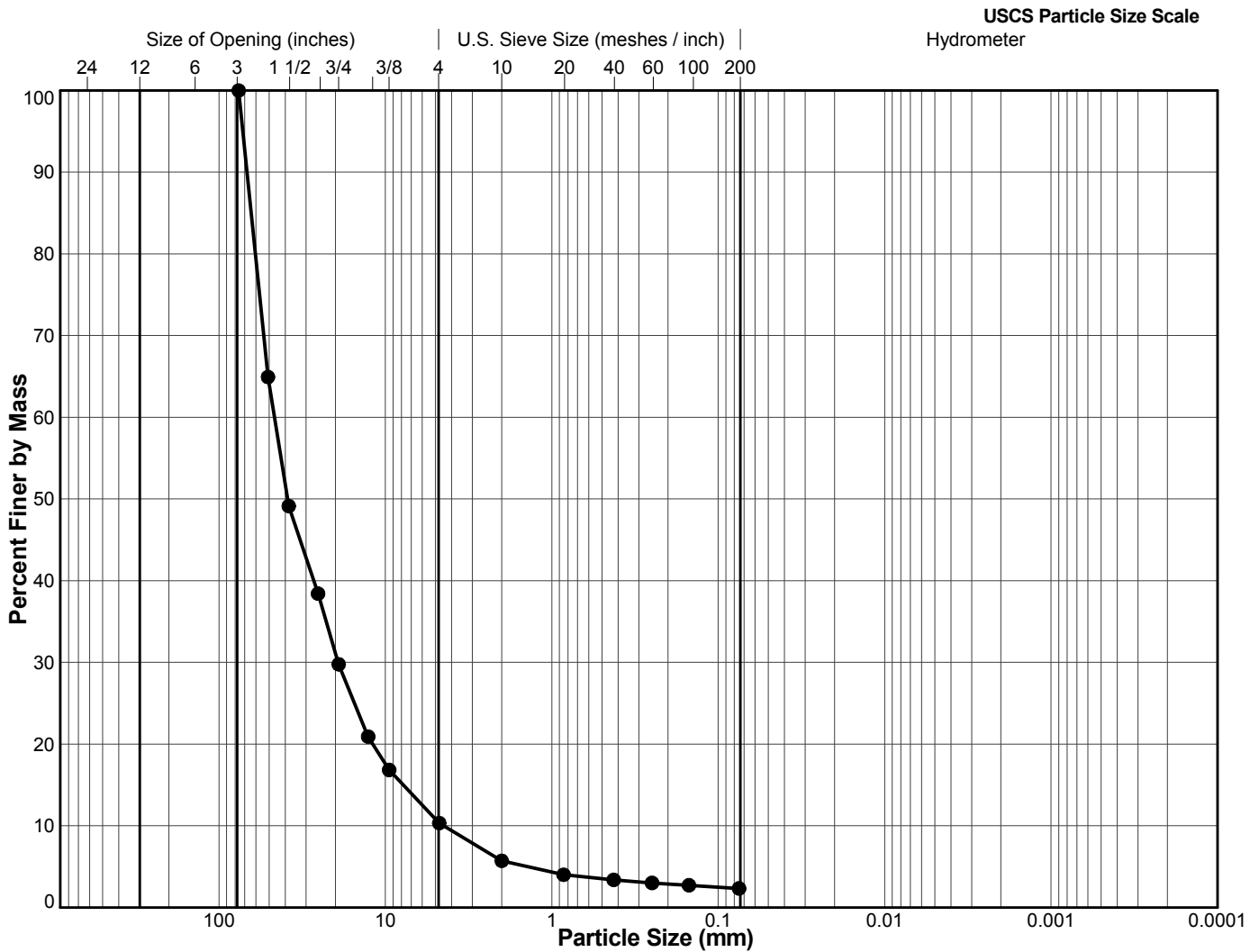
Client: Tetra Tech	ID: Section 5 Weathered Rock
Project: Aggregate Testing	Sample No.: N/A
Location: Eagle Gold, Yukon	Depth Interval (m): N/A
Project No.: 11-1415-0013 Phase: 1000	Lab Schedule No.: 211

Other Remarks: N/A

Specific Gravity (assumed): _____ **Shape:** _____

Max. Particle Size Passing (mm): 76.2

Method: Split, Washed



BOULDER	COBBLE	GRAVEL		SAND			FINES (Silt, Clay)		
		Coarse	Fine	Coarse	Medium	Fine			

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SK	15/12/2011	LP	21/12/2011
Tech	Date	Checked	Date

File: N:\BUR_G\RA\HCS\PROJECTS\2011\1415\0131\1-1415-0013 PHASE 1000\11-1415-0013 PHASE 1000\11-1415-0013 PHASE 1000.GPJ Output Form_LAB_ATTERRBERG CASAGRANDE (SINGLE) Template: BC REGION TEMPLATE BETA 1.GDT Library: BC REGION LIBRARY.GLB SKin 2/12/11

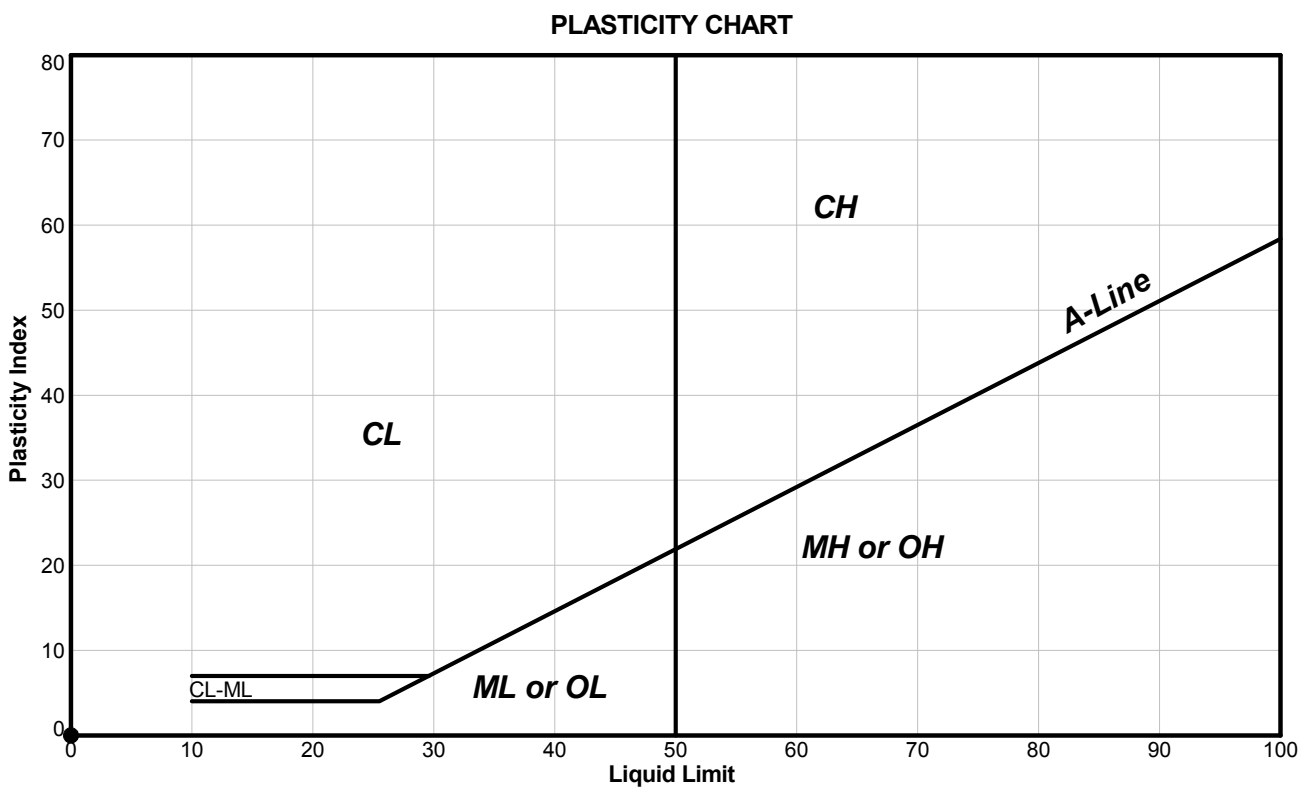
LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS	Reference(s) ASTM D 4318-10
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Client: Tetra Tech	ID: Section 4 Placer Tails
Project: Aggregate Testing	Sample No.: N/A
Location: Eagle Gold, Yukon	Depth Interval (m): N/A
Project No.: 11-1415-0013 Phase: 1000	Lab Schedule No.: 211

Classification and Definition: Non-Plastic Soil (NP).

Other Remarks: N/A

Test Method: A-Multi Point **Preparation Method:** Wet



Sym.	Sample Location	Sample Number	Depth (m)	Bottom (m)	Percent Passing #40 Sieve (%)	Liquid Limit	Plastic Limit	Plasticity Index	Natural Water Content (%)	Liquidity Index
●	Section 4 Placer Tails	N/A	0.00	0.00	12	NP	NP	NP	2.1	NP

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

SK	05/12/2011	LP	21/12/2011
Tech	Date	Checked	Date

APPENDIX M

LABORATORY TESTING RESULTS PLOTS AND TABLES

LABORATORY TESTING SUMMARIES

1.0 INTRODUCTION

Laboratory testing was completed on selected samples to characterize both soil and rock materials. Soil testing included index testing (moisture content, Atterberg limits, grain size and specific gravity) as well as proctor testing and permeability testing on potential borrow materials, and aggregate testing on potential aggregate sources. Rock testing included both, unconfined compressive strength (UCS) testing and Brazilian tensile strength (BTS) testing.

Rock testing was completed by Golder Associates of Burnaby, BC. Soil testing was completed by Golder Associates and GeoNorth Engineering Ltd, of Prince George, BC. Aggregate testing was completed by GeoNorth Engineering, Metro Testing Group of Burnaby, BC and Exova, of Edmonton, AB.

2.0 REQUESTED TESTING

The soil tests requested are tabulated in Table M-1, the rock tests requested are tabulated in Table M-2 and the aggregate tests requested are tabulated in Table M-3.

3.0 TESTING RESULTS

The grain size and hydrometer results are tabulated in Table M-4. The moisture content results are tabulated in Table M-5. The Atterberg limit results are tabulated in Table M-6. The specific gravity results are tabulated in Table M-7. The sulphate ion results are tabulated in Table M-8. The modified proctor and permeability results are tabulated in Table M-9. The UCS and BTS results are tabulated in Table M-10.

Plasticity charts for colluvium and other overburden soil samples are shown in figures M-1 and M-2 respectively.

Table M-1. Soil Samples Requested for Testing

Hole ID	Sample Number	Sample Depth From (m)	Sample Depth To (m)	Sample Type	Moisture Content	3" Minus Grain Size	Hydrometer	Atterberg Limits	Specific Gravity - soil	Modified Proctor	Permeability Testing	Sulphate in Soil
					ASTM 2216	ASTM C136	ASTM 422	ASTM 4318	ASTM 854	ASTM 698	ASTM D5856	CSA A23.2-3B
Test Pits												
TP-BGC11-50	1	2.8	3.0	GB	1	1	-	-	1	-	-	-
TP-BGC11-50	2	3.3	3.9	GB	1	1	-	-	-	-	-	-
TP-BGC11-51	1	0.2	1.44	GB	1	1	1	1	-	-	-	-
TP-BGC11-51	2	2.4	2.8	GB	1	1	-	-	-	-	-	1
TP-BGC11-51	3	4.2	4.5	GB	1	1	-	-	-	-	-	-
TP-BGC11-52	1	3.3	3.5	GB	1	1	-	-	-	-	-	-
TP-BGC11-53	1	2	2.2	GB	1	1	-	-	-	-	-	-
TP-BGC11-54	1	0.4	0.6	GB	1	1	1	1	-	-	-	-
TP-BGC11-55	1	0.4	0.6	GB	1	1	1	1	-	-	-	-
TP-BGC11-55	2	1.9	2.0	GB	1	1	-	-	-	-	-	-
TP-BGC11-57	1	0.5	1.1	GB	1	1	1	1	-	-	-	-
TP-BGC11-58	1	0.7	0.7	GB	1	1	1	1	-	-	-	-
TP-BGC11-58	2	2	2.0	GB	1	1	-	-	1	-	-	-
TP-BGC11-59	1	0.4	0.6	GB	1	1	1	1	-	-	-	-
TP-BGC11-60	1	0.7	0.9	GB	1	1	1	1	-	-	-	1
TP-BGC11-61	1	0.7	0.8	GB	1	1	1	1	-	-	-	-
TP-BGC11-61	2	1.7	1.9	GB	1	1	-	-	-	-	-	-
TP-BGC11-62	1	0.9	1.0	GB	1	1	1	1	-	-	-	-
TP-BGC11-62	2	3.5	3.7	GB	1	1	1	1	-	-	-	-
TP-BGC11-63	1	0.5	0.7	GB	1	1	1	-	-	-	-	-
TP-BGC11-64	1	1.5	1.6	GB	1	1	-	1	-	-	-	-
TP-BGC11-65	1	0.4	0.5	GB	1	1	-	-	-	-	-	-
TP-BGC11-66	1	1.3	1.3	GB	1	1	-	-	-	-	-	-
TP-BGC11-66	2	1.8	1.8	GB	1	1	1	1	-	-	-	-
TP-BGC11-67	1	0.7	0.9	GB	1	1	1	-	1	-	-	-
TP-BGC11-68	1	1.6	1.8	GB	1	1	-	-	-	-	-	-
TP-BGC11-69	1	1.7	1.9	GB	1	1	1	1	-	-	-	-
TP-BGC11-69	2	2.8	2.9	GB	1	1	1	1	-	-	-	-
TP-BGC11-71	1	1.3	1.6	GB	1	1	1	1	-	-	-	-
TP-BGC11-72	1	2.4	2.6	GB	1	1	-	-	-	-	-	-
TP-BGC11-72	2	0.8	1.0	GB	1	1	1	1	-	-	-	-
TP-BGC11-73	1	0.8	1.0	GB	1	1	1	1	-	-	-	-
TP-BGC11-74	1	1.2	1.4	GB	1	1	1	1	-	-	-	-
TP-BGC11-74	2	3	3.0	GB	1	1	1	1	-	-	-	-
TP-BGC11-74	3	4.5	4.5	GB	1	1	1	1	-	-	-	-
TP-BGC11-76	1	0.4	0.6	GB	1	1	1	-	-	-	-	-
TP-BGC11-77	1	0.4	0.5	GB	1	1	1	1	-	-	-	-
TP-BGC11-79	1	0.4	0.6	GB	1	1	1	1	-	-	-	-
TP-BGC11-79	2	1.2	1.3	GB	1	1	-	-	-	-	-	-
TP-BGC11-81	1	0.7	1.0	GB	1	1	1	-	-	-	-	-
TP-BGC11-82	1	4.7	4.9	GB	1	1	1	1	-	-	-	-
TP-BGC11-82	2	3	3.3	GB	1	1	-	1	-	-	-	-
TP-BGC11-82	3	6.7	7.0	GB	1	1	1	1	-	-	-	-
TP-BGC11-82	4	0.7	1.1	GB	1	1	-	1	-	-	-	-
TP-BGC11-82	5	1.9	2.1	GB	1	1	-	1	-	-	-	-
TP-BGC11-83	1	0.6	0.8	GB	1	1	1	1	-	-	-	-
TP-BGC11-84	1	0.4	0.6	GB	1	1	1	-	-	-	-	1
TP-BGC11-84	2	2.5	2.5	GB	1	1	1	1	-	-	-	-
TP-BGC11-85	1	1	1.0	GB	1	1	1	1	-	-	-	-
TP-BGC11-87	1	0.6	0.8	GB	1	1	-	-	-	-	-	-
TP-BGC11-88	1	1.5	1.8	GB	1	1	1	1	-	-	-	-
TP-BGC11-89	1	0.4	0.6	GB	1	1	-	-	-	-	-	-
TP-BGC11-89	2	1.3	1.5	GB	1	1	1	1	-	-	-	-
TP-BGC11-90	1	0.8	1.0	GB	1	1	-	-	-	-	-	-
TP-BGC11-90	2	3.5	3.6	GB	1	1	1	1	-	-	-	-
TP-BGC11-91	1	0.8	1.0	GB	1	1	1	1	-	-	-	-
TP-BGC11-91	2	1.6	1.8	GB	1	1	1	1	-	-	-	-
TP-BGC11-94	1	1.2	1.3	GB	1	1	1	1	-	-	-	-

Hole ID	Sample Number	Sample Depth From (m)	Sample Depth To (m)	Sample Type	Moisture Content	3" Minus Grain Size	Hydrometer	Atterberg Limits	Specific Gravity - soil	Modified Proctor	Permeability Testing	Sulphate in Soil
					ASTM 2216	ASTM C136	ASTM 422	ASTM 4318	ASTM 854	ASTM 698	ASTM D5856	CSA A23.2-3B
TP-BGC11-95	1	0.9	1.1	GB	1	1	-	-	-	-	-	-
TP-BGC11-95	2	2	3.0	GB	1	1	-	-	-	-	-	-
TP-BGC11-96	1	0.7	0.9	GB	1	1	-	-	-	-	-	-
TP-BGC11-96	2	1.3	1.5	GB	1	1	1	1	-	-	-	-
TP-BGC11-96	3	1.8	2.0	GB	1	1	-	-	-	-	-	-
TP-BGC11-103	1	4.5	4.5	GB	1	1	-	-	-	-	-	-
TP-BGC11-104	1	1.2	1.6	GB	1	1	-	-	-	-	-	-
TP-BGC11-106	1	0.8	1.5	GB	1	-	-	-	-	-	-	-
TP-BGC11-107	1	2.8	3.0	GB	1	1	1	-	-	-	-	-
TP-BGC11-108	1	0.9	1.2	GB	1	1	-	-	-	-	-	-
TP-BGC11-111	1	0.9	1.1	GB	1	-	-	-	-	-	-	-
TP-BGC11-112	1	1.3	1.4	GB	1	1	-	-	-	-	-	-
TP-BGC11-116	2	2.2	2.3	GB	1	1	-	1	-	-	-	-
TP-BGC11-119	1	0.9	1.1	GB	1	-	-	1	-	1	1	-
TP-BGC11-120	1	3.8	4.0	GB	1	-	-	1	-	1	1	-
TP-BGC11-121	1	2.9	3.1	GB	1	-	-	1	-	1	1	-
TP-BGC11-122	1	5.4	5.8	GB	1	-	-	1	-	1	1	-
TP-BGC11-124	1	6.2	6.4	GB	1	1	-	1	-	-	-	-
TP-BGC11-126	1	1.7	2.2	GB	1	1	-	-	-	-	-	-
TP-BGC11-128	1	2.5	3.0	GB	1	1	1	-	-	-	-	-
TP-BGC11-129	1	6.6	6.8	GB	1	1	-	1	-	-	-	-
TP-BGC11-130	1	5.8	6.0	GB	1	1	-	1	-	-	-	-
TP-BGC11-131	1	3.2	3.5	GB	1	1	-	1	-	-	-	-
TP-BGC11-133	1	4.3	4.4	GB	1	1	-	-	-	-	-	-
TP-BGC11-140	1	1	1.2	GB	1	1	-	1	-	-	-	-
Boreholes												
BH-BGC11-39	SPT1	1.15	1.6	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT2	1.6	2.1	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT3	2.25	2.75	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT4	2.75	3.2	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT5	4.6	5.05	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT6	5.53	5.98	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT7	6.22	6.67	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT8	7.16	7.61	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT9	7.75	8.20	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT10	8.71	9.16	SPT	1	1	-	-	-	-	-	-
BH-BGC11-39	SPT11	9.25	9.70	SPT	1	1	-	-	-	-	-	-
BH-BGC11-40A	1	18.78	18.83	GB	1	-	-	-	-	-	-	-
BH-BGC11-42	2	1.8	2.27	GB	1	1	1	1	-	-	-	-
BH-BGC11-42	4	5	5.32	GB	1	-	-	-	-	-	-	-
BH-BGC11-42	5	6.4	6.84	GB	1	1	1	1	1	-	-	-
BH-BGC11-42	7	9.4	9.88	GB	1	1	-	-	-	-	-	-
BH-BGC11-42	8	10.6	11.0	GB	1	-	-	-	-	-	-	-
BH-BGC11-42	12	13.3	13.65	GB	1	1	-	-	-	-	-	-
BH-BGC11-42	15	15.25	15.4	GB	1	-	-	-	-	-	-	-
BH-BGC11-42	16	15.6	15.80	GB	1	-	-	-	-	-	-	-
BH-BGC11-42	20	19.7	20.0	GB	1	1	1	-	-	-	-	-
BH-BGC11-42	22	21.55	21.9	GB	1	1	1	-	-	-	-	-
BH-BGC11-42	23	23	23.4	GB	1	-	-	-	-	-	-	-
BH-BGC11-42	S26	27.6	28.0	SPT	1	1	-	-	-	-	-	-
BH-BGC11-44	S1	0.2	0.4	GB	1	1	1	1	-	-	-	-
BH-BGC11-44	S3	1.6	2.0	GB	1	1	1	1	-	-	-	-
BH-BGC11-44	S5	4.8	5.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-44	S7	6.3	6.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-44	S11	9.6	9.9	GB	1	1	1	1	-	-	-	-
BH-BGC11-44	SPT1	2.27	2.72	SPT	1	-	-	-	-	-	-	-
BH-BGC11-44	SPT2	3.79	4.24	SPT	1	-	-	1	-	-	-	-
BH-BGC11-44	SPT5	8.38	8.85	SPT	1	-	-	-	-	-	-	-
BH-BGC11-44	SPT11	14.49	14.79	SPT	1	-	-	1	-	-	-	-
BH-BGC11-47	G3	2	2.3	GB	1	1	1	1	-	-	-	-

Hole ID	Sample Number	Sample Depth From (m)	Sample Depth To (m)	Sample Type	Moisture Content	3" Minus Grain Size	Hydrometer	Atterberg Limits	Specific Gravity - soil	Modified Proctor	Permeability Testing	Sulphate in Soil
					ASTM 2216	ASTM C136	ASTM 422	ASTM 4318	ASTM 854	ASTM 698	ASTM D5856	CSA A23.2-3B
BH-BGC11-47	G10	11.8	12.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-47	SPT1	0.75	1.2	SPT	1	1	1	1	-	-	-	-
BH-BGC11-47	SPT4	5.33	5.9	SPT	1	1	-	-	-	-	-	-
BH-BGC11-47	SPT5	6.85	7.3	SPT	1	-	-	-	-	-	-	-
BH-BGC11-47	SPT6	8.38	8.6	SPT	1	1	1	-	-	-	-	-
BH-BGC11-49	G4	3.4	3.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-49	G6	6.4	6.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-49	G8	9.4	9.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-49	G10	12.4	12.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-49	SPT1	0.76	1.2	GB	1	-	-	-	-	-	-	-
BH-BGC11-49	SPT2	2.28	2.7	GB	1	1	1	1	-	-	-	-
BH-BGC11-51	G6	7.9	8.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-51	G8	10.9	11.1	GB	1	-	-	-	-	-	-	-
BH-BGC11-51	G10	13.9	14.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-51	G12	17	17.2	GB	1	-	-	-	-	-	-	-
BH-BGC11-51	G14	20.1	20.3	GB	1	1	-	-	-	-	-	-
BH-BGC11-51	G17	24.7	24.9	GB	1	-	-	-	-	-	-	-
BH-BGC11-51	SPT1	0.76	1.2	SPT	1	-	-	1	-	-	-	-
BH-BGC11-51	SPT2	2.28	2.7	SPT	1	-	-	-	-	-	-	-
BH-BGC11-51	SPT3	3.8	4.3	SPT	1	-	-	1	-	-	-	-
BH-BGC11-51	SPT5	9.9	10.3	SPT	1	-	-	-	-	-	-	-
BH-BGC11-51	SPT4	5.33	5.8	SPT	1	-	-	-	-	-	-	-
BH-BGC11-53	G2	1.9	2.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-53	G3	3.2	3.5	GB	1	-	-	-	-	-	-	-
BH-BGC11-53	G4	4.9	5.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-53	G6	7.9	8.1	GB	1	1	-	-	-	-	-	-
BH-BGC11-53	G8	10.9	11.1	GB	1	-	-	-	-	-	-	-
BH-BGC11-53	SPT1	0.76	1.2	SPT	1	-	-	-	-	-	-	-
BH-BGC11-53	SPT2	2.28	2.7	SPT	1	1	1	1	-	-	-	-
BH-BGC11-53	SPT3	3.8	4.3	SPT	1	-	-	-	-	-	-	-
BH-BGC11-53	SPT4	6.85	7.3	SPT	1	1	1	1	-	-	-	-
BH-BGC11-53	SPT5	9.9	10.4	SPT	1	-	-	-	-	-	-	-
BH-BGC11-54	G1	29.97	30.0	Gouge	1	-	-	1	-	-	-	-
BH-BGC11-55	G4	4.9	5.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-55	G5	6.3	6.5	GB	1	-	-	-	-	-	-	-
BH-BGC11-55	SPT1	0.76	1.2	SPT	1	1	-	-	-	-	-	-
BH-BGC11-55	SPT2	2.28	2.7	SPT	1	-	-	-	-	-	-	-
BH-BGC11-55	SPT6	8.38	8.8	SPT	1	-	-	1	-	-	-	-
BH-BGC11-56	G2	1.9	2.1	GB	1	1	-	-	-	-	-	-
BH-BGC11-56	SPT1	0.76	1.2	SPT	1	-	-	-	-	-	-	-
BH-BGC11-56	SPT3	3.8	4.3	SPT	1	-	-	1	-	-	-	-
BH-BGC11-56	SPT4	5.33	5.5	SPT	1	-	-	1	-	-	-	-
BH-BGC11-56	SPT5	6.85	7.2	SPT	1	-	-	-	-	-	-	-
BH-BGC11-57	SPT1	2.28	2.7	SPT	1	-	-	-	-	-	-	-
BH-BGC11-57	SPT2	3.8	4.3	SPT	1	1	1	1	-	-	-	-
BH-BGC11-57	SPT3	5.33	5.8	SPT	1	-	-	-	-	-	-	-
BH-BGC11-57	SPT4	6.85	7.1	SPT	1	1	1	1	-	-	-	-
BH-BGC11-58	G2	1.9	2.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-58	G4	3.2	3.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-58	G6	4.4	4.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-58	G8	5.4	5.5	GB	1	1	1	1	-	-	-	-
BH-BGC11-58	G10	7.9	8.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-60	G3	2.7	2.8	GB	1	1	1	1	-	-	-	-
BH-BGC11-60	G5	3.7	3.8	GB	1	1	1	1	-	-	-	-
BH-BGC11-60	G6	4.9	5.1	GB	1	-	-	-	-	-	-	-
BH-BGC11-60	G7	6.3	6.6	GB	1	1	1	1	-	-	-	-
BH-BGC11-60	SPT2	6.85	7.0	SPT	1	1	1	-	-	-	-	-
BH-BGC11-63	G-2	1.9	2.1	GB	1	1	1	1	-	-	-	-
BH-BGC11-63	G-5	3.9	4.0	GB	1	1	1	1	-	-	-	-
BH-BGC11-63	G-6	5.9	6.1	GB	1	-	-	-	-	-	-	-

Hole ID	Sample Number	Sample Depth From (m)	Sample Depth To (m)	Sample Type	Moisture Content	3" Minus Grain Size	Hydrometer	Atterberg Limits	Specific Gravity - soil	Modified Proctor	Permeability Testing	Sulphate in Soil
					ASTM 2216	ASTM C136	ASTM 422	ASTM 4318	ASTM 854	ASTM 698	ASTM D5856	CSA A23.2-3B
BH-BGC11-63	G-10	11.6	11.7	GB	1	-	-	-	-	-	-	-
BH-BGC11-63	G-12	16.2	16.3	GB	1	1	1	1	-	-	-	-
BH-BGC11-65	SPT1	0.76	1.2	SPT	1	1	-	-	-	-	-	-
BH-BGC11-65	SPT2	1.52	2.0	SPT	1	1	-	-	-	-	-	-
BH-BGC11-65	SPT3	2.28	2.7	SPT	1	1	-	-	-	-	-	-
BH-BGC11-65	SPT4	3.04	3.5	SPT	1	1	-	-	-	-	-	-
BH-BGC11-65	SPT5	3.8	4.2	SPT	1	1	-	-	-	-	-	-
BH-BGC11-65	SPT6	4.56	5.0	SPT	1	1	-	-	-	-	-	-
BH-BGC11-65	SPT7	5.33	5.8	SPT	1	1	-	-	-	-	-	-
BH-BGC11-65	SPT8	6.09	6.5	SPT	1	1	-	-	-	-	-	-
Steiner Outcrop												
BGC-Steiner	-	-	-	-	-	1	-	-	-	-	-	-

Table M-2. Rock Samples Requested for Testing

Final Test Hole Number	Sample Number	Sample Depth From (m)	Sample Depth to (m)	UCS with Strain Measurement	Brazilian Tensile Strength	Specific Gravity - Rock
				ASTM D7012	ASTM D3967	C127
BH-BGC11-29	1	27.46	27.77	-	2	1
BH-BGC11-34	1	36.6	36.86	1	2	1
BH-BGC11-40A	2	30.98	31.3	1	2	1
BH-BGC11-40B	UCS1	21.81	22.06	1	2	1
BH-BGC11-43	1	4.24	4.48	1	2	1
BH-BGC11-45	UCS	17.8	18.04	1	2	1
BH-BGC11-46	UCS	13.11	13.65	1	2	1
BH-BGC11-62	UCS	28.59	28.84	1	1	1
BH-BGC11-69	UCS1	12.26	12.54	1	2	1

Table M-3. Aggregate Samples Requested for Testing

Final Test Hole Number	Sample Number	SOIL TYPE	Relative Density and Absorption, fine aggregate	Sulphate in Soil	Petrographic No. Coarse	Clay Lumps	Low Density Material	Material Finer than 80 µm, fine aggregate	Flat and Elongated Particles	Micro Deval		Freeze Thaw	LA Abrasion	MgSO ₄ Soundness		Detection of Alkali-Silica Reactive Aggregate by Accelerated Expansion of Mortar Bars
										Coarse Aggregate	Fine Aggregate		Small Coarse Aggregate	Coarse Aggregate	Fine Aggregate	
			CSA A23.2-6A	CSA A23.2-3B	CSA A23.2-15A	CSA A23.2-3A	CSA A23.2-4A	CSA A23.2-2A &5A	CSA A23.2-13A	CSA A23.2-29A	CSA A23.2-23A	CSA A23.2-24A	CSA A23.2-16A	CSA A23.2-9A	CSA A23.2-9A	CSA A23.2-25A
CONC-BGC11-GRD	S1	Form one aggregate sample from these 10 buckets. Sample need to be combined, then crushed and/or screened to produce fine and coarse aggregate for testing	1	1	1	1	1	1	1	1	1	1	1	1	1	1
CONC-BGC11-GRD	S2															
CONC-BGC11-GRD	S3															
CONC-BGC11-GRD	S4															
CONC-BGC11-GRD	S5															
CONC-BGC11-SSED	S1															
CONC-BGC11-SSED	S2															
CONC-BGC11-SSED	S3															
CONC-BGC11-SSED	S4															
CONC-BGC11-SSED	S5															
CONC-BGC11-01	S1	Form one aggregate sample from these 10 buckets. Sample need to be combined, then crushed and/or screened to produce fine and coarse aggregate for testing	1	1	1	1	1	1	1	1	1	1	1	1	1	1
CONC-BGC11-01	S2															
CONC-BGC11-02	S1															
CONC-BGC11-02	S2															
CONC-BGC11-03	S1															
CONC-BGC11-03	S2															
CONC-BGC11-04	S1															
CONC-BGC11-04	S2															
CONC-BGC11-16	S1															
CONC-BGC11-16	S2															
BGC Steiner			1	0	0	0	0	0	0	1	0	0	0	0	0	0

Table M-4. Grain Size and Hydrometer Results

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Cobbles (%) ¹	Gravel (%) ²	Sand (%) ³	Silt (%) ⁴	Clay (%) ⁵	Fines (%)	USCS Classification	Lithology Unit
Test Pits											
TP-BGC11-50	1	2.8	3.0	-	84	11	-	-	5	GW	COLLUVIUM
TP-BGC11-50	2	3.3	3.6	-	67	23	-	-	10	GW-GM/SW-SM	COLLUVIUM
TP-BGC11-51	1	0.2	1.4	-	32	24	34	10	44	GM-SM	COLLUVIUM
TP-BGC11-51	2	2.4	2.8	-	50	23	-	-	27	GM/SM	COLLUVIUM
TP-BGC11-51	3	4.2	4.5	-	60	34	-	-	6	GW-GM	COLLUVIUM
TP-BGC11-52	1	3.3	3.5	30	63	6	-	-	1	GW	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-53	1	2.0	2.2	-	79	18	-	-	3	GW	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-54	1	0.4	0.6	-	57	18	19	6	25	GM	COLLUVIUM
TP-BGC11-55	1	0.4	0.6	-	57	19	20	4	24	GM	COLLUVIUM
TP-BGC11-55	2	1.9	2.0	-	80	13	-	-	7	GW-GM	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-57	1	0.5	1.1	-	50	18	27	5	32	GM	COLLUVIUM
TP-BGC11-58	1	0.7	0.8	-	36	37	23	4	27	SM-GM	COLLUVIUM
TP-BGC11-58	2	2.0	2.1	-	69	25	-	-	6	GW-GM	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-59	1	0.4	0.6	-	67	20	12	1	13	GM	COLLUVIUM
TP-BGC11-60	1	0.7	0.9	-	20	25	53	2	55	ML	COLLUVIUM
TP-BGC11-61	1	0.7	0.8	-	30	32	36	2	38	SM-GM	COLLUVIUM
TP-BGC11-61	2	1.7	1.9	-	55	29	-	-	16	GM	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-62	1	0.9	1.0	-	33	23	40	4	44	GM-SM	COLLUVIUM
TP-BGC11-62	2	3.5	3.7	-	33	39	22	6	28	SM-GM/SC-GC	COLLUVIUM
TP-BGC11-63	1	0.5	0.7	-	53	23	20	4	24	GM	COLLUVIUM
TP-BGC11-64	1	1.5	1.6	-	26	60	-	-	14	SM	COLLUVIUM
TP-BGC11-65	1	0.4	0.5	-	49	36	-	-	15	GM-SM	COLLUVIUM
TP-BGC11-66	1	1.3	1.4	-	12	63	-	-	25	SM-GM	COLLUVIUM
TP-BGC11-66	2	1.8	1.9	-	2	32	51	15	66	CL	COLLUVIUM
TP-BGC11-67	1	0.7	0.9	-	45	19	28	8	36	GM-SM	COLLUVIUM
TP-BGC11-68	1	1.6	1.8	-	44	38	-	-	18	GM?	COLLUVIUM
TP-BGC11-69	1	1.7	1.9	-	-	7	90	3	93	ML	COLLUVIUM
TP-BGC11-69	2	2.8	2.9	-	47	31	19	3	22	GM-SM	COLLUVIUM
TP-BGC11-71	1	1.3	1.6	-	28	41	26	5	31	SM	COLLUVIUM
TP-BGC11-72	1	2.4	2.6	-	68	25	-	-	7	GW-GM	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-72	2	0.8	1.0	-	35	46	12	7	19	SM-GM	COLLUVIUM
TP-BGC11-73	1	0.8	1.0	-	17	50	31	2	33	SM	COLLUVIUM
TP-BGC11-74	1	1.2	1.4	-	39	26	28	7	35	GM-SM	COLLUVIUM
TP-BGC11-74	2	3.0	3.1	-	-	3	83	14	97	ML	COLLUVIUM
TP-BGC11-74	3	4.5	4.6	-	20	30	44	6	50	SM	COLLUVIUM
TP-BGC11-76	1	0.4	0.6	-	18	20	54	8	62	ML	COLLUVIUM
TP-BGC11-77	1	0.4	0.5	-	22	34	31	13	44	SM	COLLUVIUM
TP-BGC11-79	1	0.4	0.6	-	41	48	8	3	11	SW-SM/GW-GM	COLLUVIUM?
TP-BGC11-79	2	1.2	1.3	-	1	72	-	-	27	SM	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-81	1	0.7	1.0	-	65	24	10	1	11	GW-GM	COLLUVIUM
TP-BGC11-82	1	4.7	4.9	-	-	5	90	5	95	ML	ALLUVIUM
TP-BGC11-82	2	3.0	3.3	-	7	21	-	-	72	ML-SM	ALLUVIUM
TP-BGC11-82	3	6.7	7.0	-	2	20	74	4	78	ML	ALLUVIUM
TP-BGC11-82	4	0.7	1.1	-	-	20	-	-	80	ML	ALLUVIUM?
TP-BGC11-82	5	1.9	2.1	-	-	25	-	-	75	ML	ALLUVIUM
TP-BGC11-83	1	0.6	0.8	-	33	29	31	7	38	GM-SM	COLLUVIUM
TP-BGC11-84	1	0.4	0.6	-	-	20	34	46	80	ML	COLLUVIUM
TP-BGC11-84	2	2.3	2.5	-	2	4	84	10	94	ML	COLLUVIUM
TP-BGC11-85	1	0.7	1.1	-	7	42	40	11	51	ML	COLLUVIUM
TP-BGC11-87	1	0.6	0.8	-	26	68	-	-	6	SP-SW	COLLUVIUM
TP-BGC11-88	1	1.5	1.8	-	27	39	23	11	34	SM	COLLUVIUM
TP-BGC11-89	1	0.4	0.6	-	78	21	-	-	1	GW	COLLUVIUM
TP-BGC11-89	2	1.3	1.5	-	24	32	36	8	44	SM	COLLUVIUM
TP-BGC11-90	1	0.8	1.0	-	11	23	-	-	66	ML	COLLUVIUM
TP-BGC11-90	2	3.5	3.6	-	-	4	90	6	96	ML	COLLUVIUM
TP-BGC11-91	1	0.8	1.0	-	7	23	60	10	71	ML	COLLUVIUM
TP-BGC11-91	2	1.6	1.8	-	52	33	12	3	15	GM	COLLUVIUM
TP-BGC11-92	1	0.6	0.8	-	-	7	81	12	93	ML	COLLUVIUM

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Cobbles (%) ¹	Gravel (%) ²	Sand (%) ³	Silt (%) ⁴	Clay (%) ⁵	Fines (%)	USCS Classification	Lithology Unit
TP-BGC11-94	1	1.2	1.3	-	22	34	34	10	44	SM	COLLUVIUM
TP-BGC11-94	2	2.3	2.5	-	21	38	-	-	41	SM	COLLUVIUM
TP-BGC11-95	1	0.9	1.1	-	10	65	-	-	25	SM	WEATHERED INTRUSIVE ROCK
TP-BGC11-95	2	2.0	3.0	-	1	75	-	-	24	SM	WEATHERED INTRUSIVE ROCK
TP-BGC11-96	1	0.7	0.9	-	26	60	-	-	14	SM	WEATHERED INTRUSIVE ROCK
TP-BGC11-96	2	1.3	1.5	-	3	32	54	11	65	ML	WEATHERED INTRUSIVE ROCK
TP-BGC11-96	3	1.8	2.0	-	36	49	-	-	15	SM/GM	WEATHERED INTRUSIVE ROCK
TP-BGC11-103	1	4.4	4.8	-	36	48	-	-	5	SW/GW	TILL
TP-BGC11-104	1	1.2	1.6	-	60	33	-	-	7	GW	TILL
TP-BGC11-107	1	2.7	3.0	-	25	41	30	4	34	SM	COLLUVIUM
TP-BGC11-108	1	0.9	1.2	-	37	37	-	-	26	GM-SM	COLLUVIUM
TP-BGC11-112	1	1.3	1.4	-	48	33	-	-	19	GM-SM	COLLUVIUM
TP-BGC11-116	2	2.2	2.3	-	49	36	-	-	15	GC	COLLUVIUM
TP-BGC11-124	1	6.2	6.4	-	21	29	-	-	50	ML	COLLUVIUM
TP-BGC11-126	1	1.7	2.2	-	52	30	-	-	18	GC-SC/GM-SM	COLLUVIUM
TP-BGC11-128	1	2.5	3.0	-	7	32	61	0	61	ML	COLLUVIUM
TP-BGC11-129	1	6.6	6.8	-	16	31	-	-	53	ML	COLLUVIUM
TP-BGC11-130	1	5.8	6.0	-	35	43	-	-	22	SM-GM/SC-GC	COLLUVIUM
TP-BGC11-131	1	3.2	3.5	-	45	38	-	-	17	GM-SM	DEBRIS FLOW?
TP-BGC11-133	1	4.3	4.4	-	51	34	-	-	15	GM	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-140	1	1.0	1.2	-	21	34	-	-	45	ML	COLLUVIUM
Boreholes											
BH-BGC11-39	SPT1	1.2	1.6	-	0	5	-	-	95	ML	PLACER TAILINGS
BH-BGC11-39	SPT2	1.6	2.1	-	0	8	-	-	92	ML	PLACER TAILINGS
BH-BGC11-39	SPT3	2.3	2.8	-	27	28	-	-	45	SM-GM	PLACER TAILINGS
BH-BGC11-39	SPT4	2.8	3.2	-	48	33	-	-	19	GM	PLACER TAILINGS
BH-BGC11-39	SPT5	4.6	5.1	-	38	44	-	-	18	SM	PLACER TAILINGS
BH-BGC11-39	SPT6	5.5	6.0	-	25	61	-	-	14	SM	PLACER TAILINGS
BH-BGC11-39	SPT7	6.2	6.7	-	2	78	-	-	20	SM	PLACER TAILINGS
BH-BGC11-39	SPT8	7.2	7.6	-	0	81	-	-	19	SM	PLACER TAILINGS
BH-BGC11-39	SPT9	7.8	8.2	-	39	48	-	-	13	SM	PLACER TAILINGS
BH-BGC11-39	SPT10	8.7	9.2	-	2	82	-	-	16	SM	PLACER TAILINGS
BH-BGC11-39	SPT11	9.3	9.7	-	2	84	-	-	14	SM	PLACER TAILINGS
BH-BGC11-42	2	1.8	2.3	-	7	59	24	10	34	SM	ICE RICH COLLUVIUM
BH-BGC11-42	5	6.4	6.8	-	6	55	27	12	39	SM	ICE RICH COLLUVIUM
BH-BGC11-42	7	9.4	9.9	-	5	61	-	-	34	SM	ICE RICH COLLUVIUM
BH-BGC11-42	12	13.3	13.7	-	31	60	-	-	9	SW-GW	ICE RICH COLLUVIUM
BH-BGC11-42	20	19.7	20.0	-	10	64	16	10	26	SM	ICE RICH COLLUVIUM
BH-BGC11-42	22	21.6	21.9	-	4	66	18	12	30	SM	ICE RICH COLLUVIUM
BH-BGC11-42	S26	27.6	28.0	-	19	59	-	-	22	SM	ICE RICH COLLUVIUM
BH-BGC11-44	S1	0.2	0.4	-	20	35	32	13	45	SM	ALLUVIUM
BH-BGC11-44	S3	1.6	2.0	-	3	10	72	15	87	ML	ALLUVIUM
BH-BGC11-44	S5	4.8	5.1	-	33	22	35	10	45	GC-SC	ALLUVIUM
BH-BGC11-44	S7	6.3	6.6	-	29	22	49	0	49	GC-SC/GM-SM	ALLUVIUM
BH-BGC11-44	S11	9.6	9.9	-	20	40	24	16	40	GC-SC	ALLUVIUM?
BH-BGC11-47	G3	2.0	2.3	-	0	8	78	14	92	ML	ALLUVIUM?
BH-BGC11-47	G10	11.8	12.1	-	15	46	39	0	39	SC	ALLUVIUM?
BH-BGC11-47	SPT1	0.8	1.2	-	1	8	91	0	91	CL	ALLUVIUM?
BH-BGC11-47	SPT4	5.3	5.9	-	26	45	-	-	29	SM-GM	ALLUVIUM?
BH-BGC11-47	SPT6	8.4	8.6	-	13	38	49	0	49	SM	ALLUVIUM?
BH-BGC11-49	G4	3.4	3.6	-	28	42	26	4	30	SM	ALLUVIUM?
BH-BGC11-49	G6	6.4	6.6	-	33	34	31	2	33	SM-GM	ALLUVIUM?
BH-BGC11-49	G8	9.4	9.6	-	6	69	16	9	25	SM	ALLUVIUM?
BH-BGC11-49	G10	12.4	12.6	-	22	36	29	13	42	SM	ALLUVIUM?
BH-BGC11-49	SPT2	2.3	2.7	-	7	23	58	12	70	CL	ALLUVIUM?
BH-BGC11-51	G6	7.9	8.1	-	14	27	50	9	59	ML	ALLUVIUM
BH-BGC11-51	G10	13.9	14.1	-	21	53	26	0	26	SM	ALLUVIUM
BH-BGC11-51	G14	20.1	20.3	-	11	63	-	-	26	SW	ALLUVIUM
BH-BGC11-53	G2	1.9	2.1	-	19	40	38	3	41	SC	TILL
BH-BGC11-53	G4	4.9	5.1	-	41	44	14	1	15	SC-GC	TILL
BH-BGC11-53	G6	7.9	8.1	-	31	47	-	-	22	SM	TILL

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Cobbles (%) ¹	Gravel (%) ²	Sand (%) ³	Silt (%) ⁴	Clay (%) ⁵	Fines (%)	USCS Classification	Lithology Unit
BH-BGC11-53	SPT2	2.3	2.7	-	22	41	28	9	37	SC	TILL
BH-BGC11-53	SPT4	6.9	7.3	-	8	76	13	3	16	SC-GC	TILL
BH-BGC11-55	G4	4.9	5.1	-	20	41	36	3	39	SC	DEBRIS FLOW
BH-BGC11-55	SPT1	0.8	1.2	-	38	37	-	-	25	GM-SM	COLLUVIUM
BH-BGC11-56	G2	1.9	2.1	-	40	37	-	-	23	GM-SM	COLLUVIUM
BH-BGC11-57	SPT2	3.8	4.3	-	19	49	28	4	32	SC	COLLUVIUM
BH-BGC11-57	SPT4	6.9	7.1	-	27	42	23	8	31	SC-SM	COLLUVIUM
BH-BGC11-58	G2	1.9	2.1	-	11	44	40	5	45	SM	COLLUVIUM
BH-BGC11-58	G4	3.2	3.6	-	23	51	26	0	26	SM	COLLUVIUM
BH-BGC11-58	G6	4.4	4.6	-	29	54	16	1	17	SM	COLLUVIUM
BH-BGC11-58	G8	5.4	5.5	-	37	35	26	2	28	GM-SM	COLLUVIUM
BH-BGC11-58	G10	7.9	8.1	-	15	51	32	2	34	SM	COLLUVIUM
BH-BGC11-60	G3	2.7	2.8	-	20	42	25	13	38	SC	COLLUVIUM
BH-BGC11-60	G5	3.7	3.8	-	21	45	25	9	34	SC	COLLUVIUM
BH-BGC11-60	G7	6.3	6.6	-	43	30	17	10	27	SC	COLLUVIUM
BH-BGC11-60	SPT2	6.9	7.0	-	39	31	19	11	30	SC	COLLUVIUM
BH-BGC11-63	G-2	1.9	2.1	-	1	54	25	20	45	SC	ICE RICH COLLUVIUM
BH-BGC11-63	G-5	3.9	4.0	-	0	54	33	13	46	SC	ICE RICH COLLUVIUM
BH-BGC11-63	G-9	10.2	10.3	-	1	57	20	22	42	SM	ICE RICH COLLUVIUM
BH-BGC11-63	G-12	16.2	16.3	-	12	59	19	10	29	SC	ICE RICH COLLUVIUM
BH-BGC11-65	SPT1	0.8	1.2	-	26	60	-	-	14	SM	PLACER TAILINGS
BH-BGC11-65	SPT2	1.5	2.0	-	40	48	-	-	12	SM	PLACER TAILINGS
BH-BGC11-65	SPT3	2.3	2.7	-	53	38	-	-	9	GW-SW	PLACER TAILINGS
BH-BGC11-65	SPT4	3.0	3.5	-	63	29	-	-	8	GW-SW	PLACER TAILINGS
BH-BGC11-65	SPT5	3.8	4.2	-	76	19	-	-	5	GW-SW	PLACER TAILINGS
BH-BGC11-65	SPT6	4.6	5.0	-	40	44	-	-	16	SM-GM	PLACER TAILINGS
BH-BGC11-65	SPT7	5.3	5.8	-	64	26	-	-	10	GW-SW	PLACER TAILINGS
BH-BGC11-65	SPT8	6.1	6.5	-	53	38	-	-	9	GW-SW	PLACER TAILINGS
Steiner Outcrop											
BGC-Steiner	-	-	-	-	72	25	-	-	3	GW	AGGREGATE

NOTES:

1. Cobbles is considered to be particles larger than 3 inches (76.2 mm).
2. Gravel is considered to be particles smaller than 3 inches (76.2 mm) and larger than the #4 sieve (4.75 mm).
3. Sand is considered to be particles smaller than the #4 sieve (4.75 mm) and larger than the #200 sieve (0.075 mm).
4. Silt is considered to be particles smaller than the #200 sieve (0.075 mm) and larger than 0.002 mm.
5. Clay is considered to be any particles smaller than 0.002 mm.

Table M-5. Moisture Content Results

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Moisture Content (%)	Lithology Unit
Test Pits					
TP-BGC11-50	1	2.8	3.0	6	COLLUVIUM
TP-BGC11-50	2	3.3	3.6	7	COLLUVIUM
TP-BGC11-51	1	0.2	1.4	13	COLLUVIUM
TP-BGC11-51	2	2.4	2.8	7	COLLUVIUM
TP-BGC11-51	3	4.2	4.5	4	COLLUVIUM
TP-BGC11-52	1	3.3	3.5	3	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-53	1	2.0	2.2	6	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-54	1	0.4	0.6	9	COLLUVIUM
TP-BGC11-55	1	0.4	0.6	7	COLLUVIUM
TP-BGC11-55	2	1.9	2.0	6	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-57	1	0.5	1.1	8	COLLUVIUM
TP-BGC11-58	1	0.7	0.8	9	COLLUVIUM
TP-BGC11-58	2	2.0	2.1	6	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-59	1	0.4	0.6	7	COLLUVIUM
TP-BGC11-60	1	0.7	0.9	25	COLLUVIUM
TP-BGC11-61	1	0.7	0.8	12	COLLUVIUM
TP-BGC11-61	2	1.7	1.9	21	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-62	1	0.9	1.0	11	COLLUVIUM
TP-BGC11-62	2	3.5	3.7	9	COLLUVIUM
TP-BGC11-63	1	0.5	0.7	7	COLLUVIUM
TP-BGC11-64	1	1.5	1.6	5	COLLUVIUM
TP-BGC11-65	1	0.4	0.5	7	COLLUVIUM
TP-BGC11-66	1	1.3	1.4	11	COLLUVIUM
TP-BGC11-66	2	1.8	1.9	17	COLLUVIUM
TP-BGC11-67	1	0.7	0.9	7	COLLUVIUM
TP-BGC11-68	1	1.6	1.8	9	COLLUVIUM
TP-BGC11-69	1	1.7	1.9	30	COLLUVIUM
TP-BGC11-69	2	2.8	2.9	7	COLLUVIUM
TP-BGC11-71	1	1.3	1.6	14	COLLUVIUM
TP-BGC11-72	1	2.4	2.6	5	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-72	2	0.8	1.0	14	COLLUVIUM
TP-BGC11-73	1	0.8	1.0	8	COLLUVIUM
TP-BGC11-74	1	1.2	1.4	12	COLLUVIUM
TP-BGC11-74	2	3.0	3.1	36	COLLUVIUM
TP-BGC11-74	3	4.5	4.6	15	COLLUVIUM
TP-BGC11-76	1	0.4	0.6	16	COLLUVIUM
TP-BGC11-77	1	0.4	0.5	27	COLLUVIUM
TP-BGC11-79	1	0.4	0.6	10	COLLUVIUM?
TP-BGC11-79	2	1.2	1.3	16	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-81	1	0.7	1.0	17	COLLUVIUM
TP-BGC11-82	1	4.7	4.9	12	ALLUVIUM
TP-BGC11-82	2	3.0	3.3	10	ALLUVIUM
TP-BGC11-82	3	6.7	7.0	40	ALLUVIUM
TP-BGC11-82	4	0.7	1.1	29	ALLUVIUM?
TP-BGC11-82	5	1.9	2.1	27	ALLUVIUM
TP-BGC11-83	1	0.6	0.8	20	COLLUVIUM
TP-BGC11-84	1	0.4	0.6	18	COLLUVIUM
TP-BGC11-84	2	2.3	2.5	29	COLLUVIUM
TP-BGC11-85	1	0.7	1.1	17	COLLUVIUM
TP-BGC11-87	1	0.6	0.8	11	COLLUVIUM
TP-BGC11-88	1	1.5	1.8	13	COLLUVIUM
TP-BGC11-89	1	0.4	0.6	2	COLLUVIUM
TP-BGC11-89	2	1.3	1.5	14	COLLUVIUM
TP-BGC11-90	1	0.8	1.0	13	COLLUVIUM
TP-BGC11-90	2	3.5	3.6	23	COLLUVIUM
TP-BGC11-91	1	0.8	1.0	49	COLLUVIUM
TP-BGC11-91	2	1.6	1.8	6	COLLUVIUM
TP-BGC11-92	1	0.6	0.8	64	COLLUVIUM
TP-BGC11-94	1	1.2	1.3	16	COLLUVIUM
TP-BGC11-94	2	2.3	2.5	11	COLLUVIUM
TP-BGC11-95	1	0.9	1.1	13	WEATHERED INTRUSIVE ROCK
TP-BGC11-95	2	2.0	3.0	22	WEATHERED INTRUSIVE ROCK

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Moisture Content (%)	Lithology Unit
TP-BGC11-96	1	0.7	0.9	8	WEATHERED INTRUSIVE ROCK
TP-BGC11-96	2	1.3	1.5	20	WEATHERED INTRUSIVE ROCK
TP-BGC11-96	3	1.8	2.0	9	WEATHERED INTRUSIVE ROCK
TP-BGC11-103	1	4.4	4.8	3	TILL
TP-BGC11-104	1	1.2	1.6	7	TILL
TP-BGC11-106	1	0.8	1.5	11	COLLUVIUM
TP-BGC11-107	1	2.7	3.0	17	COLLUVIUM
TP-BGC11-108	1	0.9	1.2	10	COLLUVIUM
TP-BGC11-111	1	0.9	1.1	17	COLLUVIUM
TP-BGC11-112	1	1.3	1.4	14	COLLUVIUM
TP-BGC11-116	2	2.2	2.3	8	COLLUVIUM
TP-BGC11-119	1	0.9	1.1	29	ALLUVIUM?
TP-BGC11-120	1	3.8	4.0	29	ALLUVIUM?
TP-BGC11-121	1	2.9	3.1	25	ALLUVIUM?
TP-BGC11-122	1	5.4	5.8	19	PLACER TAILINGS
TP-BGC11-124	1	6.2	6.4	21	COLLUVIUM
TP-BGC11-126	1	1.7	2.2	12	COLLUVIUM
TP-BGC11-128	1	2.5	3.0	27	COLLUVIUM
TP-BGC11-129	1	6.6	6.8	23	COLLUVIUM
TP-BGC11-130	1	5.8	6.0	7	COLLUVIUM
TP-BGC11-131	1	3.2	3.5	8	DEBRIS FLOW?
TP-BGC11-133	1	4.3	4.4	9	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-140	1	1.0	1.2	16	COLLUVIUM
Boreholes					
BH-BGC11-39	SPT1	1.2	1.6	34	PLACER TAILINGS
BH-BGC11-39	SPT2	1.6	2.1	31	PLACER TAILINGS
BH-BGC11-39	SPT3	2.3	2.8	17	PLACER TAILINGS
BH-BGC11-39	SPT4	2.8	3.2	4	PLACER TAILINGS
BH-BGC11-39	SPT5	4.6	5.1	2	PLACER TAILINGS
BH-BGC11-39	SPT6	5.5	6.0	4	PLACER TAILINGS
BH-BGC11-39	SPT7	6.2	6.7	7	PLACER TAILINGS
BH-BGC11-39	SPT8	7.2	7.6	7	PLACER TAILINGS
BH-BGC11-39	SPT9	7.8	8.2	4	PLACER TAILINGS
BH-BGC11-39	SPT10	8.7	9.2	15	PLACER TAILINGS
BH-BGC11-39	SPT11	9.3	9.7	17	PLACER TAILINGS
BH-BGC11-40A	1	18.8	18.8	13	FAULT ZONE
BH-BGC11-42	2	1.8	2.3	17	ICE RICH COLLUVIUM
BH-BGC11-42	4	5.0	5.3	14	ICE RICH COLLUVIUM
BH-BGC11-42	5	6.4	6.8	14	ICE RICH COLLUVIUM
BH-BGC11-42	7	9.4	9.9	12	ICE RICH COLLUVIUM
BH-BGC11-42	8	10.6	11.0	16	ICE RICH COLLUVIUM
BH-BGC11-42	12	13.3	13.7	12	ICE RICH COLLUVIUM
BH-BGC11-42	15	15.3	15.4	7	ICE RICH COLLUVIUM
BH-BGC11-42	16	15.6	15.8	9	ICE RICH COLLUVIUM
BH-BGC11-42	20	19.7	20.0	18	ICE RICH COLLUVIUM
BH-BGC11-42	22	21.6	21.9	19	ICE RICH COLLUVIUM
BH-BGC11-42	23	23.0	23.4	15	ICE RICH COLLUVIUM
BH-BGC11-42	S26	27.6	28.0	15	ICE RICH COLLUVIUM
BH-BGC11-44	S1	0.2	0.4	14	ALLUVIUM
BH-BGC11-44	S3	1.6	2.0	23	ALLUVIUM
BH-BGC11-44	S5	4.8	5.1	14	ALLUVIUM
BH-BGC11-44	S7	6.3	6.6	24	ALLUVIUM
BH-BGC11-44	S11	9.6	9.9	17	ALLUVIUM?
BH-BGC11-44	SPT1	2.3	2.7	85	ALLUVIUM?
BH-BGC11-44	SPT2	3.8	4.2	25	ALLUVIUM?
BH-BGC11-44	SPT5	8.4	8.9	17	ALLUVIUM?
BH-BGC11-44	SPT10	13.0	13.4	12	ALLUVIUM?
BH-BGC11-44	SPT11	14.5	14.8	14	ALLUVIUM?
BH-BGC11-47	G3	2.0	2.3	27	ALLUVIUM?
BH-BGC11-47	G4	2.8	3.0	5	ALLUVIUM?
BH-BGC11-47	G10	11.8	12.1	11	ALLUVIUM?
BH-BGC11-47	SPT1	0.8	1.2	26	ALLUVIUM?
BH-BGC11-47	SPT4	5.3	5.9	10	ALLUVIUM?
BH-BGC11-47	SPT5	6.9	7.3	13	ALLUVIUM?
BH-BGC11-47	SPT6	8.4	8.6	12	ALLUVIUM?
BH-BGC11-49	G4	3.4	3.6	12	ALLUVIUM?
BH-BGC11-49	G6	6.4	6.6	13	ALLUVIUM?

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Moisture Content (%)	Lithology Unit
BH-BGC11-49	G8	9.4	9.6	26	ALLUVIUM?
BH-BGC11-49	G10	12.4	12.6	12	ALLUVIUM?
BH-BGC11-49	SPT1	0.8	1.2	11	ALLUVIUM?
BH-BGC11-49	SPT2	2.3	2.7	20	ALLUVIUM?
BH-BGC11-51	G6	7.9	8.1	17	ALLUVIUM
BH-BGC11-51	G8	10.9	11.1	14	ALLUVIUM
BH-BGC11-51	G10	13.9	14.1	11	ALLUVIUM
BH-BGC11-51	G12	17.0	17.2	11	ALLUVIUM
BH-BGC11-51	G14	20.1	20.3	11	ALLUVIUM
BH-BGC11-51	G17	24.7	24.9	7	ALLUVIUM
BH-BGC11-51	SPT1	0.8	1.2	12	ALLUVIUM
BH-BGC11-51	SPT2	2.3	2.7	10	ALLUVIUM
BH-BGC11-51	SPT3	3.8	4.3	29	ALLUVIUM
BH-BGC11-51	SPT4	5.3	5.8	9	ALLUVIUM
BH-BGC11-51	SPT5	9.9	10.3	20	ALLUVIUM
BH-BGC11-53	G2	1.9	2.1	11	TILL
BH-BGC11-53	G3	3.2	3.5	8	TILL
BH-BGC11-53	G4	4.9	5.1	4	TILL
BH-BGC11-53	G6	7.9	8.1	5	TILL
BH-BGC11-53	G8	10.9	11.1	3	TILL
BH-BGC11-53	SPT1	0.8	1.2	12	TILL
BH-BGC11-53	SPT2	2.3	2.7	10	TILL
BH-BGC11-53	SPT3	3.8	4.3	4	TILL
BH-BGC11-53	SPT4	6.9	7.3	3	TILL
BH-BGC11-53	SPT5	9.9	10.4	5	TILL
BH-BGC11-54	G1	30.0	30.0	8	WEATHERED METASEDIMENTARY ROCK
BH-BGC11-55	G4	4.9	5.1	11	DEBRIS FLOW
BH-BGC11-55	G5	6.3	6.5	12	DEBRIS FLOW
BH-BGC11-55	SPT1	0.8	1.2	9	COLLUVIUM
BH-BGC11-55	SPT2	2.3	2.7	10	COLLUVIUM
BH-BGC11-55	SPT6	8.4	8.8	7	DEBRIS FLOW
BH-BGC11-56	G2	1.9	2.1	6	COLLUVIUM
BH-BGC11-56	SPT1	0.8	1.2	5	COLLUVIUM
BH-BGC11-56	SPT3	3.8	4.3	9	COLLUVIUM
BH-BGC11-56	SPT4	5.3	5.5	7	COLLUVIUM
BH-BGC11-56	SPT5	6.9	7.2	5	COLLUVIUM
BH-BGC11-57	SPT1	2.3	2.7	10	COLLUVIUM
BH-BGC11-57	SPT2	3.8	4.3	9	COLLUVIUM
BH-BGC11-57	SPT3	5.3	5.8	10	COLLUVIUM
BH-BGC11-57	SPT4	6.9	7.1	8	COLLUVIUM
BH-BGC11-58	G2	1.9	2.1	17	COLLUVIUM
BH-BGC11-58	G4	3.2	3.6	11	COLLUVIUM
BH-BGC11-58	G6	4.4	4.6	11	COLLUVIUM
BH-BGC11-58	G8	5.4	5.5	6	COLLUVIUM
BH-BGC11-58	G10	7.9	8.1	9	COLLUVIUM
BH-BGC11-60	G3	2.7	2.8	10	COLLUVIUM
BH-BGC11-60	G5	3.7	3.8	9	COLLUVIUM
BH-BGC11-60	G6	4.9	5.1	7	COLLUVIUM
BH-BGC11-60	G7	6.3	6.6	7	COLLUVIUM
BH-BGC11-60	SPT2	6.9	7.0	6	COLLUVIUM
BH-BGC11-63	G-2	1.9	2.1	25	ICE RICH COLLUVIUM
BH-BGC11-63	G-3	2.9	3.0	19	ICE RICH COLLUVIUM
BH-BGC11-63	G-5	3.9	4.0	27	ICE RICH COLLUVIUM
BH-BGC11-63	G-6	5.9	6.1	27	ICE RICH COLLUVIUM
BH-BGC11-63	G-9	10.2	10.3	29	ICE RICH COLLUVIUM
BH-BGC11-63	G-10	11.6	11.7	17	ICE RICH COLLUVIUM
BH-BGC11-63	G-12	16.2	16.3	25	ICE RICH COLLUVIUM
BH-BGC11-65	SPT1	0.8	1.2	3	PLACER TAILINGS
BH-BGC11-65	SPT2	1.5	2.0	3	PLACER TAILINGS
BH-BGC11-65	SPT3	2.3	2.7	2	PLACER TAILINGS
BH-BGC11-65	SPT4	3.0	3.5	2	PLACER TAILINGS
BH-BGC11-65	SPT5	3.8	4.2	1	PLACER TAILINGS
BH-BGC11-65	SPT6	4.6	5.0	5	PLACER TAILINGS
BH-BGC11-65	SPT7	5.3	5.8	5	PLACER TAILINGS
BH-BGC11-65	SPT8	6.1	6.5	9	PLACER TAILINGS

Table M-6. Atterberg Limit Results

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index	Fines Classification	Lithology Unit
Test Pits								
TP-BGC11-51	1	0.2	1.4	27	22	5	ML	COLLUVIUM
TP-BGC11-54	1	0.4	0.6	21	20	1	ML	COLLUVIUM
TP-BGC11-55	1	0.4	0.6	-	-	-	Non-plastic	COLLUVIUM
TP-BGC11-57	1	0.5	1.1	28	23	5	ML	COLLUVIUM
TP-BGC11-58	1	0.7	0.8	27	23	4	ML	COLLUVIUM
TP-BGC11-59	1	0.4	0.6	24	23	1	ML	COLLUVIUM
TP-BGC11-60	1	0.7	0.9	27	26	1	ML	COLLUVIUM
TP-BGC11-61	1	0.7	0.8	24	21	3	ML	COLLUVIUM
TP-BGC11-62	1	0.9	1.0	-	-	-	Non-plastic	COLLUVIUM
TP-BGC11-62	2	3.5	3.7	21	16	5	CL-ML	COLLUVIUM
TP-BGC11-64	1	1.5	1.6	-	-	-	Non-plastic	COLLUVIUM
TP-BGC11-66	2	1.8	1.9	29	19	10	CL	COLLUVIUM
TP-BGC11-69	1	1.7	1.9	-	-	-	Non-plastic	COLLUVIUM
TP-BGC11-69	2	2.8	2.9	23	17	6	CL-ML	COLLUVIUM
TP-BGC11-71	1	1.3	1.6	28	22	6	CL-ML	COLLUVIUM
TP-BGC11-72	2	0.8	1.0	31	24	7	ML	COLLUVIUM
TP-BGC11-73	1	0.8	1.0	-	-	-	Non-plastic	COLLUVIUM
TP-BGC11-74	1	1.2	1.4	30	25	5	ML	COLLUVIUM
TP-BGC11-74	2	3.0	3.1	33	30	3	ML	COLLUVIUM
TP-BGC11-74	3	4.5	4.6	28	22	6	CL-ML	COLLUVIUM
TP-BGC11-77	1	0.4	0.5	46	29	17	ML	COLLUVIUM
TP-BGC11-79	1	0.4	0.6	26	25	1	ML	COLLUVIUM?
TP-BGC11-82	1	4.7	4.9	-	-	-	Non-plastic	ALLUVIUM
TP-BGC11-82	2	3.0	3.3	30	27	3	ML	ALLUVIUM
TP-BGC11-82	3	6.7	7.0	-	-	-	Non-plastic	ALLUVIUM
TP-BGC11-82	4	0.7	1.1	32	28	4	ML	ALLUVIUM?
TP-BGC11-82	5	1.9	2.1	28	23	5	ML	ALLUVIUM
TP-BGC11-83	1	0.6	0.8	55	44	11	MH	COLLUVIUM
TP-BGC11-84	2	2.3	2.5	29	26	3	ML	COLLUVIUM
TP-BGC11-85	1	0.7	1.1	23	22	1	ML	COLLUVIUM
TP-BGC11-88	1	1.5	1.8	26	19	7	CL-ML	COLLUVIUM
TP-BGC11-89	2	1.3	1.5	23	21	2	ML	COLLUVIUM
TP-BGC11-90	2	3.5	3.6	24	22	2	ML	COLLUVIUM
TP-BGC11-91	1	0.8	1.0	42	35	7	ML	COLLUVIUM
TP-BGC11-91	2	1.6	1.8	-	-	-	Non-plastic	COLLUVIUM
TP-BGC11-92	1	0.6	0.8	45	42	3	ML	COLLUVIUM
TP-BGC11-94	1	1.2	1.3	27	24	3	ML	COLLUVIUM
TP-BGC11-96	2	1.3	1.5	26	22	4	ML	WEATHERED INTRUSIVE ROCK
TP-BGC11-116	2	2.2	2.3	20	18	3	CL	COLLUVIUM
TP-BGC11-119	1	0.9	1.1	27	23	4	ML	ALLUVIUM?
TP-BGC11-120	1	3.8	4.0	33	24	9	ML	ALLUVIUM?
TP-BGC11-121	1	2.9	3.1	29	20	8	CL	ALLUVIUM?
TP-BGC11-122	1	5.4	5.8	25	18	7	CL	PLACER TAILINGS
TP-BGC11-124	1	6.2	6.4	32	28	4	ML	COLLUVIUM
TP-BGC11-129	1	6.6	6.8	27	22	5	ML	COLLUVIUM
TP-BGC11-130	1	5.8	6.0	-	-	-	Non-plastic	COLLUVIUM
TP-BGC11-131	1	3.2	3.5	-	-	-	Non-plastic	DEBRIS FLOW?
TP-BGC11-140	1	1.0	1.2	24	21	2	ML	COLLUVIUM
Boreholes								
BH-BGC11-42	2	1.8	2.3	30	23	7	ML	ICE RICH COLLUVIUM
BH-BGC11-42	5	6.4	6.8	27	18	9	CL	ICE RICH COLLUVIUM
BH-BGC11-44	S1	0.2	0.4	26	20	6	CL	ALLUVIUM
BH-BGC11-44	S3	1.6	2.0	-	-	-	Non-plastic	ALLUVIUM
BH-BGC11-44	S5	4.8	5.1	20	19	1	CL	ALLUVIUM
BH-BGC11-44	S7	6.3	6.6	23	21	3	CL	ALLUVIUM
BH-BGC11-44	S11	9.6	9.9	21	15	6	CL	ALLUVIUM?
BH-BGC11-44	SPT2	3.8	4.2	-	-	-	Non-plastic	ALLUVIUM?
BH-BGC11-44	SPT10	13.0	13.4	22	15	6	CL	ALLUVIUM?
BH-BGC11-44	SPT11	14.5	14.8	22	17	5	CL	ALLUVIUM?
BH-BGC11-47	G3	2.0	2.3	23	21	2	ML	ALLUVIUM?

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index	Fines Classification	Lithology Unit
BH-BGC11-47	G10	11.8	12.1	19	15	3	CL	ALLUVIUM?
BH-BGC11-47	SPT1	0.8	1.2	22	19	3	CL	ALLUVIUM?
BH-BGC11-49	G4	3.4	3.6	20	17	3	CL	ALLUVIUM?
BH-BGC11-49	G6	6.4	6.6	21	17	4	CL	ALLUVIUM?
BH-BGC11-49	G8	9.4	9.6	-	-	-	Non-plastic	ALLUVIUM?
BH-BGC11-49	G10	12.4	12.6	22	18	4	CL	ALLUVIUM?
BH-BGC11-49	SPT2	2.3	2.7	23	19	4	CL	ALLUVIUM?
BH-BGC11-51	G6	7.9	8.1	28	20	7	CL	ALLUVIUM
BH-BGC11-51	G10	13.9	14.1	19	16	3	CL	ALLUVIUM
BH-BGC11-51	SPT1	0.8	1.2	27	20	7	CL	ALLUVIUM
BH-BGC11-51	SPT3	3.8	4.3	34	34	0	ML	ALLUVIUM
BH-BGC11-53	G2	1.9	2.1	27	17	9	CL	TILL
BH-BGC11-53	G4	4.9	5.1	22	16	6	CL	TILL
BH-BGC11-53	SPT2	2.3	2.7	24	18	6	CL	TILL
BH-BGC11-53	SPT4	6.9	7.3	-	-	-	Non-plastic	TILL
BH-BGC11-54	G1	30.0	30.0	20	16	4	CL	WEATHERED METASEDIMENTARY ROCK
BH-BGC11-55	G4	4.9	5.1	22	15	7	CL	DEBRIS FLOW
BH-BGC11-55	SPT6	8.4	8.8	26	23	3	ML	DEBRIS FLOW
BH-BGC11-56	SPT3	3.8	4.3	-	-	-	Non-plastic	COLLUVIUM
BH-BGC11-56	SPT4	5.3	5.5	22	18	4	CL	COLLUVIUM
BH-BGC11-57	SPT2	3.8	4.3	20	14	6	CL	COLLUVIUM
BH-BGC11-57	SPT4	6.9	7.1	22	20	2	CL	COLLUVIUM
BH-BGC11-58	G2	1.9	2.1	21	17	4	CL	COLLUVIUM
BH-BGC11-58	G4	3.2	3.6	-	-	-	Non-plastic	COLLUVIUM
BH-BGC11-58	G6	4.4	4.6	-	-	-	Non-plastic	COLLUVIUM
BH-BGC11-58	G8	5.4	5.5	19	16	3	CL	COLLUVIUM
BH-BGC11-58	G10	7.9	8.1	-	-	-	Non-plastic	COLLUVIUM
BH-BGC11-60	G3	2.7	2.8	21	16	6	CL	COLLUVIUM
BH-BGC11-60	G5	3.7	3.8	18	17	1	CL	COLLUVIUM
BH-BGC11-60	G7	6.3	6.6	21	20	2	CL	COLLUVIUM
BH-BGC11-63	G-2	1.9	2.1	46	24	25	CI ¹	ICE RICH COLLUVIUM
BH-BGC11-63	G-5	3.9	4.0	39	24	15	CI ¹	ICE RICH COLLUVIUM
BH-BGC11-63	G-9	10.2	10.3	44	27	17	ML	ICE RICH COLLUVIUM
BH-BGC11-63	G-12	16.2	16.3	31	20	11	CI ¹	ICE RICH COLLUVIUM

NOTES:

¹CI values reported by GeoNorth for 30<Liquid Limit<50.

Table M-7. Specific Gravity Results

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Specific Gravity Rock			Specific Gravity Soil				Lithology Unit
				Oven Dry	SSD ¹	Apparent	Oven Dry (Coarse)	SSD ¹ (Coarse)	Apparent (Coarse)	Average (Fine)	
TP-BGC11-50	1	2.8	3.0	-	-	-	2.61	2.75	3.03	2.8	COLLUVIUM
TP-BGC11-58	2	2.0	2.1	-	-	-	-	-	-	2.84	WEATHERED METASEDIMENTARY ROCK
TP-BGC11-67	1	0.7	0.9	-	-	-	-	-	-	2.72	COLLUVIUM
BH-BGC11-29	1	27.5	27.8	2.71	2.73	2.77	-	-	-	-	METASEDIMENTARY ROCK
BH-BGC11-34	1	36.6	36.9	2.56	2.64	2.78	-	-	-	-	METASEDIMENTARY ROCK
BH-BGC11-40A	2	31.0	31.3	2.67	2.68	2.7	-	-	-	-	METASEDIMENTARY ROCK
BH-BGC11-40B	UCS1	21.8	22.1	2.74	2.75	2.78	-	-	-	-	METASEDIMENTARY ROCK
BH-BGC11-43	1	4.2	4.5	2.71	2.71	2.73	-	-	-	-	INTRUSIVE ROCK
BH-BGC11-45	UCS	17.8	18.0	2.76	2.78	2.81	-	-	-	-	METASEDIMENTARY ROCK
BH-BGC11-46	UCS	13.1	13.7	2.69	2.7	2.72	-	-	-	-	INTRUSIVE ROCK
BH-BGC11-62	UCS	28.6	28.8	2.59	2.63	2.69	-	-	-	-	METASEDIMENTARY ROCK
BH-BGC11-69	UCS1	12.3	12.5	2.52	2.59	2.7	-	-	-	-	METASEDIMENTARY ROCK
BH-BGC11-42	5	6.4	6.8	-	-	-	2.53	2.58	2.65	-	ICE RICH COLLUVIUM

NOTES:

SSD = Saturated surface dry.

Table M-8. Sulphate Ion Results

Hole ID	Sample Number	Depth From (m)	Depth To (m)	Total Sulphate Ion Content (%)	Lithology Unit
TP-BGC11-51	2	2.4	2.8	0.13	COLLUVIUM
TP-BGC11-60	1	0.7	0.9	0.07	COLLUVIUM
TP-BGC11-84	1	0.4	0.6	0.07	COLLUVIUM

NOTES:

1. Detection limit for the test is 0.005%.

Table M-9. Modified Proctor and Permeability Results

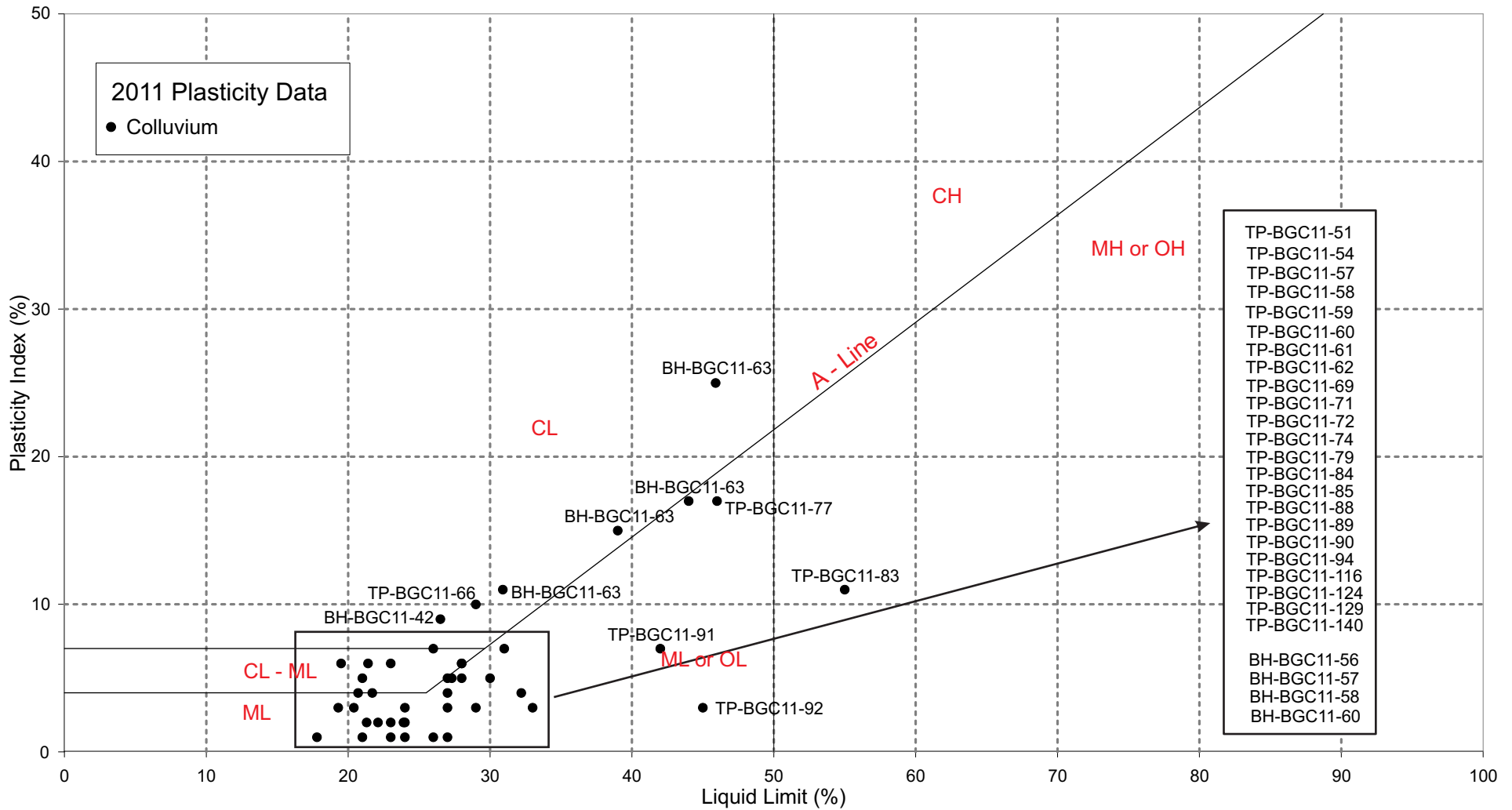
Hole ID	Sample Number	Depth From (m)	Depth To (m)	Permeability (m/s)	Modified Proctor		Lithology Unit
					Maximum Dry Density (kg/m ³)	Optimum Moisture Content (%)	
TP-BGC11-119	1	0.9	1.1	4.2E-10	1780	13.5	ALLUVIUM?
TP-BGC11-120	1	3.8	4	6.5E-10	1840	13	ALLUVIUM?
TP-BGC11-121	1	2.9	3.1	4E-10	1940	12	ALLUVIUM?
TP-BGC11-122	1	5.4	5.8	3.15E-10	2110	9	PLACER TAILINGS

Table M-10. Unconfined Compressive Strength Test and Brazilian Tensile Strength Test Results

Hole ID	Sample Number	Depth From (m)	Depth To (m)	UCS (MPa)	Young's Modulus (GPa)	Poisson Ratio	Brazilian Tensile		Lithology Unit
							Test 1 (MPa)	Test 2 (MPa)	
BH-BGC11-29	1	27.5	27.8	-	-	-	5.6	3.2	METASEDIMENTARY ROCK
BH-BGC11-34	1	36.6	36.9	53.4	46.3	0.12	9.2	11.8	METASEDIMENTARY ROCK
BH-BGC11-40A	2	31.0	31.3	164.2	51.6	0.15	13	12.2	METASEDIMENTARY ROCK
BH-BGC11-40B	UCS1	21.8	22.1	86.5	3.9	0.2	11.3	13	METASEDIMENTARY ROCK
BH-BGC11-43	1	4.2	4.5	210.1	46.8	0.21	15.6	14	INTRUSIVE ROCK
BH-BGC11-45	UCS	17.8	18.0	52.6	31.1	0.17	13.6	11.6	METASEDIMENTARY ROCK
BH-BGC11-46	UCS	13.1	13.7	154.7	56.3	0.27	15	15.3	INTRUSIVE ROCK
BH-BGC11-62	UCS	28.6	28.8	22.5	11.4	0.09	5.4	-	METASEDIMENTARY ROCK
BH-BGC11-69	UCS1	12.3	12.5	45.1	27.1	0.13	2.8	6.3	METASEDIMENTARY ROCK

FIGURES

N:\BGC\Projects\0792_Victoria_Gold\Infrastructure_2011\06_Reporting\01_Data_Report\03_Working\Appendices\Appendix M Laboratory Tabular and Graphical Summaries\All EGP Alterbergs 1 plot.pdf



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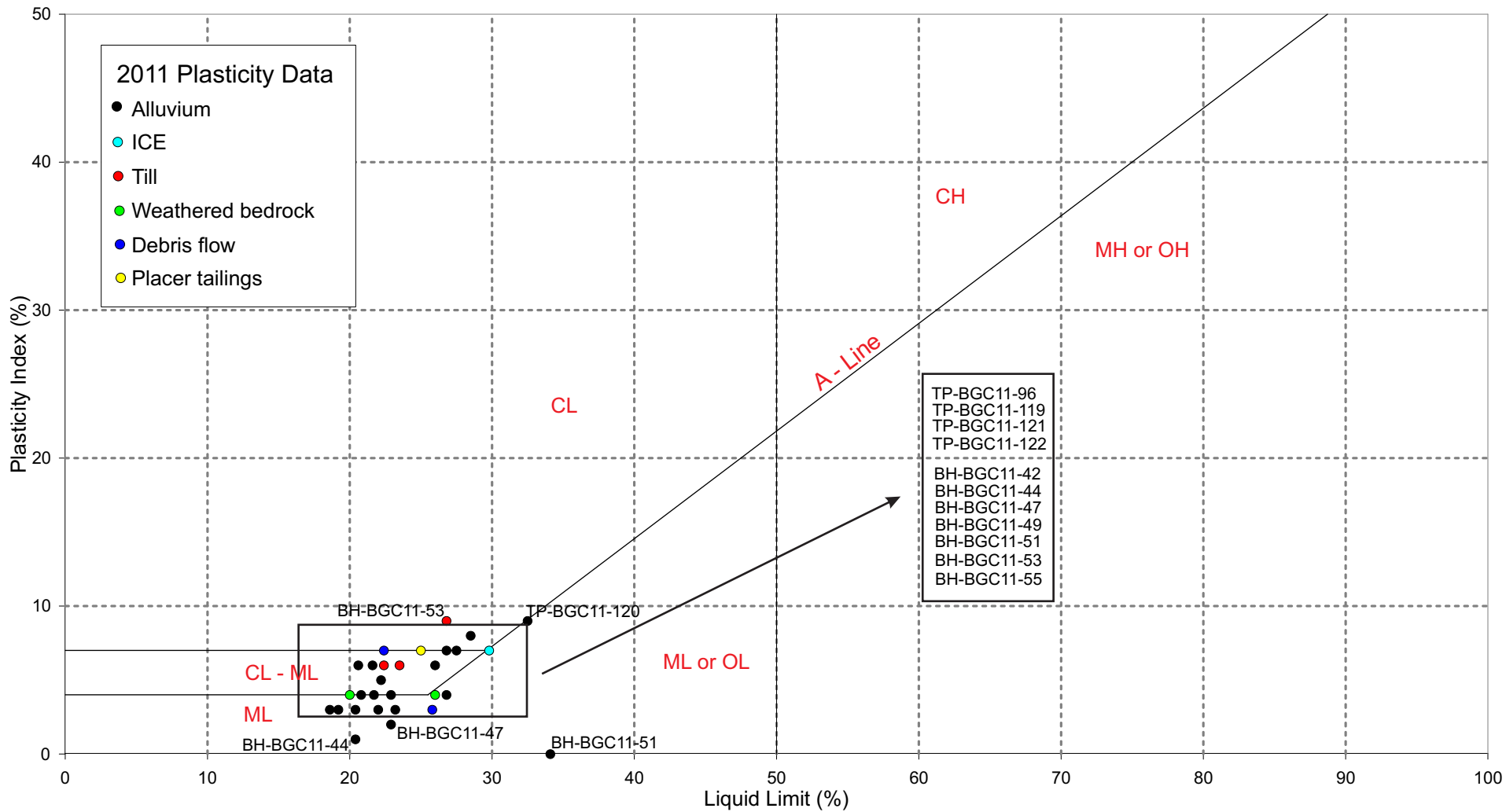
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PROJECT: **EAGLE GOLD PROJECT**
 2011 GEOTECHNICAL INVESTIGATION FOR MINE SITE INFRASTRUCTURE FACTUAL DATA REPORT

TITLE: **Plasticity Chart**
 Colluvium samples from 2011 field program

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APPENDIX N POINT LOAD TESTING

POINT LOAD TESTING

1.0 INTRODUCTION

Point load testing was carried out during the site investigation program to estimate the intact rock strength of the Metasedimentary and Intrusive bedrock. This appendix documents the findings of the point load testing undertaken during the 2011 mine site infrastructure geotechnical investigations for the Eagle Gold project.

2.0 TESTING DETAILS

All point load tests were carried out by BGC personnel on site, at the Dublin Gulch exploration camp, using a PIL-7 model point load tester manufactured by Roctest. The suggested method for determining point load strength (ISRM, 1985) was followed for all tests. Three hundred and twenty core samples of rock obtained by diamond drilling were tested; of those tests, 117 were invalid, as outlined by ISRM (1985), due to incomplete development of the fracture surface. Therefore, 203 valid tests were performed.

The details of all point load tests are reported in Tables N-1 to N-27.

3.0 REFERENCES

International Society for Rock Mechanics (ISRM). 1985. Suggested Method for Determining Point Load Strength. *Int. J. Rock Mech. Min. Sci. & Geomech. Abstr.* Vol 22, No. 2, pp. 51-60.

Table N-1: BH-BGC11-24 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
2.2	SSED	D	130	61.1	63.5	4.4	0.00417	63.5	--	1.03	1.11	1.2	V	
5.12	SSED	D	220	61.1	63.5	6.7	0.00635	63.5	--	1.58	1.11	1.8	IN	Not through platens
8.05	SSED	D	130	61.1	63.5	11.14	0.01056	63.5	--	2.62	1.11	2.9	V	
8.72	SSED	D	120	61.1	63.5	2.2	0.00209	63.5	--	0.52	1.11	0.6	IN	5 mm offset from platens
10.36	SSED	D	145	61.1	63.5	1.94	0.00184	63.5	--	0.46	1.11	0.5	V	
10.9	SSED	D	240	61.1	63.5	2.96	0.00281	63.5	--	0.70	1.11	0.8	V	
11.63	SSED	D	230	61.1	63.5	4.86	0.00461	63.5	--	1.14	1.11	1.3	V	
11.81	SSED	A	33.5	63.5	33.5	2.83	0.00268	--	52.0	0.99	1.02	1.0	IN	Not through platens
12.87	SSED	D	120	61.1	63.5	1.44	0.00137	63.5	--	0.34	1.11	0.4	V	
13.64	SSED	A	57.5	63.5	57.5	18.55	0.01759	--	68.2	3.78	1.15	4.3	IN	Not through platens
15.75	SSED	D	130	61.1	63.5	4.84	0.00459	63.5	--	1.14	1.11	1.3	V	
16.67	SSED	D	160	61.1	63.5	2.12	0.00201	63.5	--	0.50	1.11	0.6	V	
19.88	SSED	D	220	61.1	63.5	1.72	0.00163	63.5	--	0.40	1.11	0.5	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-2: BH-BGC11-25 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
3.4	SSED	D	260	61.1	63.5	11.08	0.01050	63.5	--	2.60	1.11	2.9	V	
4.01	SSED	D	140	61.1	63.5	7.48	0.00709	63.5	--	1.76	1.11	2.0	IN	Fracture 10 mm from platens
6.33	SSED	D	120	61.1	63.5	7.69	0.00729	63.5	--	1.81	1.11	2.0	IN	Fracture off platens
7.11	SSED	D	100	61.1	63.5	7.86	0.00745	63.5	--	1.85	1.11	2.1	V	
8.07	SSED	D	190	61.1	63.5	7.82	0.00741	63.5	--	1.84	1.11	2.0	V	
9.98	SSED	D	120	61.1	63.5	0.74	0.00070	63.5	--	0.17	1.11	0.2	IN	Fracture to eskers
10.88	SSED	A	48	63.5	48	3.89	0.00369	--	62.3	0.95	1.10	1.0	IN	Fracture 10 mm from platens
12	SSED	D	100	61.1	63.5	3.3	0.00313	63.5	--	0.78	1.11	0.9	V	
13.43	SSED	A	48	63.5	48	4.13	0.00392	--	62.3	1.01	1.10	1.1	IN	Fracture off platens
15.75	SSED	D	100	61.1	63.5	8.8	0.00834	63.5	--	2.07	1.11	2.3	V	
20.28	SSED	A	44	63.5	44	1.73	0.00164	--	59.6	0.46	1.08	0.5	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-3: BH-BGC11-26 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid/Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
0.95	SSED	A	58	63.5	58	10.72	0.01016	--	68.5	2.17	1.15	2.5	IN	3 mm offset from platen
12.37	SSED	D	83	61.1	63.5	16.49	0.01563	63.5	--	3.88	1.11	4.3	V	
19.56	SSED	A	390	63.5	39	8.14	0.00772	--	56.2	2.45	1.05	2.6	IN	8 mm offset from platen
20.42	SSED	A	51.5	63.5	51.5	5.93	0.00562	--	64.5	1.35	1.12	1.5	V	
20.61	SSED	A	58	63.5	58	15.62	0.01481	--	68.5	3.16	1.15	3.6	IN	Did not line up with platen
22.74	SSED	A	59.5	63.5	59.5	4.67	0.00443	--	69.4	0.92	1.16	1.1	V	
23.39	SSED	D	300	61.1	63.5	5.51	0.00522	63.5	--	1.30	1.11	1.4	V	
28.26	SSED	D	118	61.1	63.5	4.34	0.00411	63.0	--	1.04	1.11	1.1	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-4: BH-BGC11-27 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid/Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
13.5	SSED	A	55	63.5	55	12.34	0.01170	--	66.7	2.63	1.14	3.0	V	
14.67	SSED	A	51	63.5	51	13.25	0.01256	--	64.2	3.05	1.12	3.4	IN	
17.22	SSED	D	95	63.5	63.5	3.17	0.00301	63.5	--	0.75	1.11	0.8	V	
20.24	SSED	A	52	63.5	52	5.91	0.00560	--	64.8	1.33	1.12	1.5	V	
22.05	SSED	D	230	63.5	63.5	2.83	0.00268	63.5	--	0.67	1.11	0.7	IN	
24.52	SSED	A	56	63.5	56	12.05	0.01142	--	67.3	2.52	1.14	2.9	V	
25.64	SSED	D	65	63.5	63.5	5.2	0.00493	63.5	--	1.22	1.11	1.4	IN	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-5: BH-BGC11-28 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
5.57	SSED	D	88	63.5	63.5	1.19	0.00113	63.5	--	0.28	1.11	0.3	V	
8.25	SSED	A	58	63.5	58	4.56	0.00432	--	68.5	0.92	1.15	1.1	IN	
10.64	SSED	D	112	63.5	63.5	11.92	0.01130	63.5	--	2.80	1.11	3.1	V	
16.85	SSED	D	180	63.5	63.5	3.6	0.00341	63.5	--	0.85	1.11	0.9	V	
17.75	SSED	A	63.5	54	54	2.54	0.00241	--	60.9	0.65	1.09	0.7	IN	
20.16	SSED	D	155	63.5	63.5	11.68	0.01107	63.5	--	2.75	1.11	3.1	V	
21.62	SSED	A	52	63.5	52	4.5	0.00427	--	64.8	1.01	1.12	1.1	IN	
22.4	SSED	D	138	63.5	63.5	4.07	0.00386	63.5	--	0.96	1.11	1.1	V	
25.2	SSED	D	147	63.5	63.5	3.85	0.00365	63.5	--	0.91	1.11	1.0	V	
25.55	SSED	A	48	63.5	63.5	1.83	0.00173	--	71.7	0.34	1.18	0.4	IN	
25.64	SSED	D	112	63.5	63.5	1.28	0.00121	63.5	--	0.30	1.11	0.3	V	
27.8	SSED	D	244	63.5	63.5	4.1	0.00389	63.5	--	0.96	1.11	1.1	V	
28.52	SSED	D	87	63.5	63.5	9.83	0.00932	63.5	--	2.31	1.11	2.6	V	
33.08	SSED	D	125	63.5	63.5	18.85	0.01787	63.5	--	4.43	1.11	4.9	IN	
34.48	SSED	D	88	63.5	63.5	5.74	0.00544	63.5	--	1.35	1.11	1.5	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-6: BH-BGC11-29 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{4A/\pi}$ [mm] (notes 4 and 5)					
14.85	SSED	D	68	63.5	63.5	7.69	0.00729	63.5	--	1.81	1.11	2.0	V	
17.25	SSED	D	96	63.5	62.5	13.43	0.01273	62.5	--	3.26	1.11	3.6	IN	Broke 1 cm away from second platen
19.82	SSED	A	48	63.5	48	2.15	0.00204	--	62.3	0.53	1.10	0.6	V	Broke along existing microfissure
21.8	SSED	D	128	63.5	63.5	5.04	0.00478	63.5	--	1.18	1.11	1.3	V	
23.24	SSED	D	113	63.5	63.5	3.48	0.00330	63.5	--	0.82	1.11	0.9	V	Split along ??? And vertically
25.95	SSED	D	115	63.5	63.5	12.53	0.01188	63.5	--	2.95	1.11	3.3	V	
27.32	SSED	D	102	63.5	63.5	1.87	0.00177	63.5	--	0.44	1.11	0.5	V	
27.81	SSED	D	79	63.5	63.5	1.19	0.00113	63.5	--	0.28	1.11	0.3	V	
30.87	SSED	D	220	63.5	63.5	8.08	0.00766	63.5	--	1.90	1.11	2.1	V	
32.5	SSED	D	140	63.5	63.5	45.5	0.04313	63.5	--	10.70	1.11	11.9	V	Picked out of SIL altered axis
34.8	SSED	A	78	63.5	63.5	3.5	0.00332	--	71.7	0.65	1.18	0.8	IN	Foliation bed would break the one after the other by sheet
34.8	SSED	D	72	63.5	80	1.78	0.00169	63.5	--	0.42	1.24	0.3	V	
36.74	SSED	D	89	63.5	63.5	2.14	0.00203	63.5	--	0.50	1.11	0.6	V	
38.2	SSED	A	69	63.5	69	24.88	0.02359	--	74.7	4.23	1.20	5.1	IN	
38.2	SSED	D	69	63.5	63.5	16.1	0.01526	63.5	--	3.79	1.11	4.2	V	
39.38	SSED	D	150	63.5	63.5	1.62	0.00154	63.5	--	0.38	1.11	0.4	V	
40.82	SSED	A	55	63.5	55	20.55	0.01948	--	66.7	4.38	1.14	5.0	IN	Chipped out

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-7: BH-BGC11-30 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
2.95	SSED	A	81	62.5	81	6.19	0.00587	--	80.3	0.91	1.24	1.1	IN	Wedge slipped out along foliation plane
11.38	SSED	D	95	63.5	63.5	5.13	0.00486	63.5	--	1.21	1.11	1.3	V	Slight offset with platens alignment due to shape of foliation
22.8	SSED	D	97	63	61.5	1.6	0.00152	61.5	--	0.40	1.10	0.4	V	
22.83	SSED	A	54	63.5	54	5.27	0.00500	--	66.1	1.14	1.13	1.3	IN	Chipped out along foliation plane
24.3	SSED	D	110	63.5	63.5	2.92	0.00277	63.5	--	0.69	1.11	0.8	V	
30.76	SSED	D	185	63.5	63.5	3.45	0.00327	63.5	--	0.81	1.11	0.9	V	
31.5	SSED	D	215	63.5	63.5	6.75	0.00640	63.5	--	1.59	1.11	1.8	IN	Offset between platens/fracture planes
34.95	SSED	D	95	63.5	63.5	2.92	0.00277	63.5	--	0.69	1.11	0.8	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-8: BH-BGC11-31 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
28.65	SSED	D	65	61.5	61.5	0.95	0.00090	61.5	--	0.24	1.10	0.3	V	
28.86	SSED	D	70	61	61	1.07	0.00101	61	--	0.27	1.09	0.3	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-9: BH-BGC11-33 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
19.97	SSED	D	97	63.5	62.8	1.9	0.00200	62.8	--	0.51	1.11	0.5	V	
24.33	SSED	D	105	63.5	63.5	4.46	0.00400	63.50000	--	0.99	1.11	1.2	IN	Offset between platens alignment and failure plane
37.03	SSED	A	46	63.5	46	1.09	0.00100	--	61.0	0.27	1.09	0.3	IN	Chipped out
37.61	SSED	D	102	63.5	63.5	1.07	0.00100	63.50000	--	0.25	1.11	0.3	V	
38	SSED	A	48	63.5	48.5	1.09	0.00100	--	62.6	0.26	1.11	0.3	IN	Chipped out/slid along foliation

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-10: BH-BGC11-34 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
23.96	SSED	D	110	63.5	63.5	0.86	0.00082	63.5	--	0.20	1.11	0.2	IN	
27.43	SSED	D	100	63.5	63.5	2.27	0.00215	63.5	--	0.53	1.11	0.6	V	
28.63	SSED	D	150	63.5	63.5	2.2	0.00209	63.5	--	0.52	1.11	0.6	V	
30.22	SSED	A	63.5	40	40	5.13	0.00486	--	45.1	2.39	0.95	2.3	IN	
31.43	SSED	D	110	63.5	63.5	1.41	0.00134	63.5	--	0.33	1.11	0.4	V	
33.17	SSED	D	230	63.5	63.5	1.69	0.00160	63.5	--	0.40	1.11	0.4	IN	Fractured 2.5 cm away from platen along vein
36.42	SSED	D	160	63.5	63.5	5.3	0.00502	63.5	--	1.25	1.11	1.4	V	
36.58	SSED	A	63.5	20	20	5.8	0.00550	--	22.6	10.80	0.70	7.5	IN	Chipped out
36.98	SSED	D	180	63.5	63.5	7.86	0.00745	63.5	--	1.85	1.11	2.1	V	
37.58	SSED	A	63.5	20	20	44.14	0.04184	--	22.6	82.16	0.70	57.4	IN	Breaking upper rock disks

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-11: BH-BGC11-35 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{4A/\pi}$ [mm] (notes 4 and 5)					
12.36	SSED	D	71	61.1	63.5	1.45	0.00137	63.5	--	0.34	1.11	0.4	IN	4 mm off
26.01	SSED	D	158	61.1	63.5	26.83	0.02543	63.5	--	6.31	1.11	7.0	V	High quartz content, surrounding materials much weaker
29.49	SSED	D	117	61.1	63.5	1.34	0.00127	63.5	--	0.32	1.11	0.4	IN	Way off
30.33	SSED	D	164	61.1	63.5	1.71	0.00162	63.5	--	0.40	1.11	0.4	V	
19.87	SSED	A	63.5	52	52	3.21	0.00304	--	58.7	0.88	1.07	0.9	V	
31.77	SSED	D	129	61.1	63.5	3.28	0.00311	63.5	--	0.77	1.11	0.9	IN	18 mm off
34.31	SSED	D	86	61.1	63.5	1.58	0.00150	63.5	--	0.37	1.11	0.4	V	
36.32	SSED	A	63.5	50	50	4.51	0.00428	--	56.4	1.34	1.06	1.4	IN	Edges chipped off
42.62	SSED	D	97	61.1	63.5	23.34	0.02213	63.5	--	5.49	1.11	6.1	V	High in quartz, poor recover in this area so likely this is not representative

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-12: BH-BGC11-36 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
5.18	SSED	D	82	61.1	63.5	3.29	0.00312	63.5	--	0.77	1.11	0.9	V	
6.96	SSED	D	105	61.1	63.5	3.49	0.00331	63.5	--	0.82	1.11	0.9	IN	
7.43	SSED	A	63.5	53	53	30.05	0.02849	--	59.8	7.97	1.08	8.6	V	
11.63	SSED	A	63.5	55.5	55.5	2.44	0.00114	--	63.5	0.28	1.11	0.3	IN	Corner chipped off
11.38	SSED	D	96	61.1	63.5	1.2	0.00231	63.5	--	0.57	1.11	0.7	V	
14.47	SSED	D	118	61.1	63.5	3.01	0.00285	63.5	--	0.71	1.11	0.8	IN	2 mm off
17.39	SSED	D	95	61.1	63.5	2.09	0.00198	63.5	--	0.49	1.11	0.5	V	
23.08	SSED	D	149	61.1	63.5	4.64	0.00440	63.5	--	1.09	1.11	1.2	V	
28.28	SSED	D	115	61.1	63.5	1.76	0.00167	63.5	--	0.41	1.11	0.5	IN	Edge crumbled
28.97	SSED	D	120	61.1	63.5	2.51	0.00238	63.5	--	0.59	1.11	0.7	IN	12 mm off
31.7	SSED	A	63.5	54.5	54.5	6.4	0.00607	--	61.5	1.60	1.10	1.8	IN	Chipped off
35.3	SSED	D	134	63.5	63.5	1.18	0.00112	63.5	--	0.28	1.11	0.3	V	
37.05	SSED	D	215	63.5	63.5	4.08	0.00387	63.5	--	0.96	1.11	1.1	V	
38.75	SSED	D	174	63.5	63.5	3.43	0.00325	63.5	--	0.81	1.11	0.9	V	
39.28	SSED	D	210	63.5	63.5	3.38	0.00320	63.5	--	0.79	1.11	0.9	V	Failed along existing microfracture
44.14	SSED	D	87	63.5	63.5	15.57	0.01476	63.5	--	3.66	1.11	4.1	V	Slightly offset between platen alignment and failure plane
48.5	SSED	A	63.5	42.5	42.5	3.18	0.00301	--	48.0	1.31	0.98	1.3	V	Failed along mechanical fracture
48.57	SSED	D	66	63.5	63.5	8.36	0.00793	63.5	--	1.97	1.11	2.2	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-13: BH-BGC11-37 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
4.1	SSED	A	75	63.5	75	6.43	0.00610	--	77.9	1.01	1.22	1.2	IN	Failed along existing fracture
5.47	SSED	D	76	63.5	63.5	5.73	0.00543	63.5	--	1.35	1.11	1.5	V	
7.65	SSED	D	143	63.5	63.5	18.43	0.01747	63.5	--	4.33	1.11	4.8	V	
8.5	SSED	A	35	63.5	35	6.81	0.00646	--	53.2	2.28	1.03	2.3	IN	Failed along existing fracture
11.3	SSED	A	50	63.5	50	9.8	0.00929	--	63.6	2.30	1.11	2.6	V	
15.34	SSED	D	82	63.5	63.5	9.85	0.00934	63.5	--	2.32	1.11	2.6	IN	Offset between platen alignment and failure plane
16.19	SSED	D	195	63.5	63.5	12.46	0.01181	63.5	--	2.93	1.11	3.3	IN	Offset between platen alignment and failure plane
21.5	SSED	D	69	63.5	63.5	21.94	0.02080	63.5	--	5.16	1.11	5.7	V	Formeted but runs through both platens
22.5	SSED	D	135	63.5	63.5	1.1	0.00104	63.5	--	0.26	1.11	0.3	V	
25.95	SSED	A	61.5	63.5	61.5	10.4	0.00986	--	70.5	1.98	1.17	2.3	IN	Chipped out
26.65	SSED	D	160	63.5	63.5	12.6	0.01194	63.5	--	2.96	1.11	3.3	V	
30.07	SSED	A	41	63.5	41	5.54	0.00525	--	57.6	1.58	1.07	1.7	V	
34.15	SSED	D	104	63.5	63.5	2.11	0.00200	63.5	--	0.50	1.11	0.6	V	
38.04	SSED	D	142	63.5	63.5	2.26	0.00214	63.5	--	0.53	1.11	0.6	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-14: BH-BGC11-38 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{4A/\pi}$ [mm] (notes 4 and 5)					
13.35	SSED	D	180	63.5	63.5	7.09	0.00672	63.5	--	1.67	1.11	1.9	V	
21.75	SSED	D	130	63.5	63.5	0.9	0.00085	63.5	--	0.21	1.11	0.2	V	Failed along foliation
23.85	SSED	D	110	63.5	63.5	0.603	0.00057	63.5	--	0.14	1.11	0.2	V	
25.75	SSED	D	170	63.5	63.5	5.2	0.00493	63.5	--	1.22	1.11	1.4	V	
31.04	SSED	D	140	63.5	63.5	6.15	0.00583	63.5	--	1.45	1.11	1.6	V	
36.64	SSED	D	210	63.5	63.5	4.22	0.00400	63.5	--	0.99	1.11	1.1	V	Failed along foliation
36.86	SSED	D	230	63.5	63.5	5.36	0.00508	63.5	--	1.26	1.11	1.4	IN	
40.21	SSED	D	100	63.5	63.5	1.22	0.00116	63.5	--	0.29	1.11	0.3	V	
44.65	SSED	D	120	63.5	63.5	2.82	0.00267	63.5	--	0.66	1.11	0.7	IN	Along foliation
43.87	SSED	D	190	63.5	63.5	5.33	0.00505	63.5	--	1.25	1.11	1.4	IN	Along foliation
44.06	SSED	D	100	63.5	63.5	2.35	0.00223	63.5	--	0.55	1.11	0.6	V	Failed along foliation
14.99	SSED	A	60	63.5	60	2.14	0.00203	--	69.6	0.42	1.16	0.5	IN	Partly along foliation
21.68	SSED	A	65	63.5	65	2.18	0.00207	--	72.5	0.39	1.18	0.5	IN	Crumbled, pieces broke off

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-15: BH-BGC11-40A Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
7.62	SSED	D	150	63.5	63.5	5.4	0.00512	63.5	--	1.27	1.11	1.4	IN	Broke along joint, not all the way though sample
10.2	SSED	D	175	63.5	62	4.16	0.00394	62.0	--	1.03	1.10	1.1	V	
11.86	SSED	A	74	63.5	58	41.14	0.03900	--	68.5	8.32	1.15	9.6	V	Failure plane very rough but running through platens
12.39	SSED	D	160	63.5	63.5	14.275	0.01353	63.5	--	3.36	1.11	3.7	V	
15.96	SSED	D	140	63.5	63.5	16.3	0.01545	63.5	--	3.83	1.11	4.3	IN	Broke along fracture parallel to axis, not through whole diameter
18.5	SSED	D	110	63.5	63.5	14.39	0.01364	63.5	--	3.38	1.11	3.8	IN	12mm offset between failure plane and platen axis
21.53	SSED	D	135	63.5	63.5	1.72	0.00163	63.5	--	0.40	1.11	0.5	V	
22.49	SSED	D	135	63.5	63.5	7.66	0.00726	63.5	--	1.80	1.11	2.0	V	
23.8	SSED	A	49	63.5	44	17.13	0.01624	--	59.6	4.56	1.08	4.9	IN	Chipped out
25.26	SSED	D	25.26	63.5	63.5	2.03	0.00192	63.5	--	0.48	1.11	0.5	V	
26.2	SSED	D	112	63.5	63.5	1.04	0.00099	63.5	--	0.24	1.11	0.3	IN	Offset between platens axis and failure plane
27.77	SSED	A	49	63.5	49	1.74	0.00165	--	62.9	0.42	1.11	0.5	IN	Chipped out
28.6	SSED	D	130	63.5	63.5	3.78	0.00358	63.5	--	0.89	1.11	1.0	V	
30.27	SSED	D	148	63.5	63.5	8.07	0.00765	63.5	--	1.90	1.11	2.1	V	
30.87	SSED	D	190	63.5	63.5	10.67	0.01012	63.5	--	2.51	1.11	2.8	V	
31.39	SSED	D	137	63.5	63.5	6.86	0.00650	63.5	--	1.61	1.11	1.8	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-16: BH-BGC11-40B

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{4A/\pi}$ [mm] (notes 4 and 5)					
12.85	SSED	D	75	63.5	63.5	12.7	0.01204	63.5	--	2.99	1.11	3.3	V	
12.92	SSED	A	55	63.5	63.5	21.43	0.02032	--	71.7	3.96	1.18	4.7	IN	Chipped out
13.35	SSED	D	105	63.5	63.5	2.85	0.00270	63.5	--	0.67	1.11	0.7	V	
16.93	SSED	D	141	63.5	63.5	3.85	0.00365	63.5	--	0.91	1.11	1.0	V	Broke along foliation and along existing microfracture
19.16	SSED	D	197	63.5	63.5	15.94	0.01511	63.5	--	3.75	1.11	4.2	V	
21.68	SSED	D	225	63.5	63.5	19.5	0.01849	63.5	--	4.58	1.11	5.1	IN	Broke 3cm away from axis of platens
22.11	SSED	D	110	63.5	63.5	4.55	0.00431	63.5	--	1.07	1.11	1.2	IN	Broke along existing microfracture
25	SSED	A	47	63.5	63.5	3.96	0.00375	--	71.7	0.73	1.18	0.9	IN	Chipped
25.42	SSED	D	170	63.5	63.5	7.9	0.00749	63.5	--	1.86	1.11	2.1	V	
30.37	SSED	D	195	63.5	63.5	8.04	0.00762	63.5	--	1.89	1.11	2.1	V	
30.97	SSED	D	180	63.5	63.5	2.03	0.00192	63.5	--	0.48	1.11	0.5	V	Along foliation
31.67	SSED	A	80	63.5	80	2.13	0.00202	--	80.4	0.31	1.24	0.4	V	Failed in three pieces
37.47	SSED	A	70	63.5	70	2.08	0.00197	--	75.2	0.35	1.20	0.4	IN	Layer slipped off along foliation plane
38.19	SSED	D	190	63.5	63.5	24.92	0.02362	63.5	--	5.86	1.11	6.5	V	Half is intrusive, still foliated
38.83	SSED	D	170	63.5	63.5	5.1	0.00483	63.5	--	1.20	1.11	1.3	V	Failed along foliation
39.75	SSED	A	60	63.5	60	0.96	0.00091	--	69.6	0.19	1.16	0.2	IN	Piece slid off along foliation
43.71	SSED	D	150	63.5	63.5	1.35	0.00128	63.5	--	0.32	1.11	0.4	V	Along foliation
45.66	SSED	D	140	63.5	63.5	13.29	0.01260	63.5	--	3.12	1.11	3.5	V	Along foliation

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-17: BH-BGC11-43 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
4.05	SINT	D	165	63.5	63.5	29.55	0.02801	63.5	--	6.95	1.11	7.7	V	
4.16	SINT	D	140	63.5	63.5	41.37	0.03922	63.5	--	9.73	1.11	10.8	V	
4.59	SINT	D	125	63.5	63.5	28.48	0.02700	63.5	--	6.70	1.11	7.5	V	
4.97	SINT	D	140	63.5	63.5	38.6	0.03659	63.5	--	9.08	1.11	10.1	V	
6.6	SINT	D	175	63.5	63.5	35.96	0.03409	63.5	--	8.45	1.11	9.4	V	
13.41	SINT	D	210	63.5	63.5	40.66	0.03855	63.5	--	9.56	1.11	10.6	V	
14.95	SINT	D	160	63.5	63.5	21.69	0.02056	63.5	--	5.10	1.11	5.7	IN	Broke along axis after chipping several times
16.06	SINT	D	185	63.5	63.5	37.03	0.03510	63.5	--	8.71	1.11	9.7	V	
20.63	SSED	A	50	63.5	56	23.55	0.02233	--	67.3	4.93	1.14	5.6	V?	Load axis wasn't along center of core, but break was along load axis
20.65	SSED	D	150	63.5	63.5	12.47	0.01182	63.5	--	2.93	1.11	3.3	IN	Along foliation
22.17	SSED	D	165	63.5	63.5	12.89	0.01222	63.5	--	3.03	1.11	3.4	IN	Along foliation
23.47	SSED	D	185	63.5	63.5	33.29	0.03156	63.5	--	7.83	1.11	8.7	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-18: BH-BGC11-45 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
5.36	SSED	D	97	63.5	63.5	0.72	0.00068	63.5	--	0.17	1.11	0.2	V	
5.7	SSED	A	43	63.5	43.5	3.47	0.00329	--	59.3	0.94	1.08	1.0	IN	Did not break through platten, chipped out
6.22	SSED	D	65	63.5	63.5	16.7	0.01583	63.5	--	3.93	1.11	4.4	V	
7.2	SSED	D	84	63.5	63.5	18.34	0.01739	63.5	--	4.31	1.11	4.8	V	
8.1	SSED	D	260	63.5	63.5	19.72	0.01869	63.5	--	4.64	1.11	5.2	V	
8.43	SSED	A	60	63.5	57	13.44	0.01274	--	67.9	2.76	1.15	3.2	IN	Chipped out parallel to fabric
10.99	SSED	A	55	63.5	55	6.81	0.00646	--	66.7	1.45	1.14	1.7	IN	Chipped out at bottom
11.08	SSED	D	120	63.5	63.5	3.97	0.00376	63.5	--	0.93	1.11	1.0	V	
11.38	SSED	D	150	63.5	63.5	3.73	0.00354	63.5	--	0.88	1.11	1.0	V	
11.42	SSED	A	77	63.5	77	8.23	0.00780	--	78.9	1.25	1.23	1.5	IN	Chipped out at top
12.63	SSED	D	170	63.5	63.5	4.19	0.00397	63.5	--	0.99	1.11	1.1	V	
13.79	SSED	A	60	63.5	47	7.26	0.00688	--	61.6	1.81	1.10	2.0	V	
14.09	SSED	D	120	63.5	63.5	4.2	0.00398	63.5	--	0.99	1.11	1.1	IN	
17.32	SSED	D	120	63.5	63.5	12.1	0.01147	63.5	--	2.84	1.11	3.2	V	UCS sample 17.38 to 17.64
18.21	SSED	A	80	63.5	63.5	9.48	0.00899	--	71.7	1.75	1.18	2.1	V	
18.61	SSED	D	160	63.5	63.5	6.09	0.00577	63.5	--	1.43	1.11	1.6	V	
19.96	SSED	A	45	63.5	44.5	20.26	0.01921	--	60.0	5.34	1.09	5.8	IN	Maxed out, no more travel
20.18	SSED	D	80	63.5	63.5	70.6	0.06693	63.5	--	16.60	1.11	18.5	V	
20.3	SSED	A	66	63.5	56	13.97	0.01324	--	67.3	2.93	1.14	3.3	IN	Chipped out at top

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-19: BH-BGC11-46 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{4A/\pi}$ [mm] (notes 4 and 5)					
4.62	INT	D	120	63.5	63.5	23.09	0.02189	63.5	--	5.43	1.11	6.0	V	40% of break surface stained
6.08	INT	D	220	63.5	63.5	11.53	0.01093	63.5	--	2.71	1.11	3.0	V	40% of break surface stained
7.68	INT	D	138	63.5	63.5	28.57	0.02708	63.5	--	6.72	1.11	7.5	IN	Did not break
10.18	INT	D	198	63.5	63.5	21.48	0.02036	63.5	--	5.05	1.11	5.6	IN	Chipped at platen
12.94	INT	D	303	63.5	63.5	2.63	0.00249	63.5	--	0.62	1.11	0.7	V	Break along plane of weakness
13.05	INT	D	147	63.5	63.5	39.38	0.03733	63.5	--	9.26	1.11	10.3	V	Broke in intact
14.39	INT	D	153	63.5	63.5	35.44	0.03360	63.5	--	8.33	1.11	9.3	V	
15.79	INT	D	252	63.5	63.5	10.33	0.00979	63.5	--	2.43	1.11	2.7	V	60% iron staining on surface of break
18.01	INT	D	196	63.5	63.5	7.35	0.00697	63.5	--	1.73	1.11	1.9	V	Along iron infilled joint 2mm thick
18.84	SSED	D	138	63.5	63.5	11.36	0.01077	63.5	--	2.67	1.11	3.0	V	80% iron staining along surface of break
19.84	SSED	D	183	63.5	63.5	24.61	0.02333	63.5	--	5.79	1.11	6.4	V	Fresh break

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-20: BH-BGC11-48 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{4A/\pi}$ [mm] (notes 4 and 5)					
13.41	SINT	D	82	63.5	63.5	1.45	0.00137	63.5	--	0.34	1.11	0.4	V	Broke along plane 25 degrees, 100% red iron staining
13.69	SINT	D	113	63.5	63.5	3.54	0.00336	63.5	--	0.83	1.11	0.9	V	100% iron stained face
17.11	SSED	D	263	63.5	63.5	1.06	0.00100	63.5	--	0.25	1.11	0.3	V	Broke along 3 paths, all valid
24.22	SSED	D	156	63.5	63.5	0.73	0.00069	63.5	--	0.17	1.11	0.2	V	Broke along 3 paths, all valid
30.47	SSED	D	228	63.5	63.5	2.45	0.00232	63.5	--	0.58	1.11	0.6	IN	Broke and slipped along plane of weakness

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-21: BH-BGC11-50 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
16.61	SSED	D	130	63.5	63.5	4.88	0.00463	63.5	--	1.15	1.11	1.3	V	Along foliation
18.1	SSED	D	135	63.5	63.5	2.13	0.00202	63.5	--	0.50	1.11	0.6	IN	Along foliation
18.19	SSED	A	34	63.5	34	4.42	0.00419	--	52.4	1.52	1.02	1.6	IN	Crumbled at point of load
22.58	SSED	D	150	63.5	63.5	23.44	0.02222	63.5	--	5.51	1.11	6.1	V	
24.9	SSED	D	195	63.5	63.5	18.09	0.01715	63.5	--	4.25	1.11	4.7	V	
25.23	SSED	D	220	63.5	63.5	15.65	0.01484	63.5	--	3.68	1.11	4.1	V	Along foliation
26.93	SSED	D	175	63.5	63.5	9.26	0.00878	63.5	--	2.18	1.11	2.4	IN	Along foliation
28.8	SSED	D	120	63.5	63.5	21.18	0.02008	63.5	--	4.98	1.11	5.5	V	
30.32	SSED	D	160	63.5	63.5	12.37	0.01173	63.5	--	2.91	1.11	3.2	IN	Fracture 3 mm offset from bottom platen
30.64	SSED	D	173	63.5	63.5	13.29	0.01260	63.5	--	3.12	1.11	3.5	IN	Fracture 7mm offset from bottom platen
34.43	SSED	D	180	63.5	63.5	17.31	0.01641	63.5	--	4.07	1.11	4.5	IN	Along foliation
35.12	SSED	D	165	63.5	63.5	12.43	0.01178	63.5	--	2.92	1.11	3.3	V	Along foliation
36.5	SSED	D	145	63.5	63.5	4.87	0.00462	63.5	--	1.14	1.11	1.3	IN	Along foliation
38.76	SSED	D	135	63.5	63.5	8.35	0.00792	63.5	--	1.96	1.11	2.2	V	Along foliation
40.49	SINT	D	200	63.5	63.5	28.56	0.02707	63.5	--	6.71	1.11	7.5	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A=W*D$.
6. I_s = Uncorrected point load strength.
7. F= Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-22: BH-BGC11-52 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
14.54	SSED	A	63	63.5	56	7.26	0.00688	--	67.3	1.52	1.14	1.7	V	
15.73	SSED	D	109	63.5	63.5	8.45	0.00801	63.5	--	1.99	1.11	2.2	V	
17.84	SSED	D	248	63.5	63.5	17.88	0.01695	63.5	--	4.20	1.11	4.7	V	
17.87	SSED	A	46	63.5	46	21.09	0.01999	--	61.0	5.38	1.09	5.9	IN	Break almost to bottom platen (4mm)
19.66	SSED	D	83	63.5	63.5	3.5	0.00332	63.5	--	0.82	1.11	0.9	V	
21.37	SSED	A	57	63.5	53	5.83	0.00553	--	65.5	1.29	1.13	1.5	IN	Chipped out
21.88	SSED	D	162	63.5	63.5	1.23	0.00117	63.5	--	0.29	1.11	0.3	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-23: BH-BGC11-54 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
7.62	SSED	D	124	63.5	63.5	3.76	0.00356	63.5	--	0.88	1.11	1.0	IN	Broke along oblique existing fracture
8.93	SSED	A	55	63.5	51.5	3.65	0.00346	--	64.5	0.83	1.12	0.9	V	Ran through platens but followed stepped plane along existing fracs
21.08	SSED	A	51	63.5	51	1.05	0.00100	--	64.2	0.24	1.12	0.3	IN	Chipped off
22.2	SSED	D	75	63.5	63.5	1.06	0.00100	63.5	--	0.25	1.11	0.3	V	
23.9	SSED	D	155	63.5	63.5	1.13	0.00107	63.5	--	0.27	1.11	0.3	V	
27.91	SSED	D	77	63.5	61	1.41	0.00134	61.0	--	0.36	1.09	0.4	V	
32.36	SSED	D	91	63.5	61	0.94	0.00089	61.0	--	0.24	1.09	0.3	V	
34.92	SSED	D	98.5	63.5	60.5	0.97	0.00092	60.5	--	0.25	1.09	0.3	V	Along foliation
38.03	SSED	D	75	62	62	1.73	0.00164	62.0	--	0.43	1.10	0.5	IN	Broke along existing microfracture
39.21	SSED	D	93.7	62.5	62.5	0.96	0.00091	62.5	--	0.23	1.11	0.3	V	
40.3	SSED	D	140	61	61	1.09	0.00103	61.0	--	0.28	1.09	0.3	V	
40.65	SSED	D	91	61	61	0.95	0.00090	61.0	--	0.24	1.09	0.3	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-24: BH-BGC11-59 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
7.02	SSED	A	40	63.5	36	4.74	0.00449	--	54.0	1.54	1.03	1.6	V	Broke into several pieces - difficult to tell if broke at platens
11.06	SSED	D	190	63.5	63.5	1.4	0.00133	63.5	--	0.33	1.11	0.4	IN	Along foliation; 6mm offset from one of the platens
12	SSED	D	150	63.5	63.5	3.13	0.00297	63.5	--	0.74	1.11	0.8	IN	Along foliation; 15mm offset from one platen, FE stained fracture
18.9	SSED	D	210	63.5	63.5	4.82	0.00457	63.5	--	1.13	1.11	1.3	IN	Partially along foliation
19.79	SSED	D	230	63.5	63.5	1.36	0.00129	63.5	--	0.32	1.11	0.4	V	Along foliation
20.84	SSED	D	130	63.5	63.5	0.99	0.00094	63.5	--	0.23	1.11	0.3	V	Along healed fracture
21.01	SSED	A	40	63.5	35	3.63	0.00344	--	53.2	1.22	1.03	1.3	IN	Crumbled at platens
29.13	SSED	D	125	63.5	63.5	1.37	0.00130	63.5	--	0.32	1.11	0.4	IN	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-25: BH-BGC11-62 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
3.28	SSED	D	86	63.5	63.5	2.24	0.00212	63.5	--	0.53	1.11	0.6	V	Along foliation
5.4	SSED	D	97	63.5	63.5	4.84	0.00459	63.5	--	1.14	1.11	1.3	V	Along foliation
5.82	SSED	D	85	63.5	63.5	3.65	0.00346	63.5	--	0.86	1.11	1.0	V	Along foliation
7.05	SSED	A	54	63.5	52.5	3.21	0.00304	--	65.15107	0.72	1.13	0.8	IN	Chipped off the side
7.84	SSED	A	55	63.5	55	2.82	0.00267	--	66.68425	0.60	1.14	0.7	IN	4 mm offset between failure plane and area of platen
7.87	SSED	D	140	63.5	60	0.9	0.00085	60.0	--	0.24	1.09	0.3	V	Along foliation
11.16	SSED	D	84	63.5	63.5	4.22	0.00400	63.5	--	0.99	1.11	1.1	V	
13.46	SSED	A	52	63.5	52	4.62	0.00438	--	64.84009	1.04	1.12	1.2	V	
15.51	SSED	D	130	63.5	63.5	4.37	0.00414	63.5	--	1.03	1.11	1.1	IN	Along foliation 1 cm offset
17.55	SSED	A	54	63.5	54	6.51	0.00617	--	66.07525	1.41	1.13	1.6	IN	Chipped off the side
22.55	SSED	A	72	63.5	72	6.03	0.00572	--	76.29712	0.98	1.21	1.2	IN	Rough stepped failure path by side
23.47	SSED	D	142	63.5	63.5	2.35	0.00223	63.5	--	0.55	1.11	0.6	V	Along foliation
24.95	SSED	D	91	63.5	63.5	1.63	0.00155	63.5	--	0.38	1.11	0.4	IN	Chipped off along foliation
28.5	SSED	D	162	63.5	63.5	1.15	0.00109	63.5	--	0.27	1.11	0.3	V	Rock also crumbled on one side
28.92	SSED	A	63	63.5	59	7.12	0.00675	--	69.06658	1.41	1.16	1.6	V	All foliation beds sheared under compressive stress
32.6	SSED	D	126	63.5	63.5	4.53	0.00429	63.5	--	1.07	1.11	1.2	V	
33.44	SSED	A	50	63.5	48	18.18	0.01723	--	62.29634	4.44	1.10	4.9	IN	Chipped off the side
34.75	SSED	D	105	63.5	63.5	2.39	0.00227	63.5	--	0.56	1.11	0.6	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-26: BH-BGC11-64 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
16.66	INT	D	140	63.5	61.7	2.05	0.00194	61.7	--	0.51	1.10	0.6	IN	Broke in many pieces
34.96	INT	D	145	63.5	63.5	4.84	0.00459	63.5	--	1.14	1.11	1.3	V	
46.36	INT	D	110	63.5	63.9	2.22	0.00210	63.9	--	0.52	1.12	0.6	IN	Broke along other weakness plane

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-27: BH-BGC11-66 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
6.4	INT	D	150	63.5	63.5	2.39	0.00227	63.5	--	0.56	1.11	0.6	IN	Broke along existing micro fractures
6.74	INT	D	185	63.5	63.5	9.81	0.00930	63.5	--	2.31	1.11	2.6	IN	Offset
11.92	INT	D	180	63.5	63	14.72	0.01395	63	--	3.52	1.11	3.9	V	
12.52	INT	D	74	63.5	62	4.55	0.00431	62	--	1.12	1.10	1.2	V	Broke in multiple pieces
15.32	INT	D	140	63.5	63.5	20.79	0.01971	63.5	--	4.89	1.11	5.4	IN	Chipped by lower platen
17.36	INT	D	250	63.5	63.5	26.11	0.02475	63.5	--	6.14	1.11	6.8	V	
18.72	INT	D	196	63.5	63.5	14.99	0.01421	63.5	--	3.52	1.11	3.9	IN	Broke along existing fracture
18.76	INT	D	196	63.5	63.5	20.73	0.01965	63.5	--	4.87	1.11	5.4	IN	Chipped by lower platen
23.35	INT	D	150	63.5	63.5	26.18	0.02482	63.5	--	6.16	1.11	6.9	V	
24.08	INT	D	205	63.5	63.5	29.48	0.02795	63.5	--	6.93	1.11	7.7	V	
27.15	INT	D	106	63.5	63.5	22.77	0.02159	63.5	--	5.35	1.11	6.0	V	
28.18	INT	D	212	63.5	63.5	30.89	0.02928	63.5	--	7.26	1.11	8.1	V	
30.72	INT	D	145	63.5	63.5	22.56	0.02139	63.5	--	5.30	1.11	5.9	IN	Chipped out
32.38	INT	D	220	63.5	63.5	2.45	0.00232	63.5	--	0.58	1.11	0.6	V	Along existing fracture plane
32.42	INT	D	105	63.5	63.5	23.78	0.02254	63.5	--	5.59	1.11	6.2	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

Table N-28: BH-BGC11-69 Point Load Tests

Sample Depth	Rock Type ¹	Test Type ²	Sample Length, L [mm]	Sample Width, W [mm]	Distance between platens, D [mm]	Gauge Reading [MPa]	Failure Load, P [MN] (note 3)	Diametral Test (Type D)	Axial Test (Type A)	$I_s = P/D_e^2$ [MPa] (note 6)	$F = (D_e/50)^{0.45}$ (note 7)	$I_{s(50)} = F(I_s)$ [MPa] (note 8)	Invalid /Valid	Comments
								$D_e = D$ [mm] (note 4)	$D_e = \sqrt{(4A/\pi)}$ [mm] (notes 4 and 5)					
6.05	SSED	D	80	63.5	63.5	1.48	0.00140	63.5	--	0.35	1.11	0.4	IN	Broke along existing fracture
6.85	SSED	A	43.5	63.5	43.5	1.81	0.00172	--	59.3	0.49	1.08	0.5	IN	Chipped off side
7.4	SSED	D	124	63.5	63.5	3.53	0.00335	63.5	--	0.83	1.11	0.9	V	Rough surface
11.6	SSED	D	212	63.5	63.5	3.47	0.00329	63.5	--	0.82	1.11	0.9	IN	Broke along existing fracture
12.13	SSED	D	129	63.5	63.5	2.94	0.00279	63.5	--	0.69	1.11	0.8	IN	Chipped at top
12.68	SSED	D	201	63.5	63.5	3.05	0.00289	63.5	--	0.72	1.11	0.8	V	
13.33	SSED	D	175	63.5	63.5	4.03	0.00382	63.5	--	0.95	1.11	1.1	IN	Chipped at top
15.36	SSED	D	115	63.5	63.5	1.44	0.00137	63.5	--	0.34	1.11	0.4	V	
18.78	SSED	A	48	63.5	42.5	7.32	0.00694	--	58.6	2.02	1.07	2.2	V	
18.95	SSED	D	196	63.5	63.5	6.85	0.00649	63.5	--	1.61	1.11	1.8	V	
21	SSED	D	100	63.5	63.5	1.15	0.00109	63.5	--	0.27	1.11	0.3	V	

1. Rock types: SSED: Surface weathered metasedimentary rocks; SINT: Surface weathered intrusive rocks; INT: Intrusive rocks.
2. Test types: D=Diametral, A=Axial, L=Lump.
3. P = Gauge reading in MPa * 0.000948.
4. D_e = Equivalent core diameter.
5. $A = W * D$.
6. I_s = Uncorrected point load strength.
7. F = Size correction factor.
8. $I_{s(50)}$ = Corrected point load strength.

APPENDIX O PLACER TAILINGS INVENTORY

1.0 INTRODUCTION

1.1. Purpose

This appendix describes a visual reconnaissance of the reworked valley bottom materials (“placer tailings”) in the vicinity of the Eagle Gold project. These materials were examined to develop opinions as to their potential for use as construction materials in development of the proposed mine.

1.2. Background

The Eagle Gold project area has been exploited by placer mining since about 1897. This historical activity has resulted in substantial volumes of reworked overburden materials (placer tailings) in valley bottoms in the region. A significant proportion of the Dublin Gulch valley bottom is filled with placer tailings, which are in places over 20 m thick. Similarly, the reach of Haggart Creek immediately adjacent to, and south of, the project area contains deposits of placer tailings.

The placer tailings consist of highly variable materials, with a wide range of grain size and density. The materials range from isolated deposits of cobbles and boulders, to gravelly sand or sandy gravel, to loose silty sand or soft sandy silt. The latter materials appear to be confined to specific areas, which are inferred from aerial photographs to have been sediment control ponds.

The placer tailings present potential constraints to development, since they are variably loose and potentially liquefiable where they exist below the water table, and are therefore not a suitable foundation for heavy structures. However, the placer tailings materials excavated in the course of foundation preparation may comprise a useful construction material, as a potential source for general fill, structural fill, concrete aggregate or heap overliner material, if suitably selected and processed through screening, crushing, and/or washing.

Suitability of the placer tailings for use as a select fill material will depend, largely, on grain size, and to a lesser degree on mineralogy and associated durability. Placer tailings with excessive fines (i.e. silt and clay) are not expected to be economically suitable for re-use. Where fines content is lower, the sand and gravel fraction is expected to be generally suitable for use as general fill, and potentially suitable for structural fill. The oversize fraction, defined in this section as all clasts greater than 75 mm, are frequently comprised of sound, durable rock. It is expected that this oversize fraction, if screened from the tailings, can be crushed to produce structural fill, and, pending the completion of laboratory testing, to also serve as a durable material for concrete aggregate and/or heap overliner.

This appendix documents a reconnaissance level survey of the placer tailings in Dublin Gulch and within the Haggart Creek valley from its confluence with Dublin Gulch to approximately 3 km south. This appendix presents visual observations of material texture as inferred from the ground surface. Intrusive investigation would be required to confirm the vertical extent of observed materials, and laboratory testing is required to confirm material properties. Some

subsurface data are available for the placer tailings in Dublin Gulch, as reported by BGC (2009 and 2011). Additional subsurface data at select test hole locations within the Dublin Gulch placer tailings are presented elsewhere in this report. There are no subsurface data available for the Haggart Creek placer tailings.

2.0 VISUAL RECONNAISSANCE

2.1. Spatial Limits of Reconnaissance

This component of the 2011 field investigations focused on placer tailings within a reasonable haul distance of the primary components of the planned project (i.e. up to about 2 km from the proposed truck shop pad). Exposed materials were classified visually at discrete, pre-selected points, including 26 locations in the Dublin Gulch valley bottom, and 34 in the Haggart Creek valley bottom. The locations of observation points are shown in Drawing 10.

2.2. Visual Classification of Surficial Materials

The observation points were selected at a number of locations, uniformly distributed across areas containing disturbed materials, as evident in available high resolution aerial photography. At each selected observation point, surficial materials were classified in accordance with standard soil descriptions, and further classified according to the Unified Soil Classification System (USCS). Finally, the approximate proportion of materials that would be retained on a 75 mm screen was estimated at each location.

Note that the classification is based on materials exposed at the immediate ground surface or at shallow depth as exposed by shallow digging. The observed texture may be different than that at depth.

3.0 DATA RESULTS

3.1. Dublin Gulch Area and Haggart Creek Placer Tailings

Visual observations of surficial materials in the Dublin Gulch valley bottom are summarized in Table O-1, and visual observations of surficial materials in the Haggart Creek valley bottom are summarized in Table O-2, below.

Table O-1. Material Observations in the Dublin Gulch Valley Bottom

Observation Point	Northing (m)	Easting (m)	Soil Description	USCS	Estimated % >75 mm
DG-0	7101354	459740	SAND, gravelly, trace to some silt, trace to some cobbles and boulders	SW	10-15
DG-1	7101270	459784	SAND and GRAVEL, cobbly, some boulders, trace silt	GW	40-50
DG-2	7101229	459716	SAND and GRAVEL, cobbly, some boulders, trace silt	GW	30-40
DG-3	7101162	459629	GRAVEL, sandy, cobbly, trace to some boulders, trace silt	GW	20-30
DG-4	7101225	459483	BOULDERS, mixed with silty sand, trace gravel	GP	50
DG-5	7101114	459484	GRAVEL, sandy, some cobbles and boulders, trace silt	GW	15-20
DG-6	7101069	459377	GRAVEL, sandy, some cobbles and boulders, trace silt	GW	15-10
DG-7	7101143	459336	SAND, silty, trace gravel, trace to some cobbles	SM	10
DG-9	7101034	459187	GRAVEL, sandy, some cobbles and boulders	GW	10-15
DG-10	7100972	459183	Area overgrown with brush; several boulders protruding at surface of inferred gravelly sand matrix	N/A	4-5
DG-11	7100889	459171	SAND, silty, trace gravel	SM	< 5
DG-12	7100883	459034	SAND, gravelly, some silt, occasional cobbles and boulders	SW	< 10

DG-13	7100950	458974	SAND, some silt, some gravel, trace cobbles and boulders	SW	< 5
DG-14	7100862	458915	SAND, silty, some gravel	SM	< 2
DG-15	7100962	458938	SAND, gravelly, trace silt	SW	1-2
DG-16	7100944	458849	GRAVEL, sandy, cobbly, occasional boulders, trace silt	GW	25
DG-17	7100949	458725	SAND and GRAVEL, trace silt, trace to some cobbles and boulders	GW	< 10
DG-18	7100889	458662	SAND, silty, some gravel	SM	1
DG-19	7100795	458734	GRAVEL, sandy, trace to some silt	GW	< 1
DG-20	7100793	458644	SAND and GRAVEL, silty	SW	< 1
DG-21	7100944	458603	SAND and GRAVEL, trace to some silt, occasional cobbles and boulders	GW	< 5
DG-22	7100959	458536	SAND, gravelly, trace silt, occasional cobbles and boulders	GW	< 5
DG-23	7101028	458506	SAND, silty, trace gravel	SM	< 1
DG-24	7100995	458417	SAND, silty, trace gravel	SM	< 1
DG-42	7099934	458289	GRAVEL, sandy, trace silt	GW	15-20
DG-47	7100891	459092	GRAVEL, cobbly, occasional boulders	GP	25

Table O-2. Material Observations in the Haggart Creek Valley Bottom.

Observation Point	Northing (m)	Easting (m)	Soil Description	USCS	Estimated % >75 mm
HC-0	7098254	458420	GRAVEL, sandy, trace to some silt, trace cobbles and boulders	GW	5-10
HC-1	7098287	458418	GRAVEL, sandy, trace to some silt, trace cobbles and boulders	GW	5-10
HC-2	7098315	458357	GRAVEL, sandy, trace to some silt, trace cobbles and boulders	GW	5-10
HC-3	7098321	458396	GRAVEL, sandy, trace to some silt, trace cobbles and boulders	GW	< 5
HC-4	7098456	458364	SAND, gravelly, trace silt, trace cobbles and boulders	SW	< 1-2
HC-5	7098480	458324	GRAVEL, sandy, cobbly, trace boulders	GW	> 15-20
HC-6	7098528	458412	SAND and GRAVEL, trace to some silt, trace cobbles and boulders	SW	2-3
HC-7	7098567	458380	SAND and GRAVEL, trace to some silt, trace cobbles and boulders	SW	2-3
HC-8	7098537	458312	GRAVEL, sandy, trace silt, trace cobbles and boulders	GW	< 5
HC-9	7098574	458294	SAND, gravelly, trace silt, trace cobbles and boulders	SW	< 5
HC-10	7098605	458400	SAND and GRAVEL, trace to some silt, trace cobbles and boulders	SW	2-3
HC-11	7098611	458306	SAND, gravelly, trace silt, trace cobbles and boulders	SW	< 2-3
HC-12	7098669	458255	GRAVEL, sandy, trace silt, trace cobbles and boulders	GW	< 2-3

HC-13	7098713	458285	GRAVEL, sandy, trace silt, trace cobbles and boulders	GW	< 5
HC-14	7098780	458295	GRAVEL, cobbly, trace boulders	GP	30-40
HC-15	7098812	458336	GRAVEL, cobbly, trace boulders	GP	30-40
HC-16	7098853	458333	GRAVEL, cobbly, some sand, trace boulders	GW	15-20
HC-17	7098907	458359	SAND, gravelly, trace silt, trace cobbles and boulders	SW	< 5
HC-18	7098964	458350	SAND, gravelly, trace to some silt	SW	< 1
HC-19	7099079	458274	SAND and GRAVEL, some silt, trace cobbles and boulders	SW	< 2-3
HC-20	7099121	458322	GRAVEL and SAND, some silt	GW	< 1-2
HC-21	7099167	458334	GRAVEL and SAND, some silt	GW	< 1-2
HC-22	7099192	458256	SAND, gravelly, some silt to silty	SW	< 2
HC-23	7099251	458257	SAND and SILT, gravelly, trace cobbles and boulders	SM	N/A
HC-24	7099267	458352	SAND, silty, some gravel	SM	< 1
HC-25	7099292	458379	no suitable materials; only thin cover of reworked silty material over undisturbed till	N/A	N/A
HC-26	7099323	458281	SAND and SILT, gravelly, trace cobbles and boulders	SM	N/A
HC-27	7099876	458323	SAND and GRAVEL, trace to some silt, trace cobbles and boulders	SW	< 4-5

HC-28	7099885	458261	SAND and GRAVEL, trace to some silt, trace cobbles and boulders	SW	< 4-5
HC-30	7100456	458222	negligible exploitable material	N/A	N/A
HC-31	7100856	458165	negligible exploitable material	N/A	N/A
HC-32	7100910	458124	in floodplain, not exploitable	N/A	N/A
HC-33	7101019	458099	not exploitable due to low elevation and proximity to Haggart Creek	N/A	N/A
HC-34	7101048	458156	not exploitable due to low elevation and proximity to Haggart Creek	N/A	N/A

4.0 PHOTOGRAPHS

Selected photographs of surficial materials within the Dublin Gulch and Haggart Creek valley bottoms are presented in the following pages. The photograph captions indicate the USCS classification of the observed material, as follows:

- SM – silty sand;
- SW – well graded sand;
- GP – poorly graded gravel; and
- GW – well graded gravel.

5.0 REFERENCES

BGC Engineering Inc. 2010. Site Facilities Geotechnical Investigation Factual Data Report. Eagle Gold Project, Victoria Gold Corporation.

BGC Engineering Inc. 2011. Eagle Gold Project 2010 Geotechnical Investigation for Mine Site Infrastructure Factual Data Report, Victoria Gold Corporation.



DG-0 - SW.



DG-1 - GW



DG-2 - GW



DG-3 - GW



DG-4 - GP



DG-5 - GW



DG-6 - GW



DG-7 - SM



DG-9 - GW



DG-11 - SM



DG-12 - SW



DG-13 - SW



DG-14 - SM



DG-15 - SW



DG-16 - GW



DG-17 - SW



DG-18 - SW



DG-19 - SW



DG-20 - SW



DG-21 - GW



DG-22 - GW



DG-23 - SM



DG-24 - SM



DG-47 - GP



HC-0 - GW



HC-1 - GW



HC-2 - GW



HC-3 - GW



HC-4 - SW



HC-5 - GW



HC-6 - SW



HC-7 - SW



HC-8 - GW



HC-9 - SW



HC-10 - SW



HC-11 - SW



HC-12 - GW



HC-13 - GW



HC-14 - GP



HC-15 - GP



HC-16 - GW



HC-17 - SW



HC-18 - SW



HC-19 - sw



HC-20 - GW



HC-21 - GW



HC-22 - SW



HC-23 - SM



HC-24 - SM



HC-26 - SM



HC-27 - SW



HC-28 - SW